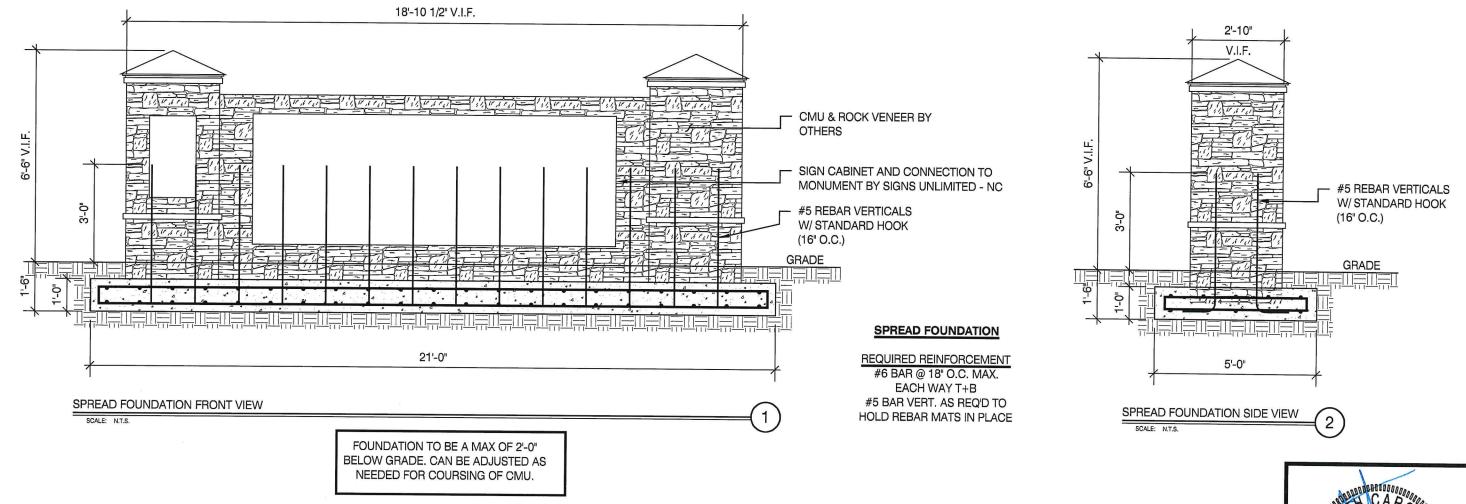
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NOTES

- SEE MANUFACTURERS DRAWINGS FOR ADDITIONAL DETAILS AND DIMENSIONS.
- 2.) SIGN CABINET AND CONNECTION BY SIGNS UNLIMITED NC.
- * CLIENT SIGNS UNLIMITED NC
- * 2018 NORTH CAROLINA BUILDING CODE
- * RISK CATEGORY II
- * 120 MPH WIND SPEED, EXP. C
- * SEISMIC DESIGN CATEGORY C
- * (1) FOOTING





MBI COMPANIES INC.

299 N. WEISGARBER RD. KNOXVILLE, TN 37919

PHONE 865.584.0999 SIGN-ENGINEER.COM 215 Brightwater Dr., Lillington, NC 27546

DRAWING TITLE:

Brightwater Campus Harnett Health

 DRAWN BY:
 CHECKED BY:
 COMM. NO.
 DATE:

 FCM
 DSA
 200149.011
 03/05/20

 REV #
 DATE
 DRAWN BY

03/05/20 DWG.

DRAWING NO.



GROUND SIGN DESIGN SPECIFICATIONS:

REFER TO SIGN COMPANY'S DRAWINGS FOR MORE DETAILS. ALL DESIGNS, DETAILING FABRICATION AND CONSTRUCTION SHALL CONFORM TO:

2018 NORTH CAROLINA BUILDING CODE

ACI

AISC

AMERICAN WELDING SOCIETY

LOCAL BUILDING CODES & ORDINANCES

CONCRETE: 3000 PSI @ 28 DAYS

STD. STEEL PIPE SECTION: ASTM A53 GRADE B (Fy=35 KSI), U.N.O.

STEEL PIPE SECTION (> 20" Ø): ASTM A252 GRADE 3 (Fy=42 KSI MIN.) U.N.O.

HSS ROUND SECTION: ASTM A500 GRADE B (Fy=42 KSI) U.N.O.

HSS SQUARE/RECTANGULAR SECTION: ASTM A500 GRADE B (Fy=46 KSI)

W SHAPES: ASTM A992 (Fy = 50 KSI)

ANCHOR BOLTS: ASTM F1554 GRADE 36 U.N.O. (ALTERNATES GRADE 55 & 105)

CONNECTION BOLTS: ASTM A325

THREADED RODS: ASTM A193 GRADE B7

STEEL ANGLES, CHANNELS, STRUCTURAL SHAPES & PLATES ASTM A36

REINFORCING: GRADE 60 ASTM A615

PROVIDE A MINIMUM OF THREE INCHES OF CONCRETE COVER OVER EMBEDDED STEEL.

THE CONTRACTOR (INSTALLER) IS RESPONSIBLE FOR THE MEANS & METHODS OF CONSTRUCTION IN REGARDS TO JOBSITE SAFETY.

NO FIELD HEATING FOR BENDING OR CUTTING OF STEEL SHALL BE ALLOWED WITHOUT THE ENGINEER'S APPROVAL.

WELDING ELECTRODES: E70XX

ALLOWABLE SOIL BEARING PRESSURE ASSUMED: 2000 PSF

ASSUMED HORIZONTAL (PASSIVE PRESSURE) ASSUMED AT 150 PSF/FT OF DEPTH. ISOLATED LATERAL BEARING FOUNDATIONS FOR SIGNS NOT ADVERSELY AFFECTED A 1/2" MOTION AT THE GROUND SURFACE DUE TO SHORT TERM LATERAL LOADS

SHALL BE PERMITTED TO BE DESIGNED USING TWO TIMES THE TABULATED CODE VALUES.

ALL FOOTINGS SHALL BEAR ON FIRM UNDISTURBED RESIDUAL SOIL AND/OR ENGINEERED EARTH.

FILL COMPACTED TO 98% OF ITS MAXIMUM DRY DENSITY AS PER ASTM D 698-70 (STANDARD PROCTOR) UNLESS NOTED OTHERWISE. THE SOIL BEARING CAPACITY IS TO BE VERIFIED BY A GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION. IF ALLOWABLE BEARING AND/OR LATERAL PRESSURE IS LESS THAN THE ABOVE ASSUMED AND/OR CALCULATED PRESSURES, THE ENGINEER SHOULD BE CONTACTED FOR RE-EVALUATION.

EXCAVATION SHALL BE FREE OF LOOSE SOIL BEFORE POURING CONCRETE. WELDERS SHALL BE CERTIFIED FOR THE TYPE OF WELDING.

ADEQUATELY BRACE POLE(S) UNTIL CONCRETE HAS SET UP FOR 14 DAYS. GROUT UNDER BASE PLATES WITH NON-SHRINK GROUT.

THIS ENGINEER DOES NOT WARRANT THE ACCURACY OF DIMENSIONS FURNISHED BY OTHERS.

ALL EXPOSED STEEL SHALL BE PAINTED WITH AN ENAMEL PAINT TO INHIBIT CORROSION.

THIS DESIGN IS FOR THE INDICATED ADDRESS ONLY, AND SHOULD NOT BE USED AT OTHER LOCATIONS WITHOUT WRITTEN PERMISSION OF THE ENGINEER. DESIGN OF DETAILS AND STRUCTURAL MEMBERS NOT SHOWN, BY OTHERS.

Building Code	2018 North Car	rolin: Importance Factor, I	1.0	Damping Ratio, β	0.005
Wind Load Criteria	ASCE 7-10	Directionality Factor, K ₁ (7)	0.85	Natural Frequency, n.	0.00 Hz
Wind Speed, V	120 mph	Topography Factor, Ka	1.0	Gust Effect Factor, G	0.85
Exposure Category	С	Base Pressure, y(q,/K,	18.8 psf	ASD Wind Load Factor, y (3)	0.6

Notes: (1) Loading values in chart below are based upon average K, values for each segment. Actual values are calculated on hidden sheet using derived V-M equations. Chart is provided for information purposes only. (2) Wind directionality (K a) factor is 0.95 for Single Pole (Round) segments instead of 0.85. The C , value from Fig. 6-21 has been increased by 0.95/0.85 to account for this variation

DEFLECTION ANALYSIS

Deflection Limit

Deflection at 0.7*W

Section Location	Түре	Height ft	Width	Horiz. Offset ft	Area sq ft	Top Elev.	Centroid ft	K,	cı	Press.	Support Pole Loads			Footing Loads			
											Trib. Factor	Shear kips	Moment k-ft	Trib. Factor	Shear klps	Moment k-ft	
																	1
2		Single Pole w/ Cabinet	6.50	19.00	890000000000000000000000000000000000000	123.5	7.0	4.1	0.85	1.43	19.4	1.0	2.4	9.8	1.0	2.4	9.8
3		None				0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0	0.0	0.0	0.0	0.0
4		None				0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0	0.0	0.0		
5		None		11000		0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0	0.0	0.0	0.0	0.0
6		None				0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0	0.0		0.0	0.0
7		None				0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0		0.0	0.0	0.0
8		None				0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0	0.0	0.0	0.0	0.0
9		None				0.0	7.0	7.0	0.85	1.46	19.8	0.0		0.0	0.0	0.0	0.0
10	Тор	None				0.0	7.0	7.0	0.85	1.46	19.8	0.0	0.0	0.0	0.0	0.0	0.0
Overall Height: 6.50 ft							Summation based upon averages above:						2.4	9.8	0.0	2.4	9.8
OUNDA						Act		eactions b					2.4	9.9		2.4	9.9

Туре	Diameter	Width	Thickness	Length ft	Depth ft	Volume CY	Del-fi	1 -	Allowable Soil	
Caisson	ft	ft	ft				Reinforcing	Status	Pressure	
Vertical Slab									1	
✓ Spread		21.00		5.00	1.00	3.89	#6 at 18 in o.c. E.W. T&B	ОК	2000 psf	

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Brightwater Campus Harnett Health

risdiction Requirement

GEOMETRY INPUT (1)