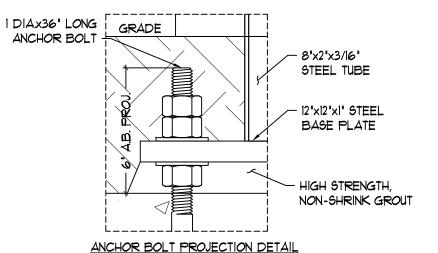
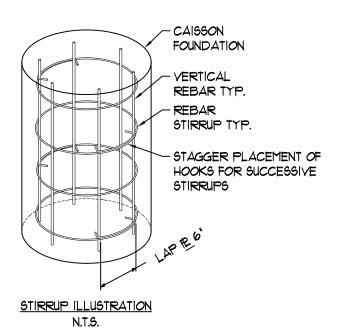
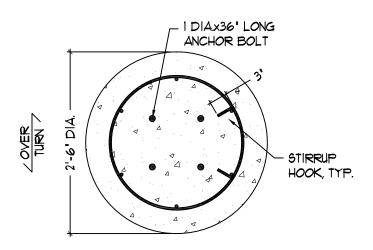


ALL STEEL BELOW GRADE (INCLUDING EXPOSED ANCHOR BOLTS) SHALL BE COATED WITH CARBOLINE BITUMASTIC 50 COAL TAR OR EQUAL AFTER INSTALLATION AND PRIOR TO LANDSCAPING







SECTION @ CAISSON

NSTALLATION ADDRESS:

JOHN DEERE (REVELS TURF & TRACTOR) 5118 RAWLS CHURCH RD. FUQUAY-YARINA, NC 27526

CLIENT:

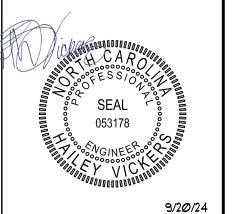


520 WEST SUMMIT HILL DR, SUITE 702 - KNOXVILLE, TN 37902 Tel (865) 693-1105 - Fax (865) 693-1106 - Toll Free (866) 218-1976

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SEAL & SIGNATURE:



9/20/24

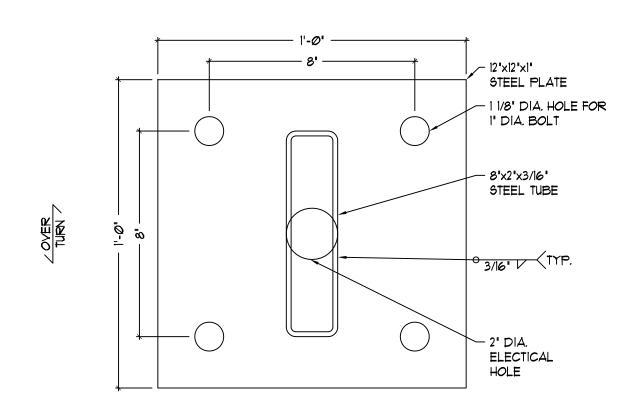


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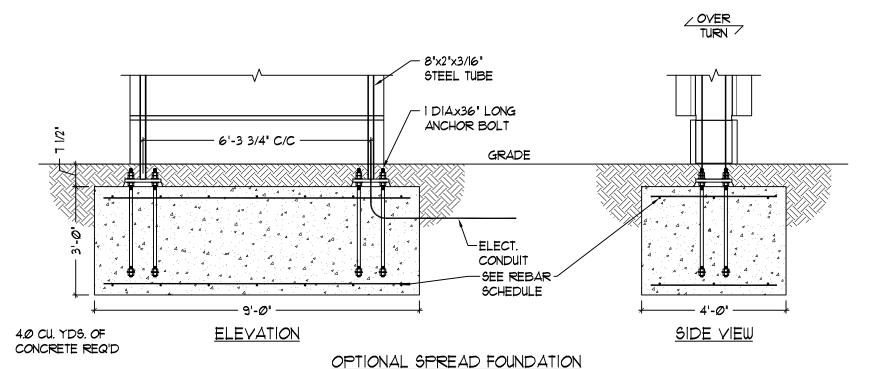
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North Carolina Certificate of Authorization No.: P-0483

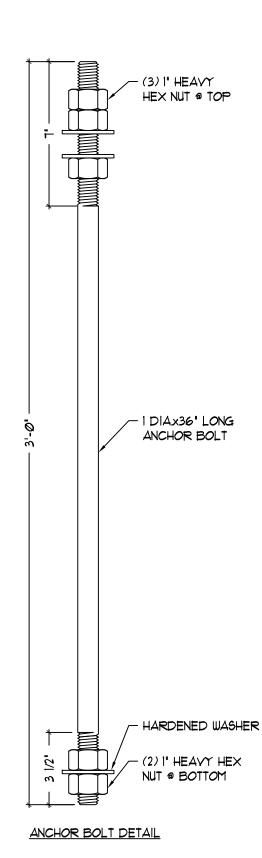
Project	Number:	Drawing Num	Drawing Number:			
24	1-0605	B2583	B258329Ø			
SHT.	OF	DATE:	BY:			
1	3	9/20/24	GHK			



BASE PLATE DETAIL



ALL STEEL BELOW GRADE (INCLUDING EXPOSED ANCHOR BOLTS) SHALL BE COATED WITH CARBOLINE BITUMASTIC 50 COAL TAR OR EQUAL AFTER INSTALLATION AND PRIOR TO LANDSCAPING. REBAR SCHEDULE SPREAD FOUNDATION DO NOT WELD REBAR 3' MIN. CONC. COVER PLACEMENT SIZE SPACING QUANTITY OVER TURN BOTTOM STEEL *****4 ٦ OVER TURN TOP STEEL #4 16" ٦ BOTTOM CROSS STEEL *****4 18' 3 #4 18" TOP CROSS STEEL 3



INSTALLATION ADDRESS:

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CLIENT:

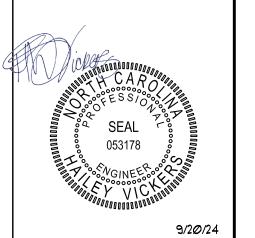


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North Carolina Certificate of Authorization No.: P-0483

Project I	Number:	Drawing Nun	Drawing Number:		
24- <i>060</i> 5		B258329Ø			
SHT.	OF	DATE:	BY:		
2	3	9/20/24	GHK		

	PROJECT #	24-0605		OWNER:	JOHN DEERE	- (REVELS TH	RF & TRACTOR)			
	September 20, 2024	24-0003		OVVINLI V.		CHURCH RD.	N & HACION)			
		B2583290					20			
	DRAWING #				FUQUAY-VA	RINA, NC 2752	20			
	WIND LOAD	20.592	PSF							
	WIND SPEED	120	MPH		PATTISON ID					
	# COLUMNS	2	NCBC 2018/IB	C 2015	520 WEST SI	JMMIT DR., ST	E 702			
	DESIGNER	GHK			KNOXVILLE,	TN				
			SHAPE	CENTROID		TOTAL		BASE		ADJUSTE
ITEM	HEIGHT	WIDTH	FACTOR	HEIGHT	AREA	FORCE	MOMENT	MOMENT	CENTROID	MONENT
======										
SIGN	6.934	7.031	1.000	3.841	49.109	1.011	3.885			
SIGN	2.344	7.031	1.000	1.289	16.479	1.351	6.692			
BASE	1.339	7.031	1.000	0.736	9.412	1.544	8.643	7.565	4.898	8.389
SUBGRADE	0.625	0.000	1.000	0.313	0.000	1.544	9.354			
OAH	10.667	0.000	1.000	0.010	0.000	1.011	5.504			
OAIT	10.007				-					
	COLLIMNI C	LCULATIONS	(CODES E	=PIPE;O=OTHE	D-T-TI IDE					
	WEGININ CA	ALCOLA HONG	(WDES F			OBLIQUE	AVAILABLE	COLUMN	OBLIQUE	
	0011841	0011845	001184	h	DESIGN					
	COLUMN	COLUMN	COLUMN	lxx	MODULUS	LOAD	FLEXURAL	CENTER	LOAD	
ITEM	WICTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	DISTANCE	FACTOR	UNTY
======										
SIGN	2.000	8.000	0.174	22.4	7.51	2.831	17.23	6.146	1.458	0.164
SIGN	2.000	8.000	0.174	22.4	7.51	4.877	17.23			0.283
BASE	2.000	8.000	0.174	22.4	7.51	6.299	17.23			0.365
SUBGRADE	2.000	8.000	0.174	22.4	7.51	6.817	17.23			0.396
	BOL	T CALCULAT	IONS							
		******	*	OBLIQUE						
		BOLT	BOLTS/	TENSION	BOLT	ALLOW.	ALLOWABLE			
ITEM	MOMENT	SPACING	PLATE	BOLT	DIA VI.	STRESS	TENSION			
======	WICHENT	======	======	======	======	======	=======			
BASE PL.	9.354	8.000	4.000	5.113	1.000	19.100	15.001			
DASEPL.	9.334	0.000	4.000	5.115	1.000	19.100	15.001			
	DI A	TE CALCULAT	IONE							

TTT 4	TENSION/	MOMENT	MONTO	DATE	PLATE	PLATE	MINIMUM			
ITEM			MCMENT	PLATE			CONTRACTOR CONTRACTOR			
	BOLT	ARM	PLATE	WIDTH	DEPTH	THICK.	THICK.			
BASE PL.	5.113	3.031	15.499	6.031	12.000	1.000	0.756			
ANCHOR BO ₋ T PR	OJECTION AND	CHOR EMBEDI	MENT	ANCHO	R BOLT MIN. L	.ENGTH				
6.000		10.172			17.000					
SPREAD FOUNDAT	TION									
MOMENT AT GRAD	E				9.354					
TOTAL FORCE					1.544	kip				
WEIGHT OF SIGN					0.852	kip				
SLAB WIDTH					9.000	ft				
SLAB LENGTH					4.000	ft				
SLAB DEPTH					3.000	ft				
SLAB WEIGHT					16.200	kip				
TOTAL WEIGHT					17.052	kip				
OVERTURNING MC	MENT				13.987					
FACTOR OF SAFE					2.438					
e = OTM/WT					0.820					
L/2 - e	********				1.180					
	'WT/(3*(L/2-e)*WIDTH)				1071					
CONCRETE					4.000	yd³				
EXCAVATION					4.000	yd ³				
MIN THICKNESS V	V/O REBAR				7.487	in				
SQRT(M*12*6*1.7*.	.75/(.178*12*WIDTH)									

DTH	0.043	
	0.025	
REBAR SIZE	4.000	
WEIGHT PER FT	0.668	
SPACING	16.000	in
AREA PER BAR	0.200	in ²
DTH	0.043	
AREA PER FT	0.156	
EST NO. REQ'D	6.750	
NUMBER REQ'D	7.000	
LENGTH	3.500	ft
WEGHT	16.366	
EDGE	6.000	
REBAR SIZE	4.000	
WEIGHT PER FT	0.668	
SPACING	16.000	in
AREA PER BAR	0.200	in ²
	0.025	
AREA PER FT	0.156	
EST NO. REQ'D	6.750	
NUMBER REQ'D	7.000	
LENGTH	3.500	ft
WEIGHT	16.366	
EDGE	6.000	
REBAR SIZE	4.000	
WEIGHT PER FT	0.668	
SPACING	18.000	in
LENGTH	8.500	ft
EST NO. REQ'D	2.667	
NUMBER REQ'D	3.000	
EDGE	6.000	in
WEIGHT	34.068	
TOTAL WEIGHT	66.800	
	9.000	ft
	4.000	ft
	9.354	FT-KIP
	1.544	KIP
M & GC		
DEPTH	150.0	PSF/F1
	566.7	
	5.667	FT.
	2.500	FT.
	6.057	FT.
	2.551	FT.
	5.573	FT.
ENT	23.783	IN.
	30.000	IN.
	-	
	REBAR SIZE WEIGHT PER FT SPACING AREA PER BAR TH AREA PER FT EST NO. REO'D NUMBER REQ'D LENGTH WEIGHT EDGE REBAR SIZE WEIGHT PER FT SPACING AREA PER BAR AREA PER REO'D LENGTH WEIGHT EST NO. REO'D NUMBER REQ'D LENGTH WEIGHT EST NO. REO'D NUMBER REQ'D LENGTH WEIGHT EDGE REBAR SIZE WEIGHT PER FT SPACING LENGTH WEIGHT EDGE WEIGHT PER FT SPACING LENGTH EST NO. REO'D NUMBER REQ'D EDGE WEIGHT PER FT TOTAL WEIGHT TOTAL WEIGHT	REBAR SIZE

General Notes:

- 1. Design is based on a 120 mph, 3 second gust wind design per the NCBC 2018/IBC 2015, Category II, Exposure C
- 2. Spread foundation is based on a presumptive safe vertical soil bearing pressure minimum of 2000 psf. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2' motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact PATTISON ID.
- 4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.
- 5. Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
- 6. Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
- 7. All voids between column base plate and foundation surface shall be completely filled with high-strength, non-shrink grout.
- 8. Anchor bolts shall meet ASTM F1554 Grade 36. Exposed surfaces shall be galvanized or coated to prevent corrosion.
- 9. All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a minimum yield strength of 46000 psi. Steel angle, channel and plate shall meet ASTM A36.
- 10. Steel welds shall be made with ET0xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
- 11. All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn-of-Nut method unless noted otherwise.
- 12. The scope of this engineer does not include onsite observations.
- 13. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
- 14. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from LINK Engineering voids this drawing in its entirety.
- 15. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

INSTALLATION ADDRESS:

JOHN DEERE (REVELS TURF & TRACTOR) 5118 RAWLS CHURCH RD. FUQUAY-VARINA, NC 27526

CLIENT

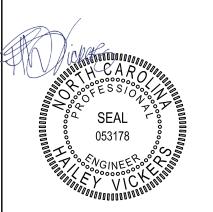


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<u> 3</u>	-/-/-	

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North Carolina Certificate of Authorization No.: P-0483

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Project N	umber:	Drawing Numl	Drawing Number:			
24-	0605	B25832	B258329Ø			
SHT. OF		DATE:	BY:			
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