

PROJECT NAME:
**NEW SHEETZ SITE
CAMERON**

Cameron, NC
PSR #214196

OWNER:
SHEETZ, INC.

5700 SIXTH AVE.
ALTOONA, PA 16602

CONSULTANT

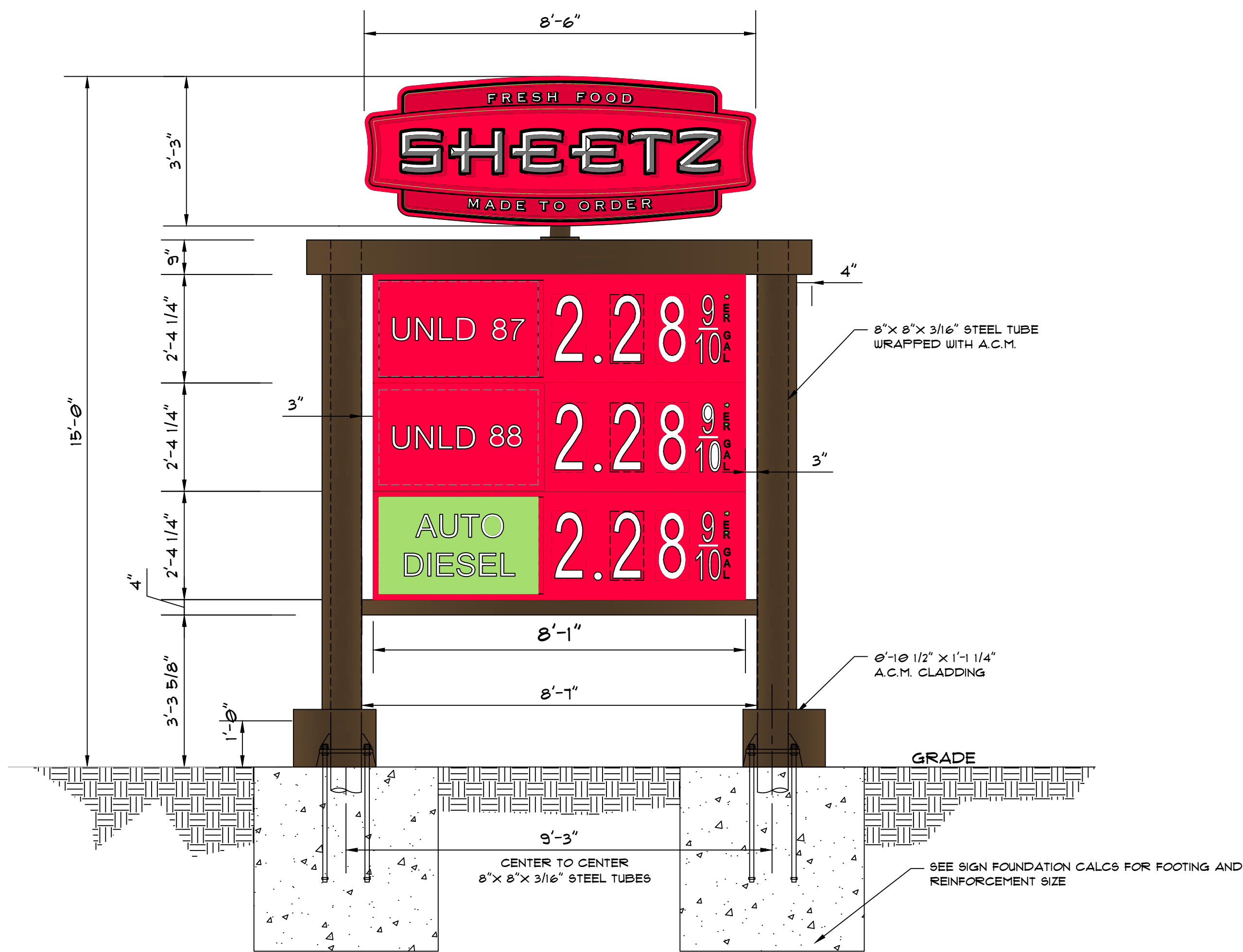
PROFESSIONAL

KEYPLAN

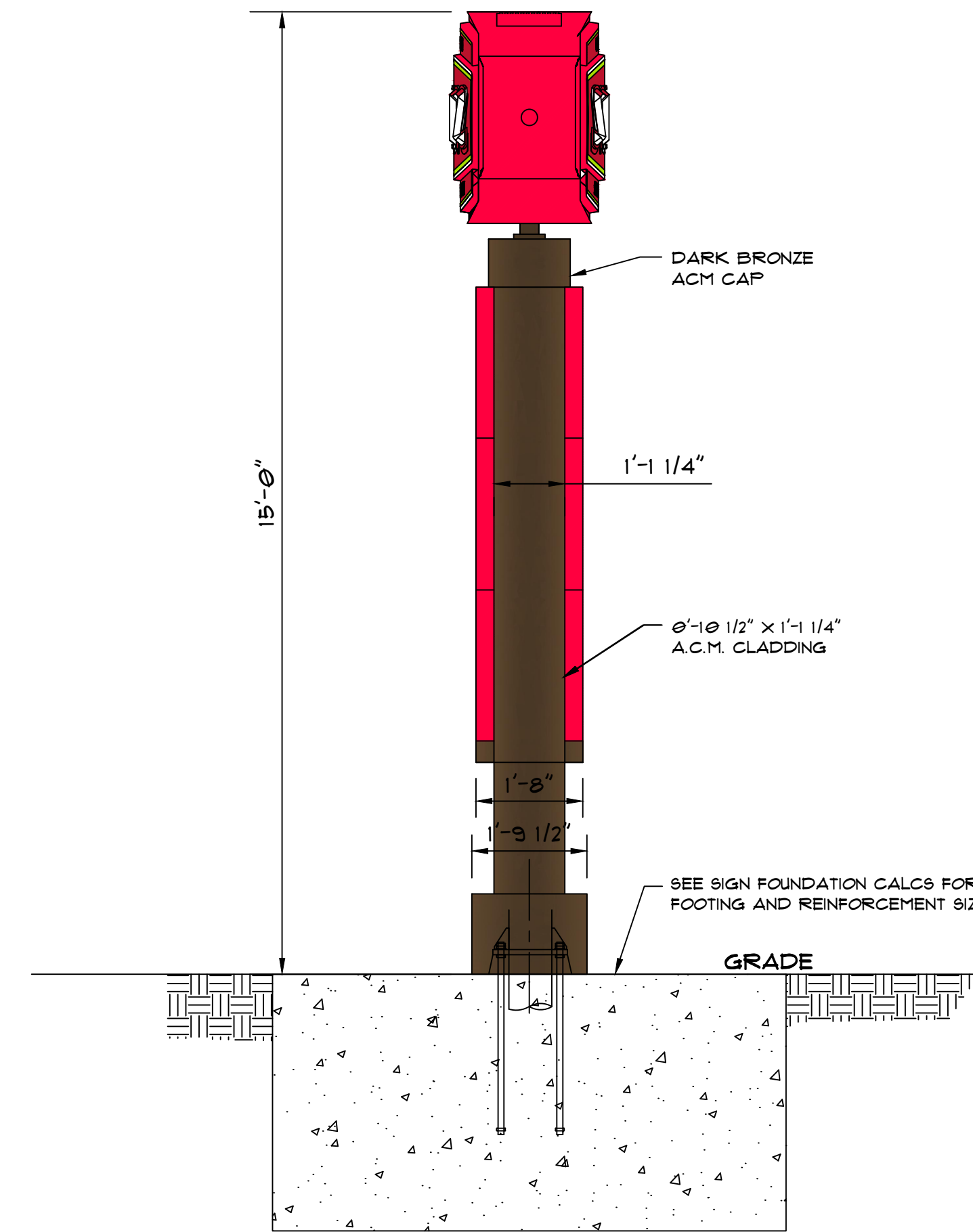
ISSUE: **7-15-2020**
PROJECT NO:
AUTHOR BY: JEB
REVIEW BY:
SHEET TITLE

POLE SIGN
DETAILS

PS1

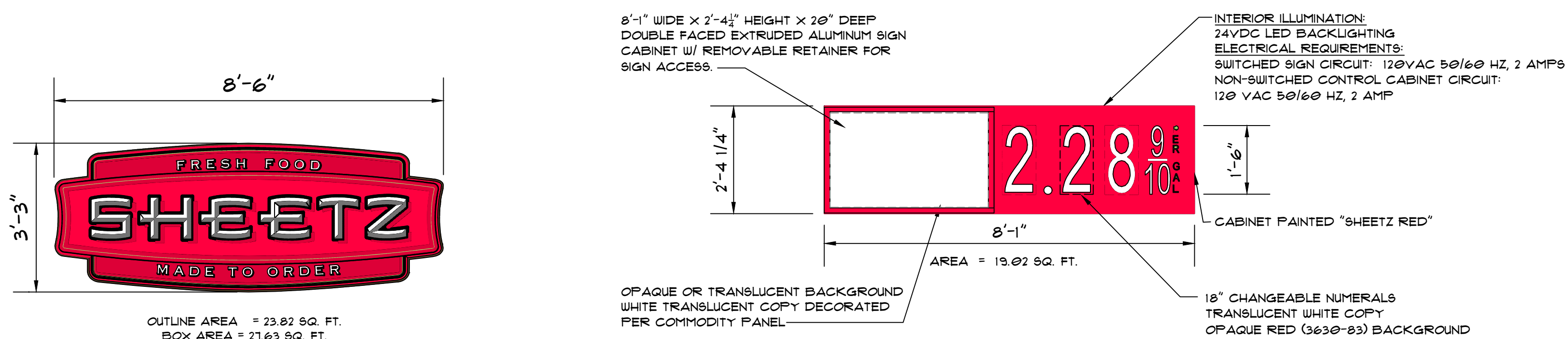


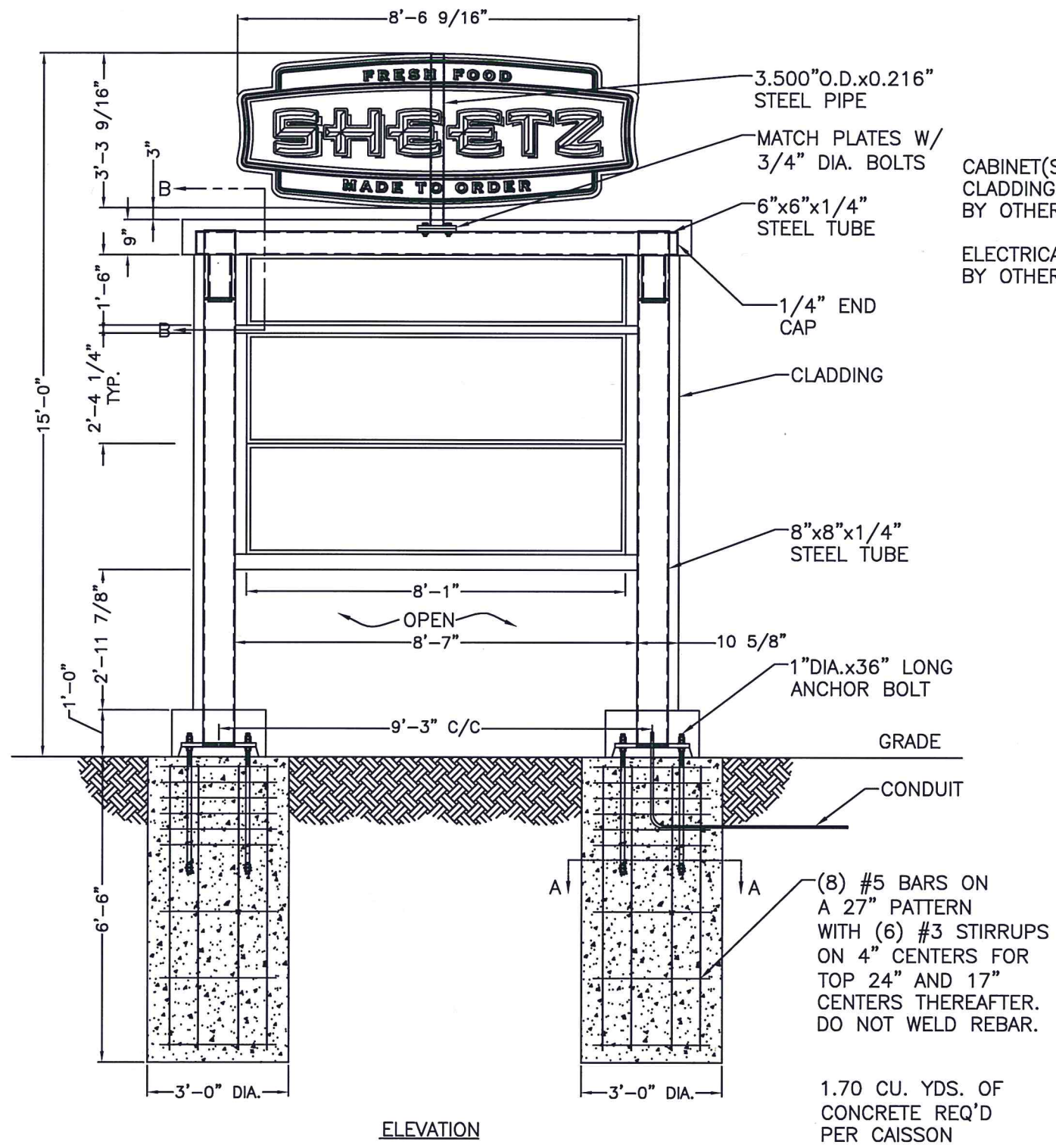
DOUBLE-FACED GAS PRICE SIGN DETAIL - PARTIAL ELEVATION
SCALE: 1/2" = 1'-0"
AREA: 84.69 SQ. FT.



DOUBLE FACED GAS PRICE SIGN SIDE ELEVATION
SCALE: 1/2" = 1'-0"

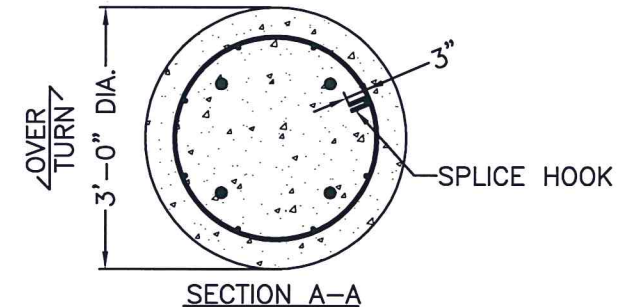
SIGN CABINET DETAILS
TOTAL SIGN AREAS: 84.69 SQ. FT.





ELEVATION

1.70 CU. YDS. OF CONCRETE REQ'D PER CAISSON



General Notes:

- Design is based on 120 mph, 3 second gust wind design per NCBC 2018/IBC 2015. Category II, Exposure C. Seismic Design Category C.
- Spread foundation is based on a presumptive safe vertical soil bearing pressure minimum of 2000 psf. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact BLAIR SIGN COMPANY.
- Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.
- Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
- Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
- All voids between column base plate and foundation surface shall be completely filled with high-strength, non-shrink grout.
- Anchor bolts shall meet ASTM F1554 Grade 36. Exposed surfaces shall be galvanized or coated to prevent corrosion.
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CABINET(S) & CLADDING DESIGN BY OTHERS
ELECTRICAL DESIGN BY OTHERS

INSTALLATION ADDRESS:

SHEETZ - #716
2201 NC 24-87
CAMERON, NC

CLIENT:

blair
IMAGE ELEMENTS
5107 KISSELL AVENUE
ALTOONA, PA 16601
PHONE (814) 949-8287
FAX (814) 949-8293

REV	DATE	DESCRIPTION
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SEAL & SIGNATURE:

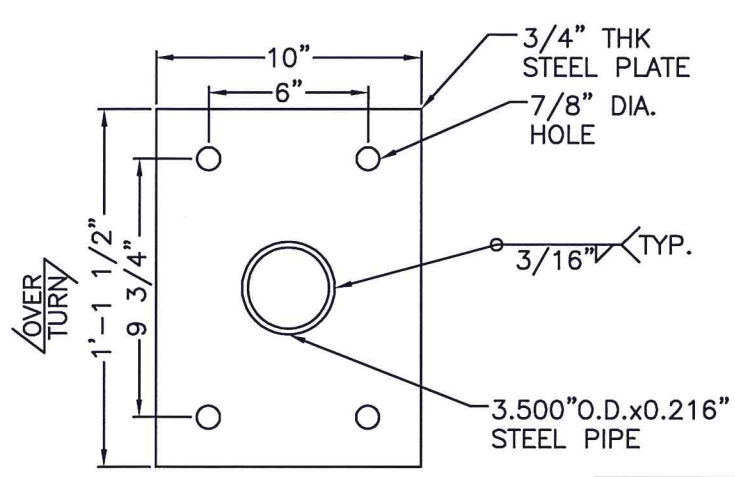


3.12.21



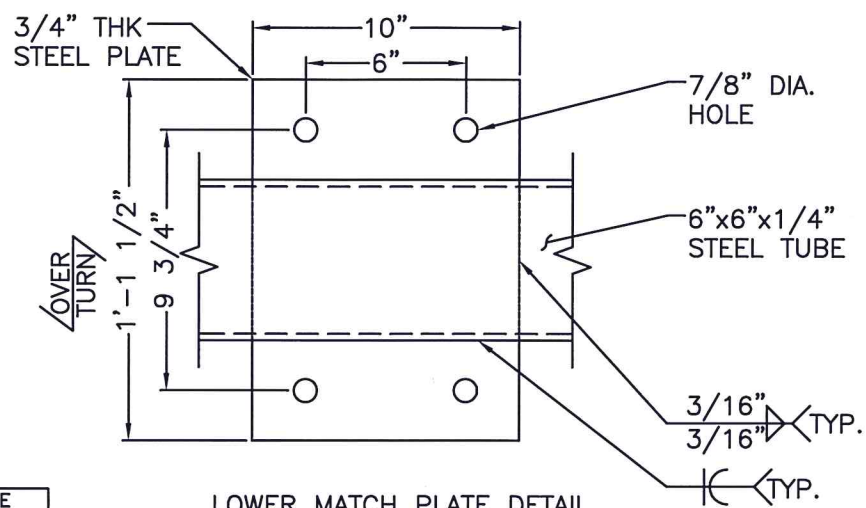
LINK Engineering, L.L.C.
135 South David Lane • Knoxville, Tennessee 37922
Phone: (865) 539-4001 • www.linkengr.com
North Carolina Certificate of Authorization No.: P-0483

Project Number: 21-0162		Drawing Number: B1071330	
SHT. 1	OF 3	DATE: 3/11/21	BY: GHK

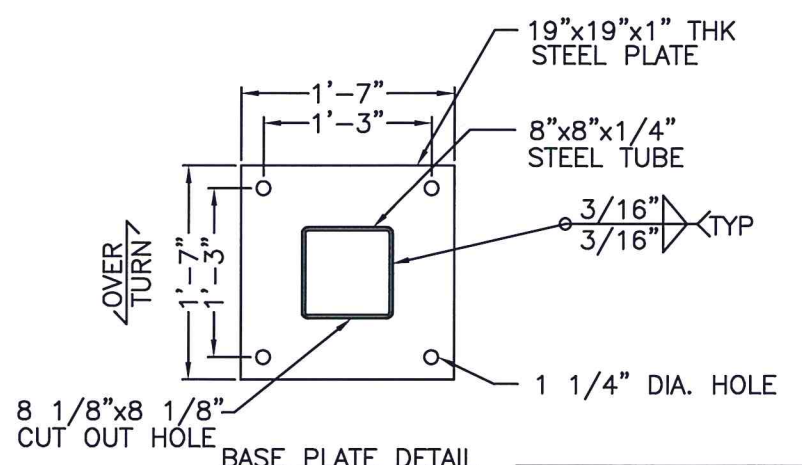


UPPER MATCH PLATE DETAIL

PARTICULAR CARE SHALL BE TAKEN TO PROVIDE SUFFICIENT PREHEAT OF THE THICKER ELEMENT FOR THE SOUNDNESS OF THE WELD. OTHERWISE, USE 1/4" MIN. WELD.

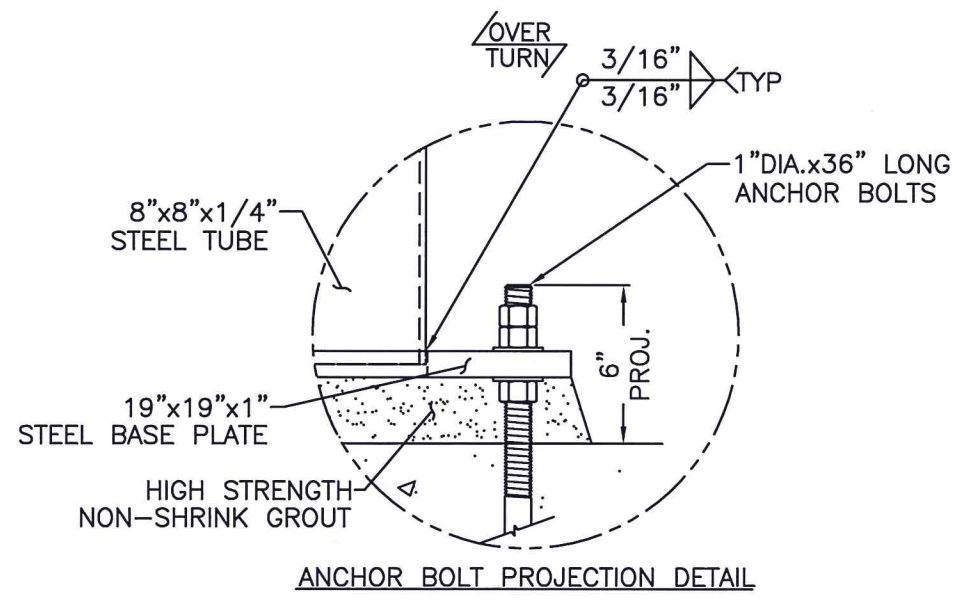


LOWER MATCH PLATE DETAIL



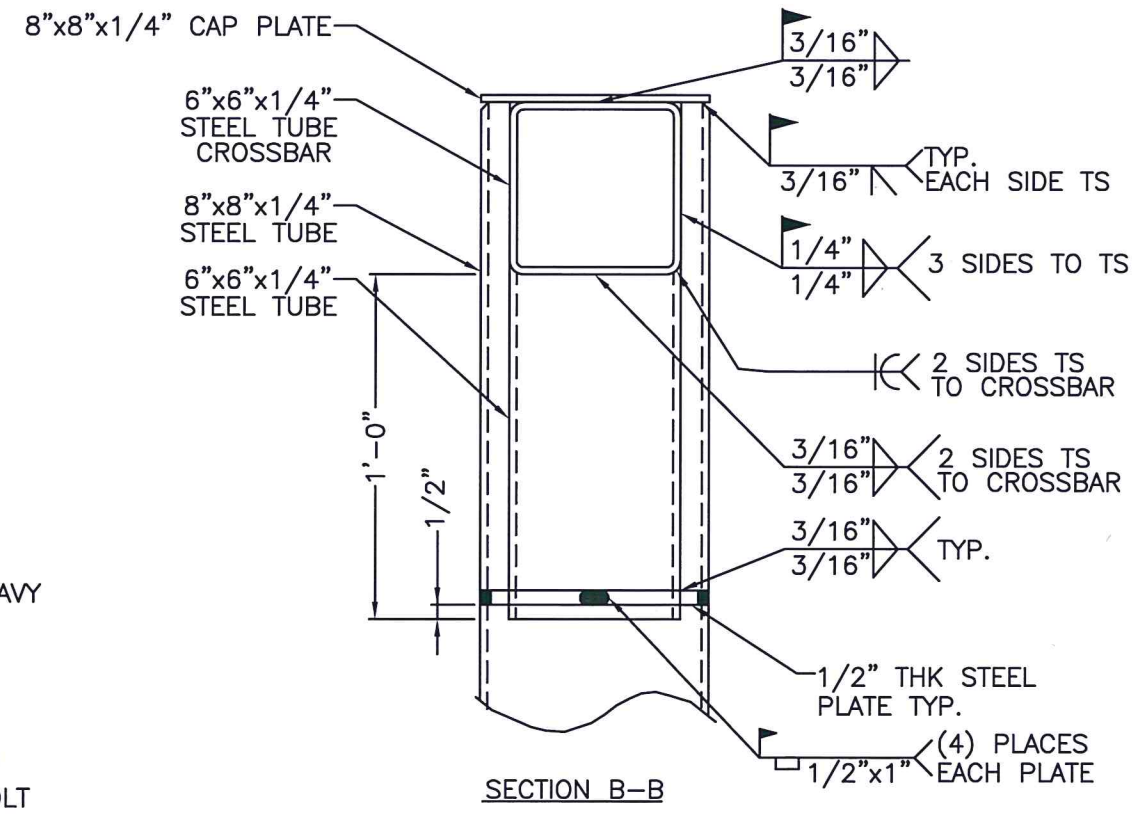
BASE PLATE DETAIL

PARTICULAR CARE SHALL BE TAKEN TO PROVIDE SUFFICIENT PREHEAT OF THE THICKER ELEMENT FOR THE SOUNDNESS OF THE WELD. OTHERWISE, USE 5/16" MIN. WELD.

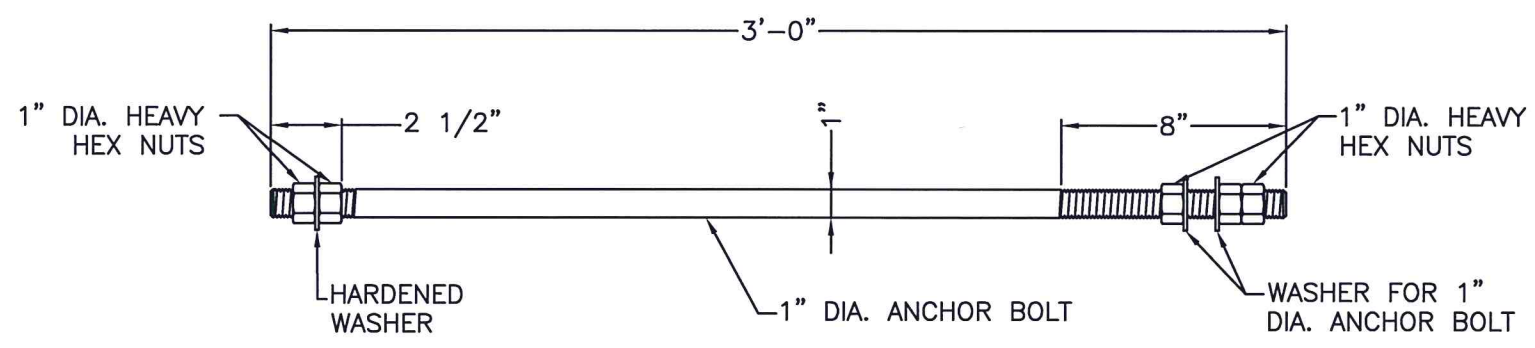


ANCHOR BOLT PROJECTION DETAIL

CUT SLOT IN TS 8X8 VERTICAL, SLEEVE IN TS 6X6, AND WELD AS SHOWN



SECTION B-B



ANCHOR BOLT DETAIL
ALL-THREADED RODS A-36 ARE ACCEPTABLE REPLACEMENT

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SEAL & SIGNATURE:

MAD N. KASHIF
NORTH CAROLINA PROFESSIONAL ENGINEER
SEAL 016218
3.12.21



LINK Engineering, L.L.C.

135 South David Lane • Knoxville, Tennessee 37922
Phone: (865) 539-4001 • www.linkengr.com
North Carolina Certificate of Authorization No.: P-0483

Project Number:		Drawing Number:	
21-0162		B1071330	
SHT.	OF	DATE:	BY:
2	3	3/11/21	GHK

PROJECT # 21-0162
 March 11, 2021
 DRAWING # B1071330
 WIND LOAD 22.116 PSF
 WIND SPEED 120 MPH
 # COLUMNS 2
 DESIGNER GHK

OWNER SHEETZ
 2201 NC 24-87
 CAMERON NC
 CLIENT: BLAIR SIGN COMPANY
 5107 KISSELL AVENUE
 ALTOONA, PA

ITEM	HEIGHT	WIDTH	SHAPE FACTOR	CENTROID HEIGHT	FACTORED AREA	TOTAL FORCE	MOMENT
SIGN	3.297	8.547	0.873	1.649	24.591	0.544	0.897
COLUMN	0.438	8.547	1.000	0.219	3.739	0.627	1.153
SIGN	7.276	10.354	1.000	3.638	75.337	2.293	11.773
COLUMNLOADING	3.990	1.771	1.000	1.995	4.880	2.401	21.136
DAH	15.000						

COLUMN CALCULATIONS (CODES P-RPEO-OTHER-T=TUBE)

ITEM	COLUMN WIDTH	COLUMN DEPTH	COLUMN WALL	lxx COLUMN	DESIGN MODULUS COLUMN	OBLIQUE LOAD MOMENT	AVAILABLE FLEXURAL STRENGTH	COLUMN CENTER DISTANCE	OBLIQUE LOAD FACTOR	UNITY
P SIGN		3.500	0.201	2.8	2.19	0.897	3.83	9.250	1.448	0.302
P COLUMN		3.500	0.201	2.8	2.19	1.153	3.83			0.369
T SIGN	8.000	8.000	0.233	70.7	17.67	8.52	44.14			0.193
T COLUMNLOADING	8.000	8.000	0.233	70.7	17.67	15.30	44.14			0.347

BOLT CALCULATIONS

ITEM	MOMENT	BOLT SPACING	BOLTS/PLATE	OBLIQUE BOLT	ALLOW. STRESS	ALLOWABLE TENSION
COLUMN	1.153	9.750	4.000	0.710	0.750	20.000
BASEPL.	21.136	15.000	4.000	6.120	1.000	19.100

PLATE CALCULATIONS

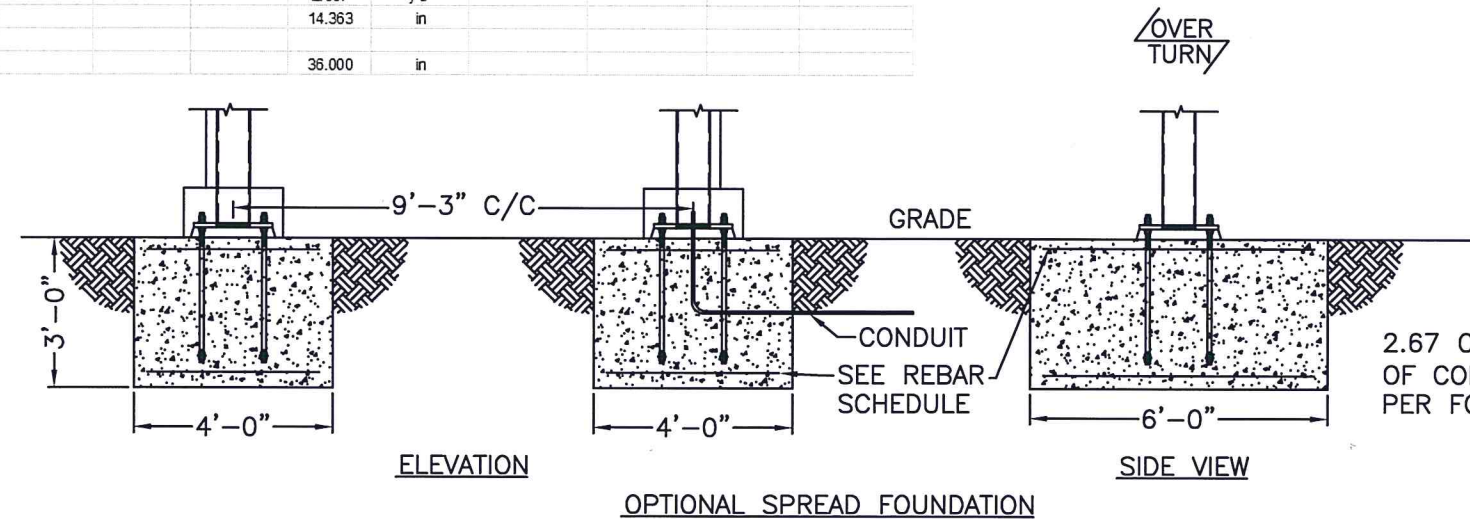
ITEM	TENSION BOLT	MOMENT ARM	MOMENT PLATE	PLATE WIDTH	PLATE DEPTH	PLATE THICK.	MINIMUM THICK.
COLUMN	0.710	3.969	2.816	5.875	13.500	0.750	0.326
BASEPL.	6.120	5.250	32.129	10.906	19.000	1.000	0.809

ANCHOR BOLT PROJECTION	ANCHOR EMBEDMENT	ANCHOR BOLT MIN LENGTH
6.000	12.175	19.000

*USE 3/8" A.B.

SPREAD FOUNDATION	
MOMENT AT GRADE	15.300
TOTAL FORCE	1.738 kip
WEIGHT OF SIGN	0.698 kip
SLAB WIDTH	4.000 ft
SLAB LENGTH	6.000 ft
SLAB DEPTH	3.000 ft
SLAB WEIGHT	10.800 kip
TOTAL WEIGHT	11.498 kip
OVERTURNING MOMENT	20.513
FACTOR OF SAFETY	1.682
e = OT/MWT	1.784
L/2 - e	1.216
SOIL PRESSURE 2*WT/(3*(L/2-e)*WIDTH)	1576
CONCRETE	2.667 yd ³
MIN. THICKNESS W/O REBAR	14.363 in
SQRT(M*12*6*1.75/(178*12*WIDTH))	
ACTUAL THICKNESS	36.000 in

BOTTOM STEEL AREA REQ'D PER FT OF WIDTH		0.142
TOP STEEL AREA REQ'D PER FT OF WIDTH		0.056
LONG BOTTOM STEEL	REBAR SIZE	4.000
	WEIGHT PER FT	0.668
	SPACING	12.000 in
	AREA PER BAR	0.200 in ²
BOTTOM STEEL AREA REQ'D PER FT OF WIDTH		0.142
	AREA PER FT	0.200
	EST NO. REQ'D	4.000
	NUMBER REQ'D	4.000
	LENGTH	5.500 ft
	WEIGHT	14.696
	EDGE	6.000
LONG TOP STEEL	REBAR SIZE	4.000
	WEIGHT PER FT	0.668
	SPACING	12.000 in
	AREA PER BAR	0.200 in ²
TOP STEEL AREA REQ'D PER FT OF WIDTH		0.056
	AREA PER FT	0.200
	EST NO. REQ'D	4.000
	NUMBER REQ'D	4.000
	LENGTH	5.500 ft
	WEIGHT	14.696
	EDGE	6.000
CROSS STEEL	REBAR SIZE	4.000
	WEIGHT PER FT	0.668
	SPACING	12.000 in
	LENGTH	3.500 ft
	EST NO. REQ'D	6.000
	NUMBER REQ'D	6.000
	EDGE	6.000 in
	WEIGHT	28.056
	TOTAL WEIGHT	57.448
FOUNDATION WIDTH		4.000 ft
FOUNDATION LENGTH		6.000 ft
CAISSON		
MOMENT		15.300 FT-KIP
FORCE		1.738 KP
REFERENCE IBC 1807.3.2 & TABLE 1806.2		
ASSUME SOIL CLASS #4 SV, SP, SM, SC, GM & GC		
LATERAL BEARING PRESSURE - PS/FT OF DEPTH	150.0	PS/FT
S1	650.0	
DEPTH	6.500	FT.
DIAMETER	3.000	FT.
	8.804	FT.
	2.085	FT.
	5.636	FT.
	27.666	IN.
ACTUAL DIAMETER	36.000	IN.
CONCRETE	1.702	CU. YD.



2.67 CU. YDS.
 OF CONC. REQ'D
 PER FOUNDATION

REBAR SCHEDULE DO NOT WELD REBAR
 SPREAD FOUNDATION 3" MIN. CONC. COVER

PLACEMENT	SIZE	SPACING	QUANTITY
LONG BOTTOM STEEL	#4	12"	4
LONG TOP STEEL	#4	12"	4
BOTTOM CROSS STEEL	#4	12"	6
TOP CROSS STEEL	#4	12"	6

INSTALLATION ADDRESS:
 SHEETZ - #716
 2201 NC 24-87
 CAMERON, NC

CLIENT:
blair
 IMAGE ELEMENTS
 5107 KISSELL AVENUE
 ALTOONA, PA 16601
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SEAL & SIGNATURE:

 MAD N. KASHIH
 3/12/21

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 North Carolina Certificate of Authorization No.: P-0483

Project Number:		Drawing Number:	
21-0162		B1071330	
SHT.	OF	DATE:	BY:
3	3	3/11/21	GHK

PROJECT NAME:
**NEW SHEETZ SITE
CAMERON**

Cameron, NC
PSR #214196

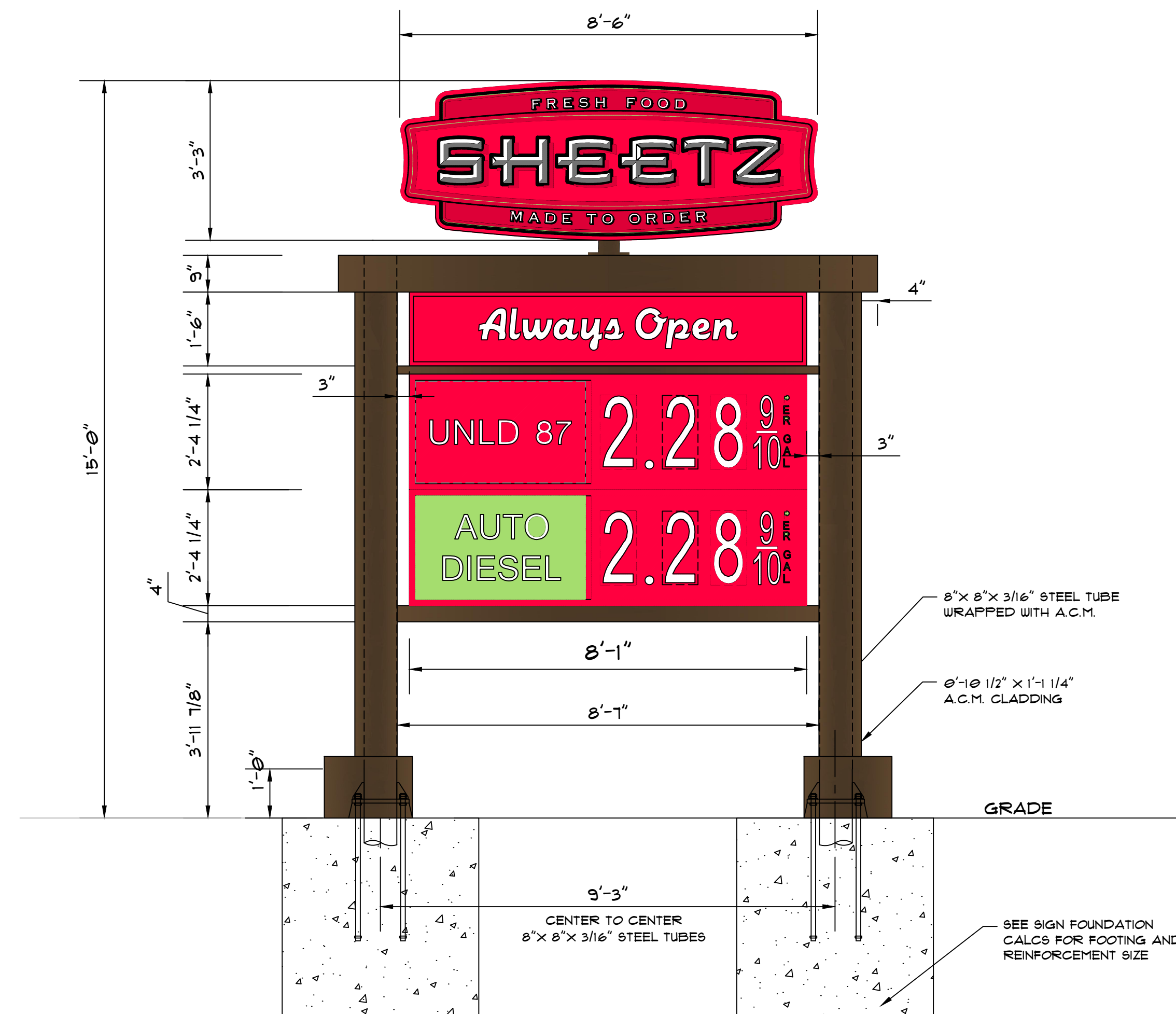
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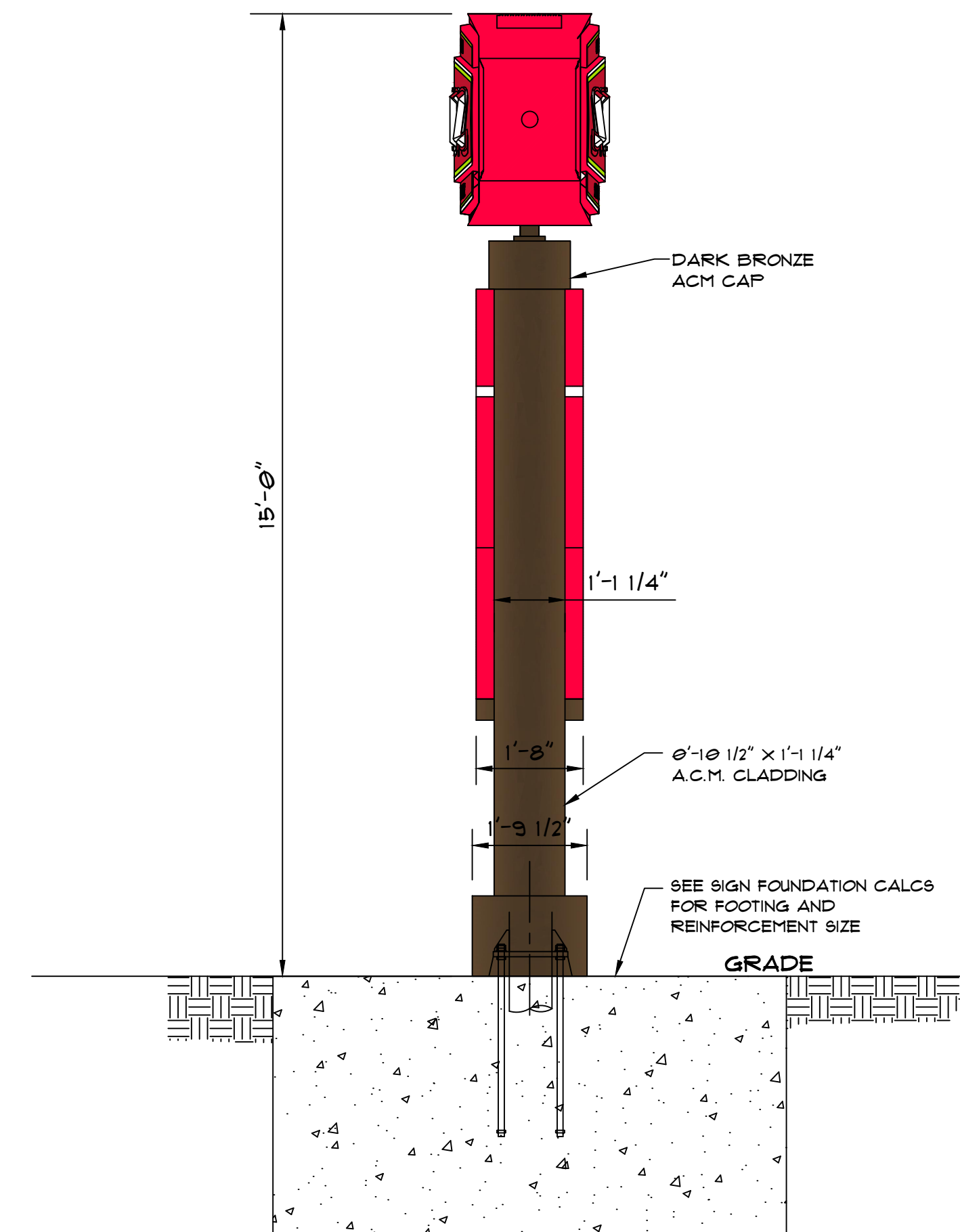
CONSULTANT

PROFESSIONAL

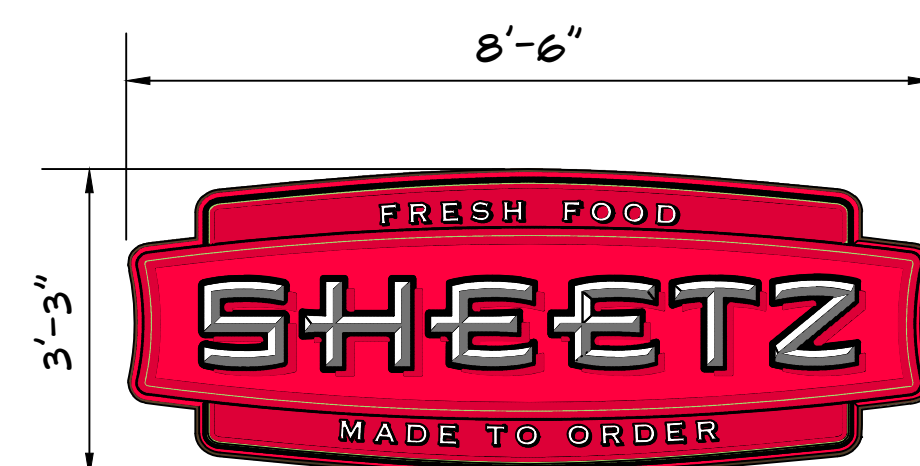
KEYPLAN



DOUBLE-FACED GAS PRICE SIGN DETAIL - PARTIAL ELEVATION
SCALE: 1/2" = 1'-0"
AREA: 96.82 SQ. FT.



DOUBLE FACED GAS PRICE SIGN SIDE ELEVATION
SCALE: 1/2" = 1'-0"



OUTLINE AREA = 23.82 SQ. FT.
BOX AREA = 21.63 SQ. FT.



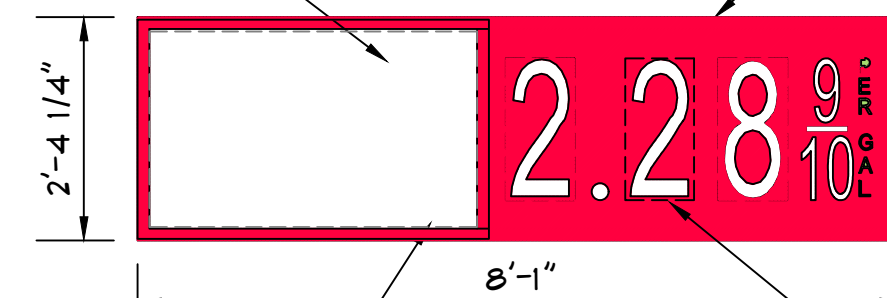
AREA: 12.13 SQ. FT.

SIGN CABINET DETAILS

TOTAL SIGN AREAS: 96.82 SQ. FT.

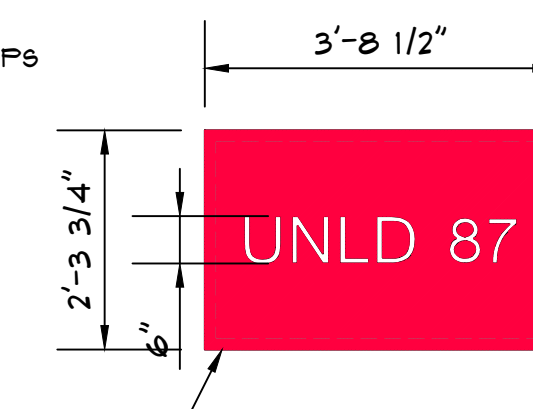
8'-1" WIDE X 2'-4 1/4" HEIGHT X 20" DEEP
DOUBLE FACED EXTRUDED ALUMINUM SIGN
CABINET W/ REMOVABLE RETAINER FOR
SIGN ACCESS.

OPAQUE OR TRANSLUCENT BACKGROUND
WHITE TRANSLUCENT COPY DECORATED
PER COMMODITY PANEL

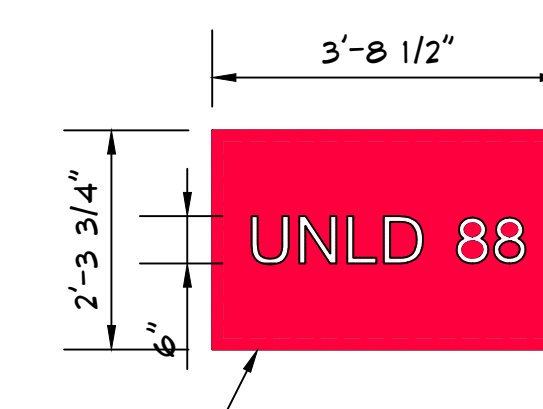


INTERIOR ILLUMINATION:
24VDC LED BACKLIGHTING
ELECTRICAL REQUIREMENTS:
SWITCHED SIGN CIRCUIT: 120VAC 50/60 HZ, 2 AMP
NON-SWITCHED CONTROL CABINET CIRCUIT:
120 VAC 50/60 HZ, 2 AMP

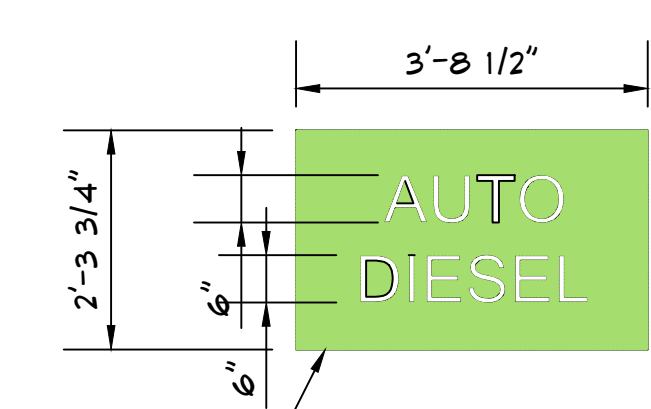
18" CHANGEABLE NUMERALS
TRANSLUCENT WHITE COPY
OPAQUE RED (3630-83) BACKGROUND



OPAQUE BACKGROUND
RED 3M 3630-83
WHITE TRANSLUCENT COPY



OPAQUE BACKGROUND
RED 3M 3630-83
WHITE TRANSLUCENT COPY

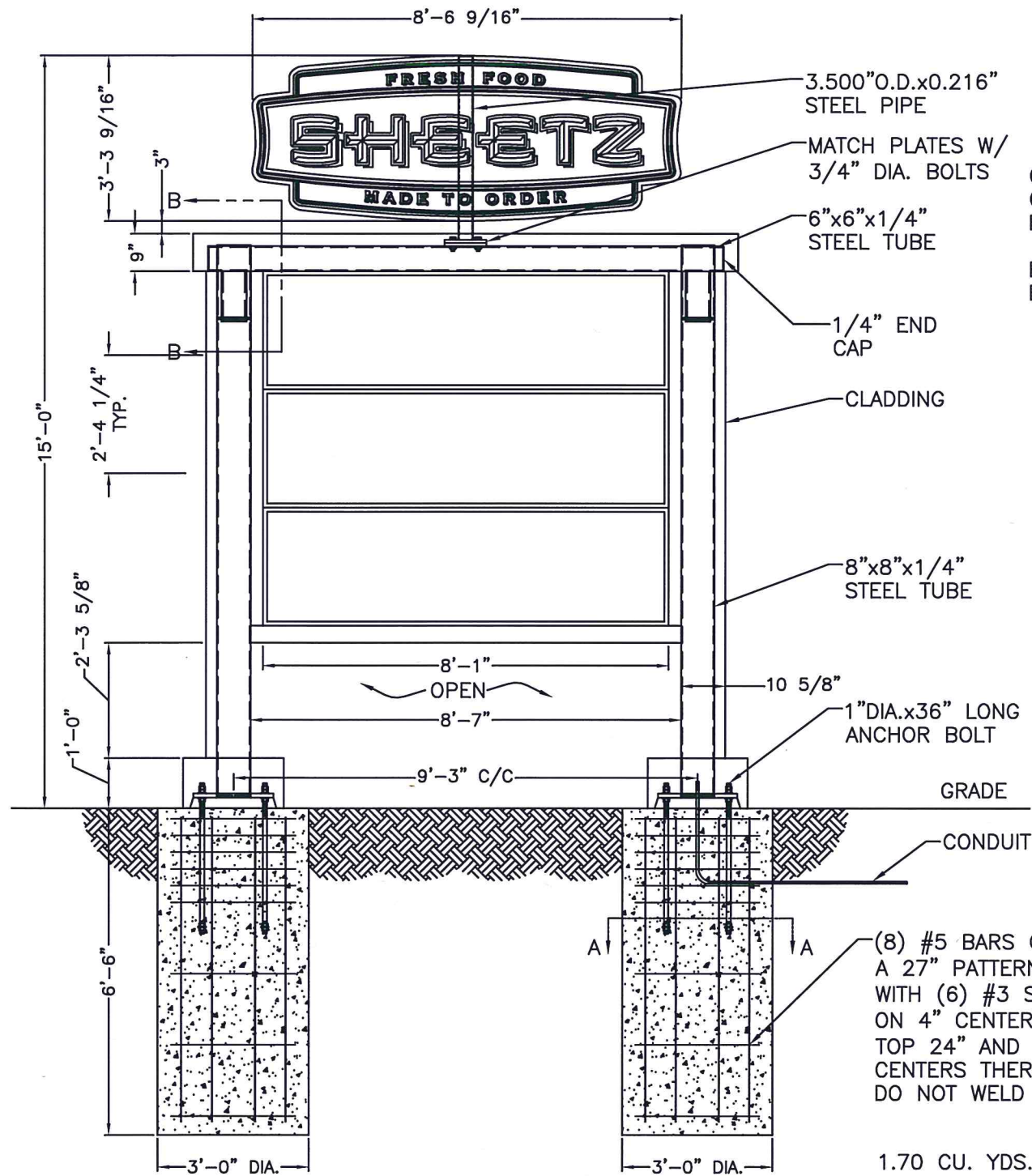


OPAQUE BACKGROUND
GREEN 3M 3630-26
WHITE TRANSLUCENT COPY

ISSUE: 7-15-2020
PROJECT NO:
AUTHOR BY: NMI
REVIEW BY:
SHEET TITLE

POLE SIGN
DETAILS

PS2



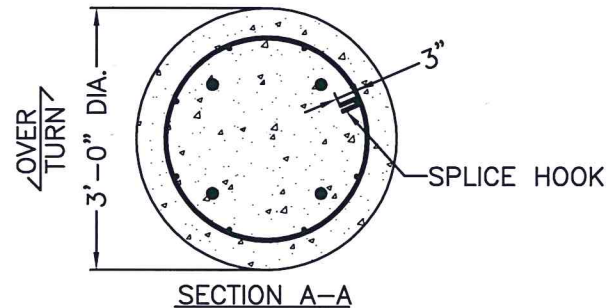
ELEVATION

CABINET(S) &
CLADDING DESIGN
BY OTHERS

ELECTRICAL DESIGN
BY OTHERS

(8) #5 BARS ON
A 27" PATTERN
WITH (6) #3 STIRRUPS
ON 4" CENTERS FOR
TOP 24" AND 17"
CENTERS THEREAFTER.
DO NOT WELD REBAR.

1.70 CU. YDS. OF
CONCRETE REQ'D
PER CAISSON



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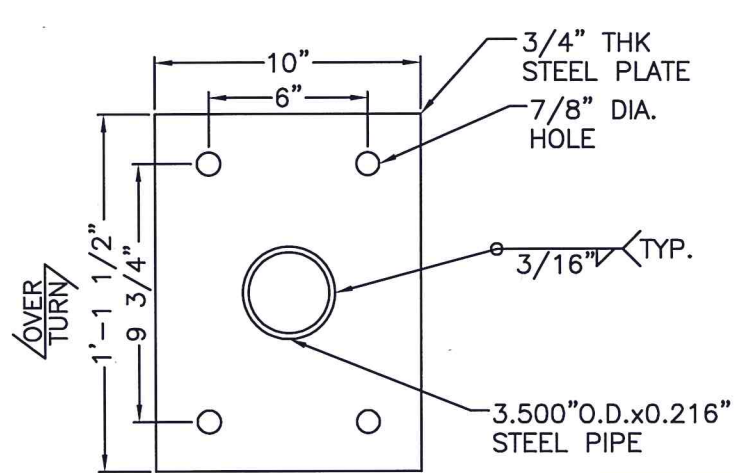
[Signature]
NORTH CAROLINA
PROFESSIONAL
SEAL
016218
ENGINEER
MAD N. KASHIE
3.12.21



LINK Engineering, L.L.C.

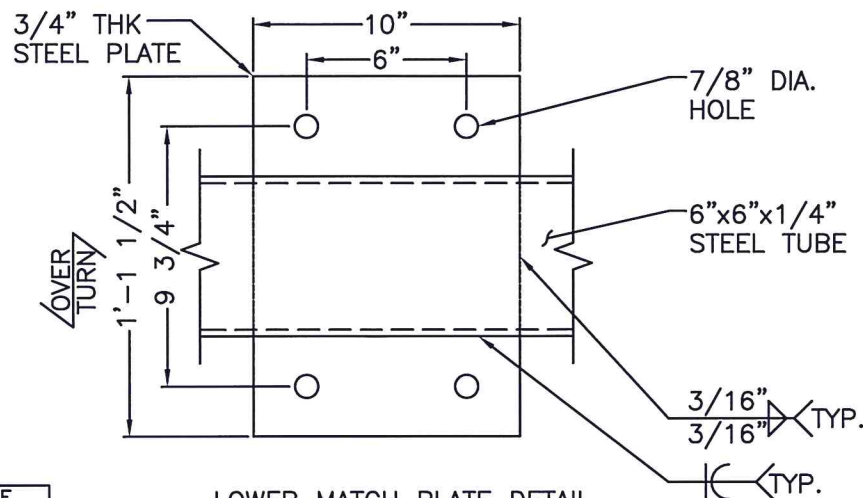
135 South David Lane • Knoxville, Tennessee 37922
Phone: (865) 530-4001 • www.linkengr.com
North Carolina Certificate of Authorization No.: P-0483

Project Number: 21-0162		Drawing Number: B1071331	
SHT. 1	OF 3	DATE: 3/11/21	BY: GHK

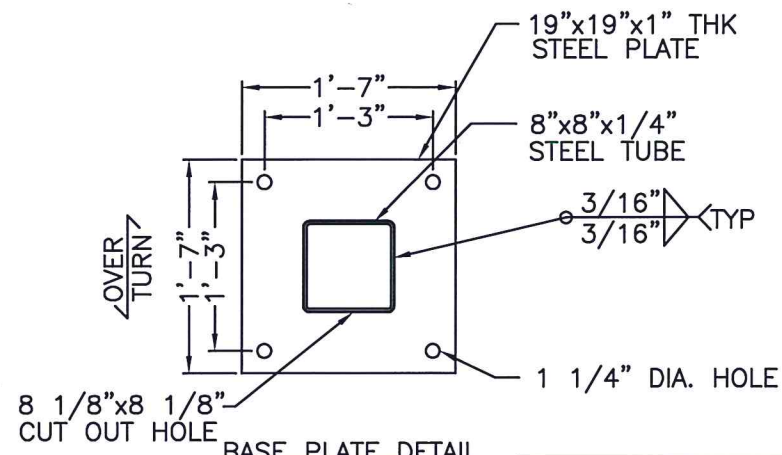


UPPER MATCH PLATE DETAIL

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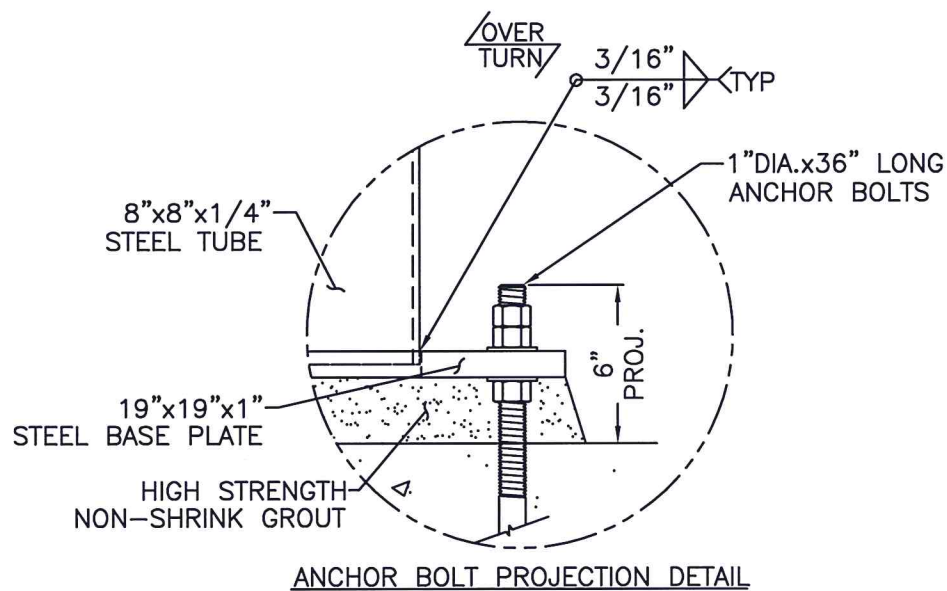


LOWER MATCH PLATE DETAIL



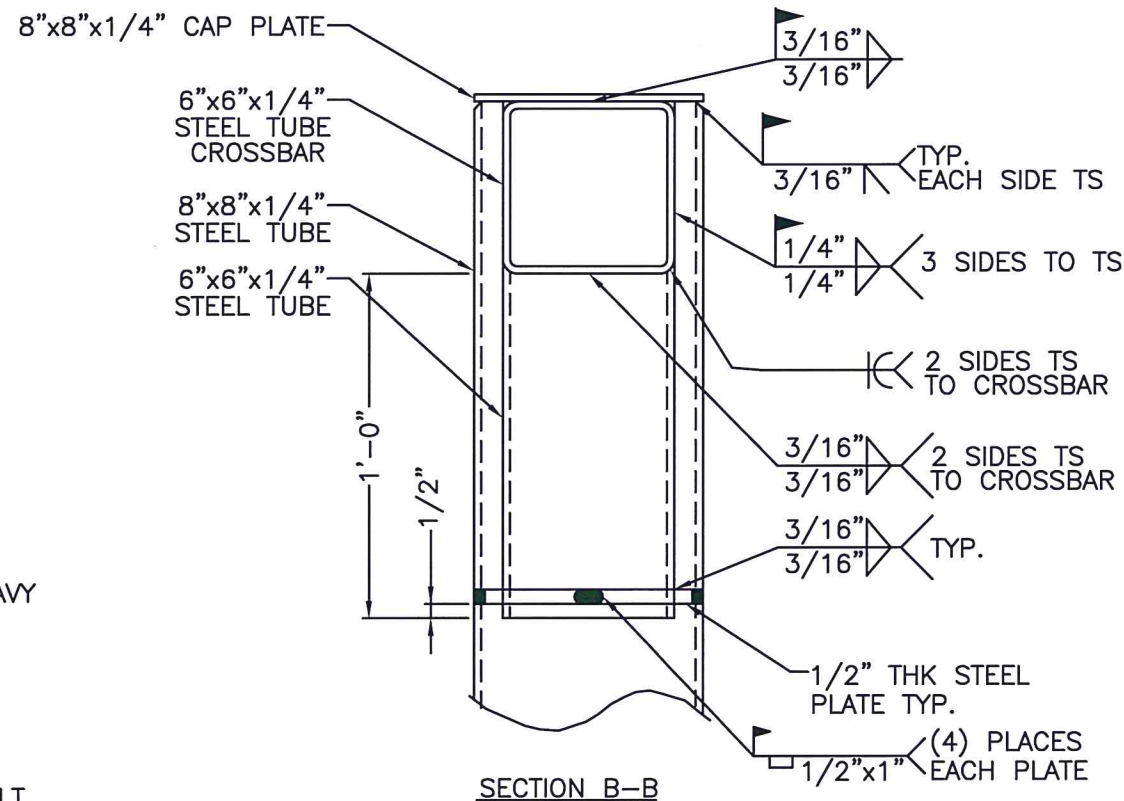
BASE PLATE DETAIL

PARTICULAR CARE SHALL BE TAKEN TO PROVIDE SUFFICIENT PREHEAT OF THE THICKER ELEMENT FOR THE SOUNDNESS OF THE WELD. OTHERWISE, USE 5/16" MIN. WELD.

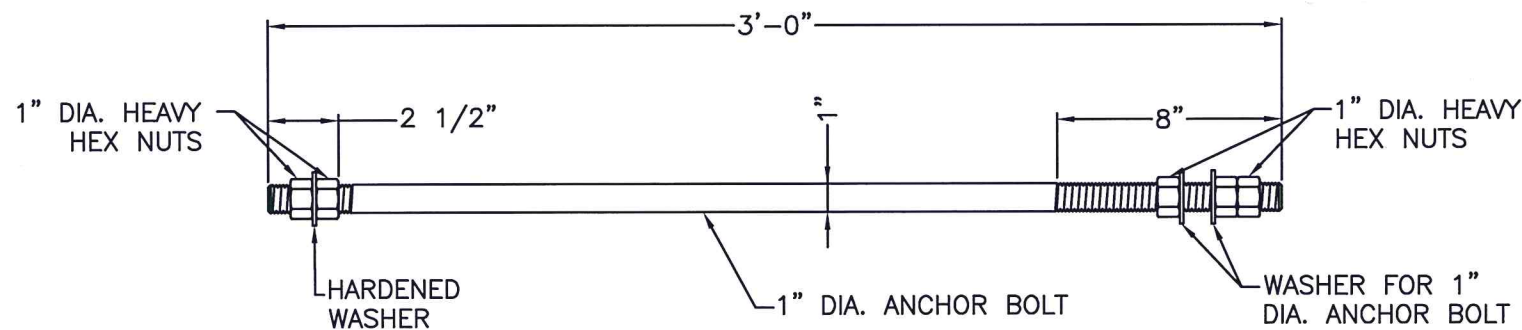


ANCHOR BOLT PROJECTION DETAIL

CUT SLOT IN TS 8X8 VERTICAL, SLEEVE IN TS 6X6, AND WELD AS SHOWN



SECTION B-B



ANCHOR BOLT DETAIL
ALL-THREADED RODS A-36
ARE ACCEPTABLE REPLACEMENT

INSTALLATION ADDRESS:

SHEETZ - #716
2201 NC 24-87
CAMERON, NC

CLIENT:

blair
IMAGE ELEMENTS
5107 KISSELL AVENUE
ALTOONA, PA 18601
PHONE (814) 949-8287
FAX (814) 949-8293

REV	DATE	DESCRIPTION
△	-/-/-	-----
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SEAL & SIGNATURE:



LINK Engineering, L.L.C.

135 South David Lane • Knoxville, Tennessee 37922
Phone: (865) 538-4001 • www.linkengr.com

North Carolina Certificate of Authorization No.: P-0483

Project Number:		Drawing Number:	
21-0162		B1071331	
SHT.	OF	DATE:	BY:
2	3	3/11/21	GHK

PROJECT #	21-0162	OWNER:	SHEETZ
March 11, 2021			2201 NC 24-87
DRAWING #	B1071331		CAMERON, NC
WIND LOAD	21.726	PSF	
WIND SPEED	120	MPH	CLIENT: BLAIR SIGN COMPANY
# COLUMNS	2	NCBC 2018/IBC 2015	5107 KISSELL AVENUE
DESIGNER	GHK		ALTOONA, PA

ITEM	HEIGHT	WIDTH	SHAPE FACTOR	CENTROID HEIGHT	FACTORED AREA	TOTAL FORCE	MOMENT
SIGN	3.297	8.547	0.873	1.649	24.591	0.534	0.881
COLUMN	0.438	8.547	1.000	0.219	3.739	0.616	1.133
SIGN	7.964	10.354	1.000	3.982	82.456	2.407	13.167
COLUMN/CLADDING	3.302	1.771	1.000	1.651	4.039	2.495	21.260
OAH	15.000						

ITEM	COLUMN WIDTH	COLUMN DEPTH	COLUMN WALL	kx	DESIGN MODULUS	OBLIQUE LOAD MOMENT	AVAILABLE FLEXURAL STRENGTH	COLUMN CENTER DISTANCE	OBLIQUE LOAD FACTOR	UNITY
P SIGN		3.500	0.201	2.8	2.19	0.881	3.83	9.250	1.448	0.295
P COLUMN		3.500	0.201	2.8	2.19	1.133	3.83			0.361
T SIGN	8.000	8.000	0.233	70.7	17.67	9.53	44.14			0.216
T COLUMN/CLADDING	8.000	8.000	0.233	70.7	17.67	15.39	44.14			0.349

ITEM	MOMENT	BOLT SPACING	BOLTS/PLATE	TENSION BOLT	BOLT DIAM.	ALLOW. STRESS	ALLOWABLE TENSION
COLUMN	1.133	9.750	4.000	0.697	0.750	20.000	8.836
BASE PL.	21.260	15.000	4.000	6.156	1.000	19.100	15.001

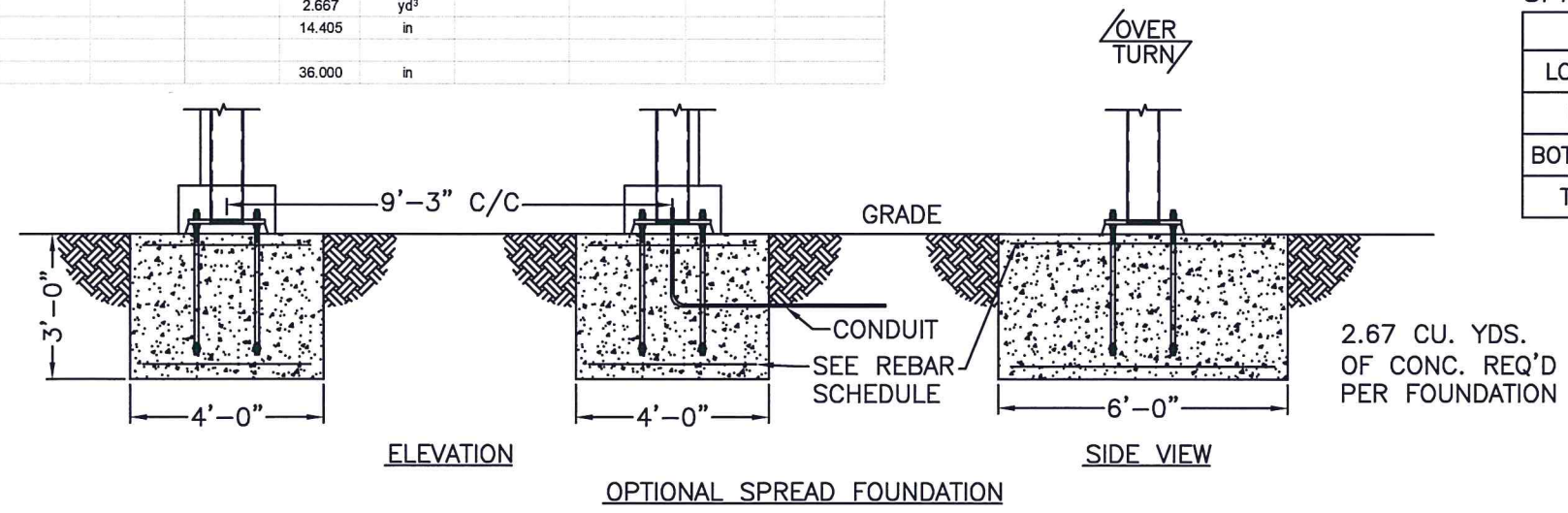
ITEM	TENSION BOLT	MOMENT ARM	MOMENT PLATE	PLATE WIDTH	PLATE DEPTH	PLATE THICK.	MINIMUM THICK.
COLUMN	0.697	3.969	2.767	5.875	13.500	0.750	0.323
BASE PL.	6.156	5.250	32.318	10.906	19.000	1.000	0.811

ANCHOR BOLT PROJECTION	6.000	ANCHOR EMBEDMENT	12.247	ANCHOR BOLT MIN. LENGTH	19.000
*USE 3/8" A.B.					
SPREAD FOUNDATION					
MOMENT AT GRADE	15.390				
TOTAL FORCE	1.806 kip				
WEIGHT OF SIGN	0.727 kip				
SLAB WIDTH	4.000 ft				
SLAB LENGTH	6.000 ft				
SLAB DEPTH	3.000 ft				
SLAB WEIGHT	10.800 kip				
TOTAL WEIGHT	11.527 kip				
OVERTURNING MOMENT	20.807				
FACTOR OF SAFETY	1.662				
e = OTM/WT	1.805				
L/2 - e	1.195				
SOIL PRESSURE 2*WT/(3*(L/2-e)*WIDTH)	1608				
CONCRETE	2.667 yd ³				
MIN. THICKNESS W/O REBAR	14.405 in				
SQRT(M*12*6*1.7*75/(.178*12*WIDTH))	36.000 in				
ACTUAL THICKNESS	36.000 in				

BOTTOM STEEL AREA REQ'D PER FT OF WIDTH	0.144
TOP STEEL AREA REQ'D PER FT OF WIDTH	0.056
LONG BOTTOM STEEL	
REBAR SIZE	4.000
WEIGHT PER FT	0.668
SPACING	12.000 in
AREA PER BAR	0.200 in ²
BOTTOM STEEL AREA REQ'D PER FT OF WIDTH	0.144
AREA PER FT	0.200
EST NO. REQ'D	4.000
NUMBER REQ'D	4.000
LENGTH	5.500 ft
WEIGHT	14.696
EDGE	6.000
LONG TOP STEEL	
REBAR SIZE	4.000
WEIGHT PER FT	0.668
SPACING	12.000 in
AREA PER BAR	0.200 in ²
TOP STEEL AREA REQ'D PER FT OF WIDTH	0.056
AREA PER FT	0.200
EST NO. REQ'D	4.000
NUMBER REQ'D	4.000
LENGTH	5.500 ft
WEIGHT	14.696
EDGE	6.000
CROSS STEEL	
REBAR SIZE	4.000
WEIGHT PER FT	0.668
SPACING	12.000 in
LENGTH	3.500 ft
EST NO. REQ'D	6.000
NUMBER REQ'D	6.000
EDGE	6.000 in
WEIGHT	28.056
TOTAL WEIGHT	57.448
FOUNDATION WIDTH	4.000 ft
FOUNDATION LENGTH	6.000 ft
CAISSON	
MOMENT	15.390 FT-KIP
FORCE	1.806 KIP
REFERENCE IBC 1807.3.2 & TABLE 1806.2	
ASSUME SOIL CLASS #4 SW, SP, SM, SC, GM & GC	
LATERAL BEARING PRESSURE - PSF/FT OF DEPTH	150.0 PSF/FT
S1	650.0
DEPTH	6.500 FT.
DIA METER	3.000 FT.
	8.522 FT.
	2.167 FT.
CALCULATED DEPTH	5.699 FT.
MINIMUM THICKNESS WITHOUT REINFORCEMENT	27.716 IN.
ACTUAL DIA METER	36.000 IN.
CONCRETE	1.702 CU. YD.

REBAR SCHEDULE DO NOT WELD REBAR
SPREAD FOUNDATION 3" MIN. CONC. COVER

PLACEMENT	SIZE	SPACING	QUANTITY
LONG BOTTOM STEEL	#4	12"	4
LONG TOP STEEL	#4	12"	4
BOTTOM CROSS STEEL	#4	12"	6
TOP CROSS STEEL	#4	12"	6



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3.12.21

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