114 W. Main Street, Clayton, North Carolina 27520



DATE: 09/16/2022

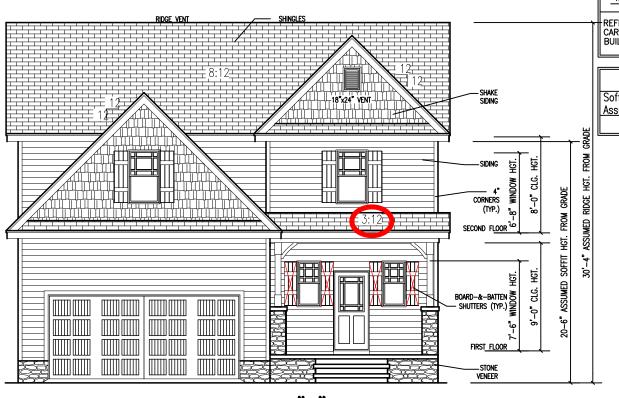
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ELEV C - CRAWL



Alton Fields Lot 7 - 159 Beebalm Run

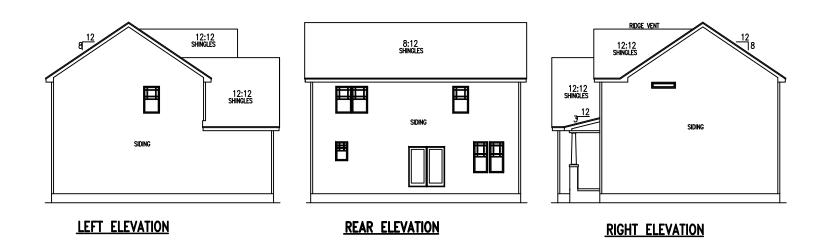


ATTIC SPACE VENTILATION $\underline{1265}$ SQ. FT. OF CLG. / 150 = $\underline{8.43}$ SQ. FT. REQUIRED REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

	MEAN ROOF HGT.		
	+ Highest Ridge Hgt. From Assumed Grade	÷2	= Mean Roof Hgt.
20'-6"	+ 30'-4"	÷2	= 25'-5" Mean Roof Hgt.

DOUBLE PAPER

FRONT ELEVATION "C"



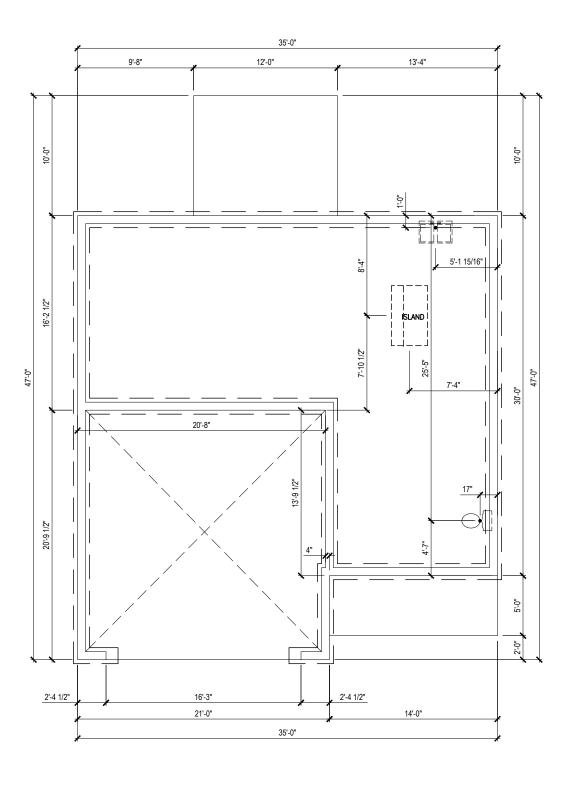
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PLUMBING

2



PLUMBING LAYOUT





Ш Z

PROJECT #: DRB2201-0265 PRINCETON

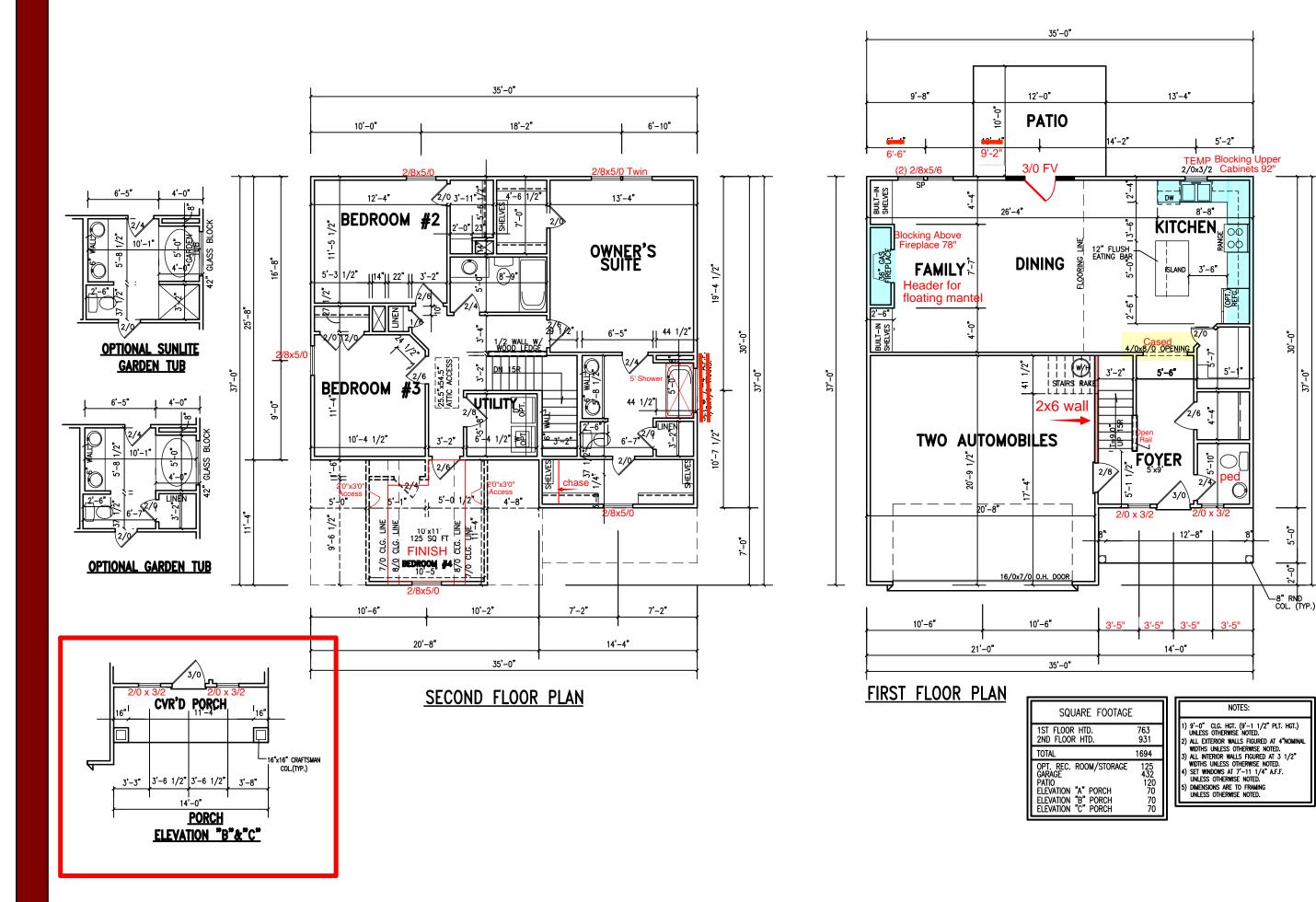
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FLOOR PLANS

OF 4



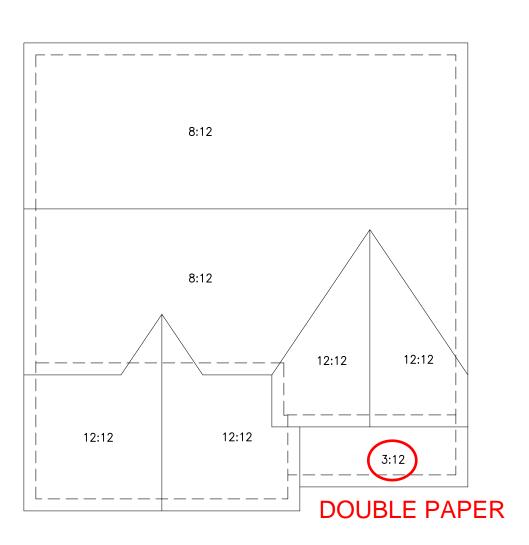
DATE: 09/16/2022

DRAWN BY: MMB

CHECKED BY: RB

ROOF

4 OF 2



ROOF FRAMING PLAN

DESIGN LOADS LIVE LOAD DEAD LOAD DEFLECTION (PSF) (PSF) FLOOR (secondary ATTIC (w/ storage) ATTIC (no access) EXTERNAL BALCONY ROOF ROOF TRUSS WIND LOAD BASED ON 120 MPH (EXPOSURE B) 35'-0" 35'-0" STRUCTURAL NOTES: 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL 9'-8" 12'-0" 13'-4" 9'-8" 12'-0" 13'-4" CODES AND REGULATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL THIS THE CONTINUE OF THE SEPTIMENT HELL MINENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS. ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI 4" CONC. SLAB ON GRADE 4" CONC. SLAB ON GRADE (IE. ILEVEL MICROLAM) ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER W (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"-8", PROVIDED HAT INE TOP OF THE WINDOW HEIGHT IS 6-6, MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1-6*. OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2410 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO) EXTERIOR LOAD CONDITIONS (UNO) REFERT TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT. ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO) ALL EXTERIOR LUMBER TO BE #2 SYP PT ALL CONCRETE, To. = 3000 PSI MIN. ALL CALLED CLUMBER TO BE #2 SYP DE TO SECTION OF THE STATE 8" MASONRY FOUNDATION WALL 8" MASONRY FOUNDATION WALL PER CODE ON A 16" WIDE X 8" THICK PER CODE ON A 16" WIDE X 8" THICK CONT. CONC. FTG., TYP. UNO CONT. CONC. FTG., TYP. UNO · 4" CONC. SLAB W/ 6x6 WI.4xWI.4 WWF OR FIBERMESH OVER 6 MIL VAPOR BARRIER OVER WASHED STONE OR COMPACTED FILL 4" CONC. SLAB W/ 6x6 WI.4xWI.4 WWF OR FIBERMESH OVER 6 MIL VAPOR BARRIER OVER WASHED STONE OR COMPACTED FILL ALL CONCRETE, fo = 3000 PSI MIN. PRESUMPTIVE BEARING CAPACITY = 2000 PSF 1/2"0 ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INFO CONCRETE OR MASONRY. PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9-0" (UNO) PROVIDE A MINIMUM OF 500# UPLIET & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF 24" X 24" X 8" 24" X 24" X 8" 16" W X 8" TH. CONC. 16" W X 8" TH. CONC. CONC. FTG. CONC. FTG. LUG FTG, TYP. -LUG FTG, TYP. -THE 2018 IRC. MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION. UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION. METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL. --... --¹∟ ___ . 20'-8" 20'-8" 5'-6 1/4" 24" X 24" X 8" 24" X 24" X 8" CONC. FTG. 30" X 30" X 10" 30" X 30" X 10" - 30" X 30" X 10" - 30" X 30" X 10" CONC. FTG. CONC. FTG. 4" CONC. SLAB W/ 6x6 4" CONC. SLAB W/ 6x6 4" CONC. SLAB W) 6x6 W) 4xW) 4 WWP OR FIBERMESH OVER 6 MIL VAPOR BARRIER OVER COMPACTED FILL OR FIRM RESIDUAL SLOPE FOR DRAINAGE. WI.4xWI.4 WWF OR FIBERMESH OVER 6 MIL VAPOR BARRIEF OVER COMPACTED FILL OR FIRM RESIDUAL SLOPE FOR DRAINAGE. B" MASONRY FOUNDATION 4" CONC. SLAB ON GRADE 4" CONC. SLAB ON GRADE WALL PER CODE W/ STONE VENEER ON A 20" WIDE X 8" THICK CONT. CONC. FTG., TYP. @ FRONT ELEV. SECURE PT 4" X 4" POST TO SECURE PT 4" X 4" POST TO 16" X 16" X 10" CONC. FTG MASONRY PIER W/ SIMP. ABU44Z W/ SIMP, ABU44Z W/ 5/8' W/ 5/8" DIA. ANCHOR BOLTS W/ 6" DIA. ANCHOR BOLTS W/ 6" MIN. EMBED., TYP. – 11'-4" PORTAL FRAME PER DETAIL B2 ON SHEET D3 PORTAL FRAME PER DETAIL B2 ON SHEET D3 16" X 16" MASONRY PIER MIN. EMBED., TYP. ON A 24" X 24" X 8" 16'-3" 2'-4 1/2" 2'-4 1/2" 2'-4 1/2" CONC. FTG., TYP. 21'-0" 14'-0" 21'-0" 14'-0" 35'-0" 35'-0" **ELEVATION "B" & "C" ELEVATION "A" FOUNDATION PLAN (STEM WALL)** PORCH ELEV. A 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC. 1/8" = 1'-0" (11"x17") 1/4" = 1'-0" (24"x36")

* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. * Any deviations or discrepancies on

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

 Please review these documents carefully.
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> TYNDALL ENGINEERING & DESIGN, P.A.



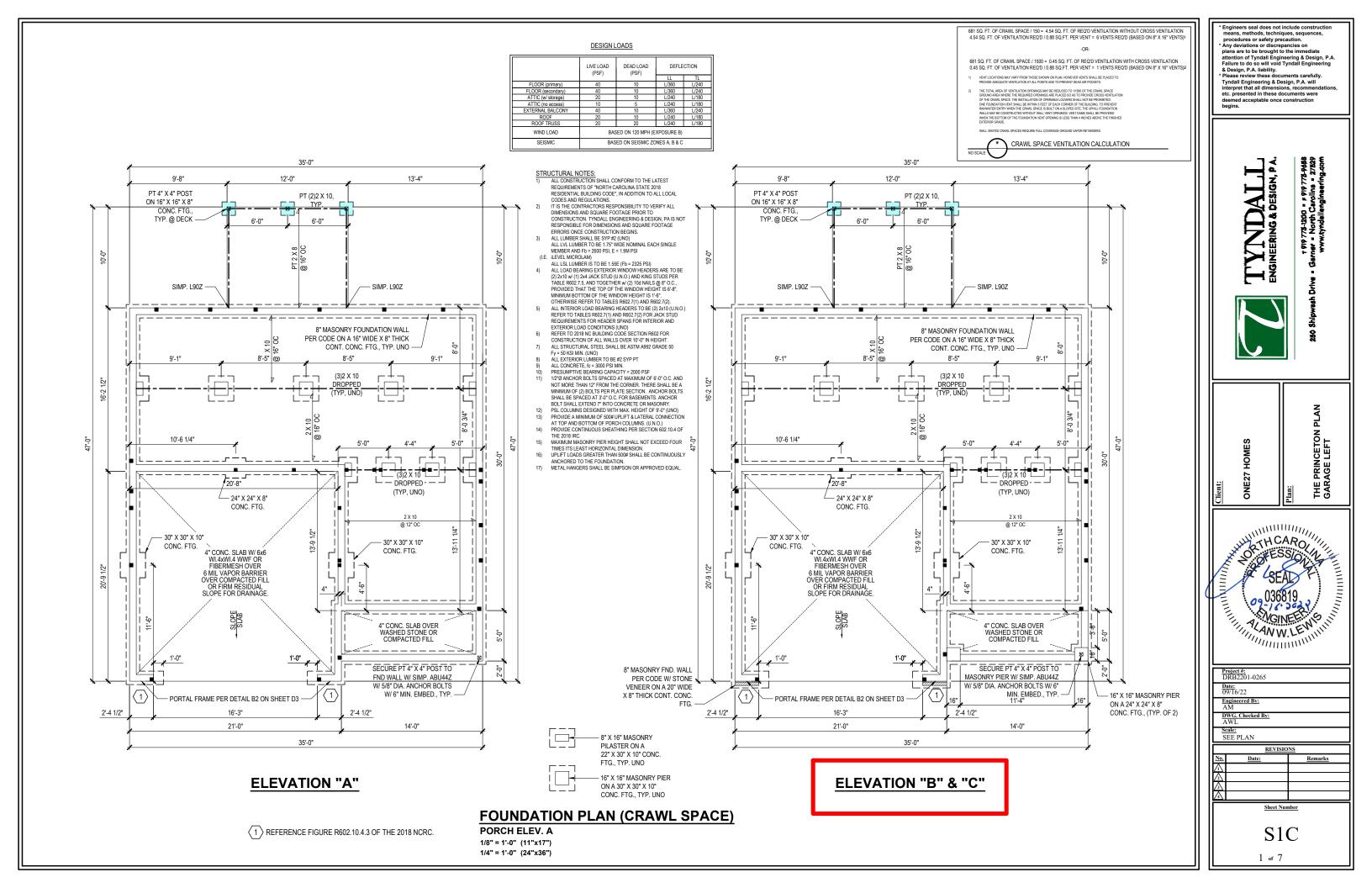
Plan: THE PRINCETON PLAN GARAGE I FET

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| Project #:
| DRB2201-0265 |
| Date:
| 09/16/22 |
| Engineered By:
| AM |
| DWG. Checked By:
| AWL |
| Scale:
| SEE PLAN |
| REVISIONS

Sheet Number

S1B



DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	` ′	, ,	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)	
SEISMIC	BAS	ED ON SEISMIC ZO	NES A, B & C	

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION, TYNDAU AND SUDANE FUOLING FINDS FOR THE OFFICE OF THE MENSIONS ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS. ALL LUMBER SHALL BE SYP #2 (UNO)

 ALL LYL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (I.E. ILEVEL MICROLAM)
- .. ILEVEL MICKOLAM).
 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w
 (1) 2x4 JACK 2X10 (JIN.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (LLN O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- (UNO)
 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT. ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)

 ALL EXTERIOR LUMBER TO BE #2 SYP PT

 ALL CONCRETE, fc = 3000 PSI MIN.

 PRESUMPTIVE BEARING CAPACITY = 2000 PSF

- PRESOMPTIVE BEARING CAPACITY = 2000 PSF 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
 PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
 UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS.

 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF
- THE 2018 NCRC.

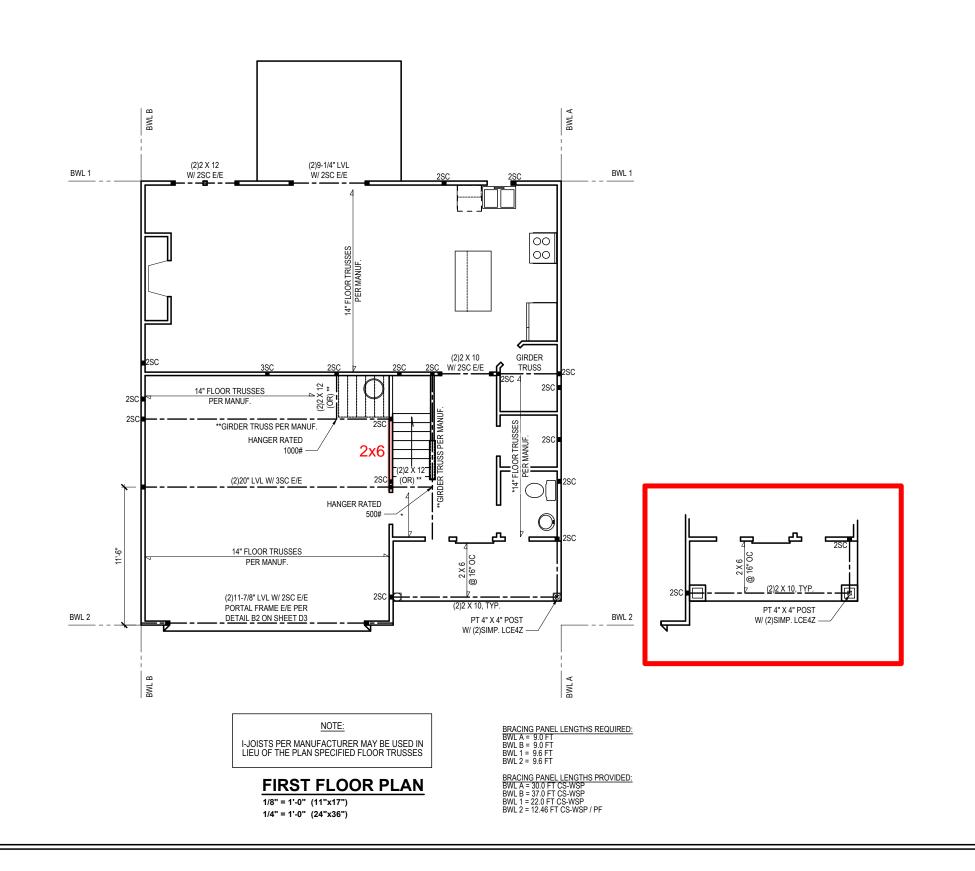
 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.
 REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- (2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0" (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING).
 SECURE W5 50 COULER NAILS (OR EQUAL PER TABLE R702.3.5)
 SPACED Ø" 70 C. AT PANEL EOGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- (3) 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ALL SHEAI HABILE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 38'S "SHEATHING SHALL BE SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6' O.C. AT PANILE LOGES AND SPACED AT 12' O.C. AT INTERMIEDIATE SUPPORTS.

 MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
- BE AS FOLLOWS:
 - S FOLLOWS:

 24* ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT

 30* ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.

 48* FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- (4) SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIET DESIGN. VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR
- (5) MINIMUM 800# HOLD-DOWN DEVICE

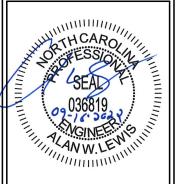


Engineers seal does not include con procedures or safety precau

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Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendetc. presented in these documents were





Project #: DRB2201-0265 Date: 09/16/22 SEE PLAN REVISIONS

Sheet Number

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	(- /	(- /	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)	
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 ALL LUMBER TO BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND
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- (I.E. ILEVEL MICROLAM)
- (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER 15 TO BE 1.55E (Fb = 2325 PSI)
 4)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
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 TOGETHER W (2) 10d NALLS @ 8' O.C., PROVIDED THAT THE TOP OF THE
- WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). I - O . O I TEXMINE REFER TO L'ABLES ROUZ. (1) AND RRUZ. (/2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES RR02.7 (1) AND RR02.7 (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- (UNO)
 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
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- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)

 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
 LEAST HORIZONTAL DIMENSION.
 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
 ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

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- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS.

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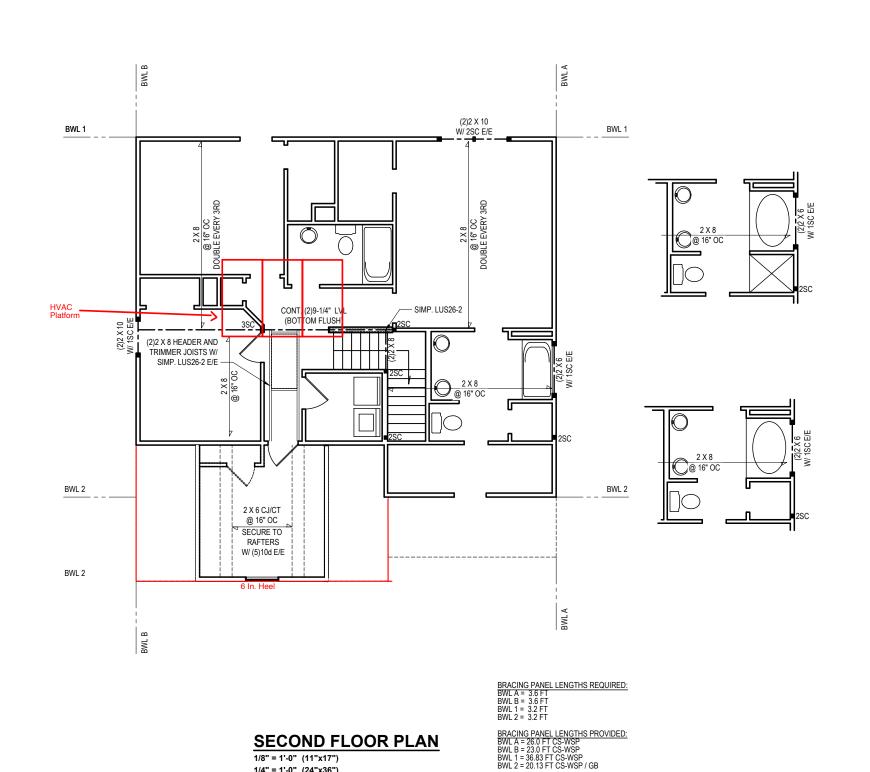
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- (3) 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS
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- (5) MINIMUM 800# HOLD-DOWN DEVICE



SECOND FLOOR PLAN

1/8" = 1'-0" (11"x17")

1/4" = 1'-0" (24"x36")

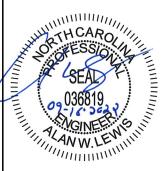
Engineers seal does not include cons procedures or safety precaution

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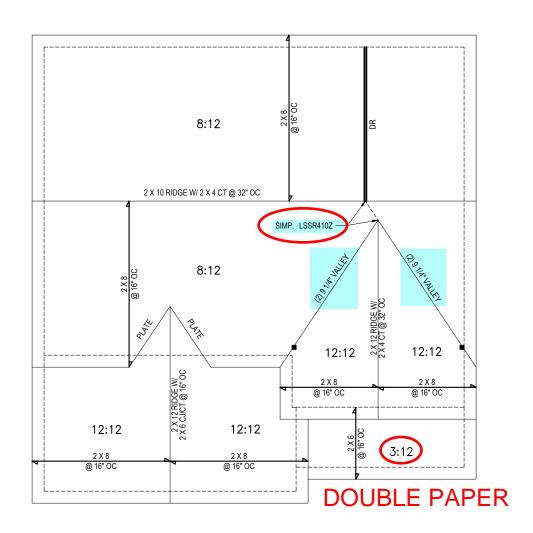
Q-14



Project #: DRB2201-0265 Date: 09/16/22 DWG. Checked B SEE PLAN REVISIONS

Sheet Number

S3



1265 SQ. FT. OF ATTIC / 300 = 5 SQ. FT. INLETS/OUTLETS REQUIRED

- CALCULATION BASED ON VENTILATORS USED AT LEAST 3:0" ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- CATHEDRAL CEILINGS SHALL HAVE A 1* MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

ATTIC VENTILATION CALCULATION

ROOF PLAN 1/8" = 1'-0" (11"x17") 1/4" = 1'-0" (24"x36")

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Project #: DRB2201-0265D Date: 9/20/2023 Engineered By: EJM Scale: SEE PLAN

	REVISIO	NS
No.	Date:	Remarks
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$\sqrt{3}$		
<u> </u>		

Sheet Number

S4

STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	(- /	(- /	TT	TL
ALL FLOORS	40	10	L/360	L/240
ATTIC (w/ walk up stairs)	30	10	L/360	L/240
ATTIC (pull down access)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD		BASED ON 120 MF	PH (EXPOSURE B)	
SEISMIC		SEISMIC ZON	NES A, B & C	

- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
 - ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LYLL LUMBER TO BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND F6 = 2600 PSI, E = 1.9M PSI (U.N.O.)
 ALL LYL LUMBER TO BE 3.5' WIDE NOMINAL EACH SINGLE MEMBER AND F6 = 2600 PSI, E = 1.9M PSI (U.N.O.)
 - ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10, (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
- ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36.
 ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2"0 x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE BND OF EACH PLATE SECTION, ANCHOR BOLTS SHALL BE SPACED AT 3-0" O.C. FOR BASEMENTS, ANCHOR BOLT SHALL EXTEND 7" INTO CONCEITE OR MASONITY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE
- WALL AND ROOF CLADDING YALUES:

 WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.

 ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

 39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12

 AND ADDRESSED FOR ROOF PITCHES 0/12 TO 1.5/12

- 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12 **MEAN ROOF HEIGHT 30'-0" OR LESS
- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION.
 TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS

CLIMATE ZONES	FENESTRATION U-FACTOR	SKYLIGHT b U-FACTOR	GLAZED FENESTRATION SHGC ^{b,k}	CEILING ^m R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE	FLOOR R-VALUE	BASEMENT ^{c, o} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE C WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5 h	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont j	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30 cont	ⁿ <u>19, or 13 + 5</u> h	13/17 <u>or</u> 13/12.5 cont	30 ^g	10/15	10	10/19

TABLE N1102.1 CLIMATE ZONES 3-5

- R-VALUES ARE MINIMUMS. U-FACTORS AND SHIGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNES OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.
- E FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.
- C 1907 WAS BE SOCRETIONED SHARE OF SHAR

- e. <u>DELETED</u> 1: BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMD LOCATIONS AS DEFINED BY <u>FIGURE N1101.7</u> AND <u>TABLE N1101.7</u>.
- g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
- SHEATHING, "15-3" MEANS R-15 CAVITY INSULATION, PLUS R3 INSULATED SHEATHING IF STRUCTURAL SHEATHING COVERS 29% OR LESS OF THE EXTERNO INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAV
- SS WALLS. THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WAL
- IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMAN CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLES WITHOUT PENALTY.

 IN ADDITION TO THE EXEMPTION IN SECTION IN 1923 3.4 MAXIMAN OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLES HAVING A SIGC NO GREATER THAN 0.70 SHALL BE EXEMPTION. FOR SERVITITIONS FOR WINNIAM SCHOOL FOR SHALL RESIDENCE ASSEMBLES WITHOUT SESSIVE AND SHALL BE EXEMPTION. FOR SERVITION FOR SHALL BE SERVITION.

- US THE ATTICKNOST DEAR.

 THE TABLE YOUR FEDURED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF. THERE THE INSULATION MUST FILL THE SPACE UP TO THE AR BAFFLE.

 R. 19 FERENASS BATTS COMPRESSED AND INSTALLED IN A NOMBALL 2 6 FRAMING CAN'TY IS DEEMED TO COMPAY. PREFICALSS BATTS RATED R-19 OR HIGHER COMPRESSED

 AND INSTALLED IN AZW MULL SUTD DEEMED TO COMPAY.
- 9. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

DEFINITIONS FOR COMMON ABBREVIATIONS

ALI	=	ALIERNAIE	WAA	-	WAXIIVIUW
CANT	=	CANTILEVER	MIN	=	MINIMUM
CJ	=	CEILING JOIST	NOM	=	NOMINAL
CMU	=	CONCRETE MASONRY UNIT	O.C.	=	ON CENTER
COL	=	COLUMN	PL	=	POINT LOAD
CONC	=	CONCRETE	PT	=	PRESSURE TREATED
CONT	=	CONTINUOUS	REINF	=	REINFORCED
CT	=	COLLAR TIE	REQD	=	REQUIRED
DBL	=	DOUBLE	RJ	=	ROOF JOIST
DIA	=	DIAMETER	RS	=	ROOF SUPPORT
DJ	=	DOUBLE JOIST	SC	=	STUD COLUMN
DR	=	DOUBLE RAFTER	SCH	=	SCHEDULE
EA	=	EACH	SPEC	=	SPECIFIED
EE	=	EACH END	THK	=	THICK
FJ	=	FLOOR JOIST	TJ	=	TRIPLE JOIST
FND	=	FOUNDATION	TRTD	=	TREATED
FTG	=	FOOTING	TYP	=	TYPICAL
GALV	=	GALVANIZED	UNO	=	UNLESS NOTED OTHERWISE
HORIZ	=	HORIZONTAL	W	=	WIDE FLANGE BEAM
HT	=	HEIGHT	WWF	=	WELDED WIRE FABRIC
MANUF	=	MANUFACTURER	XJ	=	EXTRA JOIST

MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS

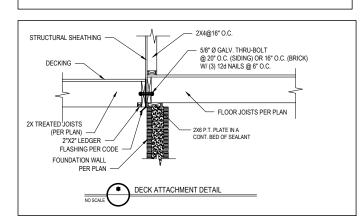
POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

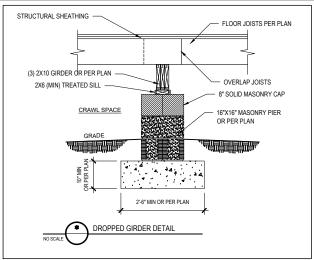
- THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
- FROM TOP OF FOOTING TO BOTTOM OF GIRDER
- DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- 2) DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF
- A. THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS
- ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4) ABOVE. LATERAL BRACING IS NOT REQUIRED. 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE
- TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED BOLT AT EACH END OF THE BRACE.

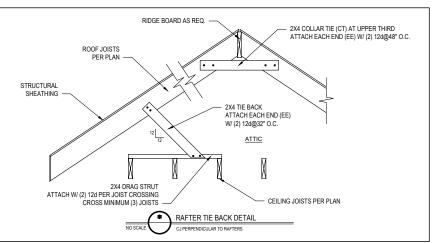
 C. FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

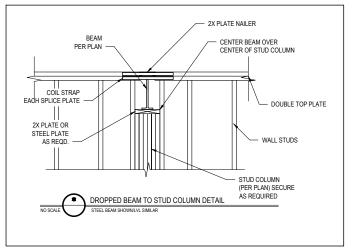
_					
	POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
I	4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
I	6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

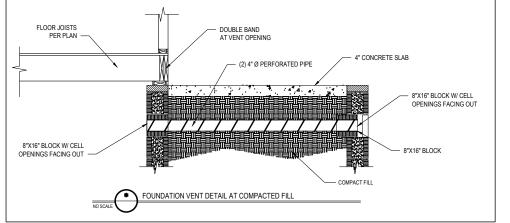
2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO
(x) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OP PARALLEL
TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS.
THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 56°0 HOT
DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.
FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.











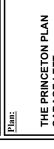
Engineers seal does not include construct procedures or safety precaution

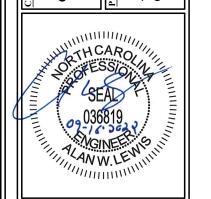
Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engir & Design, P.A. liability. Please review these docu

Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommen etc. presented in these documents were







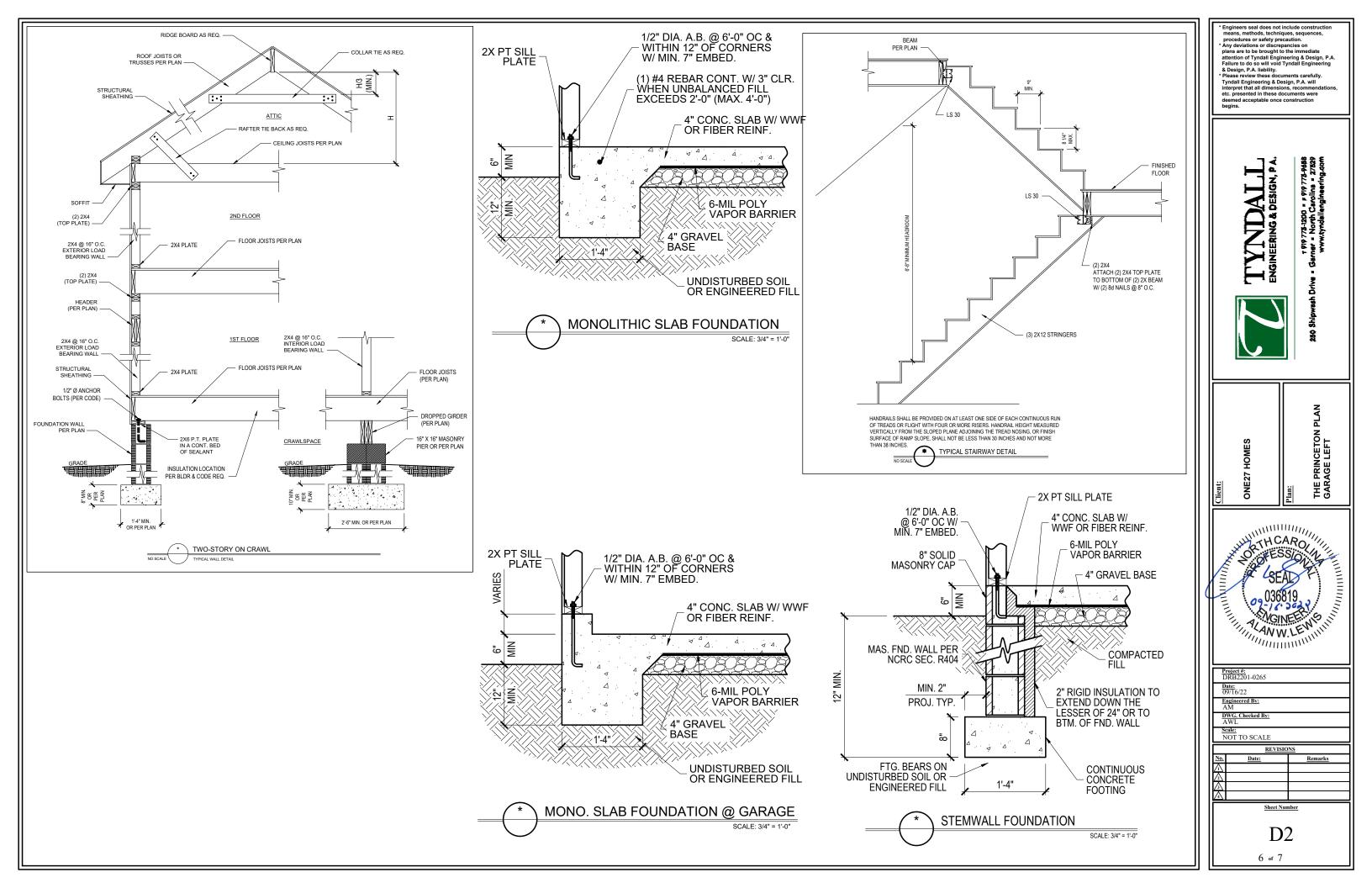


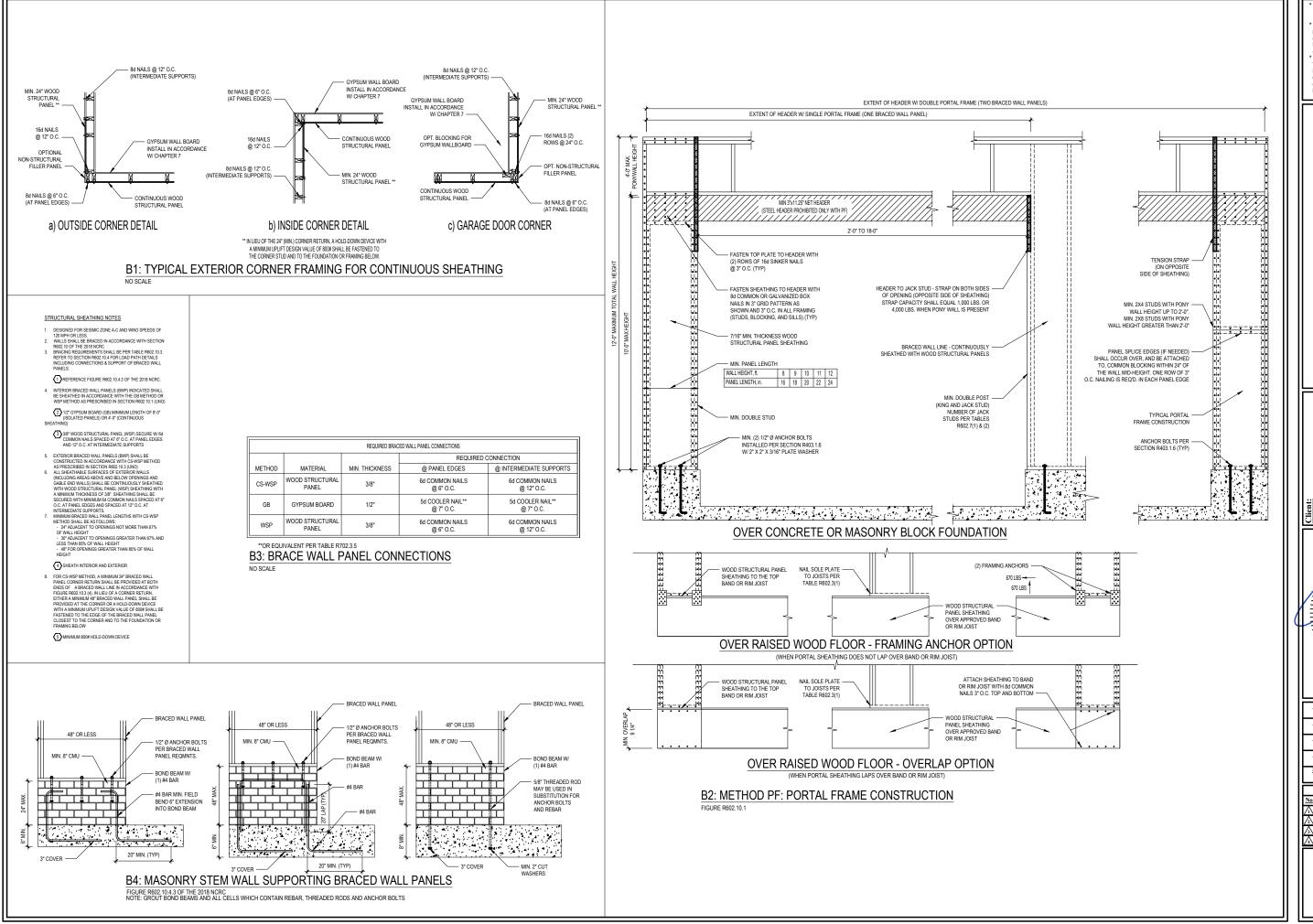
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Project #: DRB2201-0265 Date: 09/16/22 NOT TO SCALE REVISIONS

Sheet Number

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Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
Any deviations or discrepancies on plans are to be brought to the immediate

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design. P.A. liability.

* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendat etc. presented in these documents were deemed acceptable once construction

> TYNDALL ENGINEERING & DESIGN, P.A.

250 Shipwash Drive - Ga

778-1200 = #9197 North Carolina

Plan: THE PRINCETON PLAN GARAGE I FFT



ONE27

D3



DRB2201-0265

PRINCETON

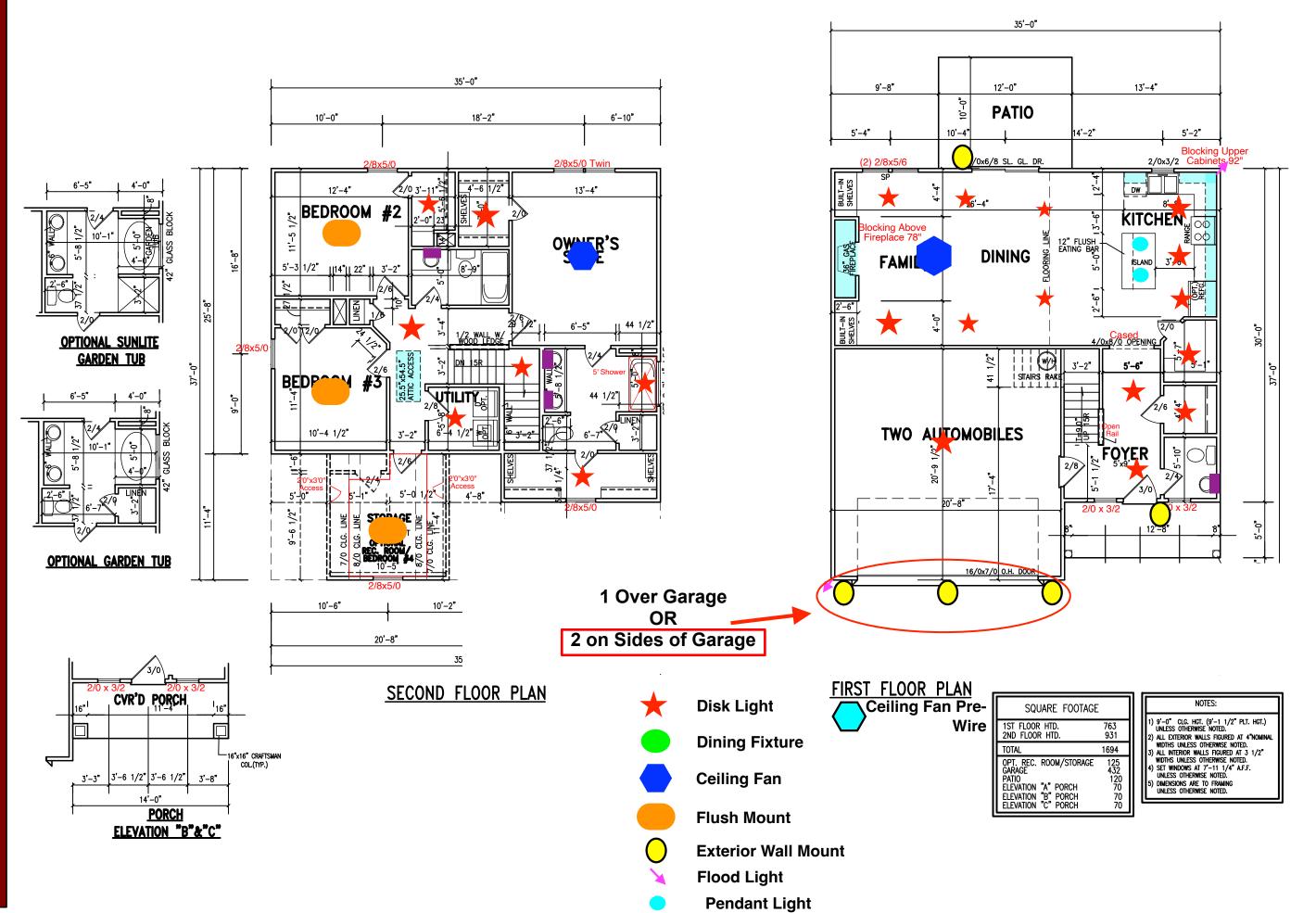
DATE: 09/16/2022

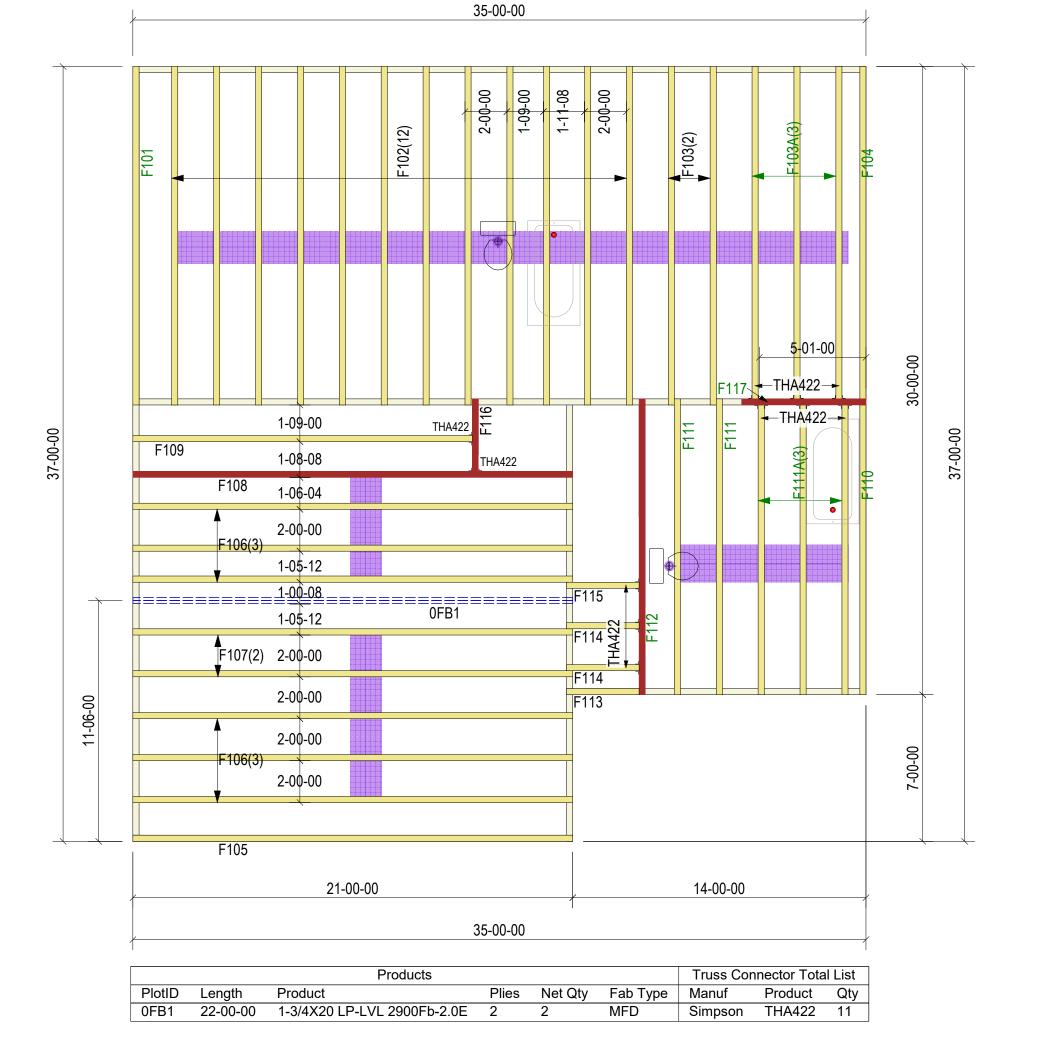
DRAWN BY: MMB

CHECKED BY:

FLOOR PLANS

3 OF 4



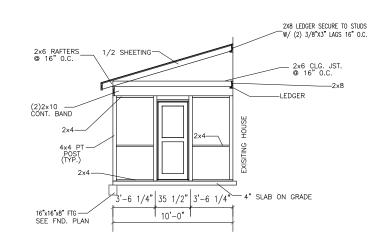




THIS IS A TRUSS PLACEMENT DIAGRAM ONL	ONLY	SHOP DRAV	SHOP DRAWING APPROVAL
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual building designer sheers for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support and columns is the responsibility of the building designer. For general guidance regarding bracing, consult "Bracing of Wood Trusses" available from the Truss Plate Institute, 583 D'Onifrio Drive, Madison, WI 53179.	at the specification of the The building designer is e. The design of the truss support eneral guidance regarding dison, WI 53179.	this Layout is the sole source for fabrication of trusses and voids. Layouts. Review and approval of this Layout must be received befor to insure against changes that will result in extra charges to you.	HIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS AYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.
		REVIEWED BY: APPROVED BY:	: DATE:
Job #: 1270114	Plan: PRINCET	PRINCETON FLOOR LAYOUT	
Customer: ONE27 HOMES	Date: 2/3/2023		Carolina Structural Systems
Site Address:	Sales Rep: RW		Roof Trusses • Floor Trusses • EWP Carolina Structural Systems
City ST 7IP	Designer: JSP		P.O. Box 157, Ether, NC 27247 225 Frame Shop Rd Star. NC 27356

DRAWING APPROVAL





SCALE 24"X36" = 1/4"=1'-0" 11"X17" = 1/8"=1'-0"

Screen Porch Detail

FILE DESIGN

DRAWN ADS CHECKED

DATE 1/15/2013 SHEET