

Mark Morris, P.E.

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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 64479

JOB: 25-9160-R01

JOB NAME: LOT 150 PROVIDENCE CREEK

Wind Code: ASCE7-16

Wind Speed: Vult= 120mph

Exposure Category: B

Mean Roof Height (feet): 35

These truss designs comply with IRC 2015 as well as IRC 2018.

20 Truss Design(s)

Trusses:

J01, J02, R01, R02, R02A, R02B, R03, R03A, R04, R05, R06, R07, R08, R09, SP01, SP02,



11/4/2025

Mark Morris

My license renewal date for the state of North Carolina is 12/31/2025

Warning !—Verify design parameters and read notes before use.

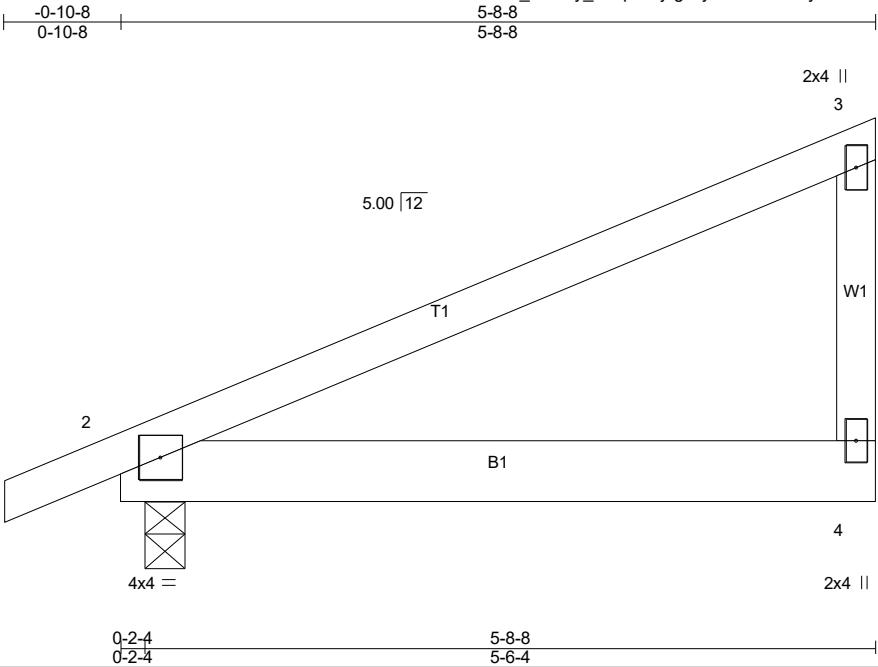
This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARINA NC 28147
25-9160-R01	J01	Jack-Closed	3	1	

Job Reference (optional) # 64479

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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.47	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.34	Vert(LL) -0.03 4-7 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Vert(CT) -0.05 4-7 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.00 2 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 27 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

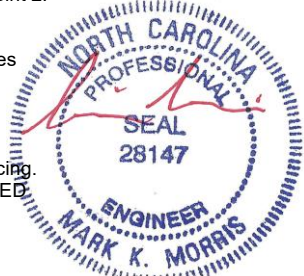
BRACING-
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied, except end verticals.
Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 4=218/Mechanical, 2=279/0-3-8 (min. 0-1-8)
Max Horz2=95(LC 13)
Max Uplift4=-60(LC 10), 2=-80(LC 10)
Max Grav4=303(LC 21), 2=364(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (11-14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 60 lb uplift at joint 4 and 80 lb uplift at joint 2.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 - Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
 - Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 - SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.



LOAD CASE(S) Standard

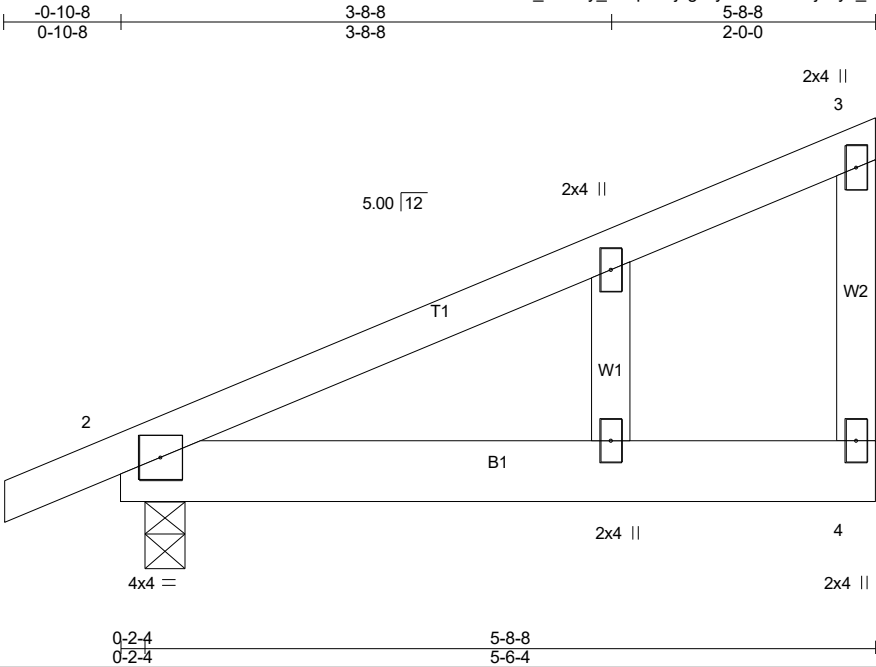
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Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	J02	Jack-Closed	1	1	

Job Reference (optional) # 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:26 2025 Page 1
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LOADING (psf)	SPACING-	CS.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.47	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.34	Vert(LL) -0.03 4-9 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.00	Vert(CT) -0.05 4-9 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.00 2 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 29 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied, except end verticals.
Rigid ceiling directly applied.

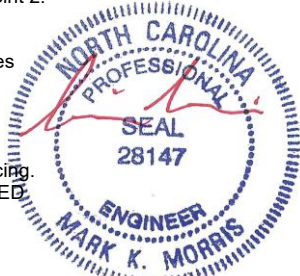
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 4=218/Mechanical, 2=279/0-3-8 (min. 0-1-8)
Max Horz 2=95(LC 13)
Max Uplift 4=-60(LC 10), 2=-80(LC 10)
Max Grav 4=303(LC 21), 2=364(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (11-14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 60 lb uplift at joint 4 and 80 lb uplift at joint 2.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 - Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
 - Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 - SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



11/4/2025

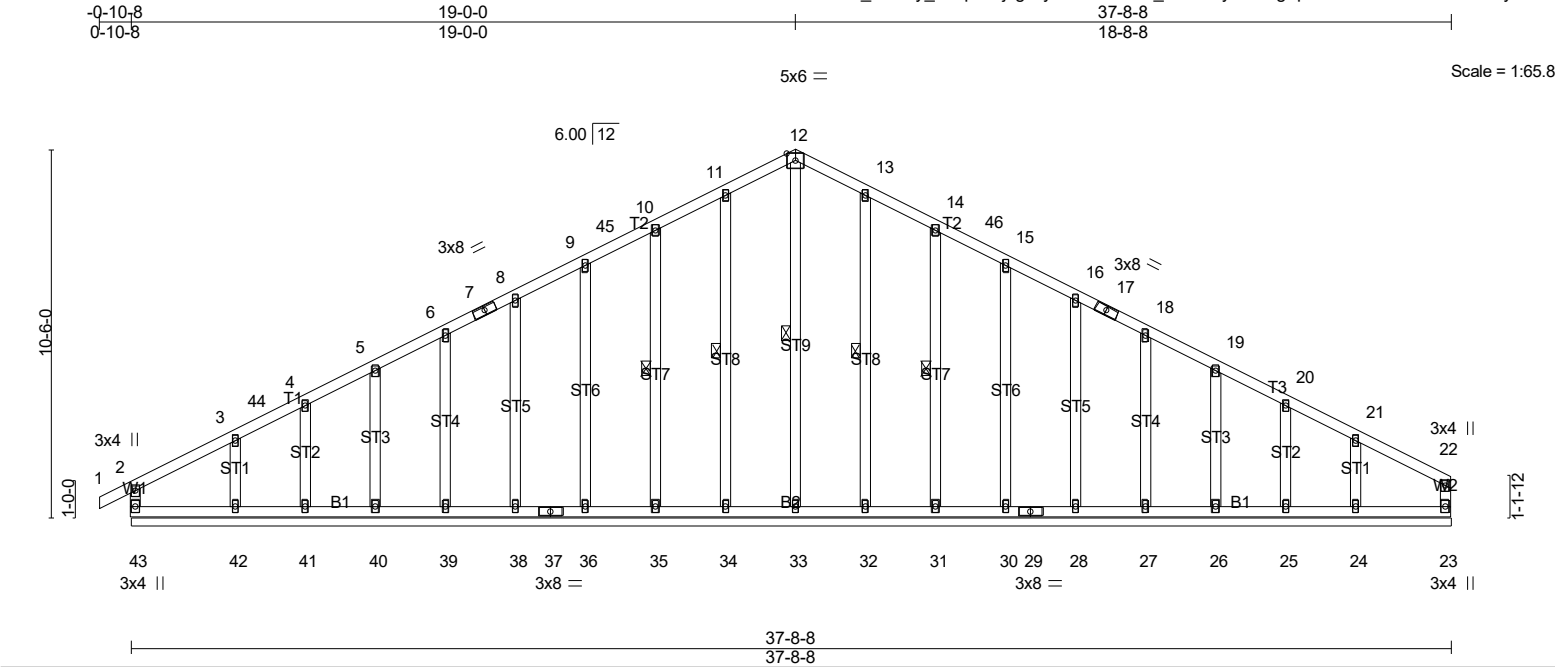
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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R01	GABLE	2	1	

Job Reference (optional) # 64479

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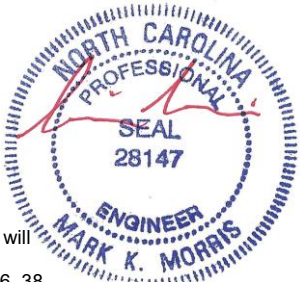
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.14	Vert(LL) -0.00 1 n/r 180		
TCDL 10.0	Lumber DOL 1.15	WB 0.14	Vert(CT) -0.00 1 n/r 80		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.00 23 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014				
				Weight: 267 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 12-33, 11-34, 10-35, 13-32, 14-31
OTHERS 2x4 SP No.3	

REACTIONS.	All bearings 37-8-8.
(lb) - Max Horz 43=158(LC 14)	
Max Uplift	All uplift 100 lb or less at joint(s) 43, 23, 34, 35, 36, 38, 39, 40, 41, 32, 31, 30, 28, 27, 26, 25 except 42=-125(LC 14), 24=-107(LC 15)
Max Grav	All reactions 250 lb or less at joint(s) 43, 23, 36, 38, 39, 40, 41, 42, 30, 28, 27, 26, 25, 24 except 33=293(LC 27), 34=290(LC 5), 35=276(LC 5), 32=290(LC 6), 31=276(LC 6)

FORCES.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	8-9=-102/251, 9-45=-120/284, 10-45=-112/292, 10-11=-139/337, 11-12=-156/374, 12-13=-156/374, 13-14=-139/337, 14-46=-112/292, 15-46=-120/284, 15-16=-102/251
WEBS	12-33=-260/79

- NOTES-** (14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 14-2-6, Corner(3R) 14-2-6 to 23-9-10, Exterior(2N) 23-9-10 to 32-9-2, Corner(3E) 32-9-2 to 37-6-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); PF=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 43, 23, 34, 35, 36, 38, 39, 40, 41, 32, 31, 30, 28, 27, 26, 25 except (jt=lb) 42=125, 24=107.



11/4/2025

LOADING CASES: Standard parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R02	Common	6	1	
Job Reference (optional)					# 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:29 2025 Page 1
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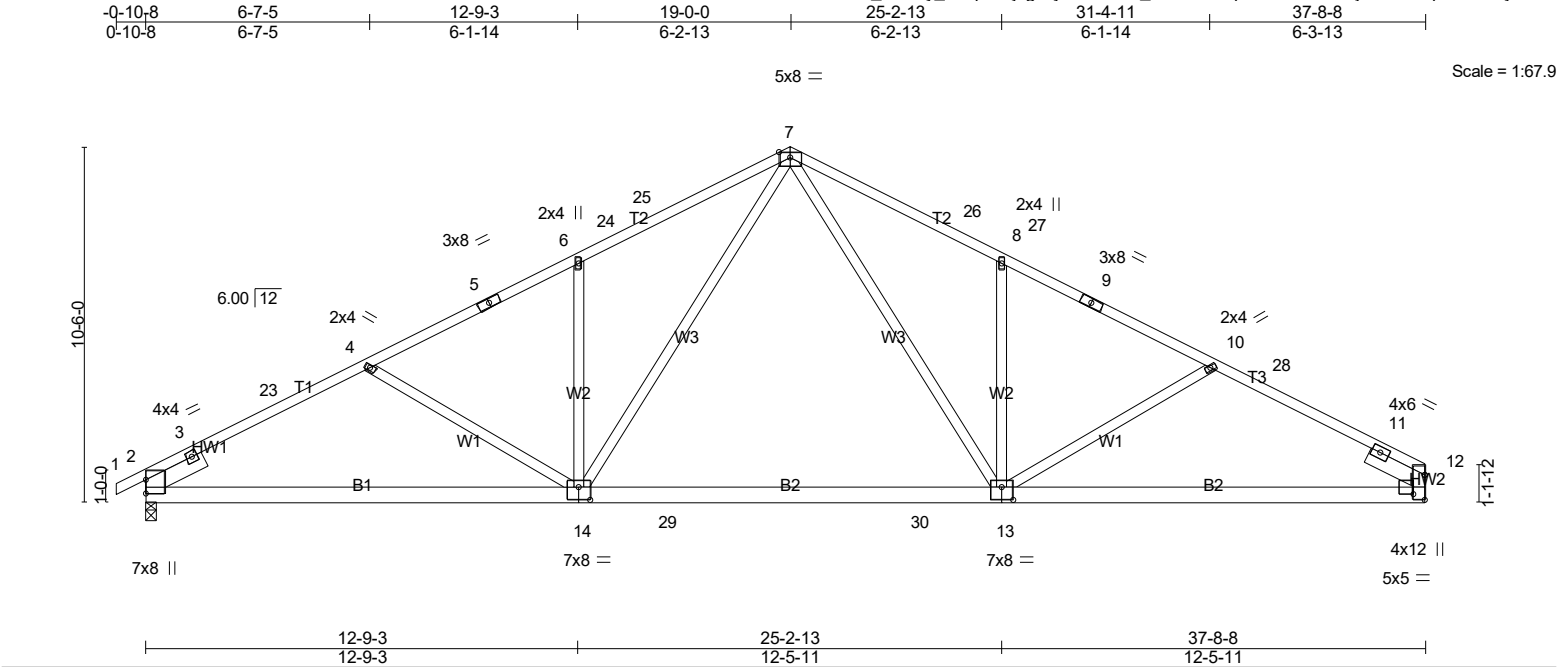


Plate Offsets (X,Y)-- [12:0-4-0,0-6-13], [13:0-4-0,0-4-8], [14:0-4-0,0-4-8]															
LOADING (psf)		SPACING-		2-0-0		CSI.		DEFL.		in (loc) l/defl L/d		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.91	Vert(LL)	-0.58	13-14	>786	240		MT20	244/190		
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.82	Vert(CT)	-0.82	13-14	>553	180					
TCDL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(CT)	0.12	12	n/a	n/a					
BCLL	0.0 *	Code IRC2021/TPI2014		Matrix-AS											
BCDL	10.0														
												Weight: 237 lb		FT = 20%	

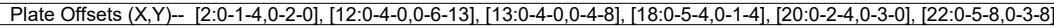
LUMBER-	BRACING-
TOP CHORD	TOP CHORD
2x4 SP No.2 *Except*	Structural wood sheathing directly applied.
T1: 2x4 SP No.1, T3: 2x4 SP SS	Rigid ceiling directly applied.
BOT CHORD	BOT CHORD
2x6 SP No.1 *Except*	
B1: 2x6 SP No.2	
WEBS	
2x4 SP No.3	
SLIDER	
Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0	

REACTIONS.	(lb/size)	12=1508/Mechanical, 2=1561/0-3-8 (min. 0-1-13)
	Max Horz 2=157(LC 14)	
	Max Uplift 12=181(LC 15), 2=200(LC 14)	

FORCES.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-882/0, 3-23=-2478/368, 4-23=-2422/391, 4-5=-2262/338, 5-6=-2194/354, 6-24=-2297/441, 24-25=-2272/445, 7-25=-2221/466, 7-26=-2197/465, 26-27=-2249/443, 8-27=-2274/440, 8-9=-2166/352, 9-10=-2233/336, 10-28=-2320/388, 11-28=-2414/367, 11-12=-483/178
BOT CHORD	2-14=-366/2130, 14-29=-96/1453, 29-30=-96/1453, 13-30=-96/1453, 12-13=-271/2060
WEBS	7-13=-223/1052, 8-13=-524/228, 6-14=-512/227, 7-14=-229/1090, 4-14=-291/187

- NOTES- (11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 14-2-6, Exterior(2R) 14-2-6 to 23-9-10, Interior(1) 23-9-10 to 32-10-14, Exterior(2E) 32-10-14 to 37-8-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=181, 2=200.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:30 2025 Page 1
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[illegible]

TOP CHORD	2x4 SP No.2 *Except*
	T3: 2x4 SP SS
BOT CHORD	2x4 SP No.2 *Except*
	B5,B7: 2x6 SP No.2
WEBS	2x4 SP No.3 *Except*
	W1: 2x6 SP No.2
SLIDER	Right 2x6 SP No.2 1-11-0

TOP CHORD	Structural wood sheathing directly applied, except end verticals.	
BOT CHORD	Rigid ceiling directly applied. Except:	
	1 Row at midpt	6-20
	6-0-0 oc bracing: 15-17	
WEBS	1 Row at midpt	7-18

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

(lb/size) 12=1583/Mechanical, 24=1634/0-3-8 (min. 0-1-15)
 Max Horz 24=147(LC 14)
 Max Uplift 12=-138(LC 15), 24=-168(LC 14)
 Max Grav 12=1604(LC 3), 24=1634(LC 1)

TOP CHORD 2-3=-2715/327, 3-29=-2896/295, 4-29=-2839/315, 4-5=-2376/278, 5-30=-2294/291,
30-31=-2250/300, 6-31=-2235/306, 6-7=-2319/390, 7-32=-2380/380, 32-33=-2435/359,
8-33=-2460/355, 8-9=-2348/266, 9-10=-2415/250, 10-34=-2498/305, 11-34=-2592/284,
11-12=-486/69, 2-24=-1552/210

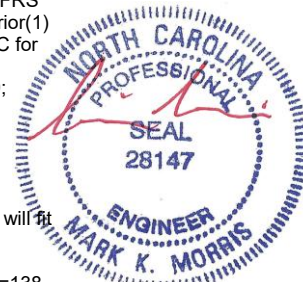
BOT CHORD 21-22=-466/2648, 20-21=-288/2539, 19-20=-327/0, 6-20=-436/185, 18-35=-21/1672,
14-35=-21/1672, 14-36=-21/1672, 13-36=-21/1672, 12-13=-199/2218

WEBS 4-21=0/313, 4-20=-658/209, 18-20=0/1731, 7-20=-265/1202, 7-15=-199/964,
13-15=-217/931, 8-13=-528/227, 17-18=-127/346, 7-17=-89/468, 2-22=-275/2230

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 14-2-6, Exterior(2R) 14-2-6 to 23-9-10, Interior(1) 23-9-10 to 32-10-14, Exterior(2E) 32-10-14 to 37-8-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) T CLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=138, 24=168.

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

Warranted to be applied directly to the document for use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



11/4/2025

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R02A	Roof Special	1	1	Job Reference (optional) # 64479

- 8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:31 2025 Page 2
ID:zSnI_VDJTy_hu?pmOjJgrKyZRJv-jr9kn0LHnOcWYu9TM?Nz43D6HbqgOZf6c?hlL7yMYe_
- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

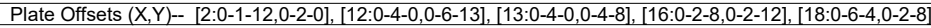
LOAD CASE(S) Standard



11/4/2025

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:32 2025 Page 1
ID:zSnI_VDJTy_hu?pmOjJgrKyZRJv-C2j6_LMvXilNA2kgvjvCcHlEm?Bw71YFrfrJsZyMYdz



Weight: 260 lb FT = 20%

TOP CHORD	Structural wood sheathing directly applied, except end verticals.	
BOT CHORD	Rigid ceiling directly applied. Except:	
	1 Row at midpt	6-16
WEBS	1 Row at midpt	7-14

11/4/2025

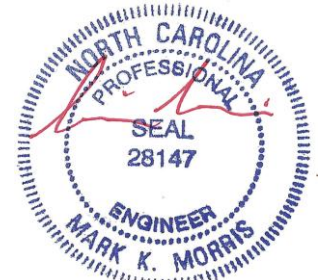
Warning 1!—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R02B	Roof Special	4	1	Job Reference (optional) # 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:32 2025 Page 2
ID:zSnI_VDJTy_hu?pmOjJgrKyZRJv-C2j6_LMvXiINA2kgvjvCcHlEm?Bw71YFrRJsZyMYdz

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



11/4/2025

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARINA NC 28147
25-9160-R01	R03	Common	5	1	
Job Reference (optional)					# 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:33 2025 Page 1
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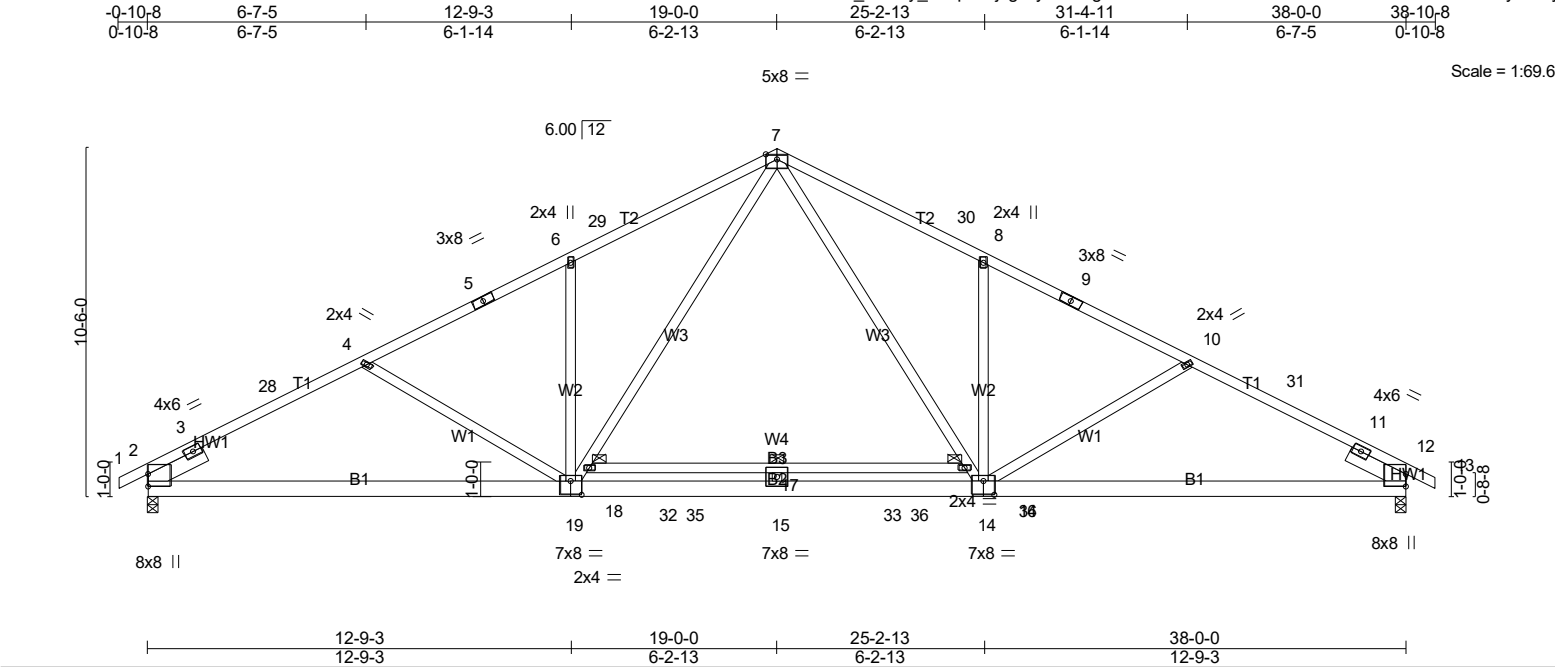


Plate Offsets (X,Y)-- [4:0-0-0,0-0-0], [12:0-0-0,0-0-0], [14:0-4-0,0-5-0], [19:0-4-0,0-5-0]		25-2-13 6-2-13		38-0-0 12-9-3	
LOADING (psf)		SPACING		CSI	
TCLL (roof)	20.0	2-0-0	Plate Grip DOL	1.15	TC 0.84
Snow (Pf)	20.0		Lumber DOL	1.15	BC 0.91
TCDL	10.0		Rep Stress Incr	YES	WB 0.65
BCLL	0.0 *		Code IRC2021/TPI2014		Matrix-AS
BCDL	10.0				
				DEFL.	
				in (loc) l/defl L/d	
				Vert(LL) -0.80 17 >568 240	
				Vert(CT) -1.18 17 >386 180	
				Horz(CT) 0.12 12 n/a n/a	
				PLATES	
				MT20	
				GRIP	
				244/190	
				Weight: 257 lb FT = 20%	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 *Except*	TOP CHORD	Structural wood sheathing directly applied. Rigid ceiling directly applied. Except: 6-0-0 oc bracing: 16-18
	T1: 2x4 SP SS	BOT CHORD	
BOT CHORD	2x6 SP No.1 *Except*		<div>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>
	B2: 2x6 SP DSS, B3: 2x4 SP No.2		
WEBS	2x4 SP No.3		
SLIDER	Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0		

REACTIONS. (lb/size) 2=1689/0-3-8 (min. 0-2-2), 12=1689/0-3-8 (min. 0-2-2)
Max Horz 2=148(LC 14)
Max Uplift 2=-142(LC 14), 12=-142(LC 15)
Max Grav 2=1795(LC 3), 12=1795(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-768/0, 3-28=-3017/528, 4-28=-2961/550, 4-5=-2832/438, 5-6=-2764/452, 6-29=-2872/564, 7-29=-2796/590, 7-30=-2796/590, 8-30=-2872/564, 8-9=-2764/452, 9-10=-2832/438, 10-31=-2961/550, 11-31=-3017/528, 11-12=-525/0
BOT CHORD 2-19=-353/2595, 19-32=-24/1852, 15-32=-24/1852, 15-33=-24/1852, 33-34=-24/1852, 14-34=-24/1852, 12-14=-353/2595
WEBS 7-16=-206/1378, 14-16=-238/1263, 8-14=-516/245, 10-14=-266/237, 18-19=-238/1263, 7-18=-206/1378, 6-19=-517/245, 4-19=-266/237, 15-17=-380/0

- NOTES-** (10)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 14-2-6, Corner(3R) 14-2-6 to 23-9-10, Exterior(2N) 23-9-10 to 34-0-14, Corner(3E) 34-0-14 to 38-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=142, 12=142.
 - 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



11/4/2025

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R03A	Roof Special	1	1	

64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:35 2025 Page 1

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-0-10-8 2-3-8 8-3-1 15-2-0 19-0-0 25-2-13 31-4-11 38-0-0 38-10-8
0-10-8 2-3-8 5-11-9 6-10-15 3-10-0 6-2-13 6-1-14 6-7-5 0-10-8

5x10 =

Scale = 1:70.7

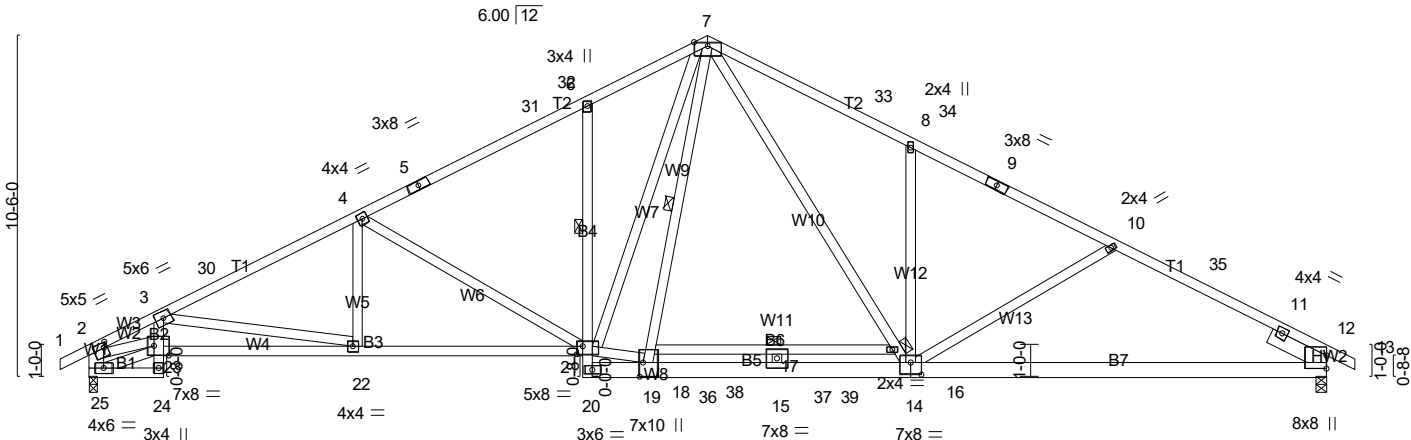


Plate Offsets (X,Y)--	[2:0-1-4,0-2-0], [14:0-4-0,0-4-8], [19:0-5-4,0-1-4], [21:0-2-4,0-3-0], [23:0-5-8,0-3-8]
-----------------------	---

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	2-0-0	TC 0.91	Vert(LL)	-0.26 17-18	>999	240	MT20	244/190
Snow (Pf)	20.0	1.15	BC 0.88	Vert(CT)	-0.45 17-18	>999	180		
TCDL	10.0	1.15	WB 0.93	Horz(CT)	0.18 12	n/a	n/a		
BCLL	0.0 *	YES	Matrix-AS						
BCDL	10.0	Code IRC2021/TPI2014							
								Weight: 274 lb	FT = 20%

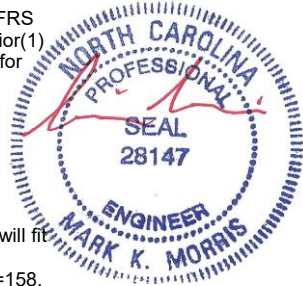
LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2 *Except* T1: 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x4 SP No.2 *Except* B5,B7: 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied. Except: 1 Row at midpt 6-21 6-0-0 oc bracing: 16-18
WEBS 2x4 SP No.3 *Except* W1: 2x6 SP No.2	WEBS 1 Row at midpt 7-19
SLIDER Right 2x6 SP No.2 1-11-0	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 12=1647/0-3-8 (min. 0-1-15), 25=1646/0-3-8 (min. 0-1-15)
Max Horz 25=-142(LC 19)
Max Uplift 12=-158(LC 15), 25=-168(LC 14)
Max Grav 12=1657(LC 3), 25=1646(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2744/324, 3-30=-2923/292, 4-30=-2867/311, 4-5=-2402/276, 5-31=-2320/290,
31-32=-2277/298, 6-32=-2262/304, 6-7=-2344/388, 7-33=-2433/381, 33-34=-2487/360,
8-34=-2511/356, 8-9=-2406/267, 9-10=-2474/251, 10-35=-2636/307, 11-35=-2692/284,
11-12=-974/0, 2-25=-1572/204
BOT CHORD 22-23=-450/2667, 21-22=-279/2564, 20-21=-321/0, 6-21=-433/184, 19-36=-3/1693,
15-36=-3/1693, 15-37=-3/1693, 14-37=-3/1693, 12-14=-175/2323
WEBS 4-22=0/311, 4-21=-660/209, 19-21=0/1743, 7-21=-262/1211, 7-16=-204/1010,
14-16=-221/979, 10-14=-285/194, 8-14=-513/226, 18-19=-145/334, 7-18=-108/454,
2-23=-272/2242

- NOTES-** (10-13)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-10-8 to 3-11-2, Interior(1) 3-11-2 to 14-2-6, Exterior(2R) 14-2-6 to 23-9-10, Interior(1) 23-9-10 to 34-0-14, Exterior(2E) 34-0-14 to 38-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=158, 25=168.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

When rock is applied directly to the bottom chord before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



11/4/2025

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R03A	Roof Special	1	1	Job Reference (optional) # 64479

- 8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:35 2025 Page 2

ID:zSnI_VDJTy_hu?pmOjJgrKyZRJv-cdPFcNPnqd7y1WSFbrSvEvNkhCDDKNZhXdfzTuyMYdw

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.
- LOAD CASE(S)
Standard
-
- 11/4/2025
- Warning !—Verify design parameters and read notes before use.** This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D’Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R04	Common	1	1	

Job Reference (optional) # 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:36 2025 Page 1
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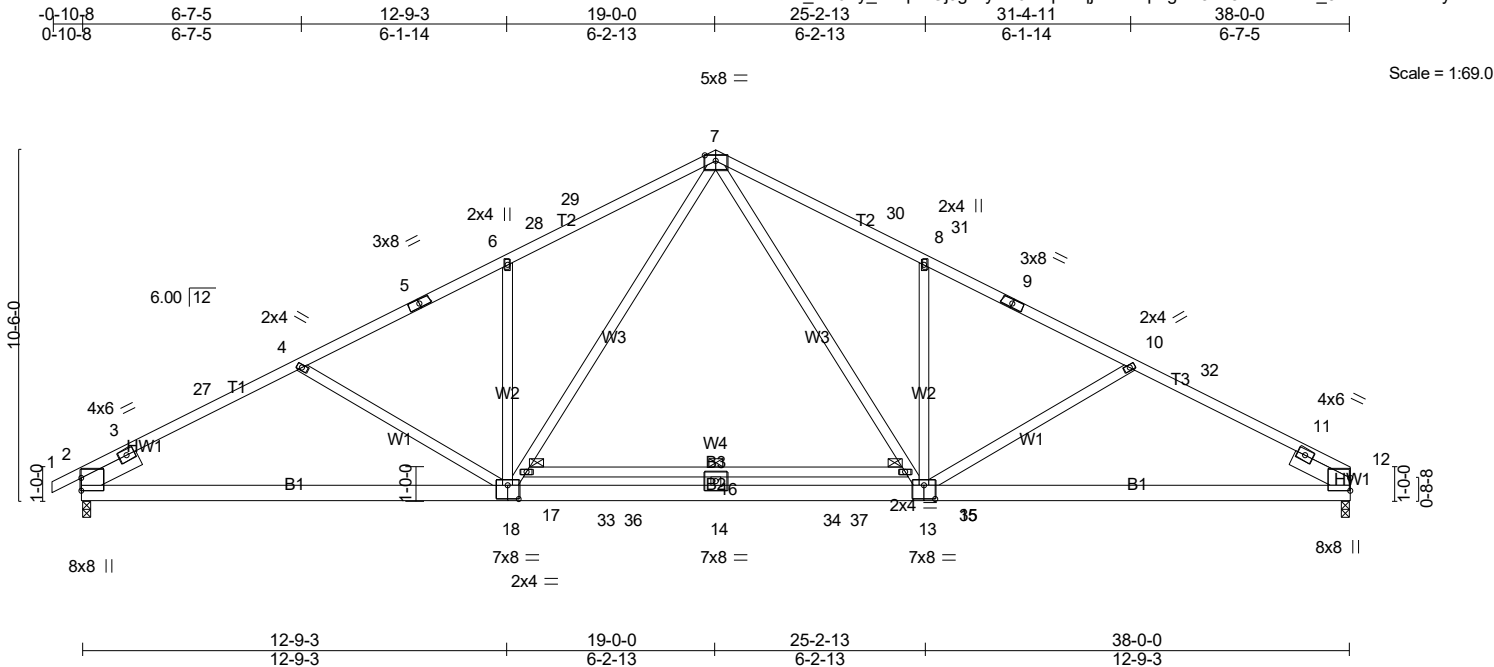


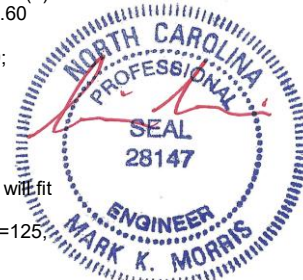
Plate Offsets (X,Y)-- [13:0-4-0,0-5-0], [18:0-4-0,0-5-0]									
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.80 16	>568	240
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-1.18 16	>386	180
TCDL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(CT)	0.12 12	n/a	n/a
BCLL	0.0 *	Code IRC2021/TPI2014		Matrix-AS					
BCDL	10.0								
								Weight: 255 lb FT = 20%	

LUMBER-	BRACING-
TOP CHORD 2x4 SP SS *Except* T2: 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied.
BOT CHORD 2x6 SP No.1 *Except* B2: 2x6 SP DSS, B3: 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied. Except: 6-0-0 oc bracing: 15-17
WEBS 2x4 SP No.3	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
SLIDER Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0	

REACTIONS. (lb/size) 12=1636/0-3-8 (min. 0-2-1), 2=1690/0-3-8 (min. 0-2-2)
Max Horz 2=154(LC 14)
Max Uplift 12=125(LC 15), 2=-142(LC 14)
Max Grav 12=1751(LC 3), 2=1796(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-768/0, 3-27=-3018/252, 4-27=-2962/275, 4-5=-2833/215, 5-6=-2765/231,
6-28=-2873/316, 28-29=-2848/319, 7-29=-2797/341, 7-30=-2798/343, 30-31=-2850/321,
8-31=-2874/318, 8-9=-2767/233, 9-10=-2834/218, 10-32=-2921/280, 11-32=-3021/258,
11-12=-564/0
BOT CHORD 2-18=-262/2596, 18-33=0/1853, 14-33=0/1853, 14-34=0/1853, 34-35=0/1853, 13-35=0/1853,
12-13=-177/2599
WEBS 7-15=-161/1380, 13-15=-192/1265, 8-13=-516/222, 10-13=-268/202, 17-18=-192/1263,
7-17=-160/1378, 6-18=-517/223, 4-18=-266/202, 14-16=-380/0

- NOTES-** (10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 14-2-6, Exterior(2R) 14-2-6 to 23-9-10, Interior(1) 23-9-10 to 33-2-6, Exterior(2E) 33-2-6 to 38-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=125, 2=142.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

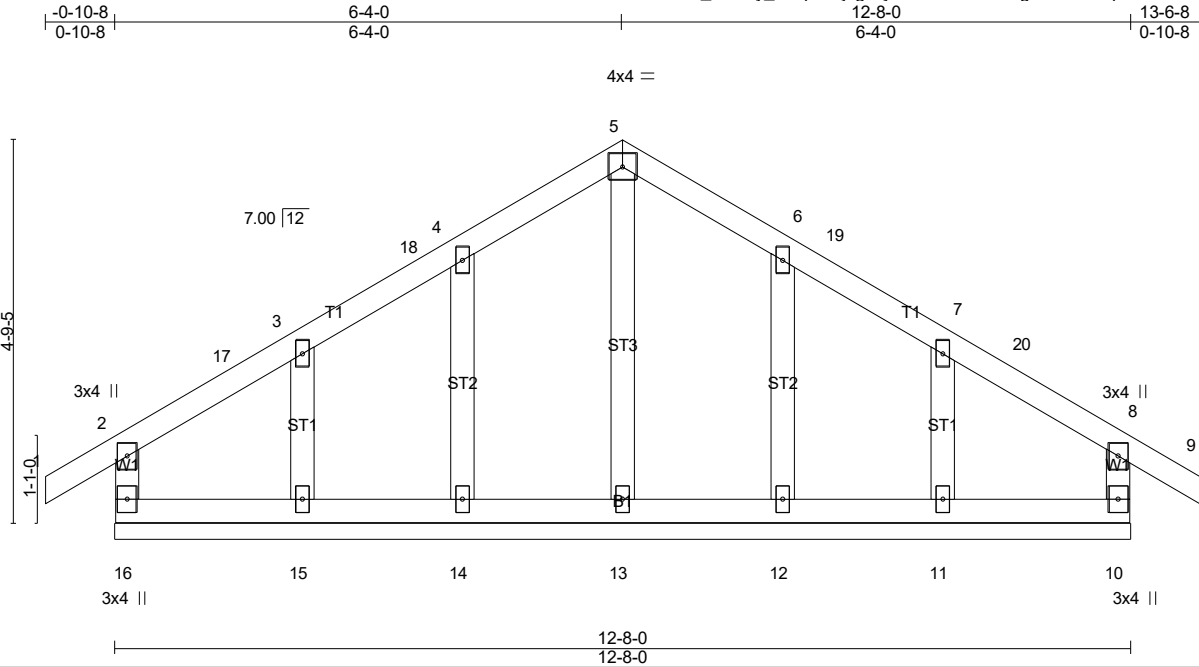


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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R05	COMMON SUPPORTED GAB	1	1	
Job Reference (optional)					# 64479

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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.12	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.06	Vert(LL) -0.00 9 n/r 180		
TCDL 10.0	Lumber DOL 1.15	WB 0.06	Vert(CT) -0.00 9 n/r 80		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.00 10 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014				
Weight: 67 lb					FT = 20%

LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
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REACTIONS. All bearings 12-8-0.
(lb) - Max Horz 16=93(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11
Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Corner(3R) 3-11-2 to 8-8-14, Corner(3E) 8-8-14 to 13-6-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.

LOAD CASE(S) Standard



11/4/2025

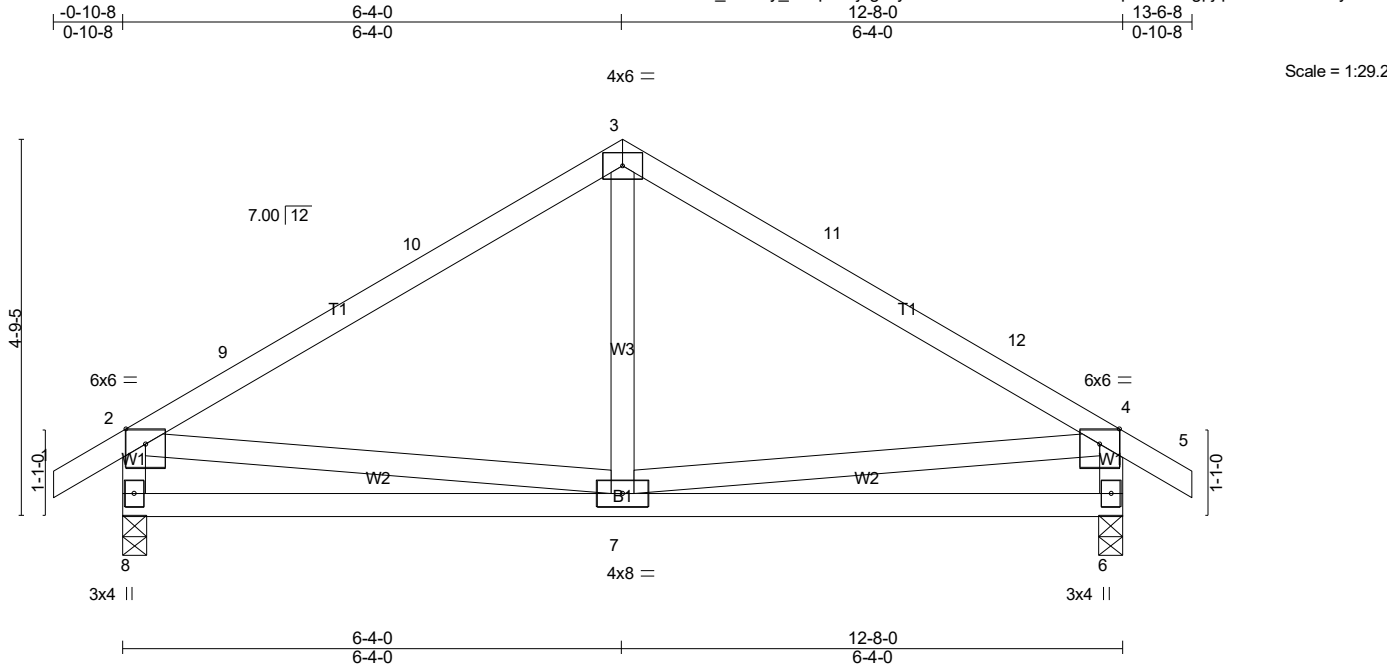
Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R06	Common	3	1	

Job Reference (optional) # 64479

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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.97	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.32	Vert(LL) -0.03 7-8 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.11	Vert(CT) -0.06 7-8 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.00 6 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 70 lb	FT = 20%

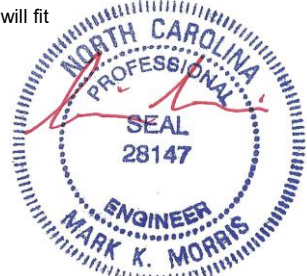
LUMBER-	BRACING-	
TOP CHORD 2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied.
WEBS 2x4 SP No.3		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 8=556/0-3-8 (min. 0-1-8), 6=556/0-3-8 (min. 0-1-8)
Max Horz 8=-93(LC 12)
Max Uplift 8=-75(LC 14), 6=-75(LC 15)
Max Grav 8=632(LC 21), 6=632(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-9=-600/171, 9-10=-461/184, 3-10=-443/200, 3-11=-443/200, 11-12=-461/184,
4-12=-600/171, 2-8=-574/274, 4-6=-574/274
BOT CHORD 7-8=-151/364, 6-7=-135/364
WEBS 2-7=-47/276, 4-7=-52/276

- NOTES-** (10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-10-8 to 3-11-2, Corner(3R) 3-11-2 to 8-8-14, Corner(3E) 8-8-14 to 13-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



11/4/2025

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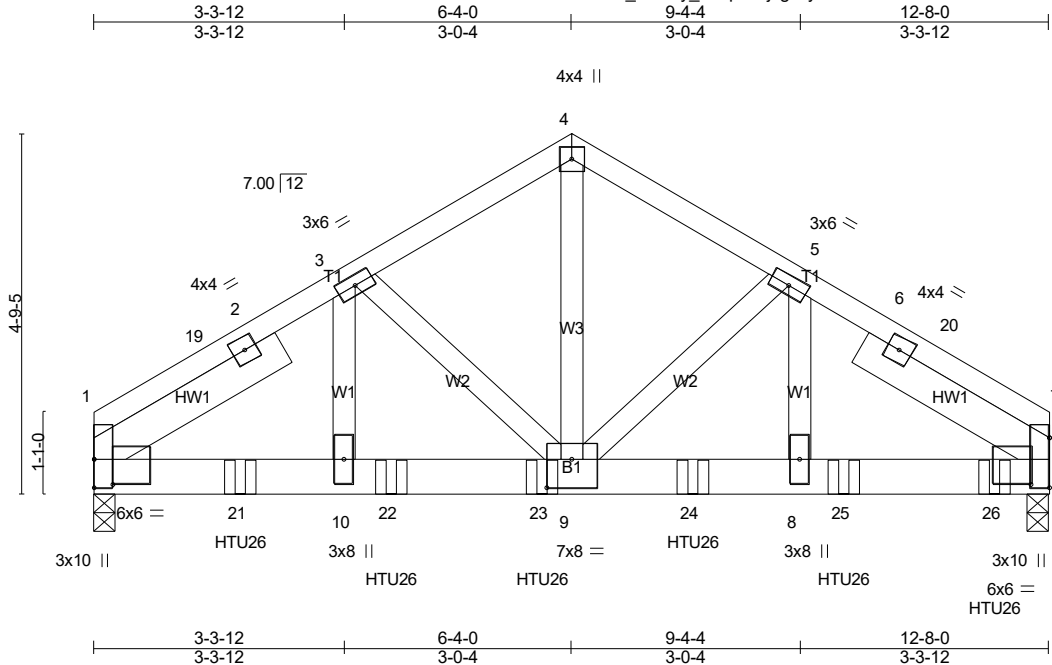
Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARINA NC 28147
25-9160-R01	R07	Common Girder	1	2	

Job Reference (optional)

64479

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Scale = 1:30.5

Plate Offsets (X,Y)-- [1:0-3-0,0-4-0], [7:0-3-0,0-7-7], [9:0-4-0,0-4-8]									
LOADING (psf)	SPACING-	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.52	Vert(LL)	-0.05	9-10	>999	240	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.76	Vert(CT)	-0.10	9-10	>999	180		
TCDL 10.0	Lumber DOL 1.15	WB 0.87	Horz(CT)	0.03	7	n/a	n/a		
BCLL 0.0 *	Rep Stress Incr NO	Matrix-MSH							
BCDL 10.0	Code IRC2021/TPI2014								
								Weight: 178 lb	FT = 20%

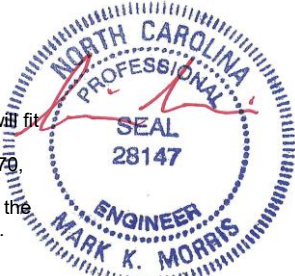
LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 4-6-12 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	
SLIDER Left 2x6 SP No.2 2-11-0, Right 2x6 SP No.2 2-11-0	

REACTIONS. (lb/size) 1=4531/0-3-8 (min. 0-2-11), 7=5448/0-3-8 (min. 0-3-4)
Max Horz 1=81(LC 38)
Max Uplift1=-570(LC 12), 7=-640(LC 13)
Max Grav 1=4604(LC 18), 7=5521(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-19=-2117/283, 2-19=-2061/286, 2-3=-5756/732, 3-4=-4593/610, 4-5=-4595/610, 5-6=-5842/733, 6-20=-2340/300, 7-20=-2396/297
BOT CHORD 1-21=-618/4754, 10-21=-618/4754, 10-22=-618/4754, 22-23=-618/4754, 9-23=-618/4754, 9-24=-566/4845, 8-24=-566/4845, 8-25=-566/4845, 25-26=-566/4845, 7-26=-566/4845
WEBS 4-9=-532/4186, 5-9=-1161/196, 5-8=-185/1627, 3-9=-1040/193, 3-10=-184/1540

- NOTES-** (12)
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-5-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=570, 7=640.
 - Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-11-4 from the left end to 11-11-4 to connect truss(es) R02 (1 ply 2x6 SP), R02B (1 ply 2x6 SP), R02A (1 ply 2x6 SP) to front face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard



11/4/2025

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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R07	Common Girder	1	2	Job Reference (optional) # 64479

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LOAD CASE(S) Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-60, 11-15=-20
Concentrated Loads (lb)
Vert: 21=-1488(F) 22=-1478(F) 23=-1478(F) 24=-1478(F) 25=-1478(F) 26=-1565(F)

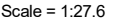


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Job Reference (optional) _____ # **044 / 9**

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Weight: 63 lb FT = 20%

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R09	Common Girder	1	2	

Job Reference (optional) # 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:43 2025 Page 1

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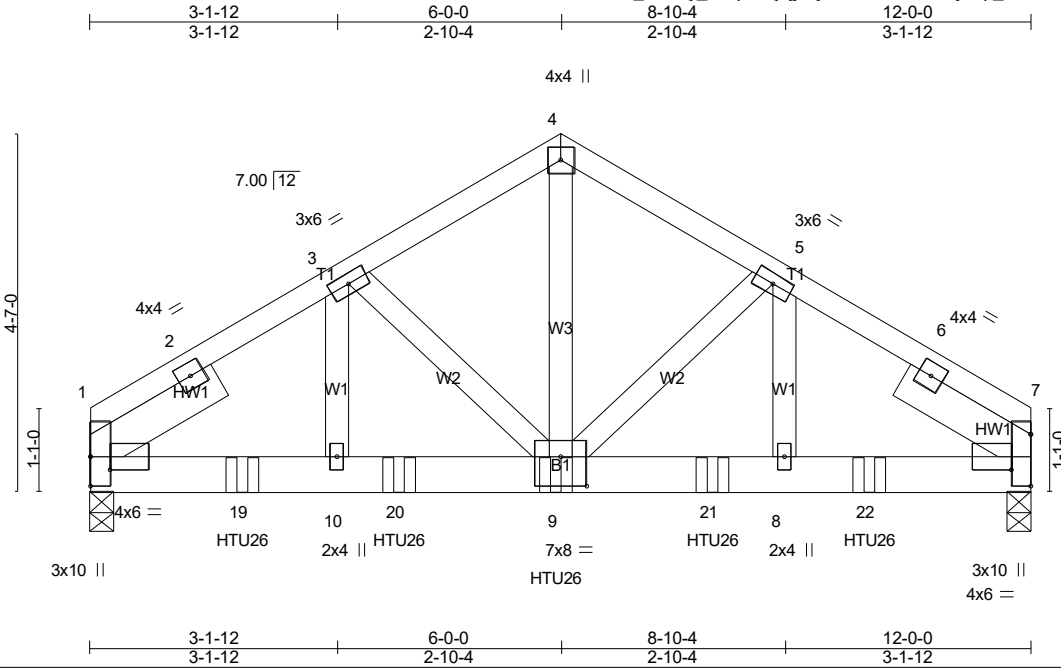


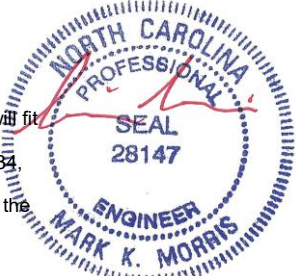
Plate Offsets (X,Y)-- [1:0-3-0,0-2-0], [7:0-3-0,0-5-7], [9:0-4-0,0-4-8]									
LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES	
TCLL (roof)	20.0	2-0-0	Plate Grip DOL	1.15	TC 0.73	in (loc)	l/defl	L/d	GRIP
Snow (Pf)	20.0		Lumber DOL	1.15	BC 0.79	Vert(LL)	-0.05 8-9	>999 240	244/190
TCDL	10.0		Rep Stress Incr	NO	WB 0.80	Vert(CT)	-0.10 8-9	>999 180	
BCLL	0.0 *		Code IRC2021/TPI2014		Matrix-MSH	Horz(CT)	0.03 7	n/a n/a	
BCDL	10.0								
					Weight: 161 lb FT = 20%				

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-2-2 oc purlins.
BOT CHORD	2x6 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		
SLIDER	Left 2x6 SP No.2 1-11-0, Right 2x6 SP No.2 1-11-0		

REACTIONS. (lb/size) 1=4230/0-3-8 (min. 0-2-9), 7=4168/0-3-8 (min. 0-2-8)
Max Horz 1=-77(LC 35)
Max Uplift1=-534(LC 12), 7=-526(LC 13)
Max Grav 1=4305(LC 18), 7=4243(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2241/279, 2-3=-5215/663, 3-4=-4303/574, 4-5=-4303/574, 5-6=-5218/664, 6-7=-2199/273
BOT CHORD 1-19=-564/4308, 10-19=-564/4308, 10-20=-564/4308, 9-20=-564/4308, 9-21=-513/4307, 8-21=-513/4307, 8-22=-513/4307, 7-22=-513/4307
WEBS 4-9=-497/3879, 5-9=-787/159, 5-8=-153/1183, 3-9=-789/158, 3-10=-151/1174

- NOTES-** (12)
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=534, 7=526.
 - Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-11-4 from the left end to 9-11-4 to connect truss(es) R02 (1 ply 2x6 SP) to front face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.



LOAD CASE(S) Standard

11/4/2025

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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	R09	Common Girder	1	2	Job Reference (optional) # 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:43 2025 Page 2
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LOAD CASE(S) Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-60, 11-15=-20
Concentrated Loads (lb)
Vert: 9=-1488(F) 19=-1488(F) 20=-1488(F) 21=-1488(F) 22=-1488(F)

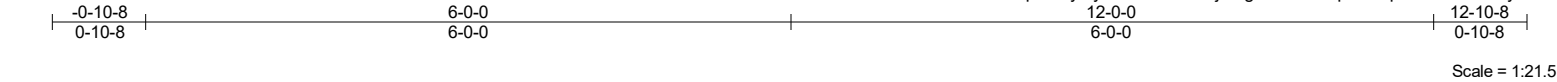


11/4/2025

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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARINA, NC 27049
25-9160-R01	SP01	COMMON SUPPORTED GAB	1	1	Job Reference (optional) # 64479

8.830 s Sep 3 2025 MiTek Industries, Inc. Tue Nov 4 14:20:44 2025 Page 1
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LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.06	MT20		244/190	
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.10				
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.01				
BCLL	0.0 *	Code IRC2021/TPI2014		Matrix-SH							
BCDL	10.0										

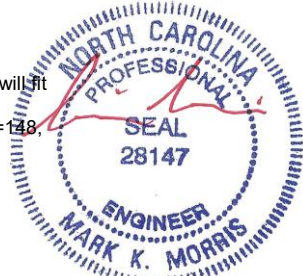
LUMBER-		BRACING-		Structural wood sheathing directly applied or 4-1-5 oc purlins. Rigid ceiling directly applied or 9-5-5 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
TOP CHORD	2x4 SP No.2	TOP CHORD		
BOT CHORD	2x4 SP No.2	BOT CHORD		
OTHERS	2x4 SP No.3			

REACTIONS. (lb/size) 2=529/0-4-0 (min. 0-1-8), 4=529/0-4-0 (min. 0-1-8)
Max Horz 2=43(LC 18)
Max Uplift 2=-148(LC 10), 4=-148(LC 11)
Max Grav 2=623(LC 21), 4=623(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-11=-786/491, 3-11=-650/503, 3-12=-650/503, 4-12=-786/491
BOT CHORD 2-13=-359/619, 6-13=-359/619, 6-14=-359/619, 4-14=-359/619
WEBS 3-6=-138/278

- NOTES-** (11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-10-8 to 3-11-2, Corner(3R) 3-11-2 to 8-0-14, Corner(3E) 8-0-14 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=-148, 4=148.

LOAD CASE(S) Standard



11/4/2025

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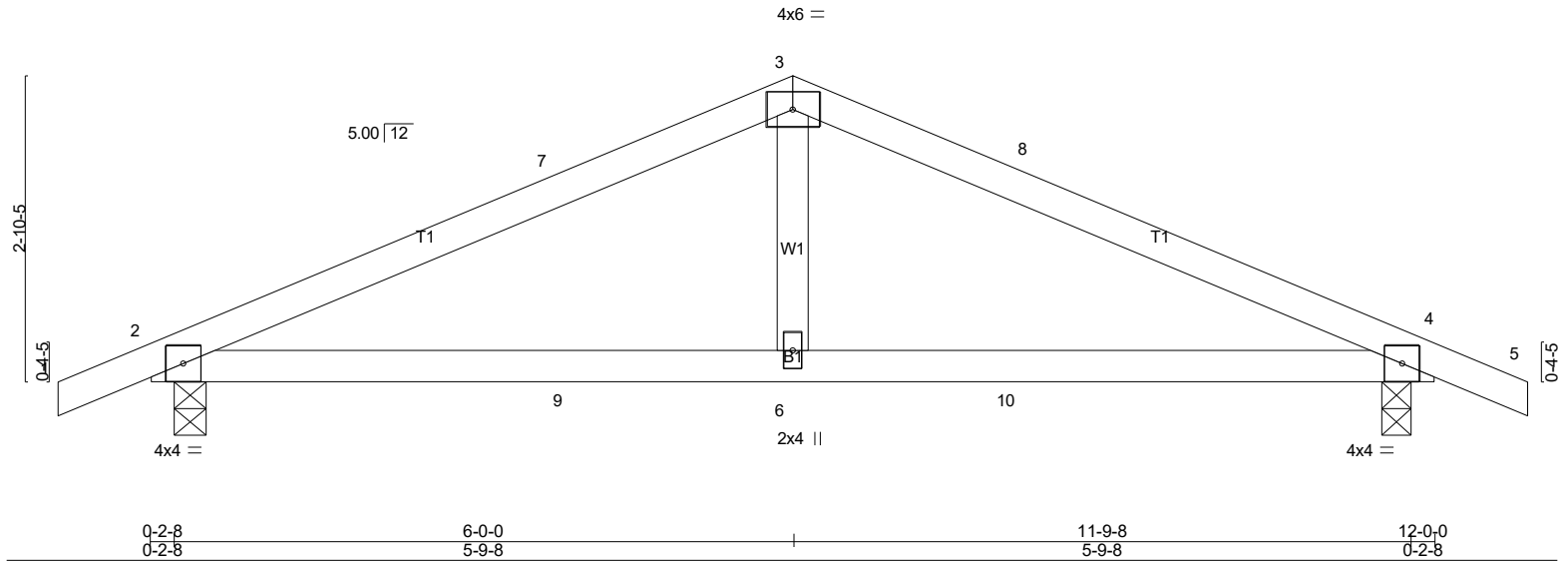
Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	SP02	COMMON	5	1	

Job Reference (optional)

64479

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Scale = 1:21.5



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.72	in (loc)	l/defl	L/d	MT20	244/190	
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.55	Vert(LL)	-0.06	4-6	>999	240	
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Vert(CT)	-0.10	4-6	>999	180	
BCLL	0.0 *	Code IRC2021/TPI2014		Matrix-SH		Horz(CT)	0.01	4	n/a	n/a	
BCDL	10.0										

Weight: 44 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-0-8 oc purlins.
Rigid ceiling directly applied or 9-9-9 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=530/0-3-8 (min. 0-1-8), 4=530/0-3-8 (min. 0-1-8)
Max Horz 2=43(LC 14)
Max Uplift 2=-148(LC 10), 4=-148(LC 11)
Max Grav 2=624(LC 21), 4=624(LC 22)

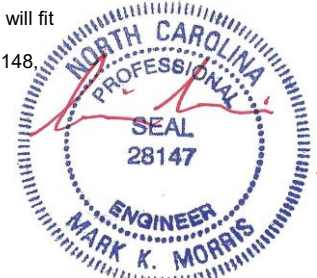
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-7=-791/457, 3-7=-655/468, 3-8=-655/468, 4-8=-791/457
BOT CHORD 2-9=-340/625, 6-9=-340/625, 6-10=-340/625, 4-10=-340/625
WEBS 3-6=-130/280

NOTES- (9)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Exterior(2R) 3-11-2 to 8-0-14, Exterior(2E) 8-0-14 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=148, 4=148.

LOAD CASE(S) Standard



11/4/2025

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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARIN
25-9160-R01	V01	Valley	1	1	

Job Reference (optional) # 64479

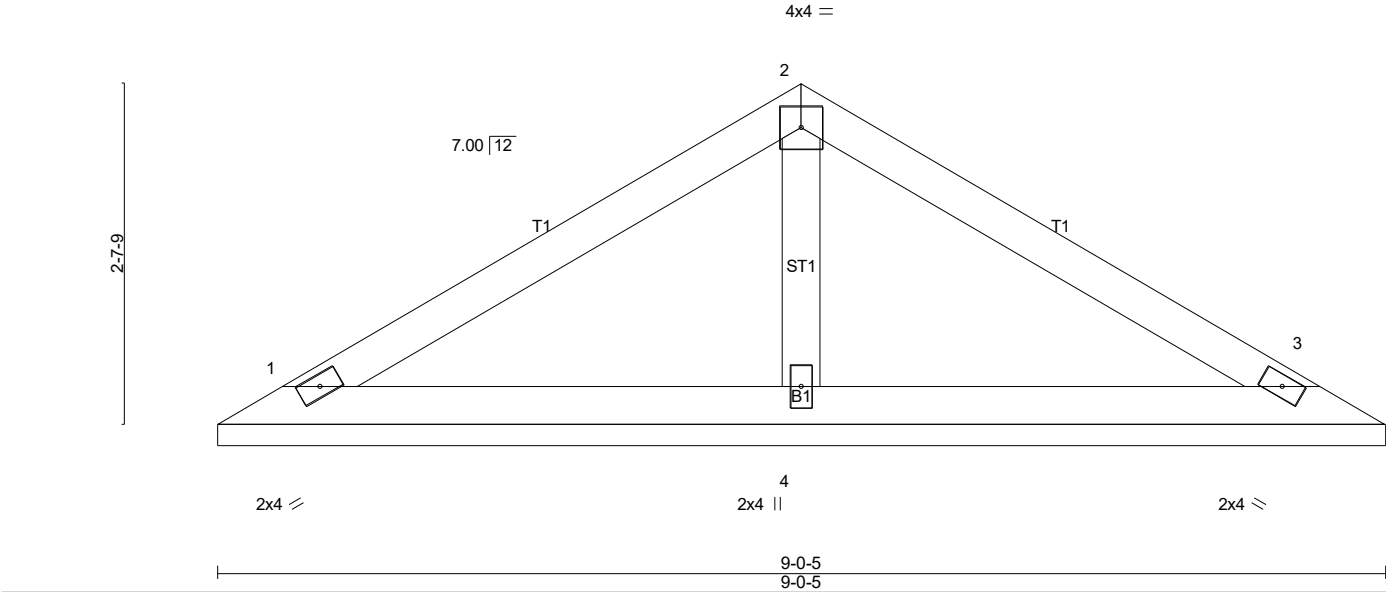
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9-0-5

4-6-2

Scale = 1:17.8



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	n/a	MT20		244/190	
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	n/a				
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00				
BCLL	0.0 *	Code IRC2021/TPI2014		Matrix-S							
BCDL	10.0										

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.3

OTHERS 2x4 SP No.3

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=150/9-0-5 (min. 0-1-8), 3=150/9-0-5 (min. 0-1-8), 4=335/9-0-5 (min. 0-1-8)

Max Horz 1=-51(LC 12)

Max Uplift1=-29(LC 14), 3=-36(LC 15), 4=-18(LC 14)

Max Grav 1=214(LC 20), 3=214(LC 21), 4=344(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard



11/4/2025

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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARINA NC 28147
25-9160-R01	V02	Valley	1	1	Job Reference (optional) # 64479

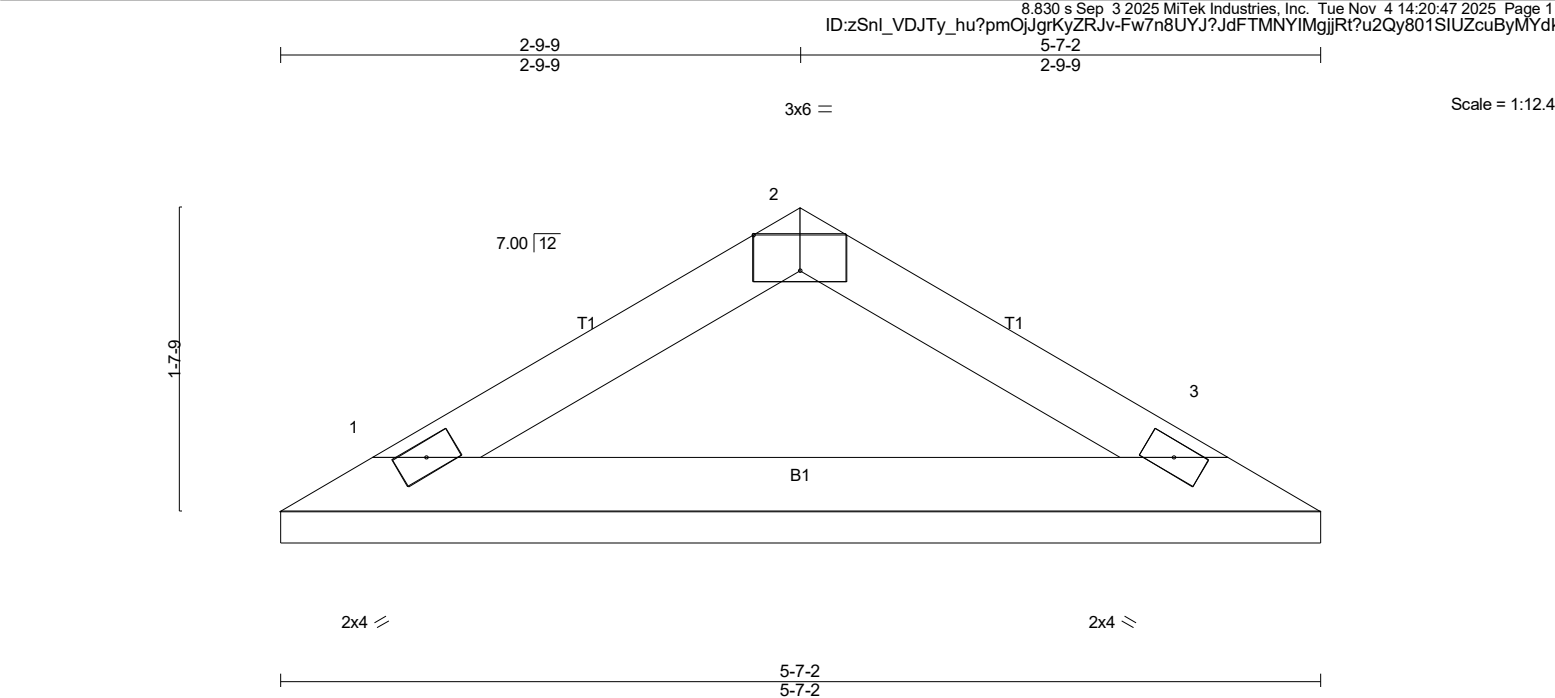


Plate Offsets (X,Y)-- [2:0-3-0,Edge]									
LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES	
TCLL (roof)	20.0	2-0-0		TC	0.13	in	(loc)	I/defl	L/d
Snow (Pf)	20.0	Plate Grip DOL	1.15	BC	0.44	n/a	-	n/a	999
TCDL	10.0	Lumber DOL	1.15	WB	0.00	n/a	-	n/a	999
BCLL	0.0 *	Rep Stress Incr	YES	Matrix-P		0.00	3	n/a	n/a
BCDL	10.0	Code IRC2021/TPI2014							
								Weight: 16 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-7-2 oc purlins.
BOT CHORD	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS. (lb/size) 1=181/5-7-2 (min. 0-1-8), 3=181/5-7-2 (min. 0-1-8)

Max Horz 1=-29(LC 10)

Max Uplift1=-21(LC 14), 3=-21(LC 15)

Max Grav 1=206(LC 20), 3=206(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (9)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



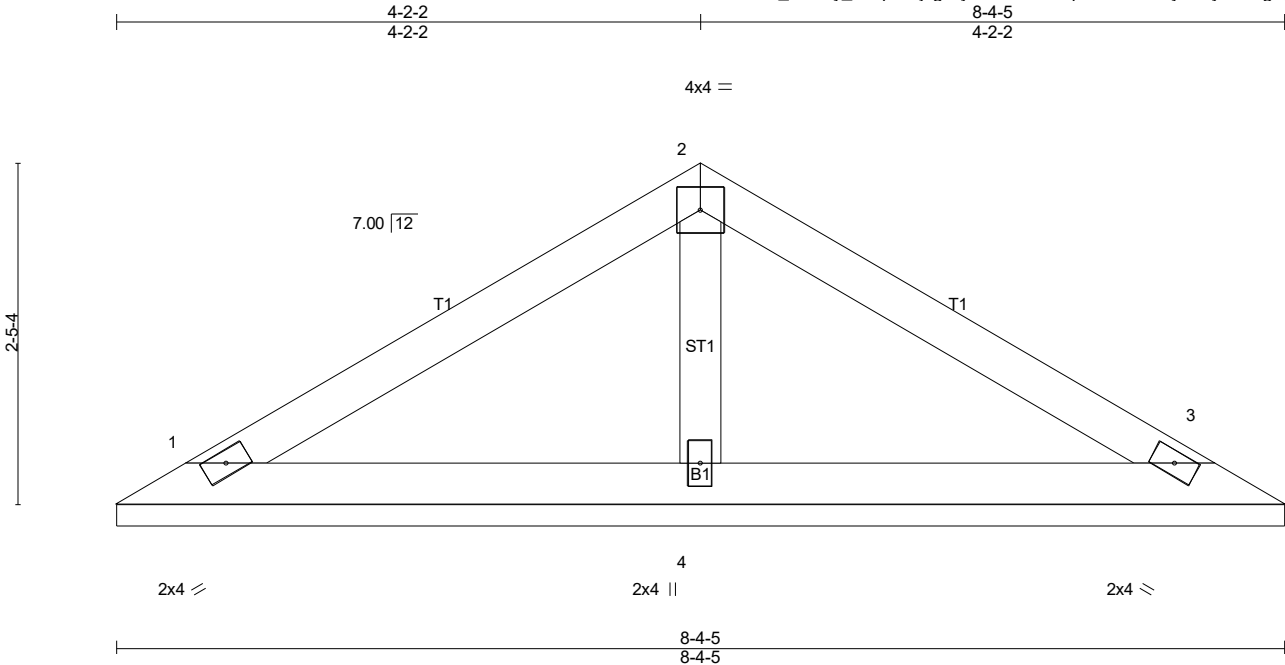
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Job	Truss	Truss Type	Qty	Ply	LOT 150 PROVIDENCE CREEK 424 PROVIDENCE CREEK DR FUQUAY-VARINA NC 28147
25-9160-R01	V03	Valley	1	1	

64479

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LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	n/a	MT20		244/190	
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	n/a				
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00				
BCLL	0.0 *	Code IRC2021/TPI2014		Matrix-P							
BCDL	10.0										

LUMBER-		BRACING-		Structural wood sheathing directly applied or 6-0-0 oc purlins.	
TOP CHORD	2x4 SP No.2	TOP CHORD		Rigid ceiling directly applied or 10-0-0 oc bracing.	
BOT CHORD	2x4 SP No.3	BOT CHORD			
OTHERS	2x4 SP No.3			MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS. (lb/size) 1=152/8-4-5 (min. 0-1-8), 3=152/8-4-5 (min. 0-1-8), 4=279/8-4-5 (min. 0-1-8)
 Max Horz 1=-47(LC 10)
 Max Uplift1=-33(LC 14), 3=-40(LC 15), 4=-2(LC 14)
 Max Grav 1=208(LC 20), 3=208(LC 21), 4=283(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (9)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

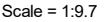
LOAD CASE(S) Standard



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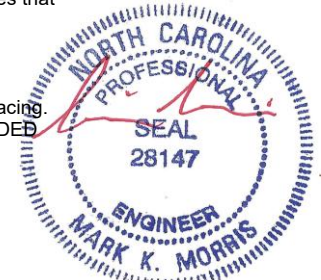
Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D’Onofrio Drive, Madison, WI 53719.

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MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

LOAD CASE(S) Standard



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