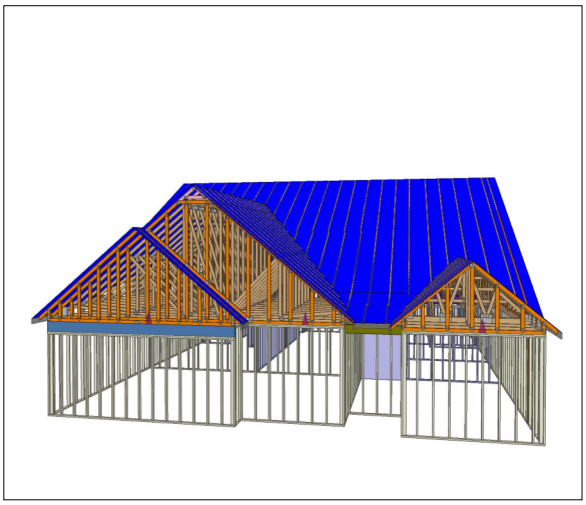
Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: DR Horton Inc

Model: 86 Eagle Creek - Kathryn - B

THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____ **Date:** _____

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.

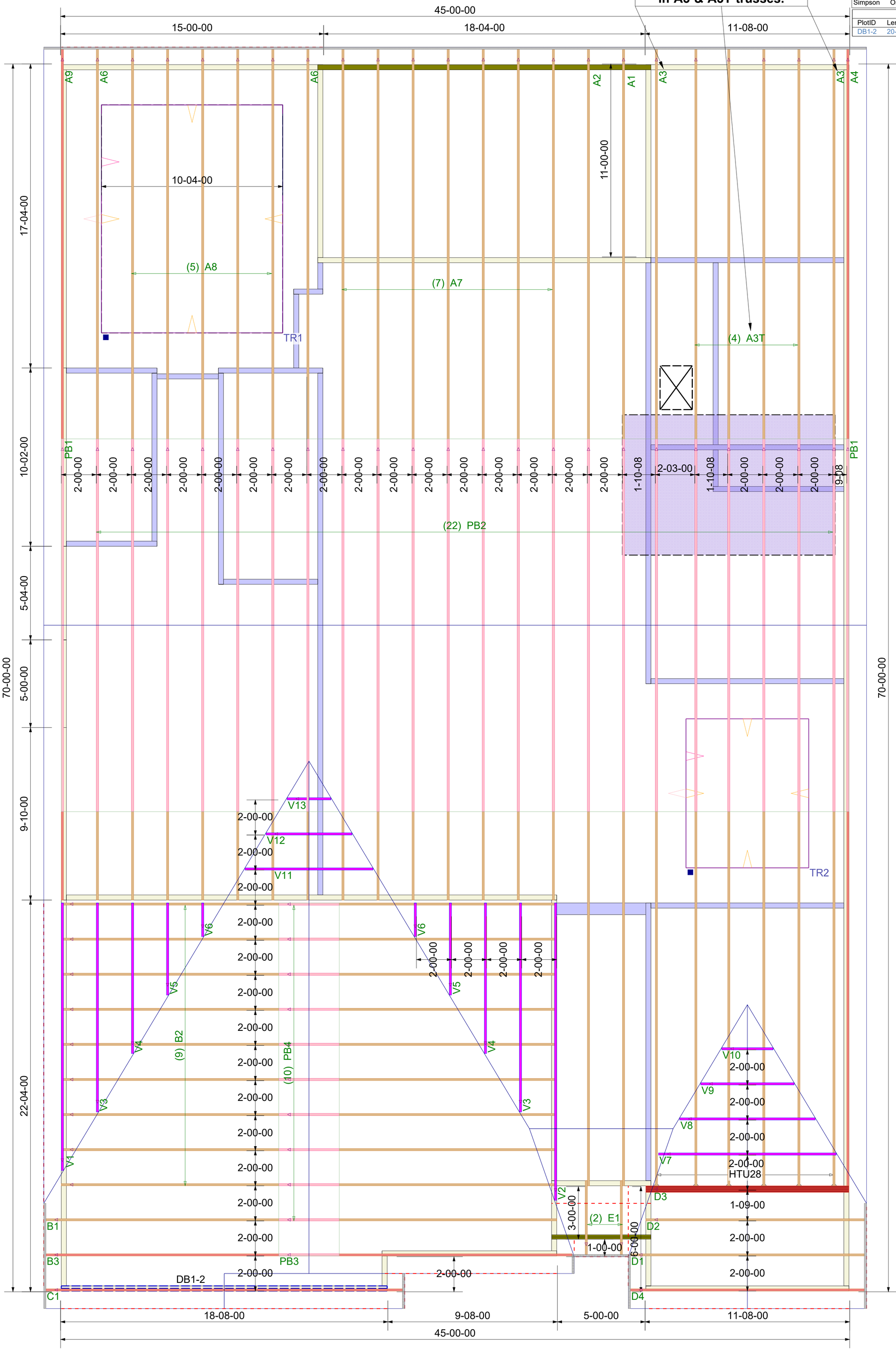
** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.

Higher deflection than usual in A3 & A3T trusses.

Truss Connector Total List		
Manuf	Product	Qty
Simpson	HTU26	6
Simpson	One H2.5A	85

Products					
PlotID	Length	Product	Plies	Net Qty	Fab Type
DB1-2	20-00-00	2.1 RigidLam SP LVL 1-3/4 x 11-7/8	2	2	FF



Truss Drawing Left End Indicator

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

** All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibilty of the bldg designer and or contractor.

General Notes:

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

Scale: NTS

Date: 11/3/2025

Designer: Nate Donaldson

Project Number: **25110007-A**

Sheet Number: **1/1**

DR Horton Inc

86 Eagle Creek - Kathryn - B

ROOF PLACEMENT PLAN

CARTER Lumber

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The disign of the tuss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179

Revisions		
00/00/00	Name	
00/00/00	Name	
00/00/00	Name	
00/00/00	Name	
00/00/00	Name	

ENGINEERING BY
TRENCO
A MiTek Affiliate

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: 25110007-A
86 Eagle Creek-Kathryn B

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I77504761 thru I77504795

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844

November 4,2025

Gilbert, Eric

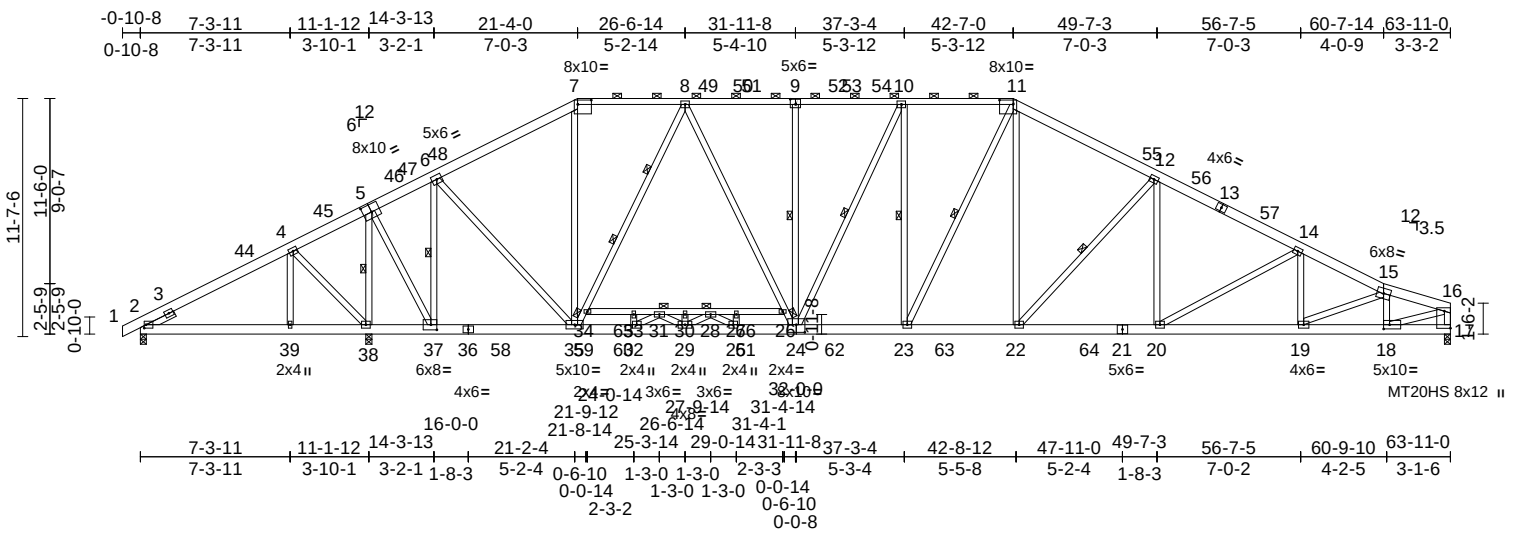
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A1	Piggyback Base	1	1	I77504761 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:10 ID:gzvdENKJOgX2Ss?L5D9RdAzEz27-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

ATTIC RESIDENTIAL LIMITED ACCESS
20 PSF. STORAGE USE ONLY.



Scale = 1:112.4

Plate Offsets (X, Y): [2:0-1-1,0-2-0], [5:0-5-0,0-4-8], [7:0-8-0,0-2-8], [9:0-3-0,0-3-0], [11:0-8-0,0-2-8], [17:Edge,0-3-8], [18:0-3-8,0-2-8], [24:0-5-0,0-4-8], [37:0-3-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.24	25-29	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.44	Vert(CT)	-0.47	25-29	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.10	17	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 566 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP 2400F 2.0E *Except* 9-11,9-7:2x4 SP No.2

BOT CHORD 2x6 SP 2400F 2.0E *Except* 34-26:2x4 SP No.2

WEBS 2x4 SP No.3 *Except* 35-7,22-11,8-35,24-8,24-9,24-10,10-23,23-11, 5-37,18-16:2x4 SP No.2

SLIDER Left 2x4 SP No.2 -- 1-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins, except end verticals, and 2-0-0 oc purlins (2-9-9 max.): 7-11.

BOT CHORD Rigid ceiling directly applied or 4-8-15 oc bracing.

WEBS 1 Row at midpt 12-22, 9-24, 10-24, 10-23, 11-23, 5-38, 6-37

WEBS 2 Rows at 1/3 pts 8-34

JOINTS 1 Brace at Jt(s): 28, 31, 34, 26

REACTIONS (size) 2=0-3-8, 17=0-3-8, 38=0-3-8
Max Horiz 2=185 (LC 18)
Max Uplift 2=-85 (LC 14), 17=-80 (LC 15)
Max Grav 2=330 (LC 46), 17=2485 (LC 6), 38=3758 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

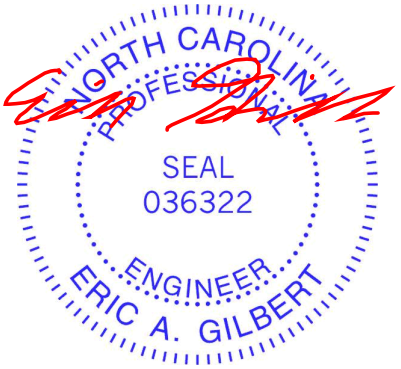
TOP CHORD 1-2=0/27, 2-4=-126/498, 4-6=-880/749, 6-7=-2517/267, 7-8=-2208/293, 8-10=-3540/386, 10-11=-3558/483, 11-12=-3838/511, 12-14=-4493/528, 14-15=-4854/533, 15-16=-3997/442, 16-17=-2575/308

BOT CHORD 2-39=-435/157, 38-39=-435/157, 37-38=-665/165, 35-37=0/736, 32-35=0/2983, 29-32=0/3694, 25-29=0/3652, 23-25=-55/3474, 22-23=-78/3251, 20-22=-249/3929, 19-20=-357/4341, 18-19=-371/3913, 17-18=-16/254, 33-34=-82/0, 31-33=-82/0, 30-31=-1478/0, 28-30=-1478/0, 27-28=-19/64, 26-27=-19/64

WEBS 7-35=0/790, 11-22=-99/1112, 32-33=0/65, 29-30=-3/47, 28-29=0/683, 25-27=0/102, 25-28=-1161/0, 34-35=-1900/158, 8-34=-1789/196, 12-20=0/517, 14-19=-46/137, 12-22=-1101/251, 8-26=-18/1301, 24-26=-60/1245, 9-24=-349/128, 10-24=-320/234, 31-32=-1071/0, 29-31=0/647, 14-20=-485/159, 10-23=-448/120, 11-23=-102/602, 15-19=0/502, 4-38=-592/186, 4-39=0/206, 5-38=-3415/214, 5-37=-182/2902, 6-37=-2589/180, 6-35=0/2166, 15-18=-1349/184, 16-18=-371/3792

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 5-6-3, Interior (1) 5-6-3 to 14-11-5, Exterior(2R) 14-11-5 to 27-8-11, Interior (1) 27-8-11 to 36-2-5, Exterior(2R) 36-2-5 to 48-11-11, Interior (1) 48-11-11 to 60-7-14, Exterior(2E) 60-7-14 to 63-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) 200.0lb AC unit load placed on the bottom chord, 26-6-14 from left end, supported at two points, 5-0-0 apart.



November 4,2025

Continued on page 2

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
8) Provide adequate drainage to prevent water ponding.
9) All plates are MT20 plates unless otherwise indicated.
10) All plates are 4x5 MT20 unless otherwise indicated.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17 and 2. This connection is for uplift only and does not consider lateral forces.
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A2	Piggyback Base	1	1	I77504762 Job Reference (optional)

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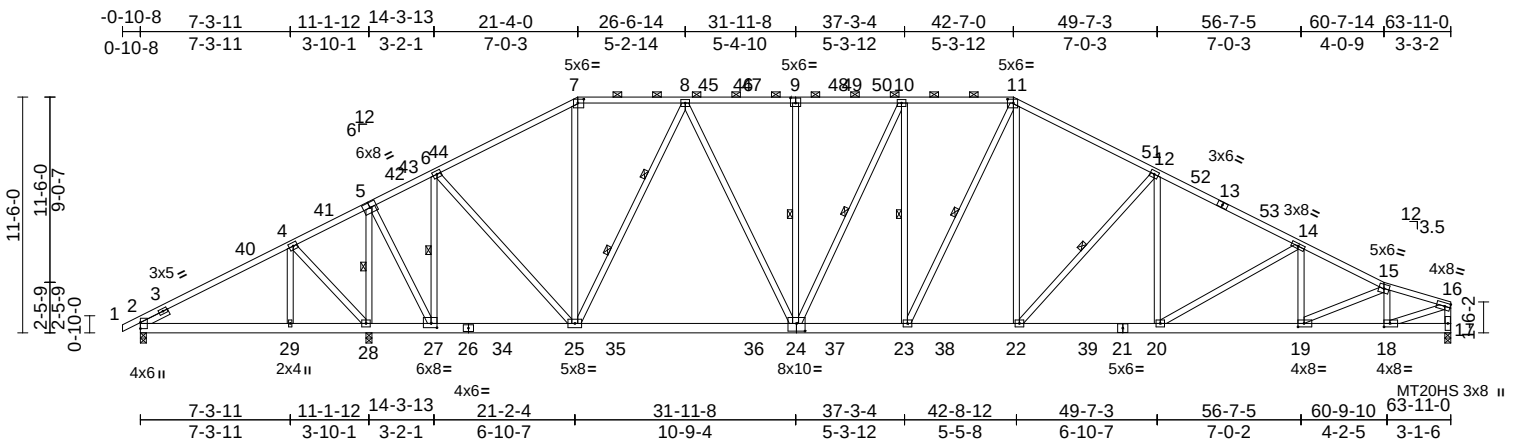


Scale = 1:112.4

Plate Offsets (X, Y): [7:0-3-8,0-2-4], [9:0-3-0,0-3-0], [11:0-3-8,0-2-4], [18:0-3-8,0-2-0], [19:0-3-8,0-2-0], [24:0-5-0,0-4-8], [27:0-3-8,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	-0.22	24-25	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.29	Vert(CT)	-0.38	24-25	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(CT)	0.08	17	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 504 lb	FT = 20%

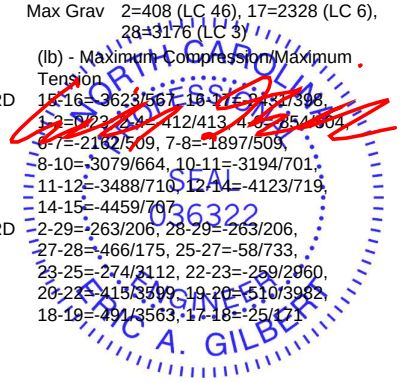
LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 25-7,22-11,24-8,24-9,24-10,10-23,23-11,8-25, 18-16:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins, except end verticals, and 2-0-0 oc purlins (4-6-6 max.): 7-11.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 2-29,28-29,27-28.
WEBS 1 Row at midpt 12-22, 9-24, 10-24, 10-23, 11-23, 5-28, 6-27
WEBS 2 Rows at 1/3 pts 8-25

REACTIONS (size) 2=0-3-8, 17=0-3-8, 28=0-3-8
Max Horiz 2=182 (LC 18)
Max Uplift 2=-74 (LC 14), 17=-168 (LC 15), 28=-132 (LC 14)
Max Grav 2=408 (LC 46), 17=2328 (LC 6), 28=3176 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 15-16=-3623/567, 16-17=-3431/398, 1-2=0/23, 2-4=-412/413, 4-5=-854/304, 6-7=-2162/509, 7-8=-1897/509, 8-10=-3079/664, 10-11=-3194/701, 11-12=-3488/710, 12-14=-4123/719, 14-15=-4459/767
BOT CHORD 2-29=-263/206, 28-29=-263/206, 27-28=-466/175, 25-27=-58/733, 23-25=-274/3112, 22-23=-259/2960, 20-22=-415/3599, 19-20=-510/3982, 18-19=-491/3563, 17-18=-25/171
WEBS 7-25=-34/616, 11-22=-96/1060, 14-19=-58/137, 12-22=-1052/236, 8-24=-93/1120, 9-24=-346/127, 10-24=-468/100, 14-20=-462/148, 10-23=-304/273, 11-23=-218/447, 15-19=-21/491, 8-25=-1605/262, 12-20=0/513, 4-29=0/261, 5-28=-2876/416, 4-28=-618/187, 6-27=-2140/384, 5-27=-361/2486, 6-25=-114/1709, 15-18=-1336/245, 16-18=-499/3547

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 5-6-3, Interior (1) 5-6-3 to 14-11-5, Exterior(2R) 14-11-5 to 27-8-11, Interior (1) 27-8-11 to 36-2-5, Exterior(2R) 36-2-5 to 48-11-11, Interior (1) 48-11-11 to 60-7-14, Exterior(2E) 60-7-14 to 63-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
7) Provide adequate drainage to prevent water ponding.
8) All plates are MT20 plates unless otherwise indicated.
9) All plates are 4x5 MT20 unless otherwise indicated.
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



November 4,2025

Continued on page 2

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17, 2, and 28. This connection is for uplift only and does not consider lateral forces.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A3	Piggyback Base	2	1	I77504763 Job Reference (optional)

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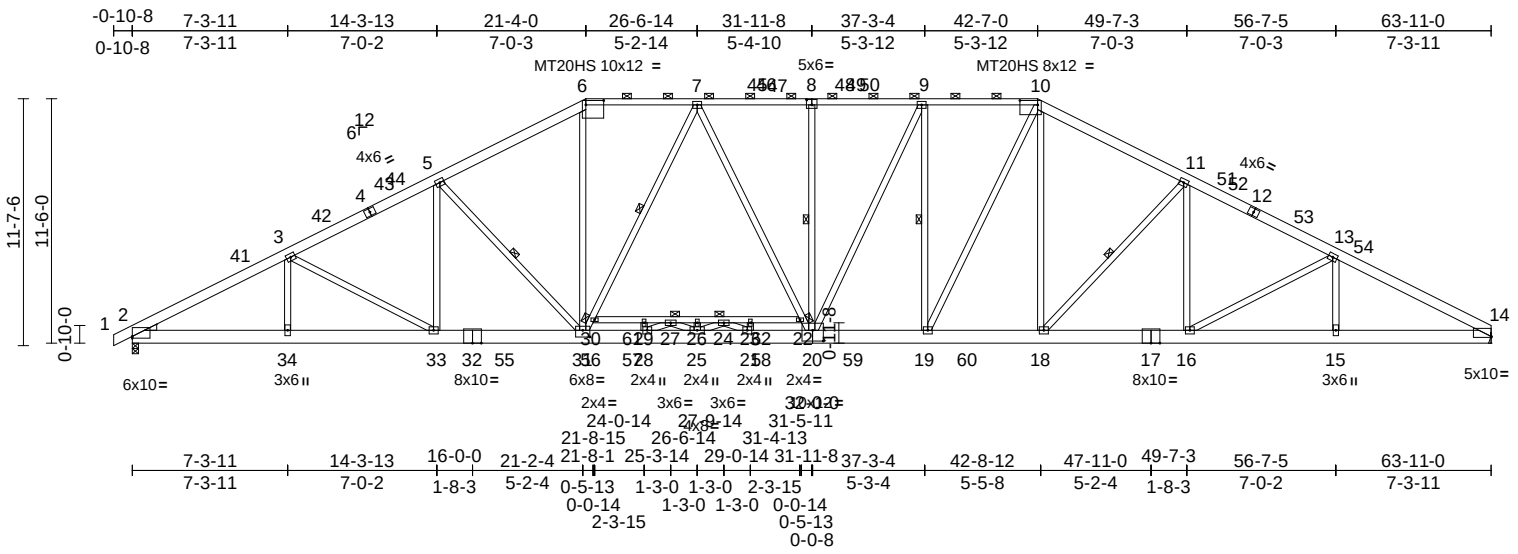
ID:E7rVnEkJT57qN7iI?w1GXqzEymp-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

ATTIC RESIDENTIAL LIMITED ACCESS
20 PSF. STORAGE USE ONLY.



Scale = 1:108.4

Plate Offsets (X, Y): [2:Edge,0-1-1], [6:0-10-0,0-2-8], [8:0-3-0,0-3-0], [10:0-10-0,0-2-8], [14:Edge,0-0-9], [20:0-6-0,0-6-0], [31:0-4-0,0-3-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	-0.35	21-25	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.67	21-25	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.17	14	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 582 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP 2400F 2.0E *Except* 6-8,8-10:2x4 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E *Except* 30-22:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 31-6,7-31,20-7,18-10,20-8,20-9,9-19,19-10:2x4 SP No.2
WEDGE Left: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins, except 2-0-0 oc purlins (3-5-0 max.): 6-10.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 29-30,27-29 5-4-10 oc bracing: 26-27 5-4-11 oc bracing: 24-26.
WEBS 1 Row at midpt 7-30, 11-18, 8-20, 5-31, 9-19
JOINTS 1 Brace at Jt(s): 24, 27, 30, 22

REACTIONS (size) 2=0-3-8, 14= Mechanical
Max Horiz 2=185 (LC 18)
Max Uplift 2=-94 (LC 14), 14=-131 (LC 15)
Max Grav 2=3265 (LC 3), 14=3093 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-3=-6702/177, 3-5=-6300/195, 5-6=-5718/171, 6-7=-4991/192, 7-9=-5563/213, 9-10=-5256/294, 10-11=-5414/310, 11-13=-6046/308, 13-14=-6441/300
BOT CHORD 2-34=-185/5879, 33-34=-185/5879, 31-33=-38/5534, 28-31=0/5234, 25-28=0/5740, 21-25=0/5755, 19-21=0/5228, 18-19=0/4622, 16-18=-86/5308, 15-16=-185/5648, 14-15=-185/5648, 29-30=-9/13, 27-29=-9/13, 26-27=-1170/0, 24-26=-1170/0, 23-24=-11/21, 22-23=-11/21
WEBS 3-34=-29/127, 6-31=0/2234, 30-31=-1026/162, 7-30=-984/186, 7-22=-87/564, 20-22=-113/519, 10-18=-92/1093, 11-18=-1146/244, 11-16=0/488, 13-16=-390/175, 28-29=-85/10, 25-26=-77/28, 24-25=0/584, 21-24=-812/0, 21-23=-58/9, 8-20=-352/128, 9-20=0/804, 27-28=-793/0, 25-27=0/596, 5-31=-1075/280, 5-33=-40/416, 3-33=-395/169, 13-15=-37/142, 9-19=-1105/38, 10-19=-43/1346

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 5-6-3, Interior (1) 5-6-3 to 12-3-8, Exterior(2R) 12-3-8 to 30-4-8, Interior (1) 30-4-8 to 33-6-8, Exterior(2R) 33-6-8 to 51-7-8, Interior (1) 51-7-8 to 57-6-5, Exterior(2E) 57-6-5 to 63-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) 200.0lb AC unit load placed on the bottom chord, 26-6-14 from left end, supported at two points, 5-0-0 apart.
7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
8) Provide adequate drainage to prevent water ponding.
9) All plates are MT20 plates unless otherwise indicated.
10) All plates are 4x5 MT20 unless otherwise indicated.

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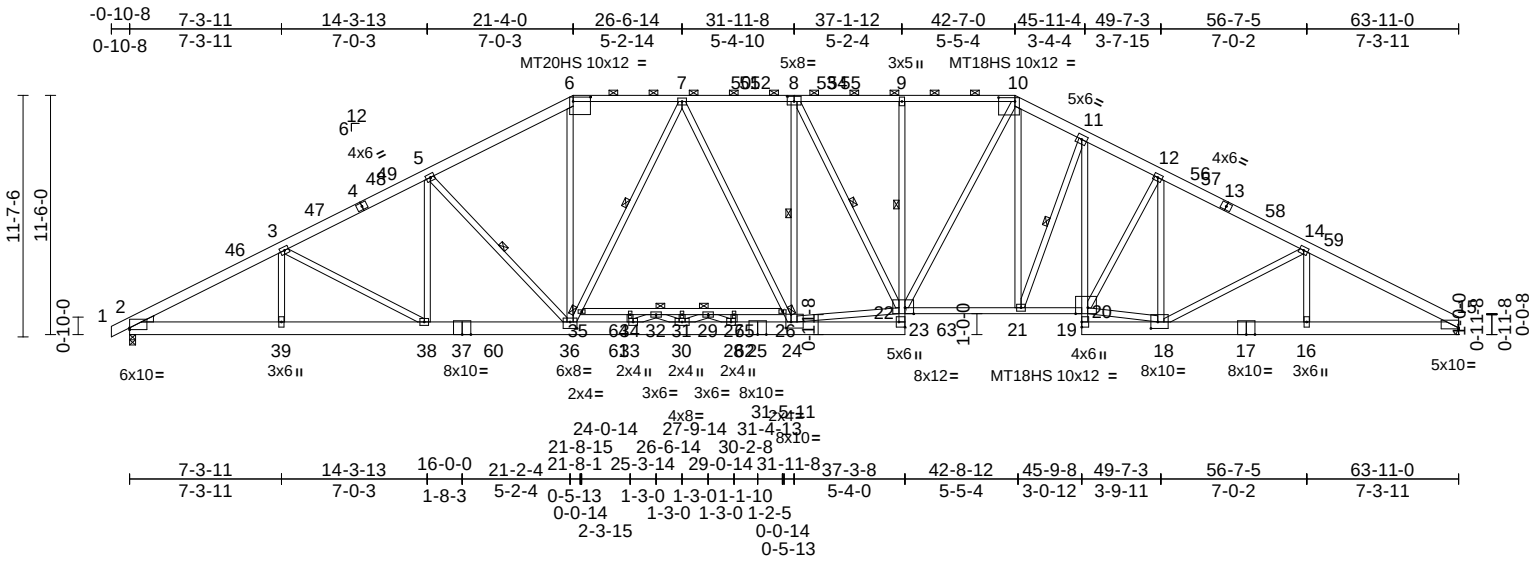
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
13) Refer to girder(s) for truss to truss connections.
14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 14.
15) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A3T	Piggyback Base	4	1	I77504764 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:12 ID:E7rVnEkJT57qN7iI?w1GXqzEymp-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1

ATTIC RESIDENTIAL LIMITED ACCESS
20 PSF. STORAGE USE ONLY.



Scale = 1:110.8

Plate Offsets (X, Y): [2:Edge,0-1-1], [6:0-10-0,0-2-8], [8:0-4-0,0-3-0], [10:0-9-8,0-2-4], [15:Edge,0-0-9], [18:0-4-0,0-3-12], [20:0-7-0,Edge], [22:0-5-0,Edge], [23:Edge,0-3-8], [24:0-3-0,0-3-8], [36:0-4-0,0-3-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.79	Vert(LL)	-0.39	28-30	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.81	Vert(CT)	-0.76	28-30	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(CT)	0.25	15	n/a	n/a	MT18HS	244/190
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 600 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP 2400F 2.0E *Except* 6-8:2x4 SP No.2, 8-10:2x4 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E *Except* 23-9,35-26:2x4 SP No.2, 22-20:2x4 SP 2400F 2.0E, 11-19:2x4 SP No.3
WEBS 2x4 SP No.3 *Except* 36-6,24-7,24-8,8-22,22-10,36-7:2x4 SP No.2, 24-22,20-18:2x4 SP No.1
WEDGE Left: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-2 oc purlins, except 2-0-0 oc purlins (2-2-0 max.): 6-10.
BOT CHORD Rigid ceiling directly applied or 5-4-11 oc bracing. Except:
1 Row at midpt 9-22
WEBS 1 Row at midpt 5-36, 8-24, 8-22, 11-21, 7-35
JOINTS 1 Brace at Jt(s): 35, 26, 32, 29

REACTIONS (size) 2=0-3-8, 15= Mechanical
Max Horiz 2=185 (LC 14)
Max Uplift 2=-94 (LC 14), 15=-131 (LC 15)
Max Grav 2=3242 (LC 3), 15=3065 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-3=-6656/177, 3-5=-6252/195, 5-6=-5667/171, 6-7=-4945/192, 7-9=-5602/284, 9-10=-5611/284, 10-11=-5745/327, 11-12=-6160/334, 12-14=-6011/306, 14-15=-6384/303
BOT CHORD 2-39=-185/5838, 38-39=-185/5838, 36-38=-38/5492, 33-36=0/5183, 30-33=0/5689, 28-30=0/5718, 24-28=0/5226, 23-24=-30/464, 22-23=-18/42, 9-22=-480/144, 21-22=0/4947, 20-21=-21/5357, 19-20=0/90, 11-20=-143/1039, 18-19=-11/522, 16-18=-188/5600, 15-16=-188/5600, 34-35=-9/12, 32-34=-9/12, 31-32=-1170/0, 29-31=-1170/0, 27-29=-46/6, 26-27=-46/6
WEBS 3-39=-30/147, 3-38=-397/169, 5-38=-40/422, 5-36=-1082/280, 6-36=0/2216, 7-26=-83/540, 24-26=-102/469, 8-24=-618/309, 22-24=0/4992, 8-22=-253/456, 10-22=-33/1317, 10-21=-135/1453, 11-21=-1222/213, 12-20=-151/409, 35-36=-1012/159, 7-35=-968/184, 12-18=-668/42, 14-18=-415/187, 18-20=-71/4867, 14-16=-54/127, 33-34=-82/10, 30-31=-79/28, 27-28=-70/6, 32-33=-794/0, 30-32=0/592, 29-30=0/568, 28-29=-780/0

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 5-6-3, Interior (1) 5-6-3 to 12-3-8, Exterior(2R) 12-3-8 to 30-4-8, Interior (1) 30-4-8 to 33-6-8, Exterior(2R) 33-6-8 to 51-7-8, Interior (1) 51-7-8 to 57-6-5, Exterior(2E) 57-6-5 to 63-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) 200.0lb AC unit load placed on the bottom chord, 26-6-14 from left end, supported at two points, 5-0-0 apart.

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7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
8) Provide adequate drainage to prevent water ponding.
9) All plates are MT20 plates unless otherwise indicated.
10) All plates are 4x5 MT20 unless otherwise indicated.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
13) Refer to girder(s) for truss to truss connections.
14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 15.
15) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	A4	Piggyback Base Supported Gable	1	1	Job Reference (optional)	I77504765

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1

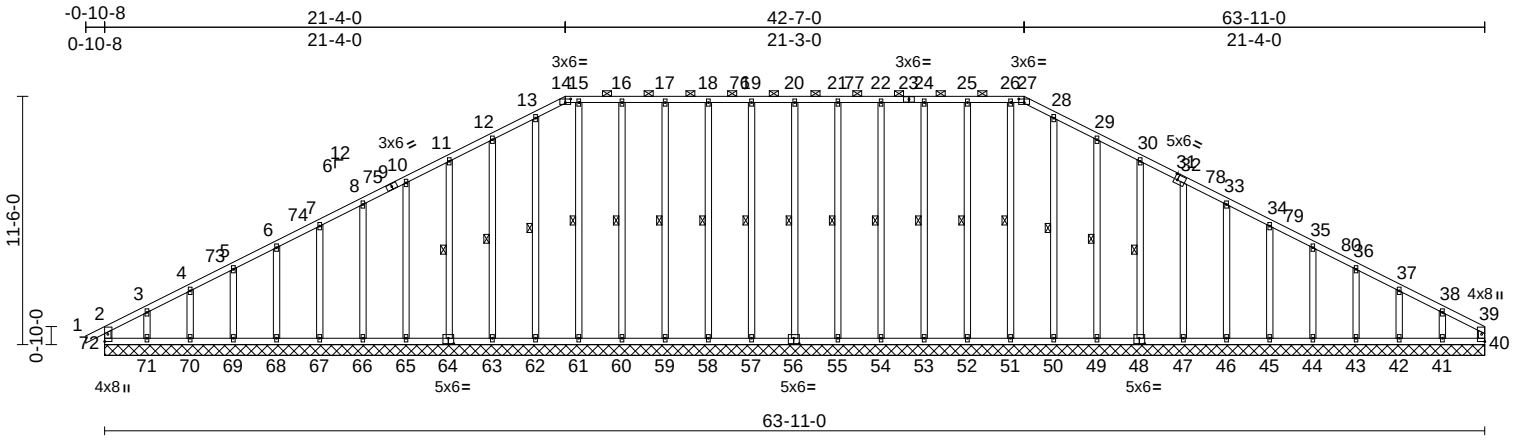

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Plate Offsets (X, Y): [14:0-3-0,0-2-0], [27:0-3-0,0-2-0], [31:0-1-12,0-3-0], [48:0-3-0,0-3-0], [56:0-3-0,0-3-0], [64:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.22	Horz(CT)	0.01	40	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 551 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3 *Except* 56-20,57-19,58-18,59-17,60-16,61-15,62-13, 55-21,54-22,53-24,52-25,51-26,50-28:2x4 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 14-27.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 20-56, 19-57, 18-58, 17-59, 16-60, 15-61, 13-62, 12-63, 11-64, 21-55, 22-54, 24-53, 25-52, 26-51, 28-50, 29-49, 30-48

REACTIONS (size) 40=63-11-0, 41=63-11-0, 42=63-11-0, 43=63-11-0, 44=63-11-0, 45=63-11-0, 46=63-11-0, 47=63-11-0, 48=63-11-0, 49=63-11-0, 50=63-11-0, 51=63-11-0, 52=63-11-0, 53=63-11-0, 54=63-11-0, 55=63-11-0, 56=63-11-0, 57=63-11-0, 58=63-11-0, 59=63-11-0, 60=63-11-0, 61=63-11-0, 62=63-11-0, 63=63-11-0, 64=63-11-0, 65=63-11-0, 66=63-11-0, 67=63-11-0, 68=63-11-0, 69=63-11-0, 70=63-11-0, 71=63-11-0, 72=63-11-0

Max Horiz 72=166 (LC 14)

Max Uplift 41=-117 (LC 15), 42=-27 (LC 15), 43=-48 (LC 15), 44=-43 (LC 15), 45=-44 (LC 15), 46=-44 (LC 15), 47=-43 (LC 15), 48=-42 (LC 15), 49=-55 (LC 15), 50=-1 (LC 15), 52=-34 (LC 10), 53=-25 (LC 11), 54=-25 (LC 10), 55=-25 (LC 11), 56=-25 (LC 10), 57=-26 (LC 11), 58=-25 (LC 10), 59=-25 (LC 11), 60=-36 (LC 10), 62=-9 (LC 14), 63=-52 (LC 14), 64=-42 (LC 14), 65=-45 (LC 14), 66=-43 (LC 14), 67=-44 (LC 14), 68=-42 (LC 14), 69=-49 (LC 14), 70=-22 (LC 14), 71=-138 (LC 14), 72=-39 (LC 10)

Max Grav 40=116 (LC 57), 41=166 (LC 59), 42=159 (LC 45), 43=160 (LC 59), 44=162 (LC 45), 45=227 (LC 45), 46=243 (LC 45), 47=241 (LC 45), 48=240 (LC 45), 49=241 (LC 45), 50=237 (LC 45), 51=187 (LC 40), 52=222 (LC 40), 53=216 (LC 40), 54=219 (LC 40), 55=189 (LC 40), 56=160 (LC 59), 57=188 (LC 40), 58=219 (LC 40), 59=216 (LC 40), 60=222 (LC 40), 61=195 (LC 56), 62=235 (LC 43), 63=241 (LC 43), 64=240 (LC 43), 65=238 (LC 43), 66=243 (LC 43), 67=221 (LC 43), 68=161 (LC 41), 69=159 (LC 58), 70=165 (LC 41), 71=152 (LC 25), 72=188 (LC 55)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-72=-148/42, 1-2=0/27, 2-3=-223/85, 3-4=-160/80, 4-5=-127/93, 5-6=-97/111, 6-7=-74/135, 7-8=-63/165, 8-10=-69/210, 10-11=-88/255, 11-12=-106/300, 12-13=-126/351, 13-14=-135/367, 14-15=-126/354, 15-16=-126/354, 16-17=-126/354, 17-18=-126/354, 18-19=-126/354, 19-20=-126/354, 20-21=-126/354, 21-22=-126/354, 22-24=-126/354, 24-25=-126/354, 25-26=-126/354, 26-27=-126/354, 27-28=-135/367, 28-29=-127/351, 29-30=-106/300, 30-32=-90/256, 32-33=-71/211, 33-34=-57/166, 34-35=-45/120, 35-36=-53/75, 36-37=-76/38, 37-38=-106/35, 38-39=-162/55, 39-40=-83/8

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BOT CHORD 71-72=-38/160, 70-71=-38/160, 69-70=-38/160, 68-69=-38/160, 67-68=-38/160, 66-67=-38/160, 65-66=-38/160, 63-65=-38/160, 62-63=-37/160, 61-62=-37/160, 60-61=-37/160, 59-60=-37/160, 58-59=-37/160, 57-58=-37/160, 55-57=-37/160, 54-55=-37/160, 53-54=-37/160, 52-53=-37/160, 51-52=-37/160, 50-51=-37/160, 49-50=-37/160, 47-49=-37/160, 46-47=-37/160, 45-46=-37/160, 44-45=-37/160, 43-44=-37/160, 42-43=-37/160, 41-42=-37/160, 40-41=-37/160

WEBS 20-56=-121/57, 19-57=-148/57, 18-58=-179/57, 17-59=-176/56, 16-60=-182/75, 15-61=-155/2, 13-62=-196/33, 12-63=-201/93, 11-64=-200/75, 10-65=-199/78, 8-66=-203/77, 7-67=-181/78, 6-68=-127/75, 5-69=-126/91, 4-70=-126/118, 3-71=-128/144, 21-55=-148/57, 22-54=-179/57, 24-53=-176/56, 25-52=-182/75, 26-51=-147/0, 28-50=-196/25, 29-49=-202/92, 30-48=-200/75, 32-47=-200/77, 33-46=-203/77, 34-45=-187/77, 35-44=-127/76, 36-43=-127/113, 37-42=-124/115, 38-41=-137/160

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 5-6-3, Exterior(2N) 5-6-3 to 14-11-5, Corner(3R) 14-11-5 to 27-11-8, Exterior(2N) 27-11-8 to 35-11-8, Corner(3R) 35-11-8 to 48-11-11, Exterior(2N) 48-11-11 to 57-4-9, Corner(3E) 57-4-9 to 63-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
8) Provide adequate drainage to prevent water ponding.
9) All plates are 2x4 MT20 unless otherwise indicated.
10) Gable requires continuous bottom chord bearing.
11) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
12) Gable studs spaced at 2-0-0 oc.
13) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
14) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 72, 25 lb uplift at joint 56, 26 lb uplift at joint 57, 25 lb uplift at joint 58, 25 lb uplift at joint 59, 36 lb uplift at joint 60, 9 lb uplift at joint 62, 52 lb uplift at joint 63, 42 lb uplift at joint 64, 45 lb uplift at joint 65, 43 lb uplift at joint 66, 44 lb uplift at joint 67, 42 lb uplift at joint 68, 49 lb uplift at joint 69, 22 lb uplift at joint 70, 138 lb uplift at joint 71, 25 lb uplift at joint 55, 25 lb uplift at joint 54, 25 lb uplift at joint 53, 34 lb uplift at joint 52, 1 lb uplift at joint 50, 55 lb uplift at joint 49, 42 lb uplift at joint 48, 43 lb uplift at joint 47, 44 lb uplift at joint 46, 44 lb uplift at joint 45, 43 lb uplift at joint 44, 48 lb uplift at joint 43, 27 lb uplift at joint 42 and 117 lb uplift at joint 41.
16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A6	Piggyback Base	2	1	I77504766 Job Reference (optional)

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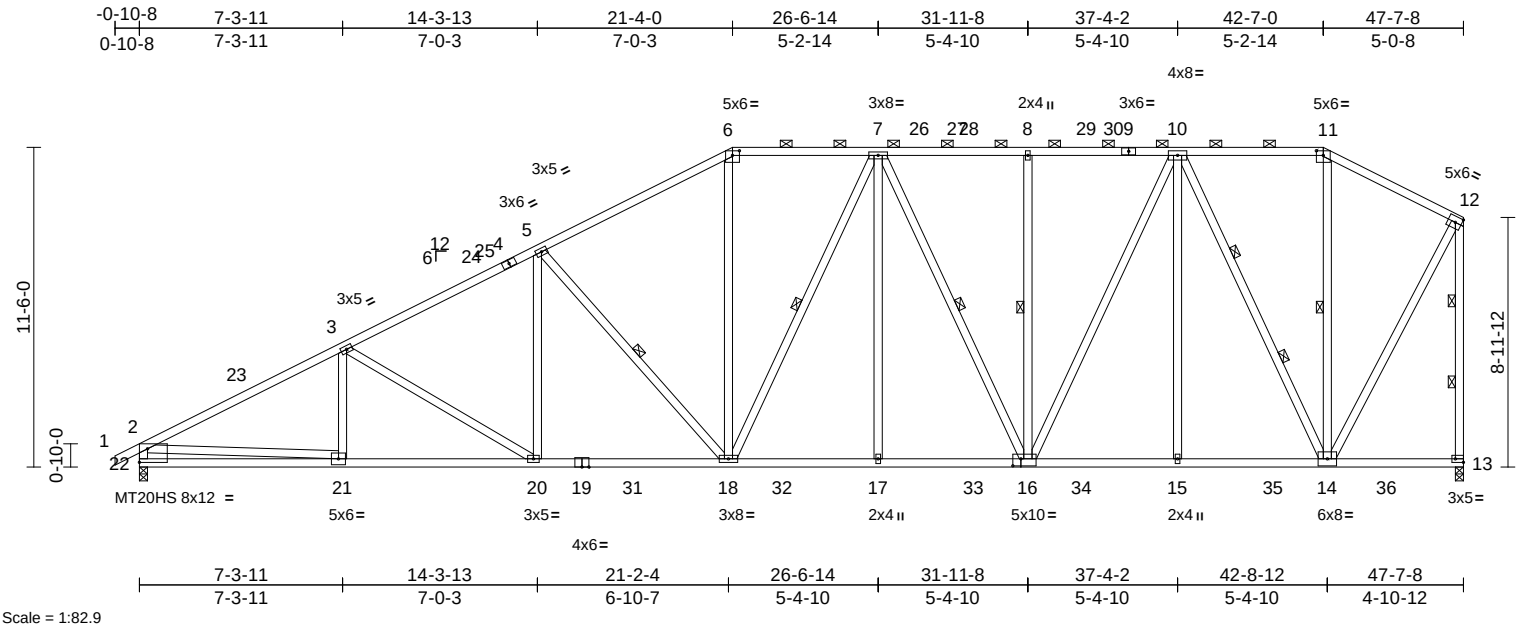

Scale = 1:82.9

Plate Offsets (X, Y): [6:0-3-0,0-2-0], [11:0-3-0,0-2-0], [13:Edge,0-1-8], [16:0-3-8,0-3-0], [22:Edge,0-5-13]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.98	Vert(LL)	-0.20	18-20	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.36	18-20	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.10	13	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 382 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.2 *Except* 3-21,20-3,5-20,18-5,14-12:2x4 SP No.3, 13-12,18-7,16-7,16-10,14-10:2x4 SP No.1

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-6 oc purlins, except end verticals, and 2-0-0 oc purlins (5-1-3 max.): 6-11.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-18, 11-14, 7-18, 7-16, 8-16
WEBS 2 Rows at 1/3 pts 12-13, 10-14

REACTIONS (size) 13=0-3-8, 22=0-3-8
Max Horiz 22=363 (LC 11)
Max Uplift 13=-150 (LC 15), 22=-257 (LC 14)
Max Grav 13=2252 (LC 46), 22=2187 (LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/27, 2-3=-4002/414, 3-5=-3592/420, 5-6=-2956/402, 6-7=-2568/401, 7-8=-2371/372, 8-10=-2371/372, 10-11=-961/276, 11-12=-1078/284, 2-22=-2255/294, 12-13=-2412/258
BOT CHORD 21-22=-374/1033, 20-21=-470/3731, 18-20=-346/3341, 17-18=-319/2764, 15-17=-319/2764, 14-15=-244/1977, 13-14=-110/154
WEBS 3-21=-54/148, 3-20=-455/145, 5-20=0/523, 5-18=-1046/229, 6-18=-39/955, 11-14=-69/275, 12-14=-180/2119, 2-21=-136/2711, 7-17=0/297, 7-18=-268/284, 7-16=-678/153, 8-16=-352/128, 10-16=-179/1247, 10-15=0/310, 10-14=-2029/213

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 3-10-10, Interior (1) 3-10-10 to 14-3-13, Exterior(2R) 14-3-13 to 28-0-13, Interior (1) 28-0-13 to 35-10-3, Exterior(2R) 35-10-3 to 42-7-0, Exterior(2E) 42-7-0 to 47-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 13. This connection is for uplift only and does not consider lateral forces.
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A7	Piggyback Base	7	1	I77504767 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:14 Page: 1
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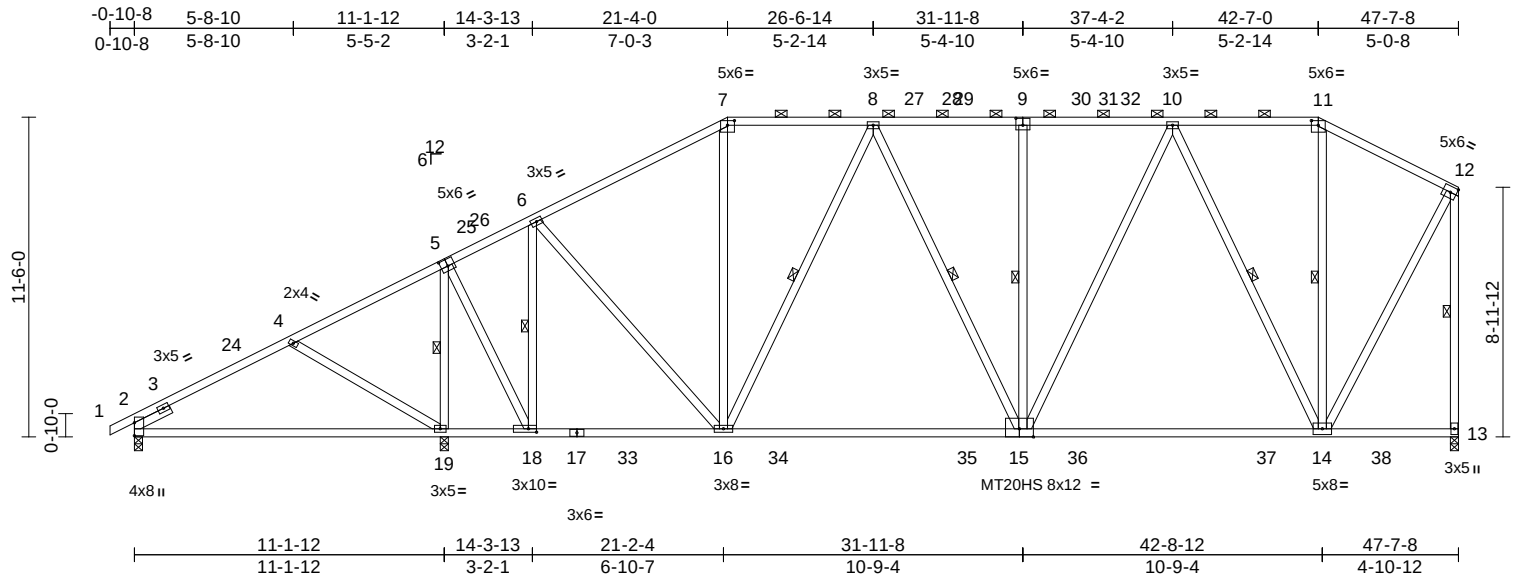

Scale = 1:82.9

Plate Offsets (X, Y): [5:0-3-0,0-3-0], [7:0-3-0,0-2-0], [9:0-3-0,0-3-0], [11:0-3-0,0-2-0], [18:0-3-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.88	Vert(LL)	-0.33	14-15	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.66	Vert(CT)	-0.53	14-15	>822	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(CT)	0.05	13	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 351 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.2 *Except* 4-19,5-19,5-18,6-18,16-6,14-12:2x4 SP No.3, 13-12:2x4 SP No.1
SLIDER Left 2x4 SP No.3 -- 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-11.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-19, 6-18, 8-16, 8-15, 9-15, 11-14, 12-13, 10-14

REACTIONS (size) 2=0-3-8, 13=0-3-8, 19=0-3-8
Max Horiz 2=357 (LC 13)
Max Uplift 2=-15 (LC 14), 13=-135 (LC 15), 19=-313 (LC 14)
Max Grav 2=606 (LC 37), 13=1810 (LC 46), 19=2083 (LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-4=-950/56, 4-6=-905/144, 6-7=-1574/255, 7-8=-1378/269, 8-10=-1710/290, 10-11=-768/247, 11-12=-860/252, 12-13=-1962/189
BOT CHORD 2-19=-379/753, 18-19=-94/376, 16-18=-163/987, 14-16=-251/1789, 13-14=-111/153
WEBS 4-19=-459/204, 5-19=-1745/315, 5-18=-175/1367, 6-18=-1155/180, 6-16=-73/829, 7-16=0/363, 8-16=-634/179, 8-15=-31/206, 9-15=-350/129, 11-14=-125/203, 12-14=-132/1719, 10-14=-1335/197, 10-15=-59/826

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 3-10-10, Interior (1) 3-10-10 to 14-3-13, Exterior(2R) 14-3-13 to 28-0-13, Interior (1) 28-0-13 to 35-10-3, Exterior(2R) 35-10-3 to 42-7-0, Exterior(2E) 42-7-0 to 47-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 19, and 13. This connection is for uplift only and does not consider lateral forces.
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A8	Piggyback Base	5	1	I77504768 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Tue Nov 04 07:42:55 Page: 1
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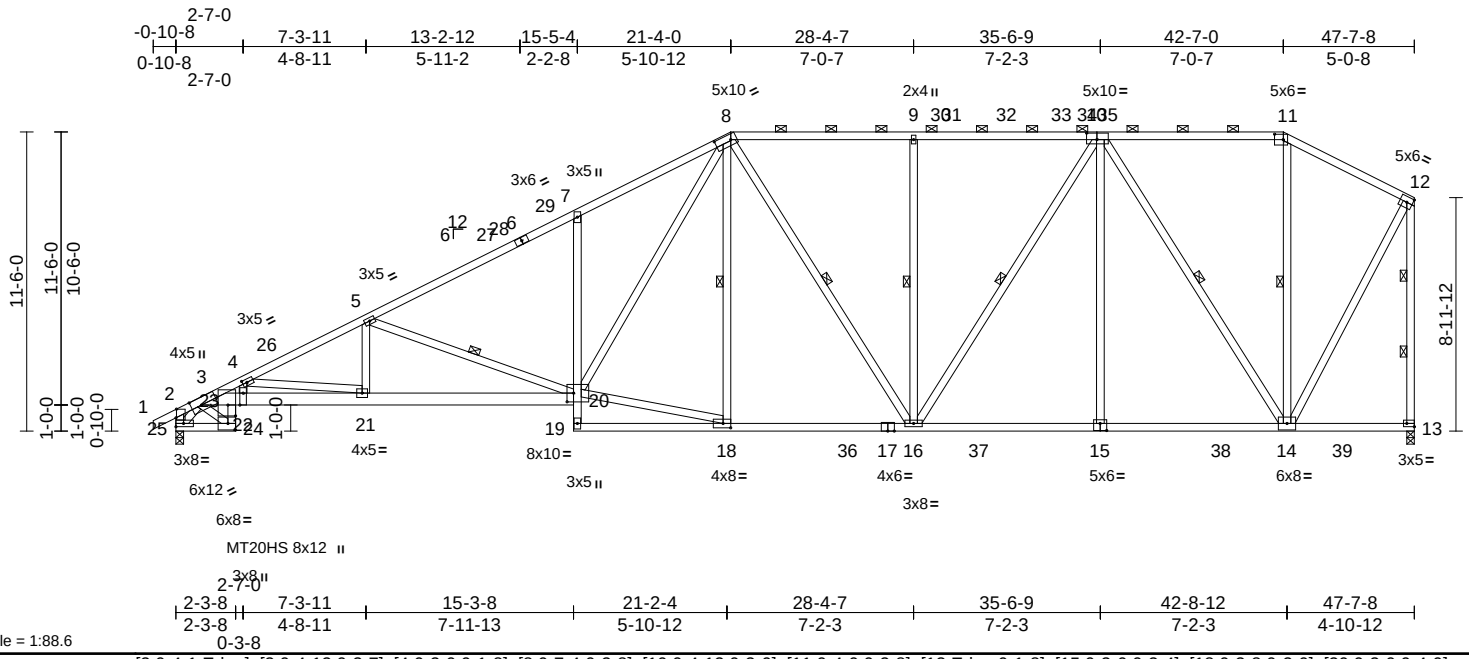

Scale = 1:88.6

Plate Offsets (X, Y): [2:0-4-1,Edge], [3:0-4-12,0-3-7], [4:0-2-0,0-1-8], [8:0-7-4,0-2-8], [10:0-4-12,0-3-0], [11:0-4-0,0-2-8], [13:Edge,0-1-8], [15:0-3-0,0-3-4], [18:0-3-8,0-2-0], [20:0-3-0,0-4-0], [22:0-5-8,Edge], [23:0-5-0,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.26	20-21	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.76	Vert(CT)	-0.48	20-21	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(CT)	0.26	13	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 377 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E *Except* 25-24,7-19:2x4 SP No.2, 24-23:2x4 SP No.1, 3-20:2x6 SP 2400F 2.0E
WEBS 2x4 SP No.2 *Except* 16-8,16-10,14-10:2x4 SP 2400F 2.0E, 13-12:2x4 SP No.1

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-13 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-8 max.): 8-11.
BOT CHORD Rigid ceiling directly applied or 9-10-6 oc bracing.
WEBS 1 Row at midpt 5-20, 8-18, 8-16, 9-16, 10-16, 10-14, 11-14
WEBS 2 Rows at 1/3 pts 12-13

REACTIONS (lb/size) 13=1893/0-3-8, 25=1955/0-3-8
Max Horiz 25=363 (LC 13)
Max Uplift 13=-150 (LC 15), 25=-257 (LC 14)
Max Grav 13=2235 (LC 46), 25=2167 (LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-607/82, 3-4=-7132/902, 4-26=-5384/591, 5-26=-5300/609, 5-27=-3961/446, 27-28=-3848/452, 6-28=-3838/454, 6-29=-3799/464, 7-29=-3781/468, 7-8=-3969/609, 8-9=-2564/391, 9-30=-2564/391, 30-31=-2564/391, 31-32=-2564/391, 32-33=-2564/391, 33-34=-2564/391, 10-34=-2564/391, 10-35=-964/276, 11-35=-964/276, 11-12=-1074/282, 2-25=-745/126, 12-13=-2407/253
BOT CHORD 24-25=-361/2197, 23-24=-488/3028, 3-23=-949/6555, 22-23=-957/6619, 21-22=-957/6619, 20-21=-641/5040, 7-20=-638/255, 18-36=-305/2672, 17-36=-305/2672, 16-17=-305/2672, 16-37=-270/2210, 15-37=-270/2210, 15-38=-269/2214, 14-38=-269/2214
WEBS 4-21=-1598/320, 5-21=0/724, 5-20=-1484/283, 18-20=-271/2524, 8-20=-359/2034, 8-16=-326/194, 9-16=-589/185, 10-16=-189/961, 10-15=0/426, 10-14=-2037/213, 12-14=-181/2127, 3-24=-3530/587, 3-25=-2272/209, 4-22=-146/917

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 3-10-10, Interior (1) 3-10-10 to 14-7-3, Exterior(2R) 14-7-3 to 28-4-7, Interior (1) 28-4-7 to 35-10-3, Exterior(2R) 35-10-3 to 42-7-0, Exterior(2E) 42-7-0 to 47-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) The Fabrication Tolerance at joint 3 = 8%
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
11) One RT4 MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 25. This connection is for uplift only and does not consider lateral forces.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	A9	Piggyback Base Supported Gable	1	1	I77504769 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:14 Page: 1
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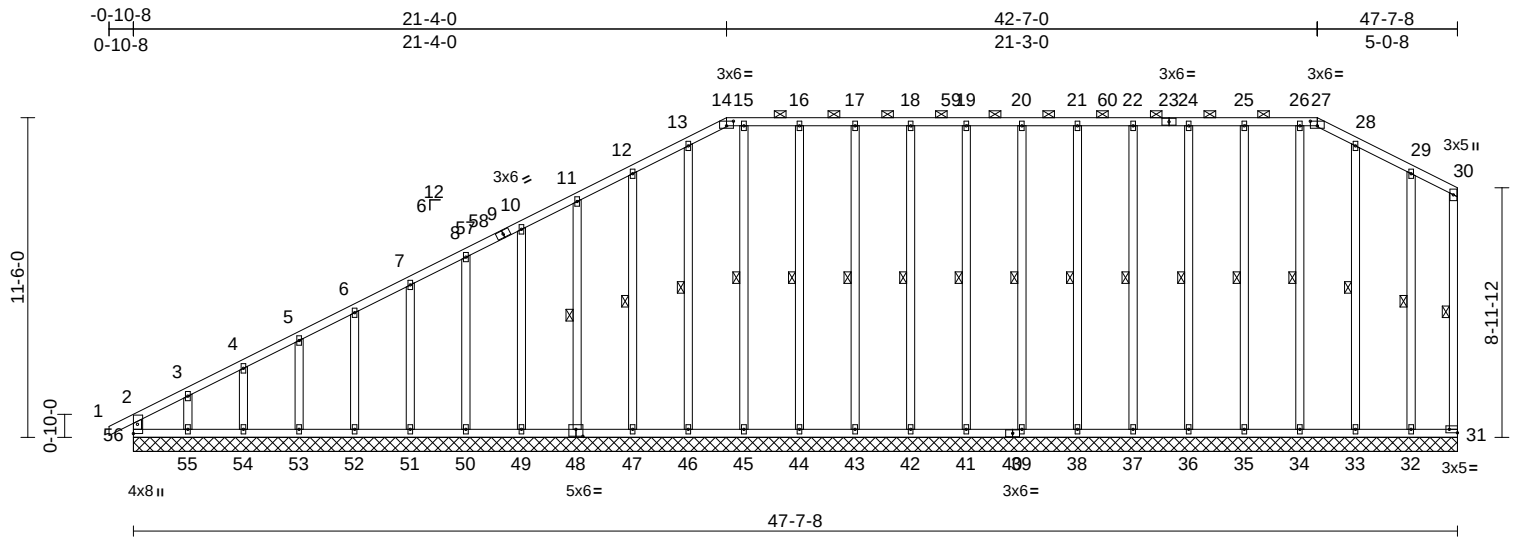


Scale = 1:82.9

Plate Offsets (X, Y): [14:0-3-0,0-2-0], [27:0-3-0,0-2-0], [31:Edge,0-1-8], [48:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.29	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(CT)	0.01	31	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 455 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 30-31:2x4 SP No.2
OTHERS 2x4 SP No.2 *Except* 47-12,48-11,49-10,50-8,51-7,52-6,53-5,54-4, 55-3,32-29:2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 14-27.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 30-31, 20-39, 19-41, 18-42, 17-43, 16-44, 15-45, 13-46, 12-47, 11-48, 21-38, 22-37, 24-36, 25-35, 26-34, 28-33, 29-32

REACTIONS (size) 31=47-7-8, 32=47-7-8, 33=47-7-8, 34=47-7-8, 35=47-7-8, 36=47-7-8, 37=47-7-8, 38=47-7-8, 39=47-7-8, 41=47-7-8, 42=47-7-8, 43=47-7-8, 44=47-7-8, 45=47-7-8, 46=47-7-8, 47=47-7-8, 48=47-7-8, 49=47-7-8, 50=47-7-8, 51=47-7-8, 52=47-7-8, 53=47-7-8, 54=47-7-8, 55=47-7-8, 56=47-7-8
Max Horiz 56=363 (LC 11)

Max Uplift 31=-35 (LC 10), 32=-31 (LC 15), 33=-44 (LC 15), 34=-28 (LC 11), 35=-33 (LC 10), 36=-25 (LC 11), 37=-25 (LC 11), 38=-25 (LC 10), 39=-25 (LC 11), 41=-25 (LC 10), 42=-25 (LC 11), 43=-25 (LC 11), 44=-35 (LC 10), 45=-38 (LC 11), 46=-20 (LC 14), 47=-50 (LC 14), 48=-43 (LC 14), 49=-45 (LC 14), 50=-43 (LC 14), 51=-44 (LC 14), 52=-42 (LC 14), 53=-51 (LC 14), 54=-14 (LC 14), 55=-172 (LC 14), 56=-29 (LC 10)
Max Grav 31=85 (LC 53), 32=227 (LC 45), 33=243 (LC 45), 34=190 (LC 40), 35=221 (LC 40), 36=216 (LC 40), 37=219 (LC 40), 38=188 (LC 40), 39=160 (LC 1), 41=188 (LC 40), 42=219 (LC 40), 43=216 (LC 40), 44=221 (LC 40), 45=191 (LC 40), 46=217 (LC 43), 47=221 (LC 43), 48=220 (LC 43), 49=221 (LC 43), 50=180 (LC 43), 51=160 (LC 58), 52=161 (LC 43), 53=159 (LC 58), 54=164 (LC 1), 55=155 (LC 51), 56=256 (LC 31)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-56=-207/144, 1-2=0/27, 2-3=-332/311, 3-4=-273/262, 4-5=-254/254, 5-6=-228/236, 6-7=-214/221, 7-8=-201/205, 8-10=-188/189, 10-11=-174/225, 11-12=-161/270, 12-13=-150/320, 13-14=-128/338, 14-15=-110/326, 15-16=-110/326, 16-17=-110/326, 17-18=-110/326, 18-19=-110/326, 19-20=-110/326, 20-21=-110/326, 21-22=-110/326, 22-24=-110/326, 24-25=-110/326, 25-26=-110/326, 26-27=-110/326, 27-28=-124/335, 28-29=-130/291, 29-30=-159/278, 30-31=-134/238

November 4,2025

Continued on page 2

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

BOT CHORD 55-56=-113/185, 54-55=-113/185, 53-54=-113/185, 52-53=-113/185, 51-52=-113/185, 50-51=-113/185, 49-50=-113/185, 47-49=-113/185, 46-47=-113/185, 45-46=-113/185, 44-45=-113/185, 43-44=-113/185, 42-43=-113/185, 41-42=-113/185, 39-41=-113/185, 38-39=-113/185, 37-38=-113/185, 36-37=-113/185, 35-36=-113/185, 34-35=-113/185, 33-34=-113/185, 32-33=-113/185, 31-32=-113/185

WEBS 20-39=-121/57, 19-41=-148/57, 18-42=-179/57, 17-43=-176/56, 16-44=-181/74, 15-45=-178/62, 13-46=-180/44, 12-47=-181/92, 11-48=-180/75, 10-49=-181/78, 8-50=-140/77, 7-51=-127/77, 6-52=-126/77, 5-53=-129/76, 4-54=-123/93, 3-55=-173/151, 21-38=-148/57, 22-37=-179/57, 24-36=-176/56, 25-35=-181/71, 26-34=-178/55, 28-33=-202/77, 29-32=-223/102

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 3-11-8, Exterior(2N) 3-11-8 to 16-6-14, Corner(3R) 16-6-14 to 25-11-8, Exterior(2N) 25-11-8 to 37-9-14, Corner(3R) 37-9-14 to 42-7-0, Corner(3E) 42-7-0 to 47-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Provide adequate drainage to prevent water ponding.
8) All plates are 2x4 MT20 unless otherwise indicated.
9) Gable requires continuous bottom chord bearing.
10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
11) Gable studs spaced at 2-0-0 oc.
12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
14) N/A
15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

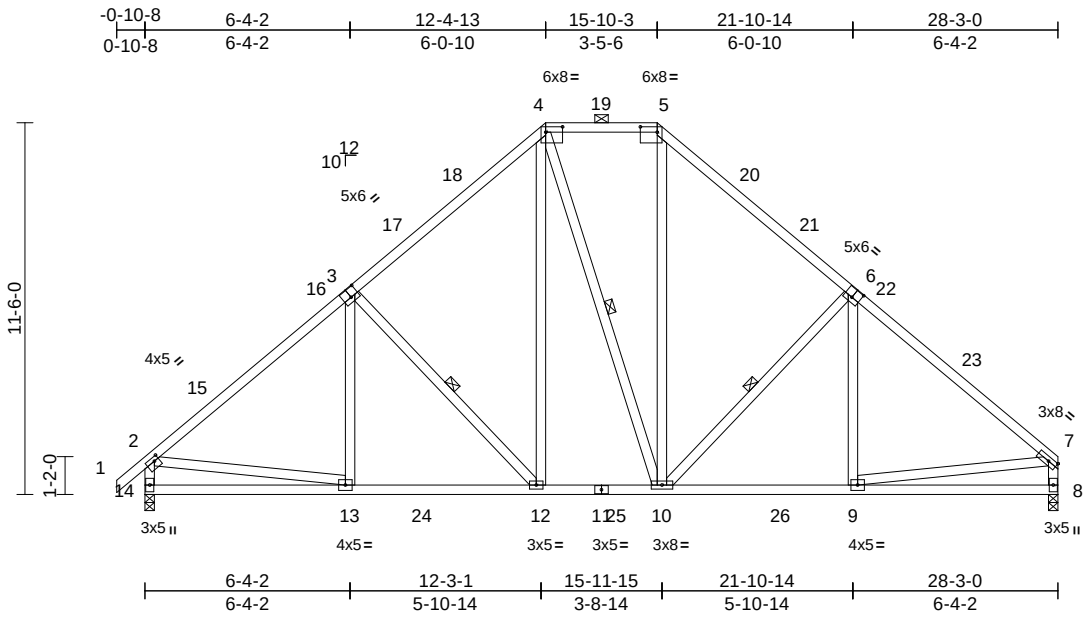
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	B1	Piggyback Base	1	1	I77504770 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:15 ID:aFloEPoPLjeLiUtS0JRAUZzEwGe-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:71.3

Plate Offsets (X, Y): [2:0-1-12,0-1-8], [3:0-3-0,0-3-4], [4:0-6-4,0-2-0], [5:0-6-4,0-2-0], [6:0-3-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.79	Vert(LL)	-0.08	12-13	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.13	12-13	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(CT)	0.03	8	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 208 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 12-4,10-4,10-5:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (5-6-12 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-12, 4-10, 6-10

REACTIONS (size) 8=0-3-8, 14=0-3-8
Max Horiz 14=287 (LC 11)
Max Uplift 8=-87 (LC 15), 14=-107 (LC 14)
Max Grav 8=1383 (LC 53), 14=1444 (LC 51)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-4=-1810/223, 4-5=-1040/232, 5-7=-1803/225, 2-14=-1478/142, 7-8=-1431/119
BOT CHORD 13-14=-295/412, 12-13=-138/1343, 10-12=-35/966, 9-10=-7/1321, 8-9=-59/188
WEBS 3-13=-23/205, 3-12=-526/215, 4-12=-98/563, 4-10=-169/174, 5-10=-86/554, 6-10=-532/217, 6-9=-29/201, 2-13=0/1093, 7-9=0/1157

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 8-1-14, Exterior(2R) 8-1-14 to 20-1-2, Interior (1) 20-1-2 to 25-1-4, Exterior(2E) 25-1-4 to 28-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 8. This connection is for uplift only and does not consider lateral forces.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

November 4,2025

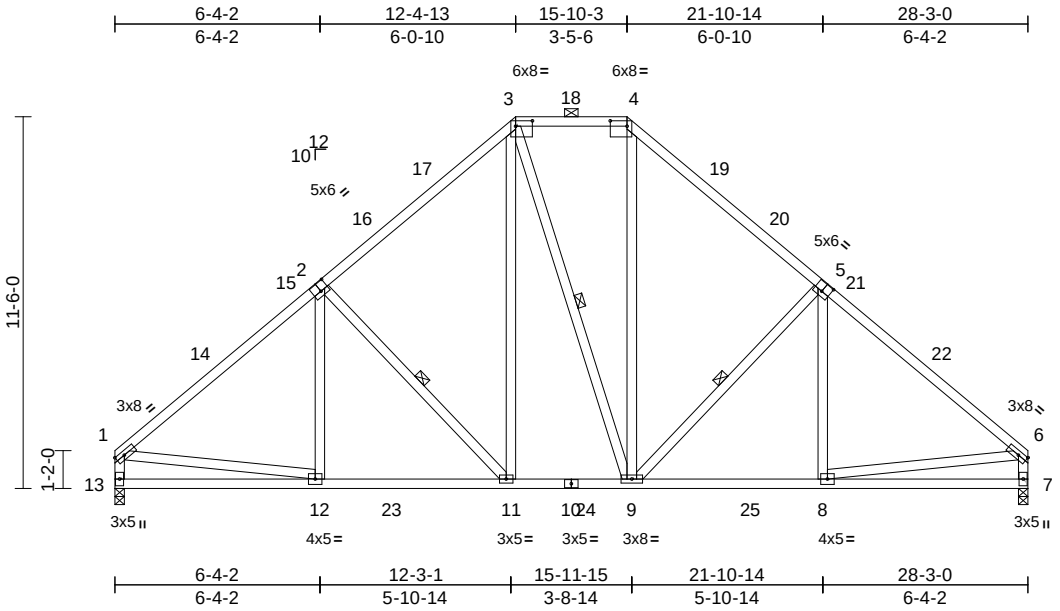
ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	B2	Piggyback Base	9	1	I77504771 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:15 ID:il5Z5UMuG9ZgrjzUGhnBuTzEwFv-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:71.3

Plate Offsets (X, Y): [2:0-3-0,0-3-4], [3:0-6-4,0-2-0], [4:0-6-4,0-2-0], [5:0-3-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.08	11-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.13	11-12	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(CT)	0.03	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 207 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 11-3,9-3,9-4:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-10 oc purlins, except end verticals, and 2-0-0 oc purlins (5-6-12 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-11, 3-9, 5-9

REACTIONS (size) 7=0-3-8, 13=0-3-8
Max Horiz 13=-273 (LC 10)
Max Uplift 7=-87 (LC 15), 13=-87 (LC 14)
Max Grav 7=1384 (LC 52), 13=1386 (LC 50)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-1810/226, 3-4=-1041/233, 4-6=-1804/226, 1-13=-1433/120, 6-7=-1432/119
BOT CHORD 12-13=-270/363, 11-12=-140/1349, 9-11=-35/967, 8-9=-6/1322, 7-8=-59/188
WEBS 2-12=-28/202, 2-11=-534/217, 3-11=-100/568, 3-9=-169/174, 4-9=-85/553, 5-9=-532/217, 5-8=-30/201, 1-12=-9/1142, 6-8=0/1158

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12 to 8-1-14, Exterior(2R) 8-1-14 to 20-1-2, Interior (1) 20-1-2 to 25-1-4, Exterior(2E) 25-1-4 to 28-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 7. This connection is for uplift only and does not consider lateral forces.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	B3	Piggyback Base Supported Gable	1	1	I77504772 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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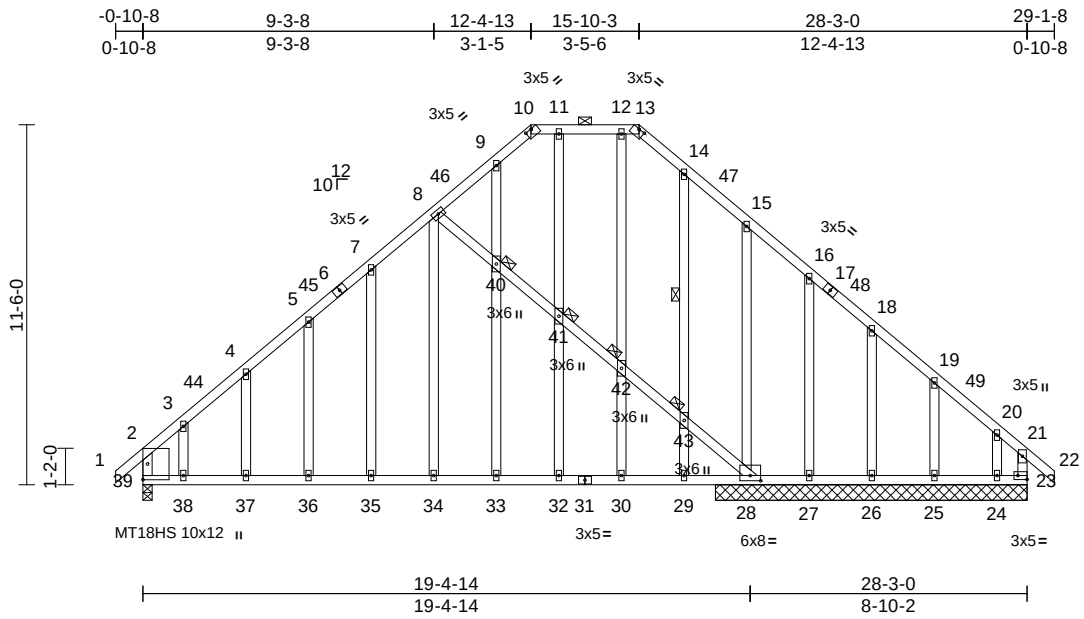

Scale = 1:73.6

Plate Offsets (X, Y): [10:0-2-8,0-0-3], [13:0-2-8,0-0-3], [23:Edge,0-1-8], [28:0-4-0,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	0.26	36-37	>868	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.23	36-37	>999	180	MT18HS	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.02	23	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 255 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 10-13.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 14-43
JOINTS 1 Brace at Jt(s): 40, 41, 42, 43

REACTIONS (size) 23=9-11-8, 24=9-11-8, 25=9-11-8, 26=9-11-8, 27=9-11-8, 28=9-11-8, 39=0-3-8
Max Horiz 39=-293 (LC 12)
Max Uplift 23=-90 (LC 13), 24=-506 (LC 15), 25=-53 (LC 15), 26=-72 (LC 15), 27=-117 (LC 15), 28=-123 (LC 14), 39=-17 (LC 14)
Max Grav 23=781 (LC 56), 24=186 (LC 13), 25=183 (LC 22), 26=208 (LC 53), 27=104 (LC 53), 28=941 (LC 41), 39=1005 (LC 41)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-39=-851/25, 1-2=0/39, 2-3=-1003/0, 3-4=-942/0, 4-5=-892/21, 5-7=-859/79, 7-8=-817/129, 8-9=-454/68, 9-10=-353/74, 10-11=-289/71, 11-12=-289/71, 12-13=-289/71, 13-14=-397/66, 14-15=-463/39, 15-16=-381/29, 16-18=-420/20, 18-19=-415/33, 19-20=-441/62, 20-21=-612/99, 21-22=0/39, 21-23=-533/76
BOT CHORD 38-39=-116/693, 37-38=-116/693, 36-37=-116/693, 35-36=-116/693, 34-35=-116/693, 33-34=-116/693, 32-33=-116/693, 30-32=-116/693, 29-30=-116/693, 28-29=-116/693, 27-28=-88/377, 26-27=-88/377, 25-26=-88/377, 24-25=-88/377, 23-24=-88/377
WEBS 3-38=-16/65, 4-37=-73/67, 5-36=-85/71, 7-35=-72/56, 8-34=-133/374, 9-40=-15/74, 11-41=-38/50, 12-42=0/61, 14-43=-110/71, 15-28=-295/89, 16-27=-166/119, 18-26=-164/98, 19-25=-158/96, 20-24=-101/284, 8-40=-713/297, 40-41=-671/270, 41-42=-690/279, 42-43=-695/274, 28-43=-743/303, 33-40=-70/72, 32-41=-1/62, 30-42=0/69, 29-43=-53/42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 9-3-8, Corner(3R) 9-3-8 to 18-10-3, Exterior(2N) 18-10-3 to 26-1-8, Corner(3E) 26-1-8 to 29-1-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Provide adequate drainage to prevent water ponding.
8) All plates are MT20 plates unless otherwise indicated.
9) All plates are 2x4 MT20 unless otherwise indicated.
10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
11) Gable studs spaced at 2-0-0 oc.
12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

November 4,2025

Continued on page 2

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Edenton, NC 27932

14) N/A

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	C1	Common Supported Gable	1	1	Job Reference (optional)	I77504773

Carter Components (Sanford, NC), Sanford, NC - 27332,

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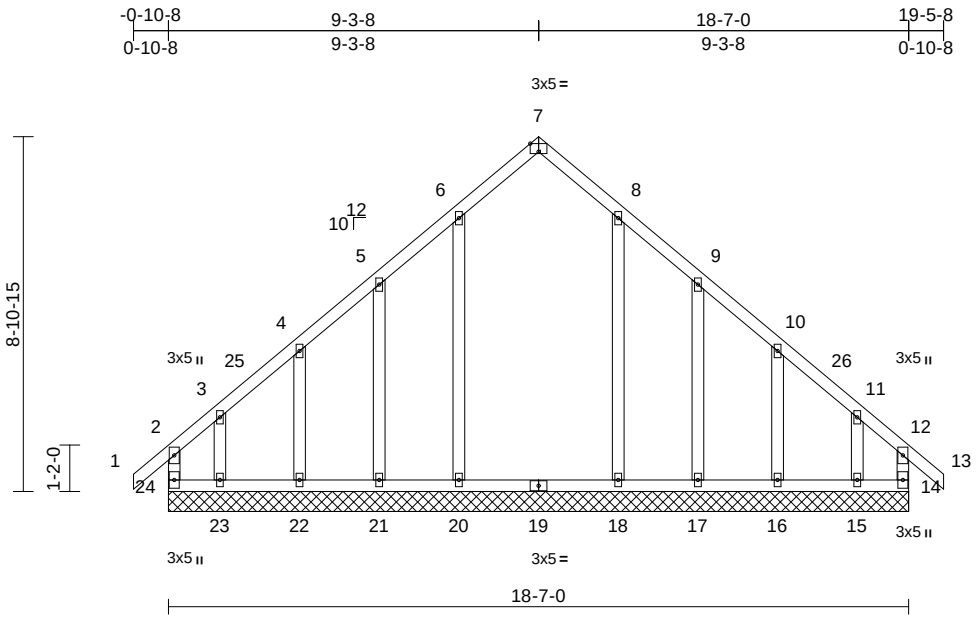

Scale = 1:57.8

Plate Offsets (X, Y): [7:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(CT)	0.01	14	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 120 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)
14=18-7-0, 15=18-7-0, 16=18-7-0, 17=18-7-0, 18=18-7-0, 20=18-7-0, 21=18-7-0, 22=18-7-0, 23=18-7-0, 24=18-7-0
Max Horiz 24=232 (LC 13)
Max Uplift 14=-89 (LC 13), 15=-272 (LC 15), 16=-45 (LC 15), 17=-118 (LC 15), 20=-1 (LC 14), 21=-116 (LC 14), 22=-45 (LC 14), 23=-275 (LC 14), 24=-101 (LC 12)
Max Grav 14=317 (LC 28), 15=193 (LC 13), 16=218 (LC 26), 17=176 (LC 22), 18=370 (LC 6), 20=370 (LC 5), 21=176 (LC 21), 22=218 (LC 25), 23=199 (LC 12), 24=321 (LC 27)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	2-24=-234/77, 1-2=0/39, 2-3=-295/136, 3-4=-167/89, 4-5=-135/61, 5-6=-127/75, 6-7=-158/104, 7-8=-158/104, 8-9=-127/75, 9-10=-130/53, 10-11=-163/81, 11-12=-291/126, 12-13=0/39, 12-14=-231/70
BOT CHORD	23-24=-108/281, 22-23=-108/281, 21-22=-108/281, 20-21=-108/281, 18-20=-108/281, 17-18=-108/281, 16-17=-108/281, 15-16=-108/281, 14-15=-108/281
WEBS	6-20=-260/44, 5-21=-151/155, 4-22=-154/99, 3-23=-138/195, 8-18=-260/40, 9-17=-151/155, 10-16=-156/97, 11-15=-121/212

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 6-3-8, Corner(3R) 6-3-8 to 12-3-8, Exterior(2N) 12-3-8 to 16-5-8, Corner(3E) 16-5-8 to 19-5-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable requires continuous bottom chord bearing.
9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 24, 89 lb uplift at joint 14, 1 lb uplift at joint 20, 116 lb uplift at joint 21, 45 lb uplift at joint 22, 275 lb uplift at joint 23, 118 lb uplift at joint 17, 45 lb uplift at joint 16 and 272 lb uplift at joint 15.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	D1	Common	1	1	Job Reference (optional)	I77504774

Carter Components (Sanford, NC), Sanford, NC - 27332,

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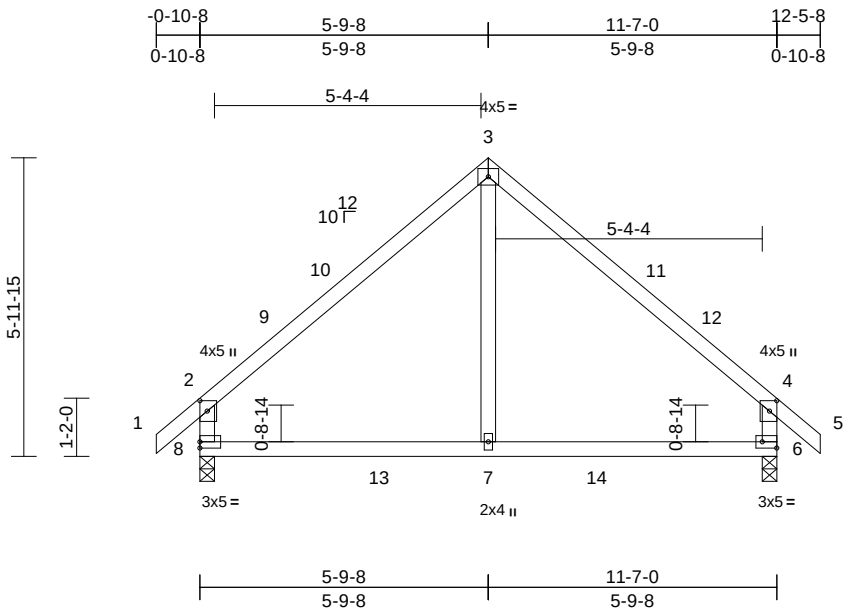

Scale = 1:46.3

Plate Offsets (X, Y): [2:0-2-8,0-1-12], [4:0-2-8,0-1-12], [6:Edge,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.64	Vert(LL)	-0.04	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.07	7-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 54 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2 *Except* 7-3:2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 6=0-3-8, 8=0-3-8
Max Horiz 8=163 (LC 13)
Max Uplift 6=-49 (LC 15), 8=-49 (LC 14)
Max Grav 6=632 (LC 6), 8=632 (LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-572/141, 3-4=-573/140, 4-5=0/39, 2-8=-551/199, 4-6=-553/196
BOT CHORD 7-8=-2/347, 6-7=-2/347
WEBS 3-7=0/302

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-9-8, Exterior(2R) 2-9-8 to 8-9-8, Interior (1) 8-9-8 to 9-5-8, Exterior(2E) 9-5-8 to 12-5-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 6. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	D2	Common	1	1	Job Reference (optional)	I77504775

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:16
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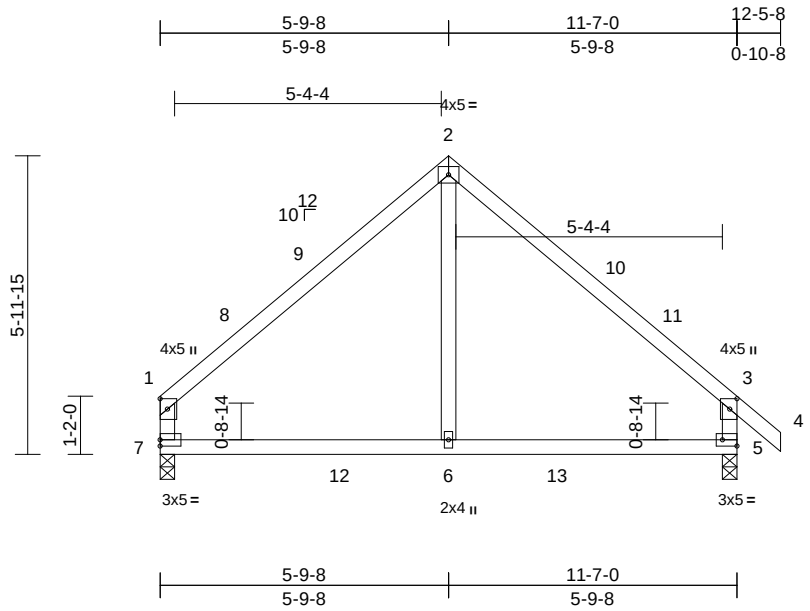
Page: 1



Scale = 1:46.3

Plate Offsets (X, Y): [3:0-2-8,0-1-12], [5:Edge,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.05	5-6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.42	Vert(CT)	-0.08	5-6	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 52 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2 *Except* 6-2:2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 5=0-3-8, 7=0-3-8
Max Horiz 7=-156 (LC 10)
Max Uplift 5=-48 (LC 15), 7=-29 (LC 14)
Max Grav 5=633 (LC 6), 7=579 (LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-565/136, 2-3=-573/136, 3-4=0/39, 1-7=-502/146, 3-5=-552/194
BOT CHORD 6-7=-2/348, 5-6=-2/348
WEBS 2-6=0/295

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Exterior(2R) 3-1-12 to 8-9-8, Interior (1) 8-9-8 to 9-5-8, Exterior(2E) 9-5-8 to 12-5-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	D3	Common Girder	1	3	I77504776 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:16 ID:5keisCuUTTGnIvooWYPHQwzEw4v-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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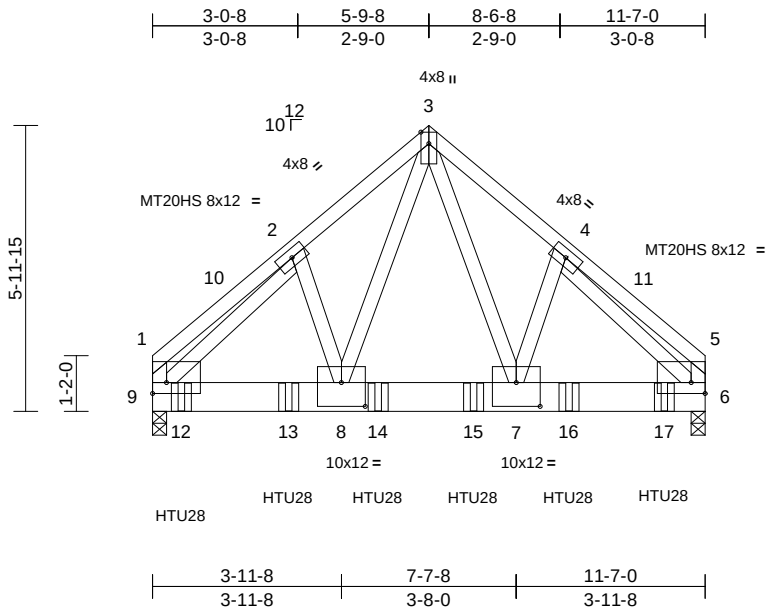

Scale = 1:48.3

Plate Offsets (X, Y): [1:Edge,0-2-12], [5:Edge,0-2-12], [7:0-6-0,0-6-0], [8:0-6-0,0-6-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.03	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.06	7-8	>999	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.72	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 287 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 9-1,6-5:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 6=0-3-8, 9=0-3-8
Max Horiz 9=138 (LC 11)
Max Uplift 6=-451 (LC 13), 9=-459 (LC 12)
Max Grav 6=9527 (LC 6), 9=9733 (LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1993/144, 2-3=-7864/461, 3-4=-7921/464, 4-5=-2075/148, 1-9=-1422/117, 5-6=-1476/120
BOT CHORD 8-9=-302/5686, 7-8=-205/4435, 6-7=-256/5730
WEBS 3-7=-313/5220, 4-7=-95/1199, 3-8=-306/5081, 2-8=-96/1201, 2-9=-6438/297, 4-6=-6411/296

NOTES

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 4 rows staggered at 0-4-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
6) Unbalanced snow loads have been considered for this design.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 6. This connection is for uplift only and does not consider lateral forces.
11) Use Simpson Strong-Tie HTU28 (20-16d Girder, 26-10dx1 1/2 Truss) or equivalent spaced at 2-3-0 oc max. starting at 0-7-4 from the left end to 10-8-12 to connect truss(es) to back face of bottom chord.
12) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-60, 3-5=-60, 6-9=-20
Concentrated Loads (lb)
Vert: 12=-2708 (B), 13=-2702 (B), 14=-2702 (B), 15=-2702 (B), 16=-2702 (B), 17=-2705 (B)

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	D4	Common Supported Gable	1	1	Job Reference (optional)	I77504777

Carter Components (Sanford, NC), Sanford, NC - 27332,

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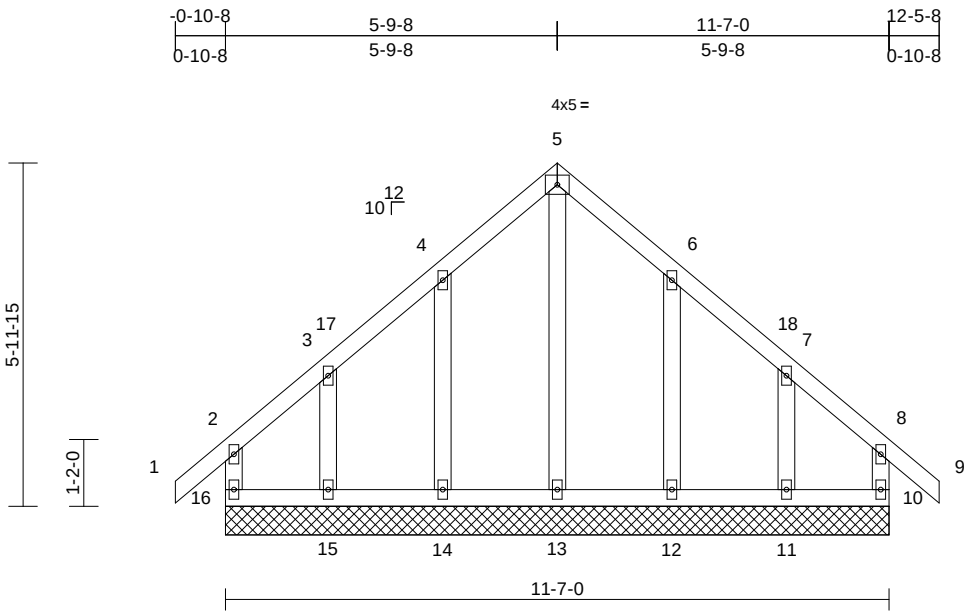

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(CT)	0.00	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 71 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 10=11-7-0, 11=11-7-0, 12=11-7-0, 13=11-7-0, 14=11-7-0, 15=11-7-0, 16=11-7-0
Max Horiz 16=163 (LC 13)
Max Uplift 10=-61 (LC 11), 11=-110 (LC 15), 12=-71 (LC 15), 14=-70 (LC 14), 15=-112 (LC 14), 16=-74 (LC 10)
Max Grav 10=153 (LC 25), 11=191 (LC 26), 12=279 (LC 22), 13=191 (LC 28), 14=279 (LC 21), 15=197 (LC 25), 16=163 (LC 26)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	2-16=-130/151, 1-2=0/39, 2-3=-98/99, 3-4=-65/181, 4-5=-116/287, 5-6=-116/287, 6-7=-64/183, 7-8=-85/87, 8-9=0/39, 8-10=-122/140
BOT CHORD	15-16=-80/102, 14-15=-80/102, 13-14=-80/102, 12-13=-80/102, 11-12=-80/102, 10-11=-80/102
WEBS	5-13=-282/47, 4-14=-238/142, 3-15=-156/145, 6-12=-238/140, 7-11=-146/159

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 2-9-8, Corner(3R) 2-9-8 to 8-9-8, Exterior(2N) 8-9-8 to 9-5-8, Corner(3E) 9-5-8 to 12-5-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable requires continuous bottom chord bearing.
9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 16, 61 lb uplift at joint 10, 70 lb uplift at joint 14, 112 lb uplift at joint 15, 71 lb uplift at joint 12 and 110 lb uplift at joint 11.

LOAD CASE(S) Standard

NORTH CAROLINA PROFESSIONAL SEAL 036322 ENGINEER ERIC A. GILBERT

November 4,2025

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	E1	Monopitch	2	1	Job Reference (optional)	I77504778

Carter Components (Sanford, NC), Sanford, NC - 27332,

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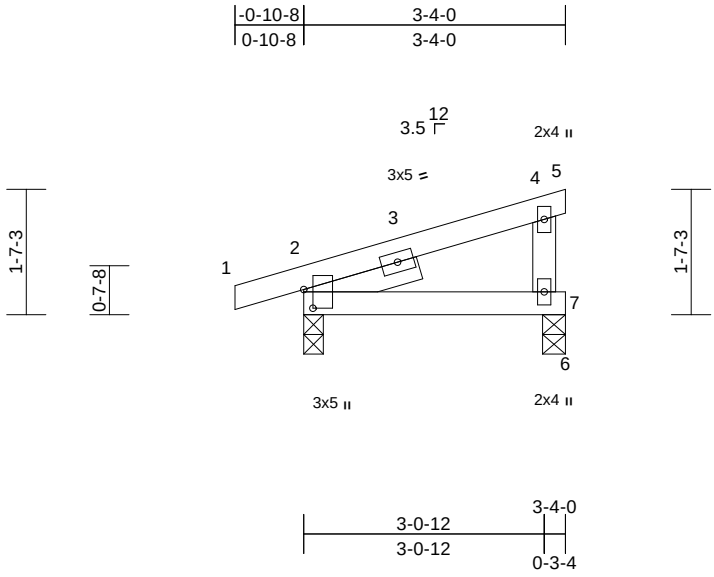

Scale = 1:29.3

Plate Offsets (X, Y): [2:0-2-14,0-1-6]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	0.01	7-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	-0.01	7-10	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 15 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-4-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-0, 7=0-3-8
Max Horiz 2=50 (LC 13)
Max Uplift 2=-51 (LC 10), 7=-25 (LC 14)
Max Grav 2=245 (LC 21), 7=180 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/21, 2-4=-93/36, 4-5=-6/0, 4-7=-127/75
BOT CHORD	2-7=-62/102, 6-7=0/0

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	PB1	Piggyback	2	1	I77504779 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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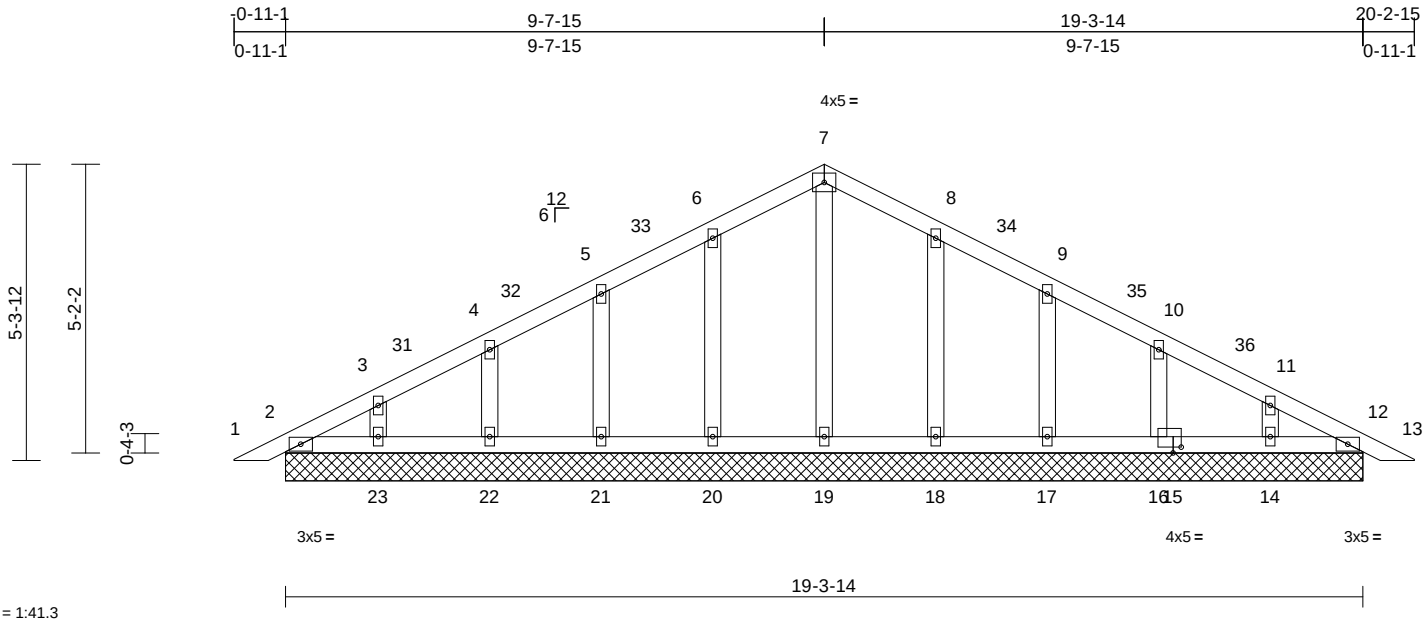

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Plate Offsets (X, Y): [15:0-1-12,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 96 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=19-3-14, 12=19-3-14, 14=19-3-14, 16=19-3-14, 17=19-3-14, 18=19-3-14, 19=19-3-14, 20=19-3-14, 21=19-3-14, 22=19-3-14, 23=19-3-14
Max Horiz 2=81 (LC 18)
Max Uplift 2=-9 (LC 15), 14=-42 (LC 15), 16=-44 (LC 15), 17=-44 (LC 15), 18=-45 (LC 15), 20=-46 (LC 14), 21=-44 (LC 14), 22=-44 (LC 14), 23=-43 (LC 14)
Max Grav 2=103 (LC 21), 12=103 (LC 22), 14=152 (LC 37), 16=177 (LC 22), 17=228 (LC 22), 18=246 (LC 22), 19=146 (LC 28), 20=246 (LC 21), 21=228 (LC 21), 22=177 (LC 21), 23=152 (LC 36)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/17, 2-3=-93/40, 3-4=-70/51, 4-5=-50/70, 5-6=-56/94, 6-7=-68/132, 7-8=-68/132, 8-9=-56/84, 9-10=-47/43, 10-11=-45/21, 11-12=-59/26, 12-13=0/17
BOT CHORD 2-23=-20/77, 22-23=-20/77, 21-22=-20/77, 20-21=-20/77, 19-20=-20/77, 18-19=-20/77, 17-18=-20/77, 16-17=-20/77, 14-16=-20/77, 12-14=-20/77
WEBS 7-19=-106/0, 6-20=-206/83, 5-21=-187/67, 4-22=-137/69, 3-23=-117/61, 8-18=-206/83, 9-17=-187/68, 10-16=-137/69, 11-14=-117/61

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-4-3 to 3-4-3, Interior (1) 3-4-3 to 7-7-8, Exterior(2R) 7-7-8 to 13-7-8, Interior (1) 13-7-8 to 17-10-13, Exterior(2E) 17-10-13 to 20-10-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable requires continuous bottom chord bearing.
9) Gable studs spaced at 2-0-0 oc.
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
12) N/A
13) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	PB2	Piggyback	22	1	Job Reference (optional)	I77504780

Carter Components (Sanford, NC), Sanford, NC - 27332,

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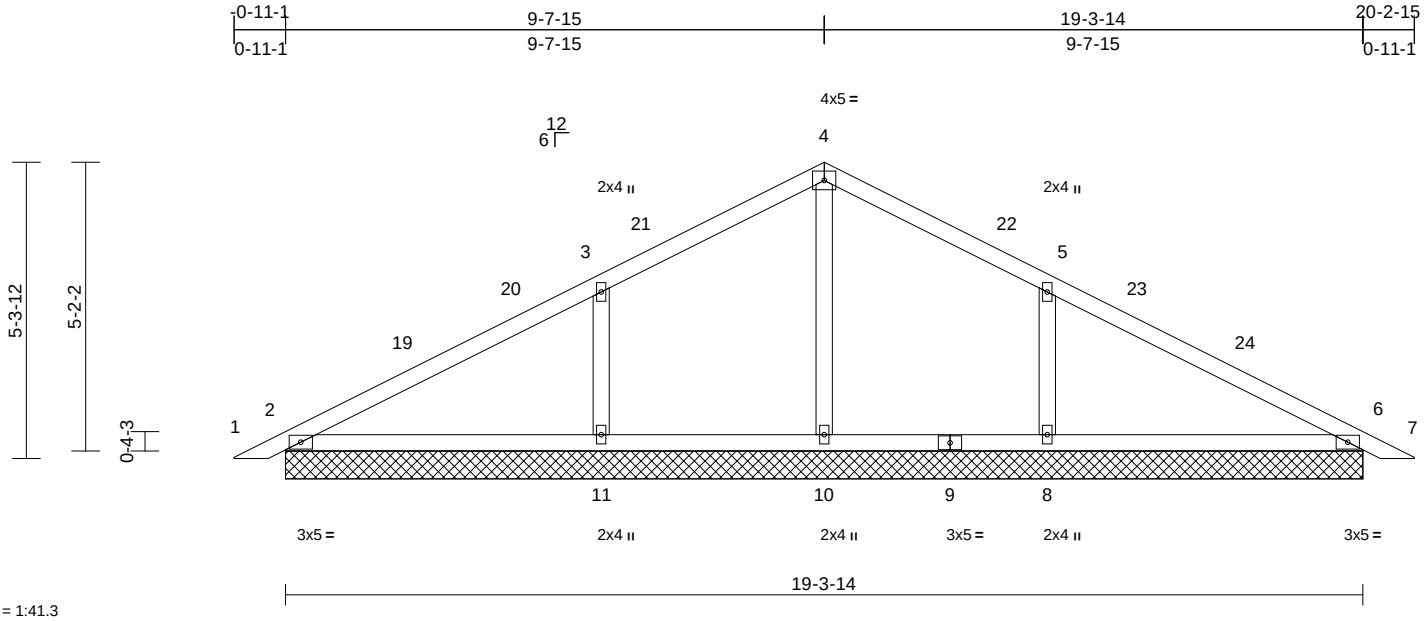

Scale = 1:41.3

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.43	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.28	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 79 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=19-3-14, 6=19-3-14, 8=19-3-14, 10=19-3-14, 11=19-3-14
Max Horiz 2=81 (LC 18)
Max Uplift 2=-17 (LC 14), 6=-32 (LC 15), 8=-135 (LC 15), 11=-136 (LC 14)
Max Grav 2=250 (LC 1), 6=250 (LC 1), 8=610 (LC 22), 10=191 (LC 22), 11=610 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/17, 2-3=-112/69, 3-4=-161/135, 4-5=-161/135, 5-6=-103/47, 6-7=0/17
BOT CHORD 2-11=-21/81, 10-11=0/81, 8-10=0/81, 6-8=0/81
WEBS 4-10=-149/0, 3-11=-465/180, 5-8=-465/180

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-4-3 to 3-4-3, Interior (1) 3-4-3 to 7-7-8, Exterior(2R) 7-7-8 to 13-7-8, Interior (1) 13-7-8 to 17-10-13, Exterior(2E) 17-10-13 to 20-10-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 4-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
11) N/A
12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	PB3	Piggyback	1	1	I77504781 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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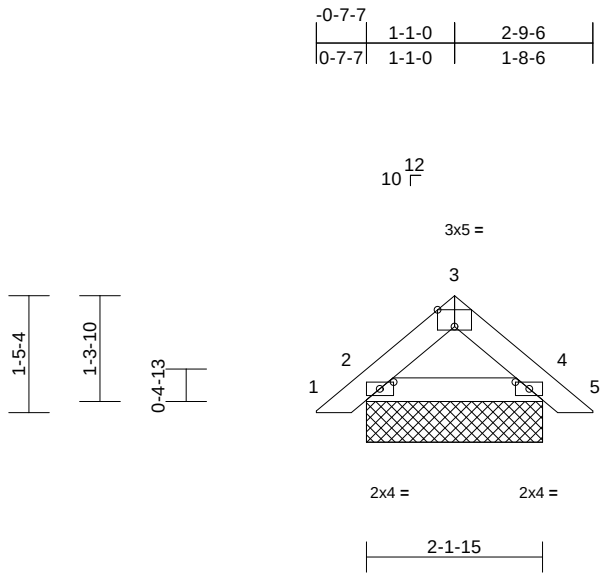

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Plate Offsets (X, Y): [2:0-2-1,0-1-0], [3:0-2-8,Edge], [4:0-2-1,0-1-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 10 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 2=2-1-15, 4=2-1-15
Max Horiz 2=29 (LC 13)
Max Uplift 2=-7 (LC 14), 4=-14 (LC 15)
Max Grav 2=148 (LC 21), 4=137 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=-67/36, 3-4=-67/38, 4-5=0/22
BOT CHORD 2-4=-46/46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
11) N/A
12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	PB4	Piggyback	10	1	Job Reference (optional)	I77504782

Carter Components (Sanford, NC), Sanford, NC - 27332,

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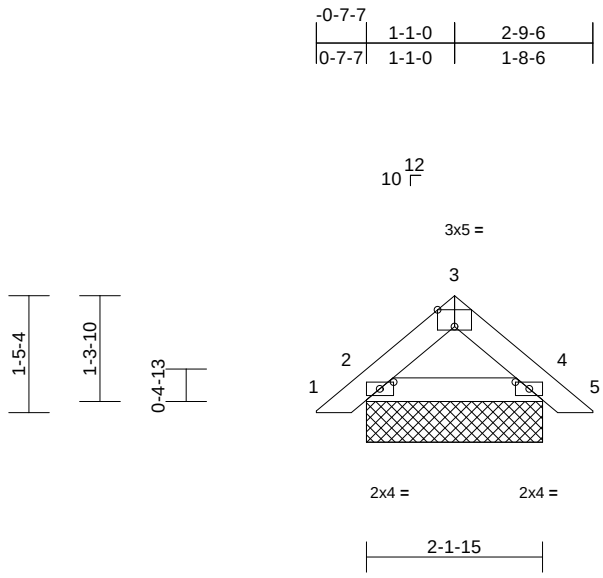

Scale = 1:28.3

Plate Offsets (X, Y): [2:0-2-1,0-1-0], [3:0-2-8,Edge], [4:0-2-1,0-1-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 10 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=2-1-15, 4=2-1-15
Max Horiz 2=29 (LC 13)
Max Uplift 2=-14 (LC 14), 4=-11 (LC 15)
Max Grav 2=136 (LC 21), 4=142 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=-66/38, 3-4=-67/36, 4-5=0/22
BOT CHORD 2-4=-3/46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 4-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
11) N/A
12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V1	Valley	1	1	Job Reference (optional)	I77504783

Carter Components (Sanford, NC), Sanford, NC - 27332,

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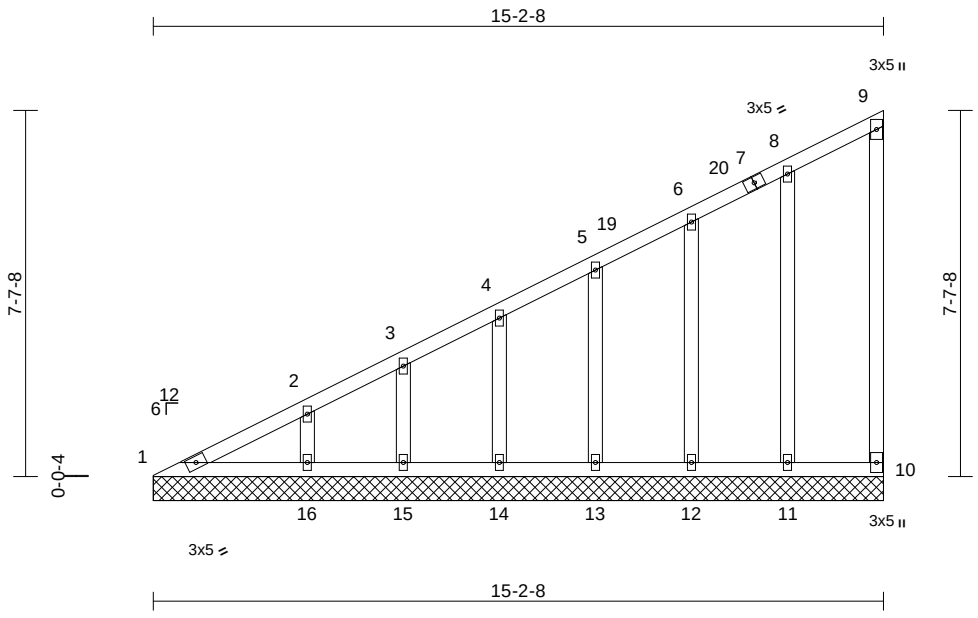

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.20	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horiz(TL)	0.00	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 89 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 15-2-8.

(lb) - Max Horiz 1=274 (LC 11)
Max Uplift All uplift 100 (lb) or less at joint(s) 10, 11, 12, 13, 14, 15, 16
Max Grav All reactions 250 (lb) or less at joint (s) 1, 10, 11, 12, 13, 14, 15 except 16=260 (LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-246/263

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-0-8 to 3-3-0, Exterior(2N) 3-3-0 to 12-1-4, Corner(3E) 12-1-4 to 15-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint (s) 10, 11, 12, 13, 14, 15, 16.

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	V2	Valley	1	1	I77504784
					Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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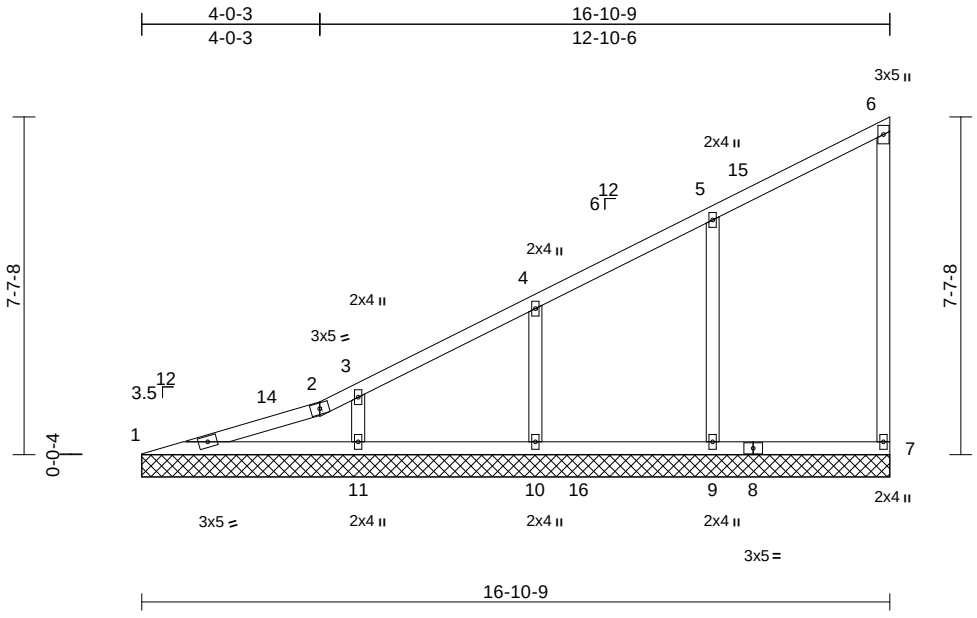


Scale = 1:52

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.33	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horiz(TL)	0.00	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 76 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=16-10-9, 7=16-10-9, 9=16-10-9, 10=16-10-9, 11=16-10-9
Max Horiz 1=276 (LC 11)
Max Uplift 1=-25 (LC 10), 7=-35 (LC 11), 9=-93 (LC 14), 10=-92 (LC 14), 11=-49 (LC 14)
Max Grav 1=216 (LC 20), 7=168 (LC 24), 9=445 (LC 5), 10=334 (LC 24), 11=479 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-471/208, 2-3=-210/224, 3-4=-194/197, 4-5=-161/169, 5-6=-133/117, 6-7=-121/52
BOT CHORD	1-11=-92/442, 10-11=-92/130, 9-10=-92/130, 7-9=-92/130
WEBS	5-9=-292/146, 4-10=-237/144, 3-11=-317/108

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-14 to 3-0-14, Interior (1) 3-0-14 to 13-9-11, Exterior(2E) 13-9-11 to 16-9-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 4-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 35 lb uplift at joint 7, 93 lb uplift at joint 9, 92 lb uplift at joint 10 and 49 lb uplift at joint 11.

LOAD CASE(S) Standard

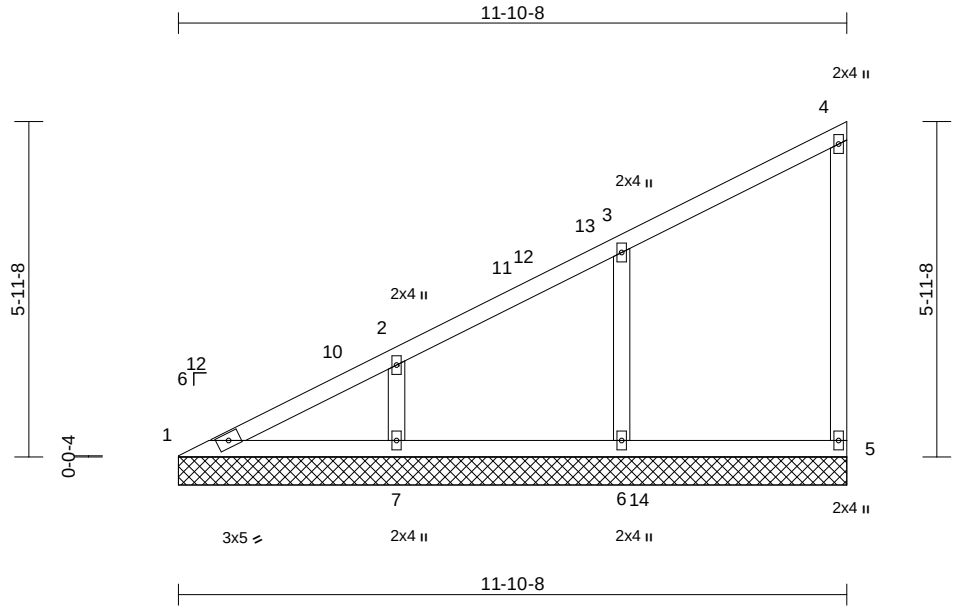
November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	I77504785
25110007-A	V3	Valley	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:17 ID:RmdJFgIpll77zrclq_xeBhzEw?D-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:40.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.16	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 52 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=11-10-8, 5=11-10-8, 6=11-10-8, 7=11-10-8
Max Horiz 1=212 (LC 11)
Max Uplift 5=-28 (LC 11), 6=-39 (LC 14), 7=-83 (LC 14)
Max Grav 1=153 (LC 25), 5=207 (LC 5), 6=486 (LC 5), 7=362 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-267/196, 2-3=-140/158, 3-4=-120/102, 4-5=-153/42
BOT CHORD	1-7=-70/264, 6-7=-70/102, 5-6=-70/102
WEBS	3-6=-388/166, 2-7=-267/126

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-8 to 3-0-8, Interior (1) 3-0-8 to 7-6-5, Exterior(2R) 7-6-5 to 11-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 4-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 5, 39 lb uplift at joint 6 and 83 lb uplift at joint 7.

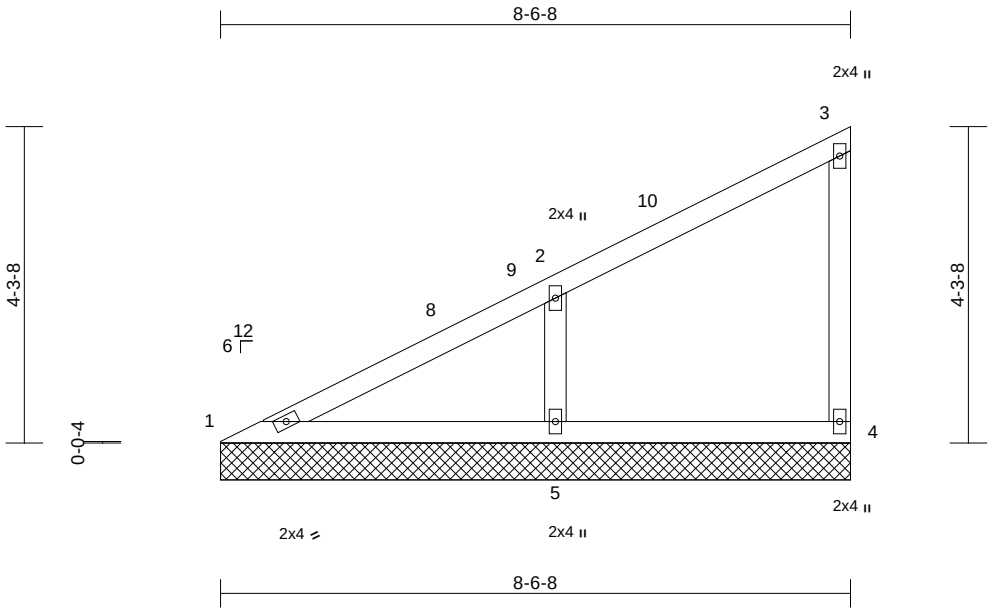
LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V4	Valley	2	1	Job Reference (optional)	I77504786

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:17 ID:RmdJFgIpII77zrclq_xeBhzEw?D-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.19	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 34 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=8-6-8, 4=8-6-8, 5=8-6-8
Max Horiz 1=150 (LC 11)
Max Uplift 4=-21 (LC 11), 5=-94 (LC 14)
Max Grav 1=143 (LC 1), 4=165 (LC 20), 5=555 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-242/156, 2-3=-104/85, 3-4=-142/50
BOT CHORD 1-5=-50/268, 4-5=-50/73
WEBS 2-5=-428/241

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-8 to 3-0-8, Interior (1) 3-0-8 to 4-2-5, Exterior(2R) 4-2-5 to 8-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 4-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 4 and 94 lb uplift at joint 5.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	V5	Valley	2	1	I77504787 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:17 ID:RmdJFgIpll77zrclq_xeBhzEw?D-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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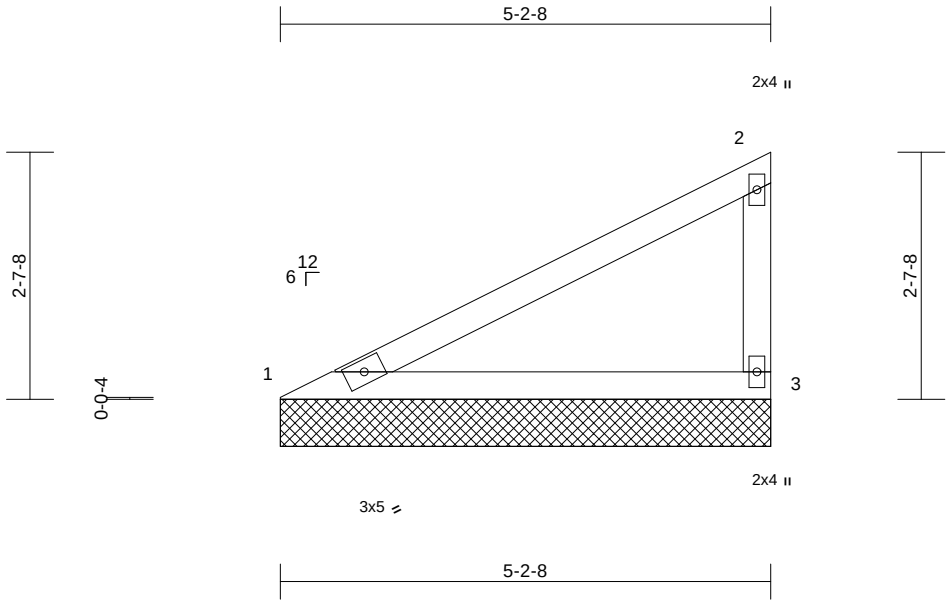

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.52	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.55	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 18 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=5-2-8, 3=5-2-8
Max Horiz 1=87 (LC 11)
Max Uplift 1=-21 (LC 14), 3=-43 (LC 14)
Max Grav 1=292 (LC 20), 3=292 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-504/134, 2-3=-199/104
BOT CHORD 1-3=-175/441

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 4-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 3 and 21 lb uplift at joint 1.

LOAD CASE(S) Standard

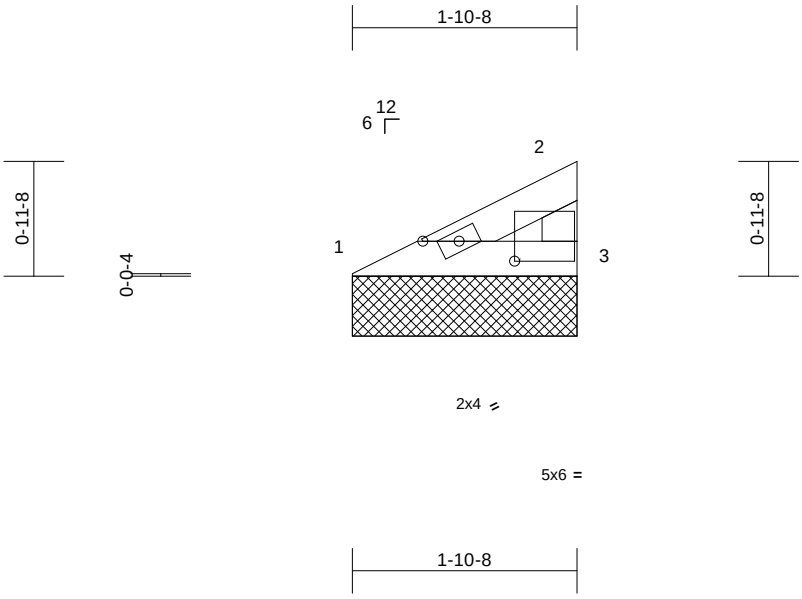
November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	V6	Valley	2	1	I77504788 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:17 ID:RmdJFgIpll77zrclq_xeBhzEw?D-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:19.2

Plate Offsets (X, Y): [3:0-9-3,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.03	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 5 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=1-10-8, 3=1-10-8
Max Horiz 1=25 (LC 11)
Max Uplift 1=-8 (LC 14), 3=-14 (LC 14)
Max Grav 1=87 (LC 20), 3=87 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-98/28, 2-3=-44/20
BOT CHORD 1-3=-36/81

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 4-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 3 and 8 lb uplift at joint 1.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V7	Valley	1	1	Job Reference (optional)	I77504789

Carter Components (Sanford, NC), Sanford, NC - 27332,

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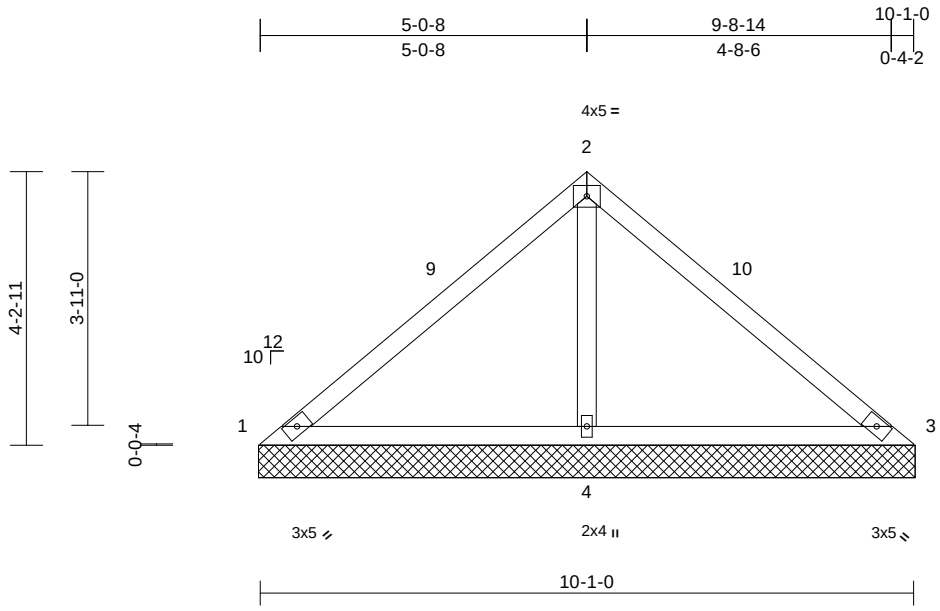

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.46	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.22	Horiz(TL)	0.01	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 38 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=10-1-10, 3=10-1-10, 4=10-1-10
Max Horiz 1=95 (LC 13)
Max Uplift 1=-67 (LC 21), 3=-67 (LC 20), 4=-121 (LC 14)
Max Grav 1=86 (LC 20), 3=86 (LC 21), 4=846 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-131/421, 2-3=-131/421
BOT CHORD 1-4=-278/187, 3-4=-278/187
WEBS 2-4=-705/295

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Exterior(2R) 3-0-0 to 7-1-10, Exterior(2E) 7-1-10 to 10-1-10 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 1, 67 lb uplift at joint 3 and 121 lb uplift at joint 4.
11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V8	Valley	1	1	Job Reference (optional)	I77504790

Carter Components (Sanford, NC), Sanford, NC - 27332,

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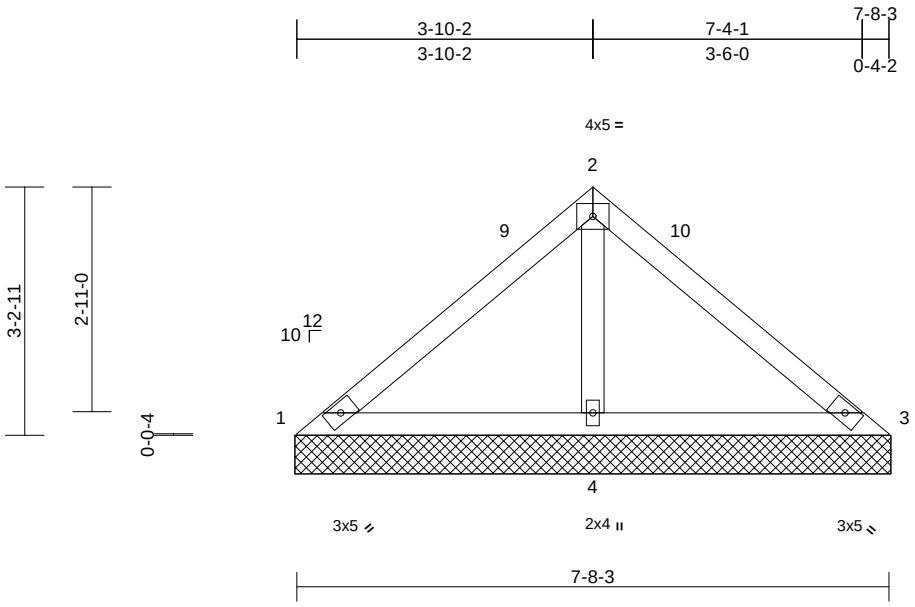

Scale = 1:29.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.30	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 29 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-8-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=7-8-13, 3=7-8-13, 4=7-8-13
Max Horiz 1=-71 (LC 10)
Max Uplift 1=-31 (LC 21), 3=-31 (LC 20), 4=-85 (LC 14)
Max Grav 1=102 (LC 20), 3=102 (LC 21), 4=597 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-106/268, 2-3=-106/268
BOT CHORD 1-4=-209/167, 3-4=-209/167
WEBS 2-4=-480/225

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Exterior(2R) 3-0-0 to 4-8-13, Exterior(2E) 4-8-13 to 7-8-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 31 lb uplift at joint 3 and 85 lb uplift at joint 4.
11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V9	Valley	1	1	Job Reference (optional)	I77504791

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:18
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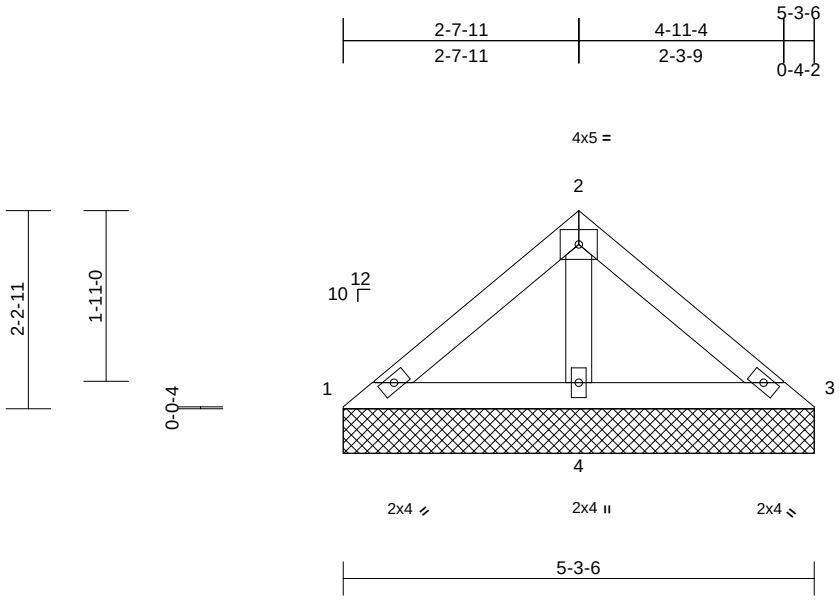

Scale = 1:25.8

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 19 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=5-3-6, 3=5-3-6, 4=5-3-6
Max Horiz 1=-48 (LC 10)
Max Uplift 3=-6 (LC 15), 4=-39 (LC 14)
Max Grav 1=93 (LC 20), 3=93 (LC 21), 4=333 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-84/122, 2-3=-84/122
BOT CHORD 1-4=-99/100, 3-4=-99/100
WEBS 2-4=-245/116

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 3 and 39 lb uplift at joint 4.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	V10	Valley	1	1	I77504792
					Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:18 Page: 1
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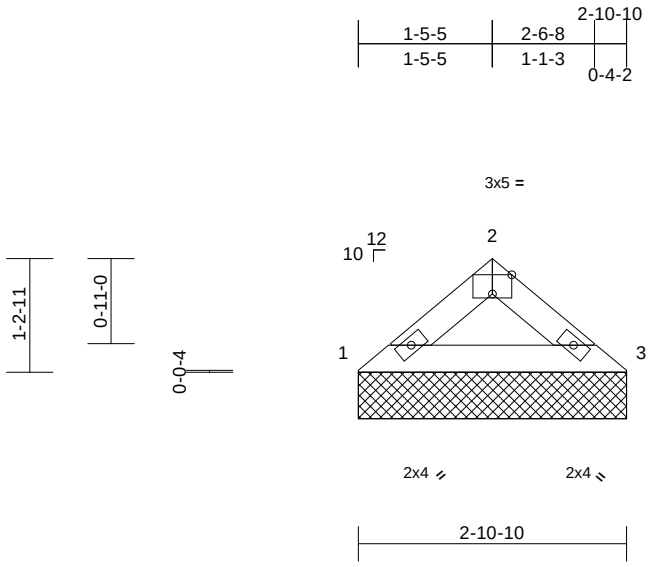

Scale = 1:24.8

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.06	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 9 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=2-10-10, 3=2-10-10
Max Horiz 1=24 (LC 13)
Max Uplift 1=-10 (LC 14), 3=-10 (LC 15)
Max Grav 1=132 (LC 20), 3=132 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-169/70, 2-3=-169/70
BOT CHORD 1-3=-39/121

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1 and 10 lb uplift at joint 3.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V11	Valley	1	1	Job Reference (optional)	I77504793

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:18 Page: 1
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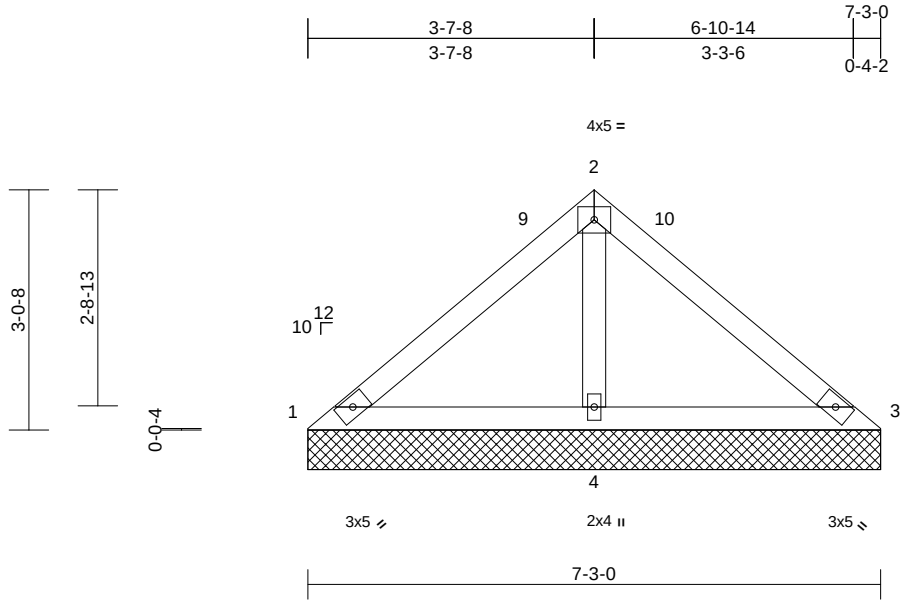


Scale = 1:29.2

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.27	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 27 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=7-3-0, 3=7-3-0, 4=7-3-0
Max Horiz 1=-67 (LC 10)
Max Uplift 1=-17 (LC 21), 3=-17 (LC 20), 4=-74 (LC 14)
Max Grav 1=105 (LC 20), 3=105 (LC 21), 4=534 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-89/230, 2-3=-89/230
BOT CHORD 1-4=-180/152, 3-4=-180/152
WEBS 2-4=-422/200

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-5 to 3-0-5, Exterior(2R) 3-0-5 to 4-3-5, Exterior(2E) 4-3-5 to 7-3-5 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1, 17 lb uplift at joint 3 and 74 lb uplift at joint 4.

LOAD CASE(S) Standard

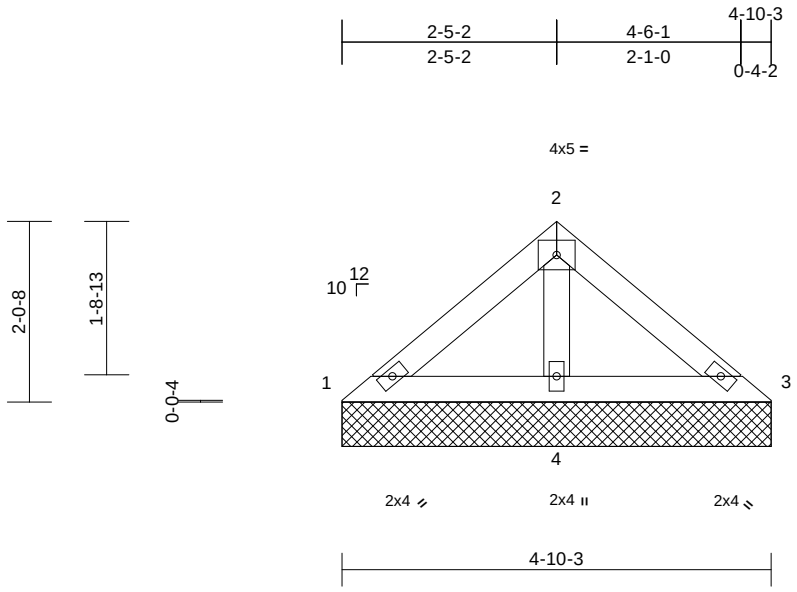
November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B
25110007-A	V12	Valley	1	1	I77504794 Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Mon Nov 03 11:53:18 ID:p31oUbLOMlKiby_S1xy5cszEwVR-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:26

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 17 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=4-10-3, 3=4-10-3, 4=4-10-3
Max Horiz 1=-43 (LC 10)
Max Uplift 3=-7 (LC 15), 4=-33 (LC 14)
Max Grav 1=88 (LC 20), 3=88 (LC 21), 4=295 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-81/103, 2-3=-81/103
BOT CHORD 1-4=-84/88, 3-4=-84/88
WEBS 2-4=-210/97

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 3 and 33 lb uplift at joint 4.

LOAD CASE(S) Standard

November 4,2025

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	86 Eagle Creek-Kathryn B	
25110007-A	V13	Valley	1	1	Job Reference (optional)	I77504795

Carter Components (Sanford, NC), Sanford, NC - 27332,

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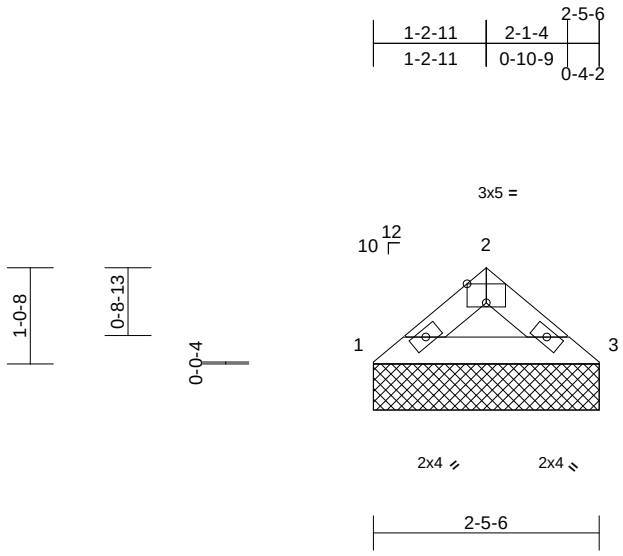


Scale = 1:25

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 7 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=2-5-6, 3=2-5-6
Max Horiz 1=-20 (LC 10)
Max Uplift 1=-8 (LC 14), 3=-8 (LC 15)
Max Grav 1=110 (LC 20), 3=110 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-139/60, 2-3=-139/60
BOT CHORD 1-3=-32/99

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 4-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 1 and 8 lb uplift at joint 3.

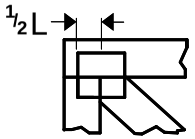
LOAD CASE(S) Standard

November 4,2025

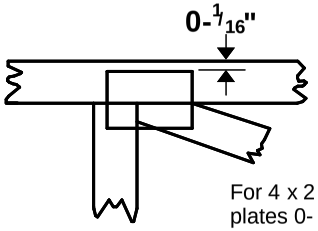
ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

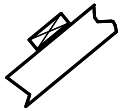
* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

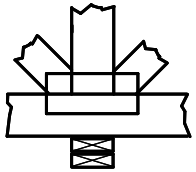
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

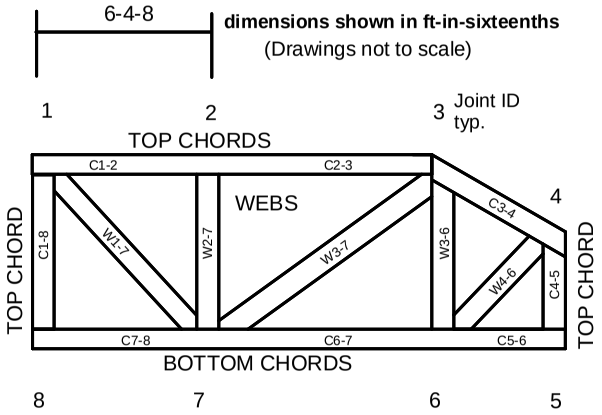


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek®

ENGINEERING BY TRENCO
A MiTek Affiliate

MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.