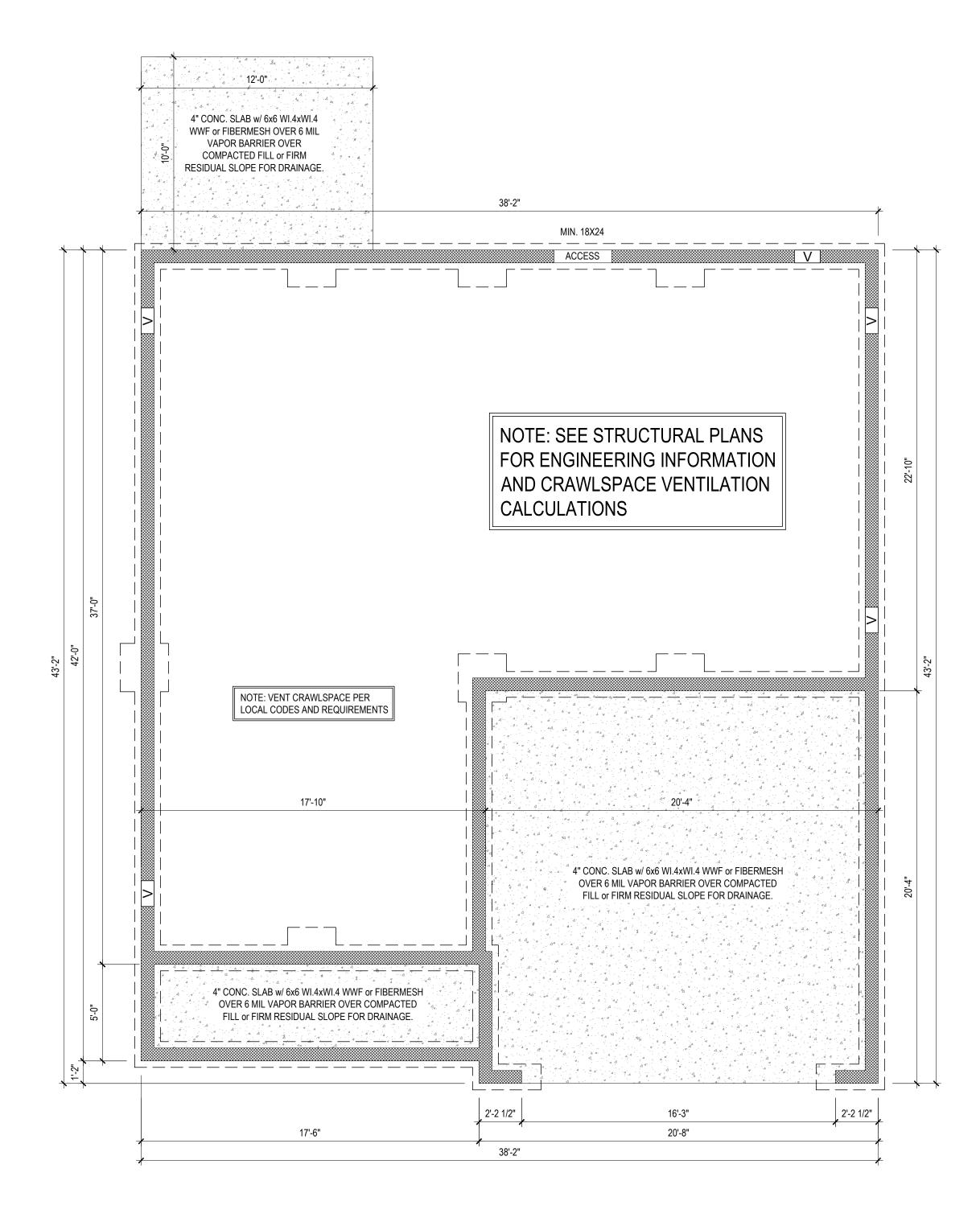
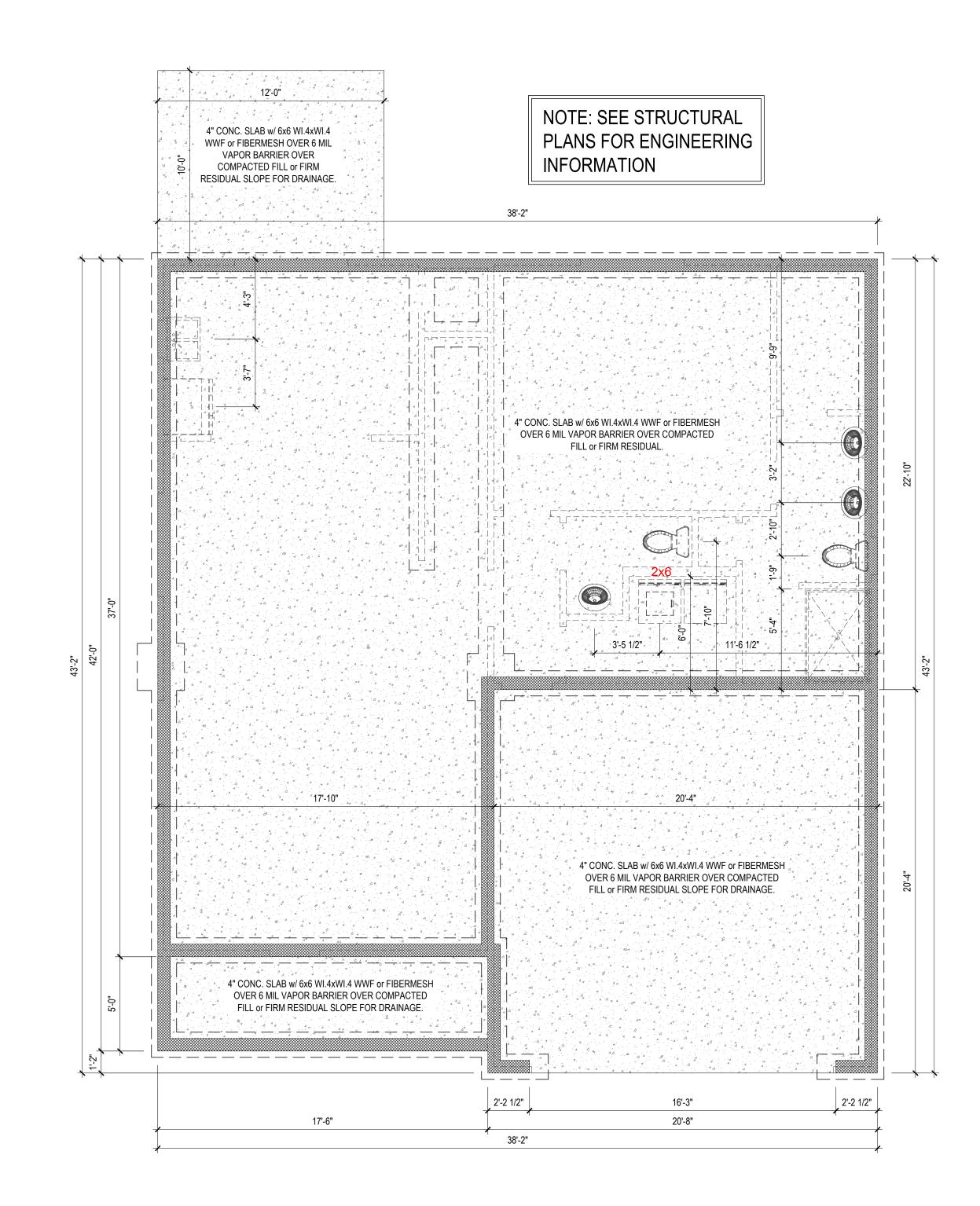
DRB2301-0476 B

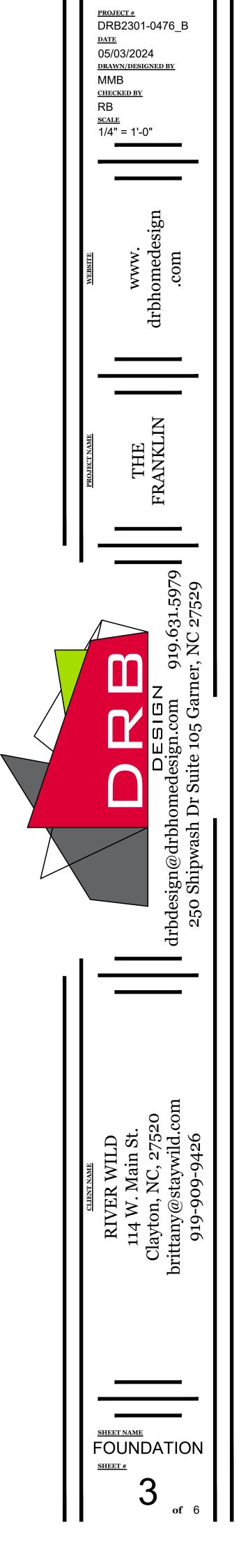


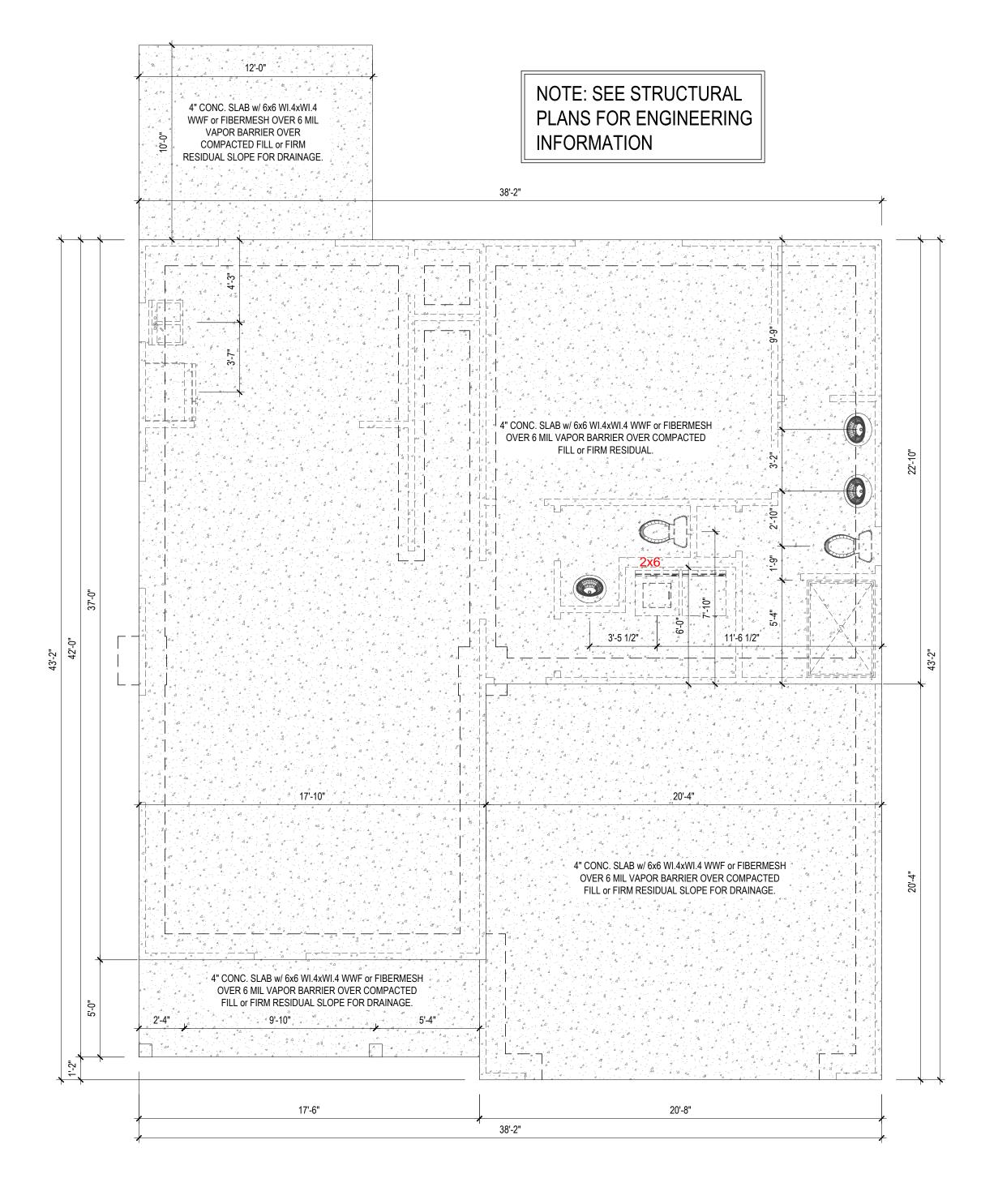
FOUNDATION PLAN - A&B 1/4" = 1'-0"

- DRB DESIGN assumes no liability for any home constructed from this plan.
- 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.
- 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN. 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- Communication is imperfect and every contingency cannot be anticipated.
- Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.
- 8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences.
- 9. Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB DESIGN of responsibility for any and all consequences arriving out of such changes.
- 10. Written dimensions on these plans always have precedence over scaled dimensions. 11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square
- footage errors once construction has begun. 12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.



FOUNDATION PLAN - A&B 1/4" = 1'-0" STEM WALL





FOUNDATION PLAN - A&B 1/4" = 1'-0" MONOLITHIC SLAB

- 1. DRB DESIGN assumes no liability for any home constructed from this plan.
- 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.
- 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.
 Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- 6. Communication is imperfect and every contingency cannot be anticipated.
- Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB
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- A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all
- responsibilities for all consequences.

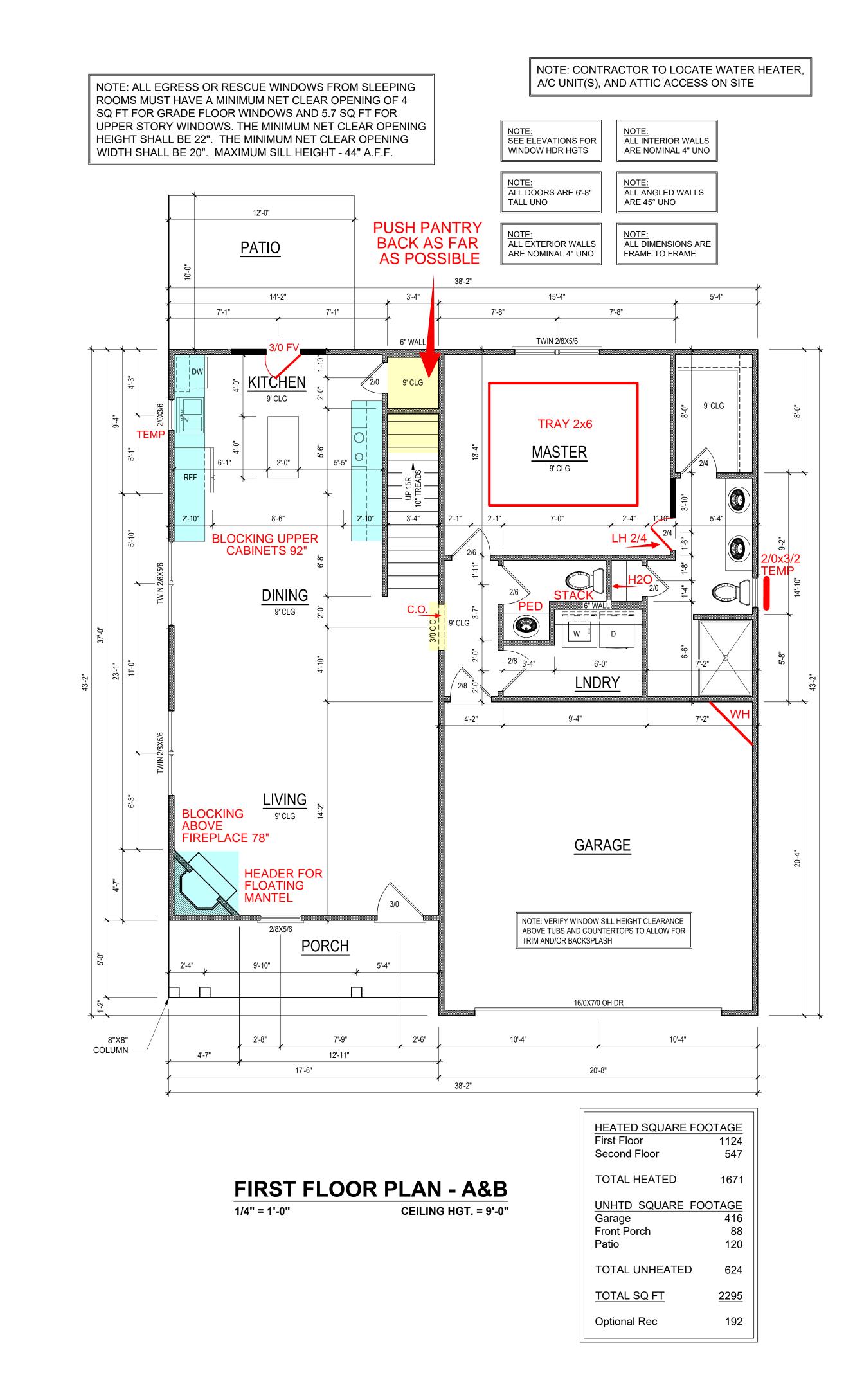
 9. Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB
- DESIGN of responsibility for any and all consequences arriving out of such changes.

 10. Written dimensions on these plans always have precedence over scaled dimensions.

 11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to
- footage errors once construction has begun.

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DRB2301-0476_B

05/03/2024

MMB

RB

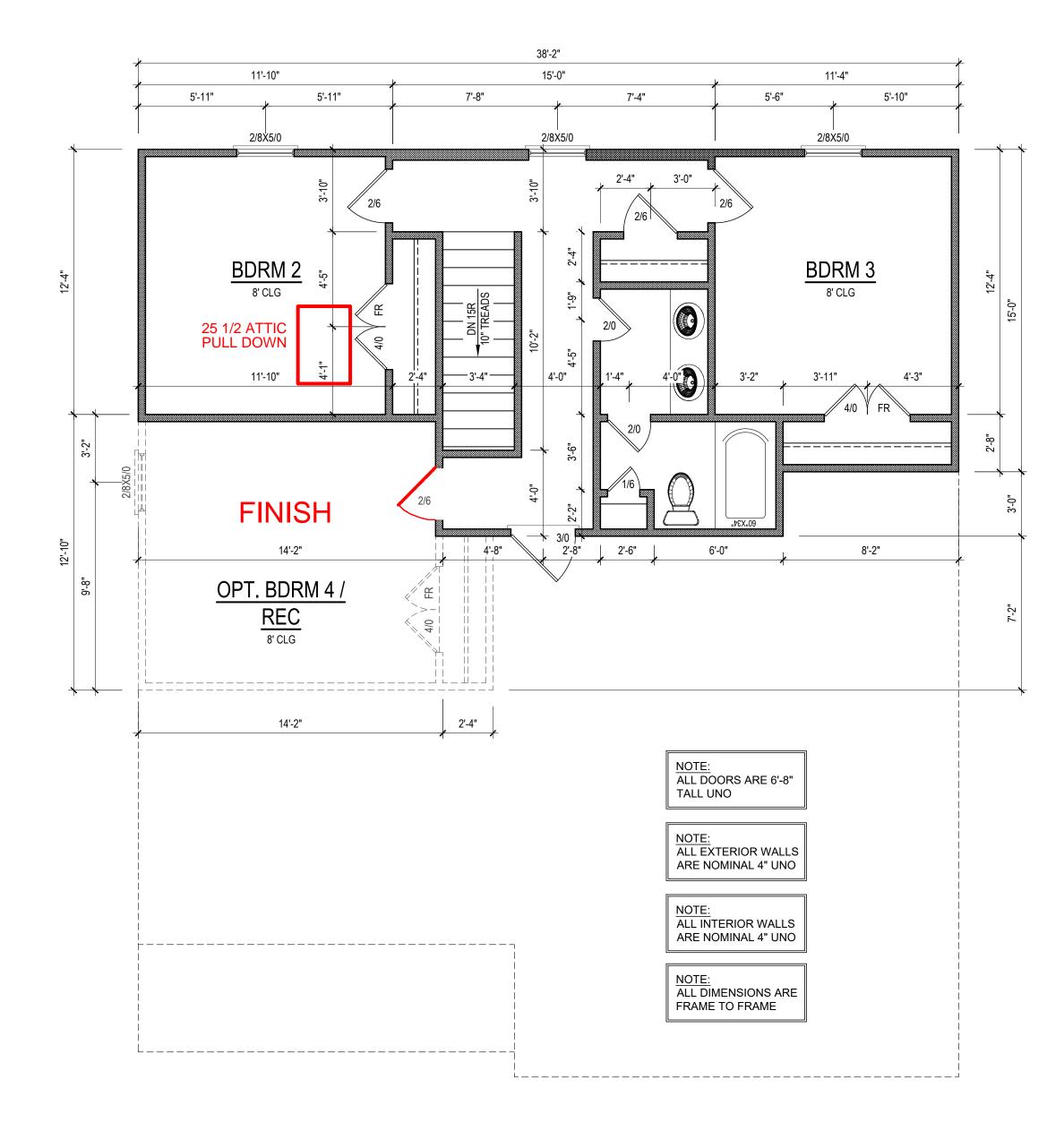
SCALE

CHECKED BY

1/4" = 1'-0"

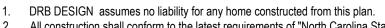
FND_1ST FL

DRAWN/DESIGNED BY

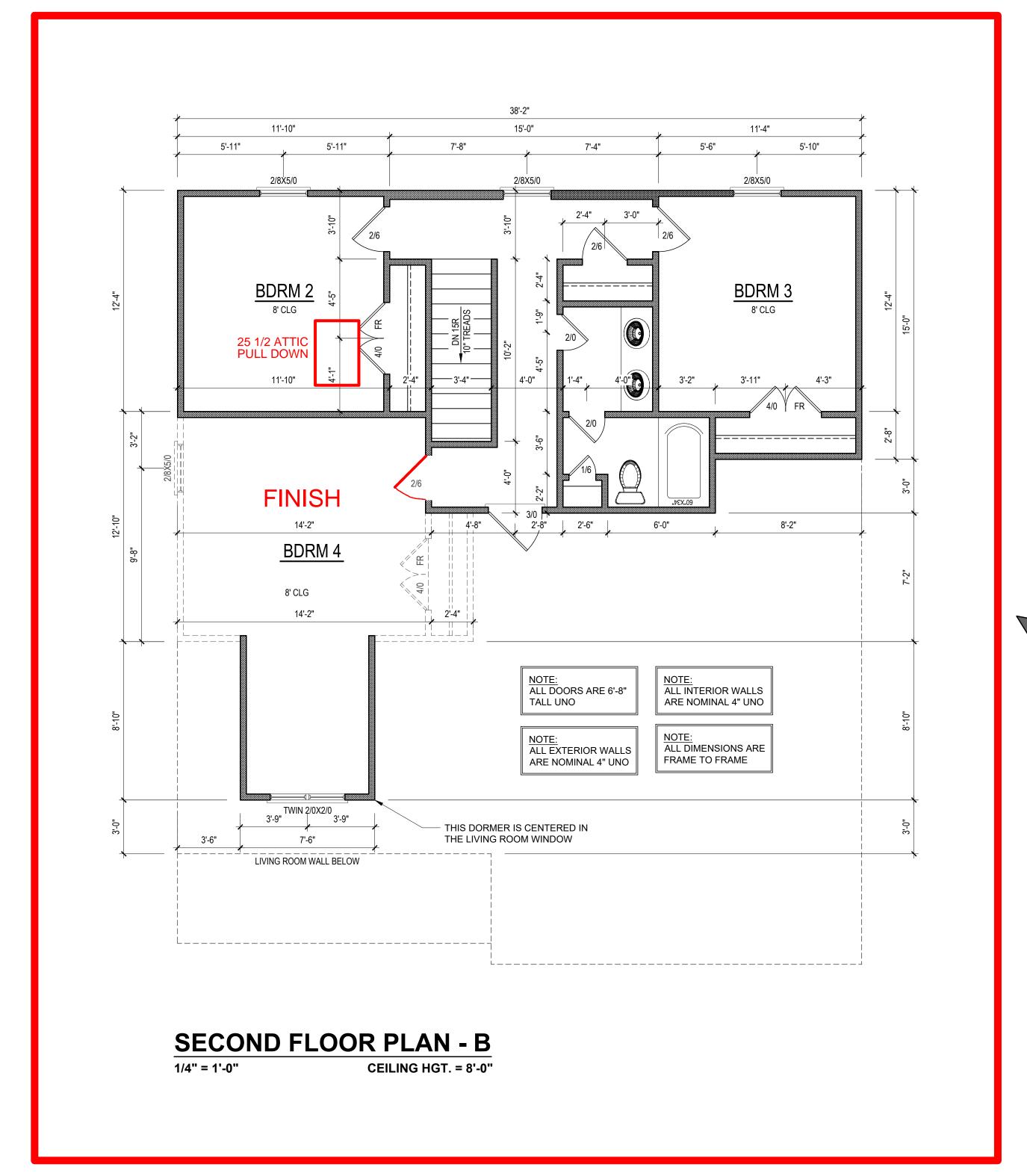


SECOND FLOOR PLAN - A

CEILING HGT. = 8'-0" 1/4" = 1'-0"



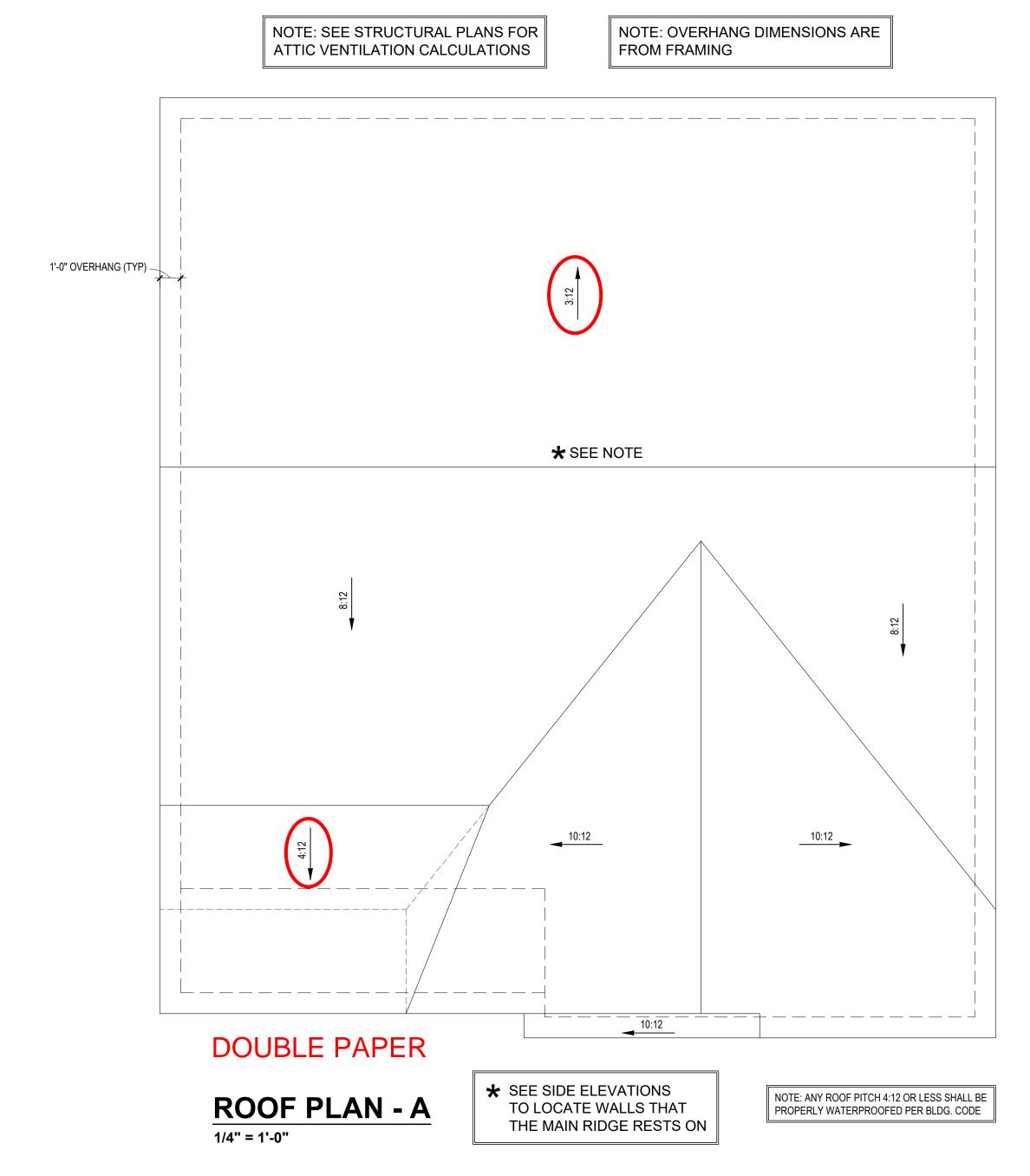
- 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.
- 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- 4. Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN. 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- 6. Communication is imperfect and every contingency cannot be anticipated. 7. Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB
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- 8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all
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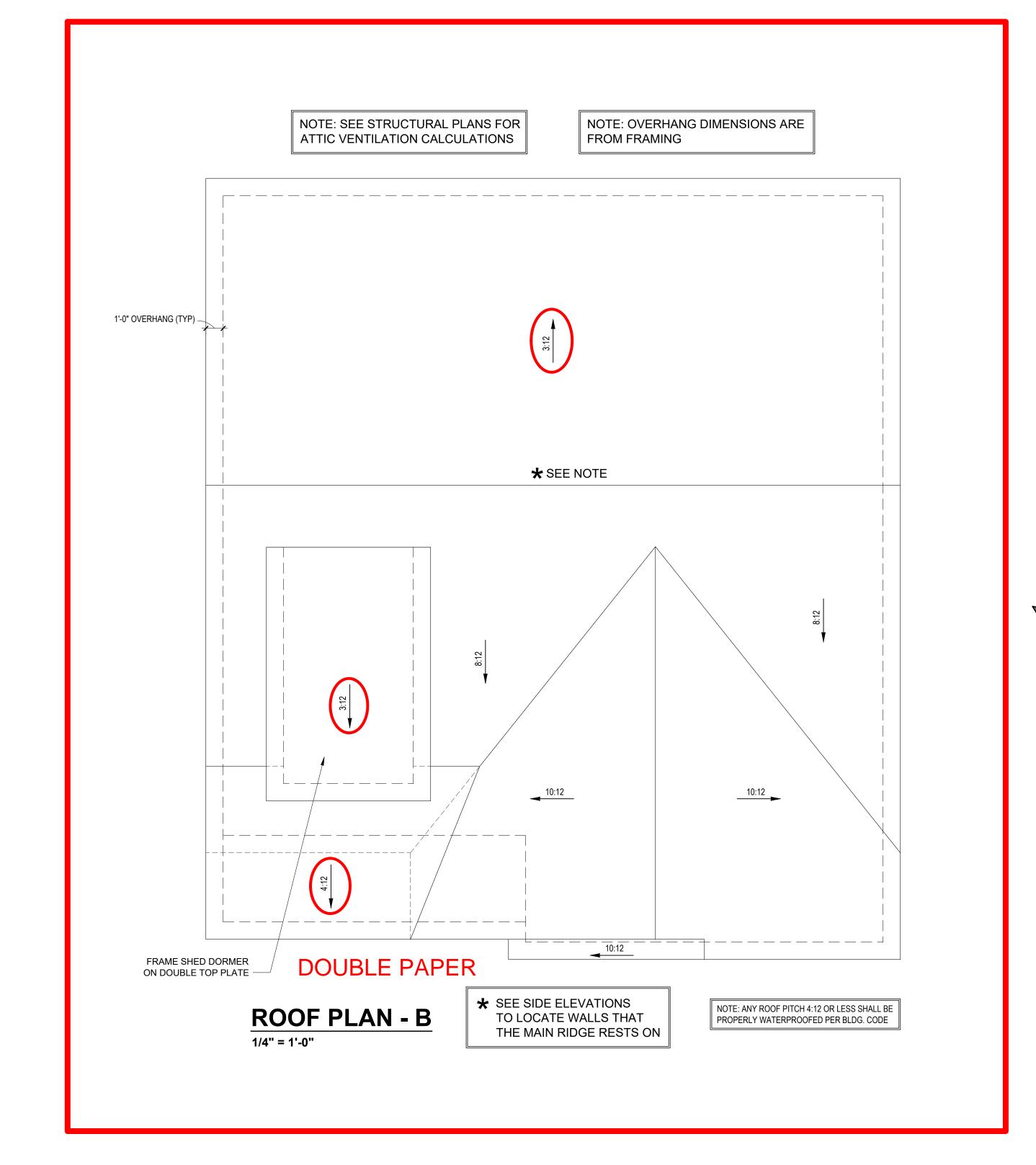


<u>PROJECT #</u> DRB2301-0476_B 05/03/2024 DRAWN/DESIGNED BY MMB CHECKED BY

SHEET NAME

2ND_FLOOR





DRB2301-0476_B

05/03/2024 DRAWN/DESIGNED BY

MMB CHECKED BY

footage errors once construction has begun.

DRB DESIGN assumes no liability for any home constructed from this plan. 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.

3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.

Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN. 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.

Communication is imperfect and every contingency cannot be anticipated.

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12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

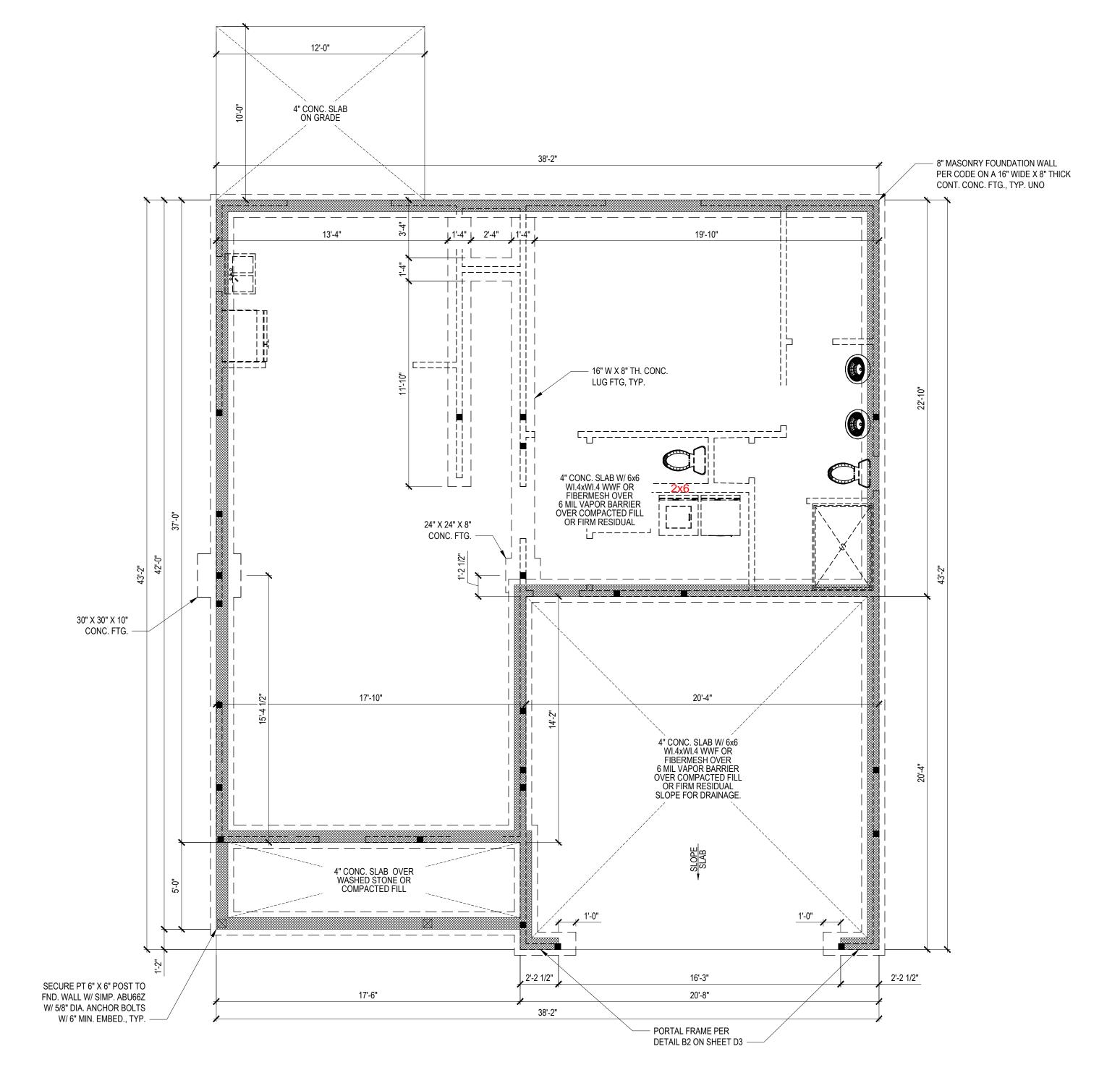
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	(. 5.)	(. 5.)	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 3) ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND
 Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)
 - (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER IS TO BE 1.55F (Fb = 2325 PSI) (OR GREATER)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER) ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT.
 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)

 13) PROVIDE A MINIMUM OF 500# LIPLIET & LATERAL CONNECTION
- 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.

 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



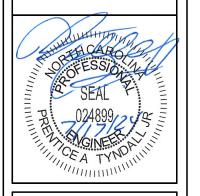
FOUNDATION PLAN - A&B

1/4" = 1'-0" STEM WALL

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

*Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

*Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

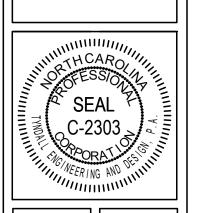


ENGINEERING & DESIGN, P.A.

T \$19 773-1200 - # \$19 773-9458

280 Shipwash Drive - Garmer - North Carolina - 27829

www.syndallangineering.com



RIVER WILD 114 W. MAIN ST. CLAYTON, NC, 27520 THE FRANKLIN

OUNDATION PLAN STEM WALL

Project #:
DRB2301-0476B

Date:
05/13/2024

Engineered By:
JA

DWG. Checked By:
AM

Scale:
SEE PLAN

REVISIONS

REVISIONS

No. Date: Remarks

1
2
3

Sheet Number

S1A

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION			
	(* 5.)	(* 5. /	LL	TL		
FLOOR (primary)	40	10	L/360	L/240		
FLOOR (secondary)	40	10	L/360	L/240		
ATTIC (w/ storage)	20	10	L/240	L/180		
ATTIC (no access)	10	5	L/240 L/180			
EXTERNAL BALCONY	40	10	L/360	L/240		
ROOF	20	10	L/240	L/180		
ROOF TRUSS	20	20	L/240	L/180		
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)					
SEISMIC	BAS	ED ON SEISMIC ZO	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

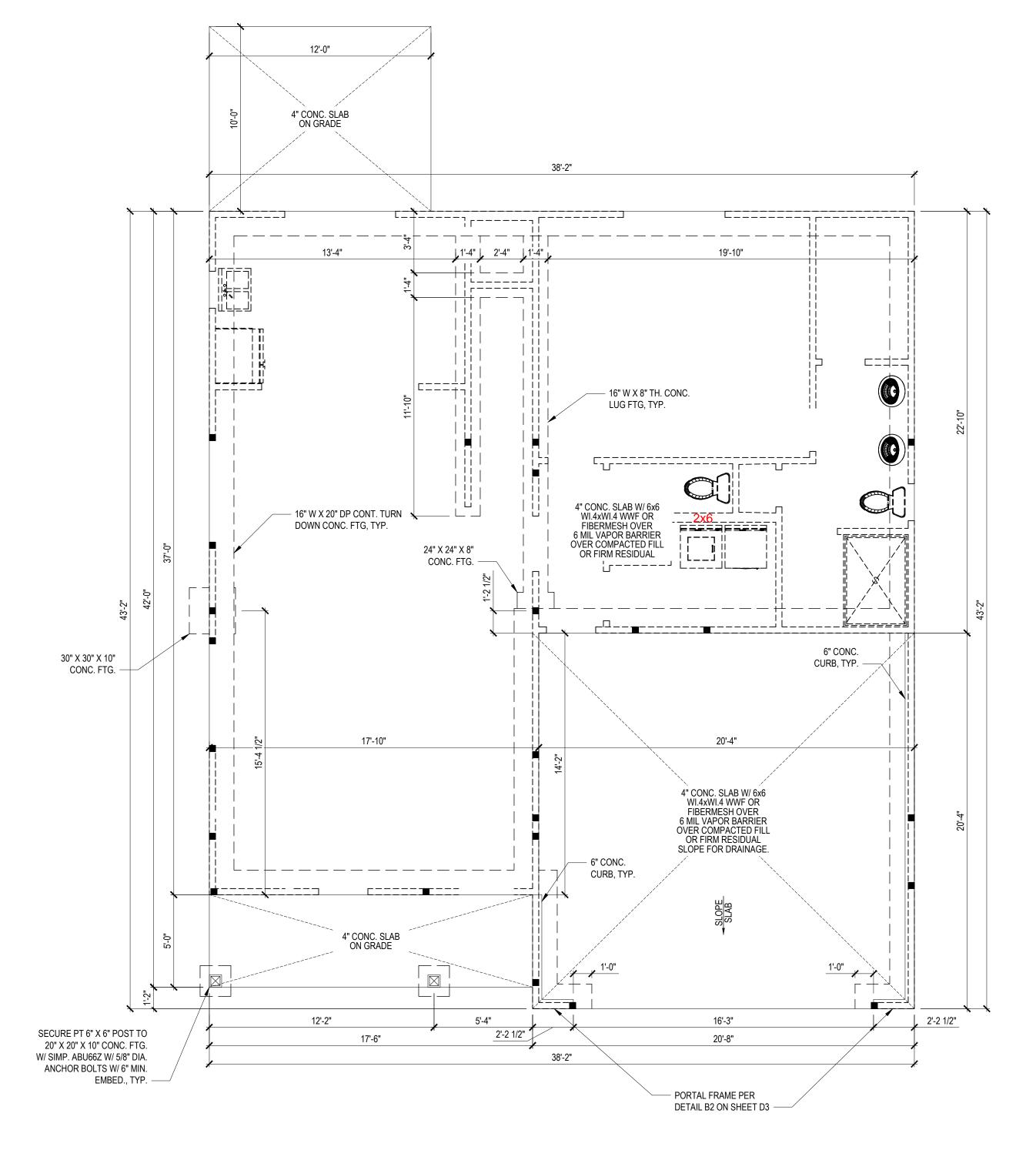
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- 3) ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND
 Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)
 (I.E. iLEVEL MICROLAM)
 - ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
- ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)

 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
 TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE
 WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS
- 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).

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- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT 9) ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
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 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.

 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



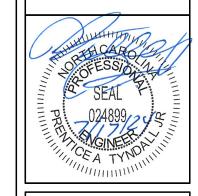
FOUNDATION PLAN - A&B

1/4" = 1'-0" MONOLITHIC SLAB

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

*Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

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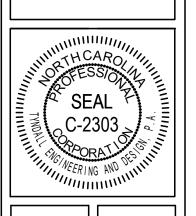


ENGINEERING & DESIGN, P.A.

1 919 775-1200 = # 919 775-9468

250 Shipwash Drive = Garmer = North Caroline = 27829

www.tyndellengineering.com



RIVER WILD 114 W. MAIN ST. CLAYTON, NC, 27520

FOUNDATION PLAN MONOLITHIC SLAB

DRB2301-0476B

Date:
05/13/2024

Engineered By:
JA

DWG. Checked By:
AM

Scale:
SEE PLAN

REVISION

 No.
 Date:
 Remarks

 1
 2

 3
 3

Sheet Number

S1B

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE		
			LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BAS	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS
AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL
ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS
AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

3) ALL LUMBER SHALL BE SYP #2 (UNO)
ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND
Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)

(I.E. iLEVEL MICROLAM)
ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)

4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).

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6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.

7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)

8) ALL EXTERIOR LUMBER TO BE #2 SYP PT

9) ALL CONCRETE, fc = 3000 PSI MIN.

10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF

11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.

12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP

AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018

15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY

ANCHORED TO THE FOUNDATION.

17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

*NOTE: SECURE 4-PLY W/ 1/2"Ø THRU-BOLTS @ 24" O.C. (OR EQUIV. STRUCTURAL SCREWS)

ADDITIONAL JOISTS

-INSTALL AN ADDITIONAL JOIST UNDER NON-LOAD BEARING WALLS, BUILT-INS, AND CABINETRY ABOVE THAT ARE PARALLEL TO THE FRAMING SYSTEM ON THIS PAGE, TYP. UNO, BUILDER TO INSTALL AS REQUIRED, VIF DIMENSIONS

1671 SQ. FT. OF CRAWL SPACE / 150 = 11.14 SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION 11.14 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 13.0 VENTS REQ'D (BASED ON 8" X 16" VENTS)¹

-OR-

1671 SQ. FT. OF CRAWL SPACE / 1500 = 1.11 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 1.11 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 2.0 VENT REQ'D (BASED ON 8" X 16" VENTS)2

1) VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON PLAN, HOWEVER VENTS SHALL BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS AND TO PREVENT DEAD AIR POCKETS.

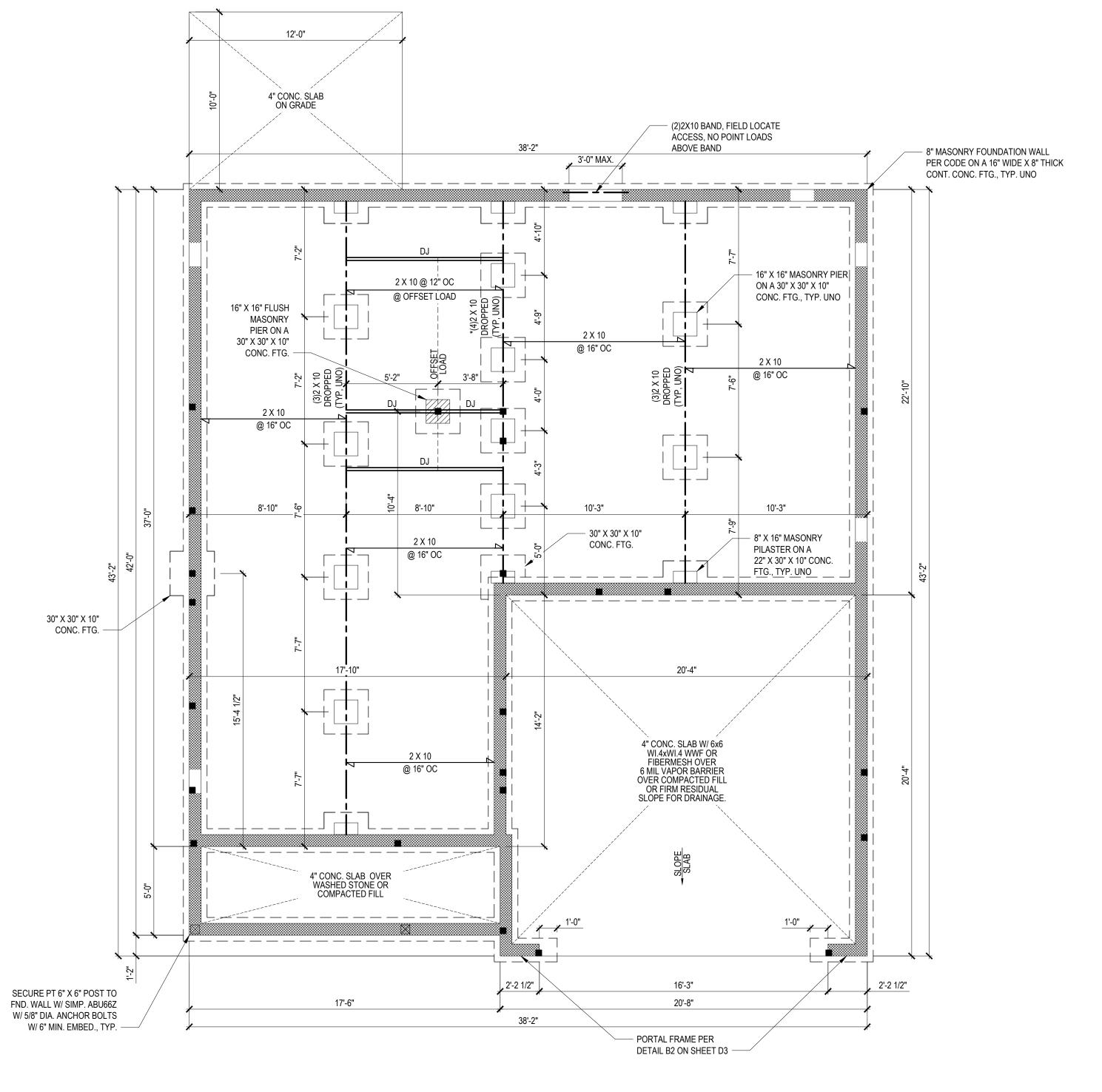
2) THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 OF THE CRAWL SPACE GROUND AREA WHERE THE REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS VENTILATION OF THE CRAWL SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. ONE FOUNDATION VENT SHALL BE WITHIN 3 FEBT OF EACH CORNER OF THE BUILDING. TO PREVENT RAINWATER ENTRY WHEN THE CRAWL SPACE IS BUILT ON A SLOPED SITE, THE UPHILL FOUNDATION WALLS MAY BE CONSTRUCTED WITHOUT WALL VENT OPENINGS. VENT DAMS SHALL BE PROVIDED WHEN THE BOTTOM OF THE FOUNDATION VENT OPENING IS LESS THAN 4 INCHES ABOVE THE FINISHED EXTERIOR GRADE.

WALL VENTED CRAWL SPACES REQUIRE FULL COVERAGE GROUND VAPOR RETARDERS.

* CRAWL SPACE VENTILATION CALCULATION

ADDITIONAL JOISTS

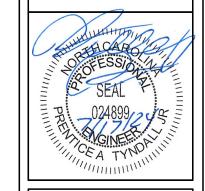
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FOUNDATION PLAN - A&B

1/4" = 1'-0" CRAWL SPACE

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
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ENGINEERING & DESIGN, P.A.

1 919 775-1200 - # 919 775-9655

250 Shipwesh Drive - Gerner - North Caroline - 27529

www.tyndellengineering.com



RIVER WILD 114 W. MAIN ST. CLAYTON, NC, 27520 THE FRANKLIN

> FOUNDATION PLAN CRAWL SPACE

Project #:

DRB2301-0476B

Date:

05/13/2024

Engineered By:

JA

DWG. Checked By:

AM

AM
Scale:
SEE PLAN

Sheet Number

51**C**

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
		, ,	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240 L/180 L/360 L/240		
EXTERNAL BALCONY	40	10			
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BAS	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN

ADDITION TO ALL LOCAL CODES AND REGULATIONS. 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM)

ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)

ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER) 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS

1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS

6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.

ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)

ALL EXTERIOR LUMBER TO BE #2 SYP PT

ALL CONCRETE, fc = 3000 PSI MIN. PRESUMPTIVE BEARING CAPACITY = 2000 PSF

1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR

12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)

14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018

15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.

17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- (1) REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- $\overline{2}$ 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN

67% OF WALL HEIGHT - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT

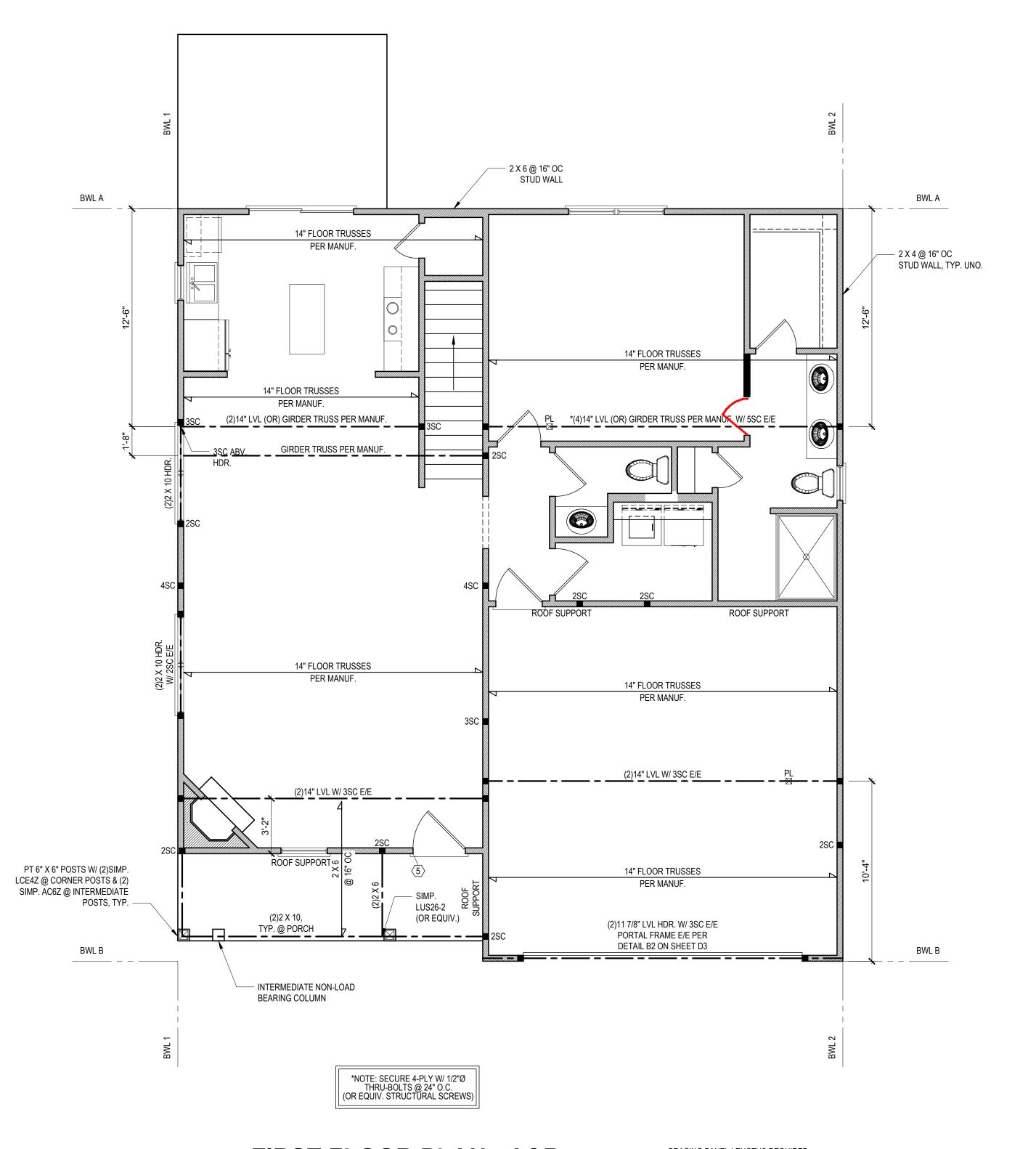
4 SHEATH INTERIOR & EXTERIOR

8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.

(5) MINIMUM 800# HOLD-DOWN DEVICE

ADDITIONAL TRUSSES

-INSTALL AN ADDITIONAL TRUSS UNDER NON-LOAD BEARING WALLS, BUILT-INS, AND CABINETRY ABOVE THAT ARE PARALLEL TO THE FRAMING SYSTEM ON THIS PAGE, TYP. UNO, BUILDER TO INSTALL AS REQUIRED, VIF DIMENSIONS



FIRST FLOOR PLAN - A&B

1/4" = 1'-0"

CEILING HGT. = 9'-0"

BRACING PANEL LENGTHS REQUIRED: BWL A = 12.3 FT BWL B = 12.3 FT BWL 1 = 10.9 FT BWL 2 = 10.9 FT

BRACING PANEL LENGTHS PROVIDED:
BWL A = 26.67 FT CS-WSP
BWL B = 17.83 FT CS-WSP
BWL 1 = 30.17 FT CS-WSP
BWL 2 = 40.50 FT CS-WSP

KING STUD SCHEDULE

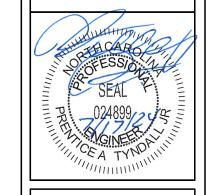
	MIN. # OF FULL HEIGHT STUDS (KING) E.E. OF OPENING PER WALL DEPTH				
	OI OF LINING FL	IN WALL DEFIII			
HEADER SPAN (FT)	2 X 4 STUD WALL	2 X 6 STUD WALL			
UP TO 3'-0"	1	1			
3'-1" TO 6'-0"	2	1			
6'-1" TO 9'-0"	3	2			
9'-1" TO 12'-0"	4	2			
10' 1" TO 10' 0"	6	r			

TABLE DENOTES REQUIRED MINIMUM NUMBER OF STUDS EE OF HEADER, TYP UNO

NUMBER OF KING STUDS LISTED ABOVE ARE BASED 10' NOMINAL WALL HEIGHT, STUD SPACING OF 16" O.C., AND WIND LOAD OF 120 MPH (EXPOSURE B)

HEADER SPANS IN TABLE ARE BASED ON ROUGH OPENINGS. INTERPOLATION BETWEEN SPAN VALUES IS PERMITTED, ROUND UP NUMBER OF KING STUDS, EXTRAPOLATION IS PROHIBITED. CONTACT TYNDALL ENGINEERING AND DESIGN IF HEADER SPANS EXCEED TABLE VALUES

12'-1" TO 18'-0"

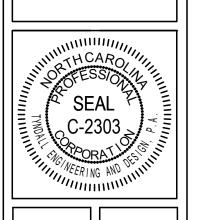


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DRB2301-0476B 05/13/2024 Engineered By: DWG. Checked By: AMSEE PLAN

REVISIONS No. Date:

Sheet Number

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	,	, ,	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240 L/180		
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

STRUCTURAL NOTES:

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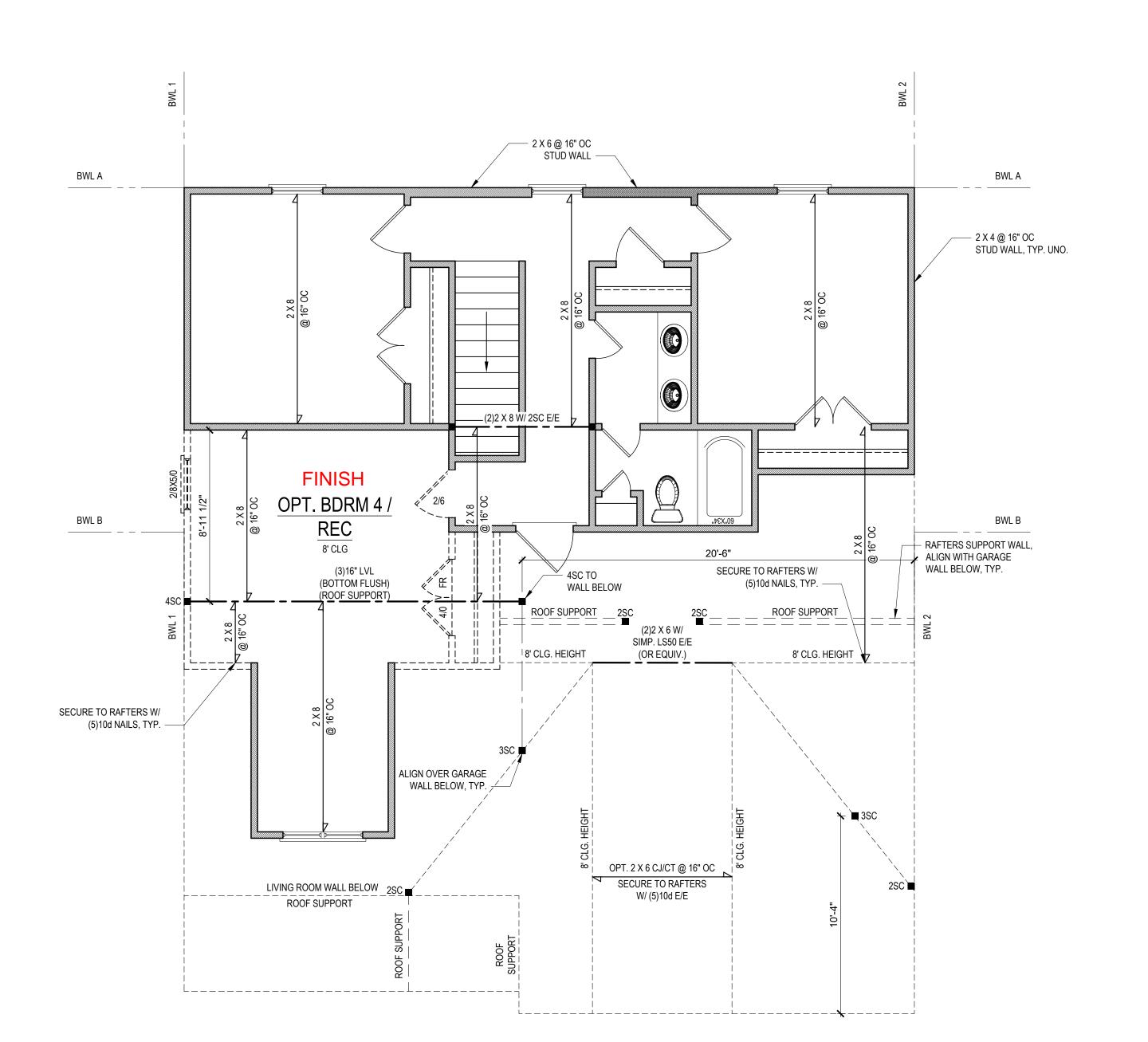
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- 3) ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND
 Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)
 (I.E. ¡LEVEL MICROLAM)
 - ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
- ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)

 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
 TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE
 WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS
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- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT 9) ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 11) PRESUMPTIVE BEARING CAPACITY = 2000 PSF

 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROPUE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

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 (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING).
 SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5)
 SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND
 BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 24" ADJACENT TO OPENINGS NOT MORE THAN
 - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- $\langle 4 \rangle$ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- (5) MINIMUM 800# HOLD-DOWN DEVICE



SECOND FLOOR PLAN - B

CEILING HGT. = 8'-0"

1/4" = 1'-0"

BRACING PANEL LENGTHS REQUIRED: BWL A = 2.5 FT

BWL B = 35.17 FT CS-WSP

BWL 1 = 18.00 FT CS-WSP BWL 2 = 18.00 FT CS-WSP
 KING STUD SCHEDULE

 MIN. # OF FULL HEIGHT STUDS (KING) E.E. OF OPENING PER WALL DEPTH

 HEADER SPAN (FT)
 2 X 4 STUD WALL

 UP TO 3'-0"
 1
 1

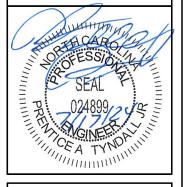
 3'-1" TO 6'-0"
 2
 1

 6'-1" TO 9'-0"
 3
 2

 9'-1" TO 12'-0"
 4
 2

 12'-1" TO 18'-0"
 6
 3

- a. TABLE DENOTES REQUIRED MINIMUM NUMBER OF STUDS EE OF HEADER, TYP UNO ON PLANS
- b. NUMBER OF KING STUDS LISTED ABOVE ARE BASED 10' NOMINAL WALL HEIGHT, STUD SPACING OF 16" O.C., AND WIND LOAD OF 120 MPH (EXPOSURE B)
- c. HEADER SPANS IN TABLE ARE BASED ON ROUGH OPENINGS. INTERPOLATION
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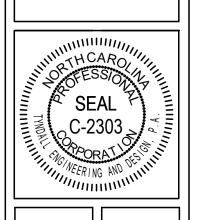
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RIVER WILD 114 W. MAIN ST. CLAYTON, NC, 27520 THE FRANKLIN

> 2ND FLOOR HEADER 2ND FLR. CLG. FRAMING

DRB2301-0476B

Date:
05/13/2024

Engineered By:
JA

DWG. Checked By:
AM

SEE PLAN

REVISIONS

No. Date: Remarks

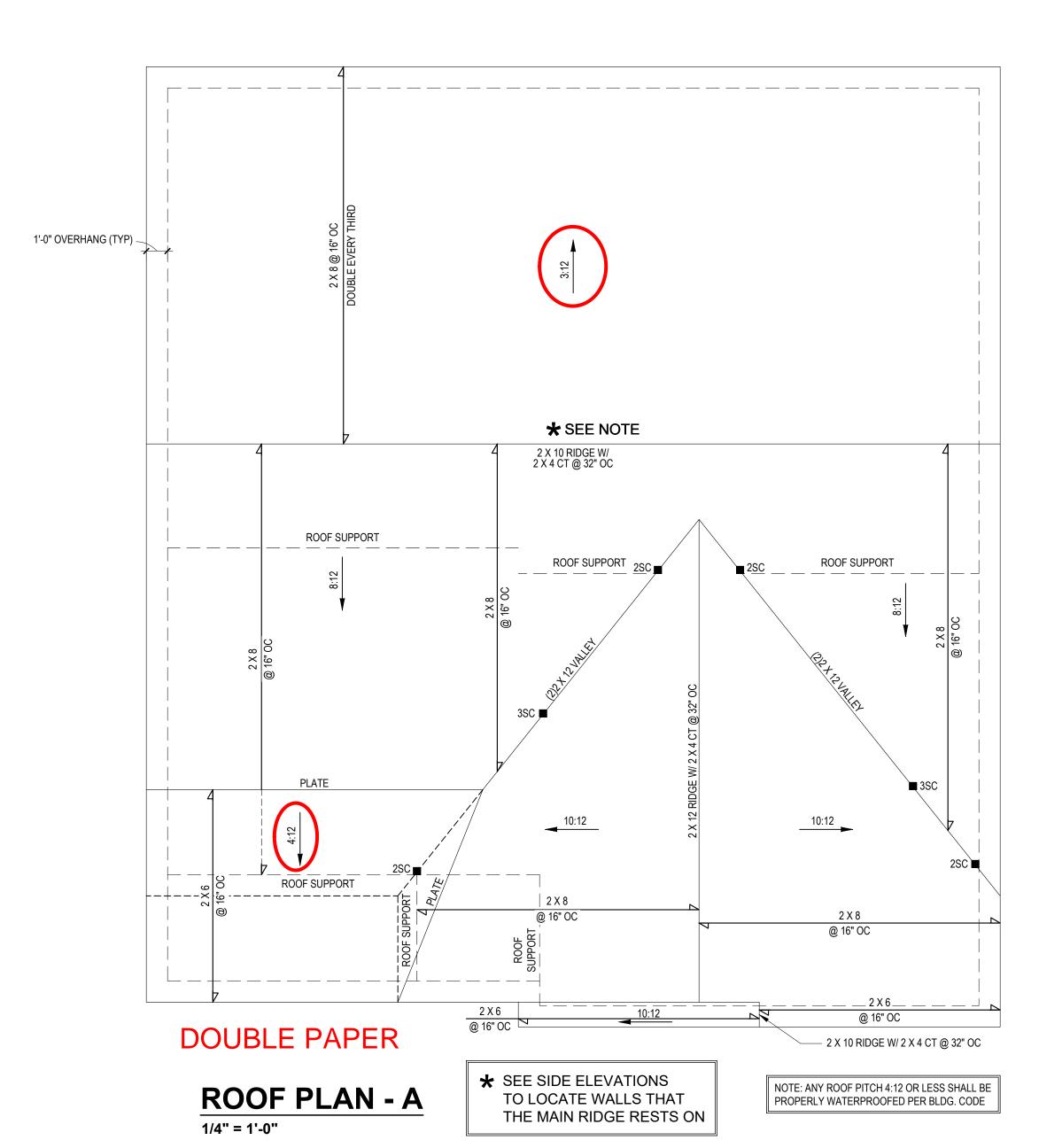
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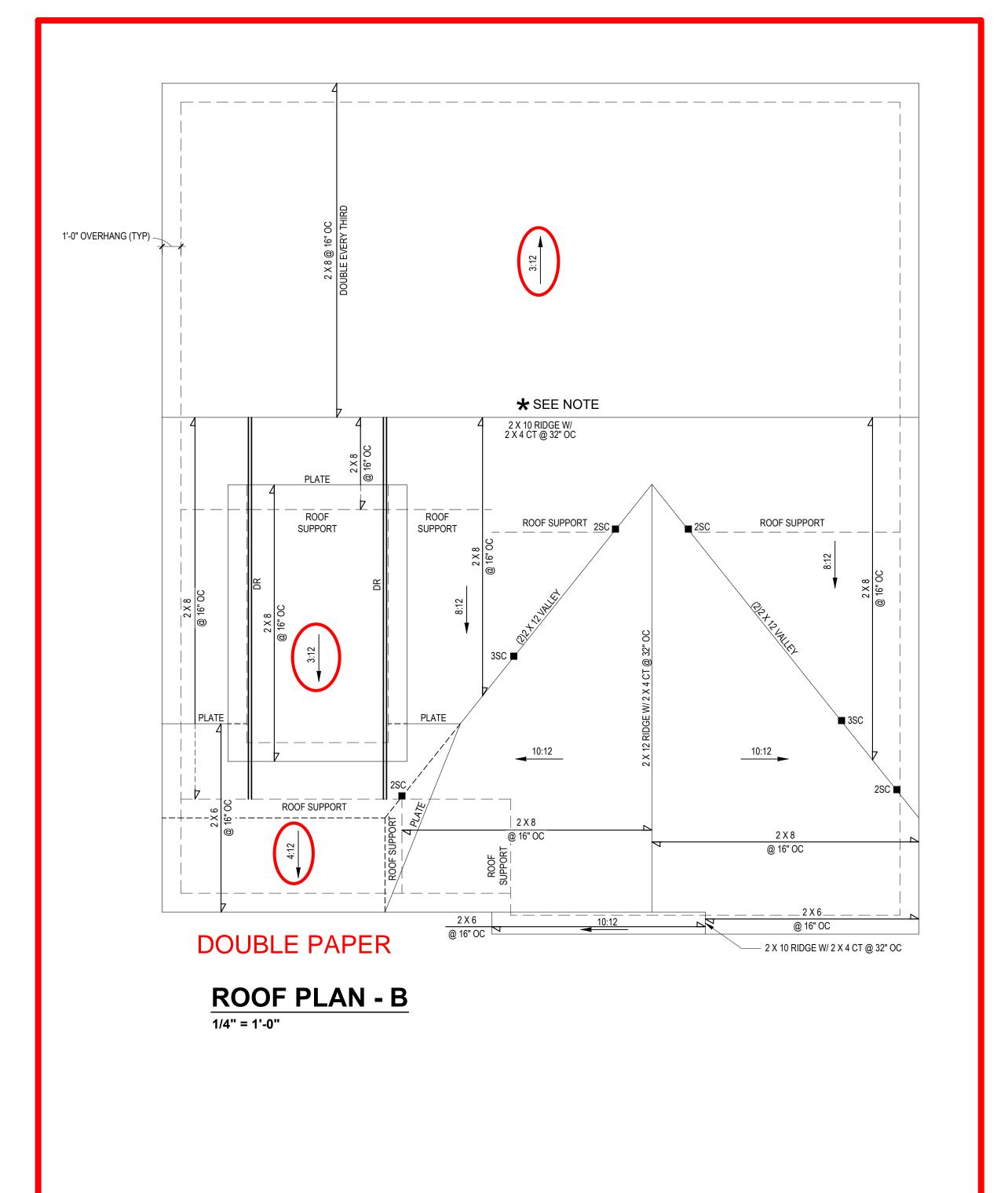
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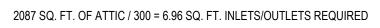
	LIVE LOAD DEAD LOAD (PSF) (PSF)		DEFLECTION		
	(* 5.)	(* 5. /	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
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WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
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- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER) ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER) 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
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- ALL EXTERIOR LUMBER TO BE #2 SYP PT
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- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
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- ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



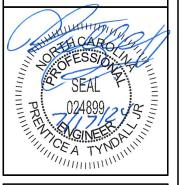




- CALCULATION BASED ON VENTILATORS USED AT LEAST 3-0" ABOVE
 THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED
- CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

* ATTIC VENTILATION CALCULATION NO SCALE

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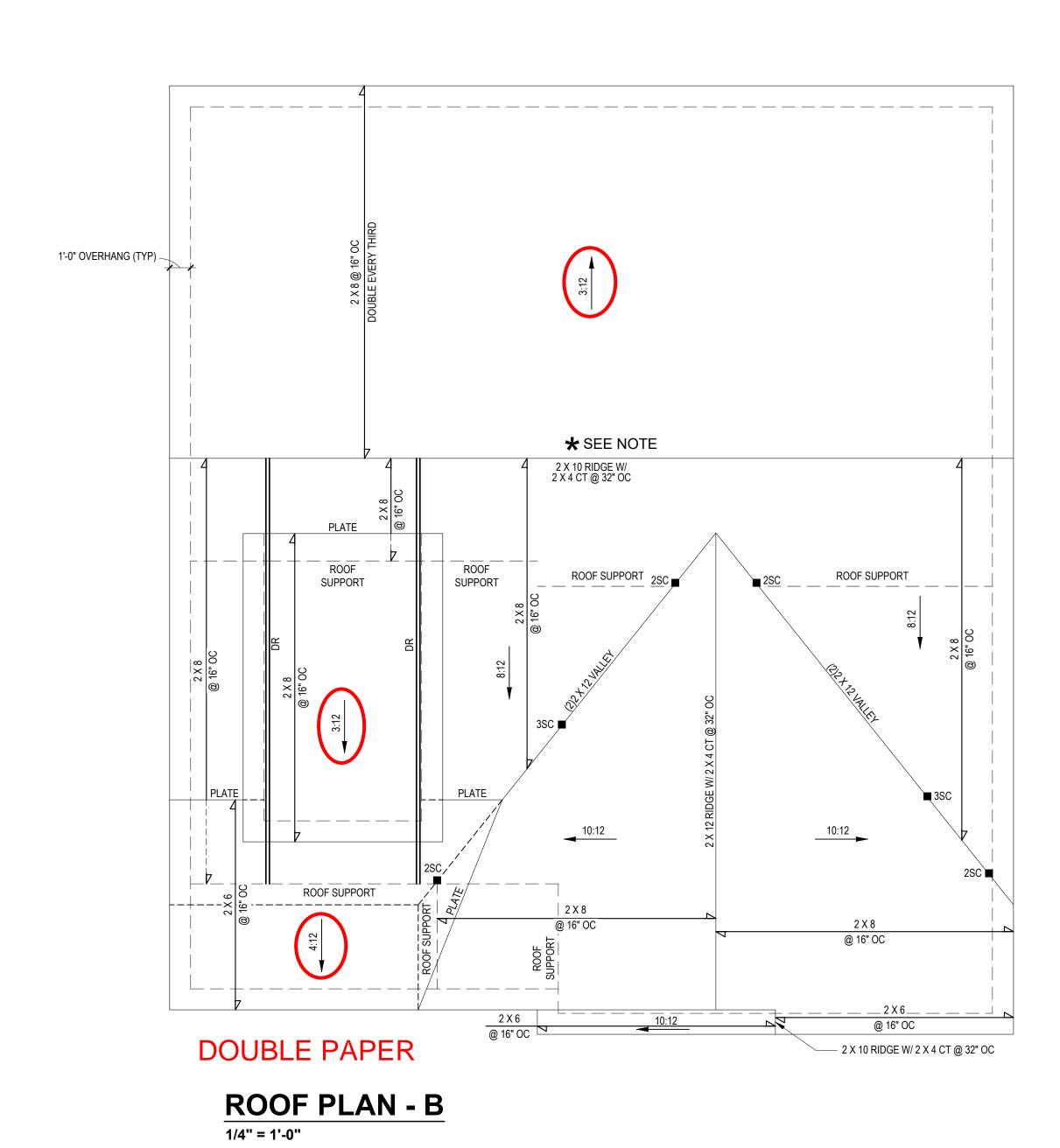
RIVER WILD 114 W. MAIN ST. LAYTON, NC, 27520

DRB2301-0476B 05/13/2024 Engineered By: DWG. Checked By:

SEE PLAN

REVISIONS

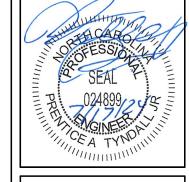
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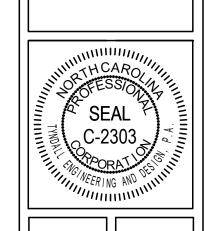


*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

*Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

*Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.





RIVER WILD 114 W. MAIN ST. CLAYTON, NC, 27520

Project #: DRB2301-0476B Date: 05/13/2024

Scale: SEE PLAN REVISIONS

Sheet Number

STRUCTURAL NOTES

1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION			
	(*)	(* 3.)	LL	TL		
ALL FLOORS	40	10	L/360	L/240		
ATTIC (w/ walk up stairs)	30	10	L/360	L/240		
ATTIC (pull down access)	20	10	L/240	L/180		
ATTIC (no access)	10	5	L/240	L/180		
EXTERNAL BALCONY	40	10	L/360	L/240		
ROOF	20	10	L/240	L/180		
ROOF TRUSS	20	20	L/240	L/180		
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)					
SEISMIC		SEISMIC ZONES A, B & C				

- 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- 6) ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO. ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL. ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.) ALL LSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.) ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2"Ø x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- 12) WALL AND ROOF CLADDING VALUES:
- WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE. ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
- 39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12
- 36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
- **MEAN ROOF HEIGHT 30'-0" OR LESS
- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 14) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 20) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

			GLAZED		WOOD	MASS		BASEMENT ","	SLAB ^a	CRAWL SPACE
CLIMATE	FENESTRATION	SKYLIGHT b	FENESTRATION	CEILING ^m	FRAMED WALL	WALL	FLOOR	WALL	R-VALUE	WALL
ZONES	U-FACTOR b, J	U-FACTOR	SHGC b, <u>k</u>	R-VALUE	R-VALUE	R-VALUE i	R-VALUE	R-VALUE	AND DEPTH	R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5	<u>5/13 or</u> 5/10 cont	19	<u>5/13</u> ^f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont	15 or 13 + <u>2.5</u> h	<u>5/13 or</u> 5/10 cont	19	<u>10/15</u>	10	<u>10/15</u>
5	0.35	0.55	NR	38 or 30 cont	ⁿ 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 ⁹	10/15	10	<u>10/19</u>
TABLE NAMES A SUBMETE TONIES OF										

TABLE N1102.1 CLIMATE ZONES 3-5

INSULATION PLUS R-2.5 SHEATHING.

- a. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS, WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.
 - b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT
 - (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION. c. "10/15" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME
 - OR R-15 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL. d. $\underline{\mathsf{FOR}\,\mathsf{MONOLITHIC}\,\mathsf{SLABS}}$, insulation shall be applied from the inspection gap downward to the bottom
 - $\underline{\text{OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION}\\$ SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. R-5 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS.
 - e. <u>DELETED</u>
 - $f. \ \ \text{BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY} \ \underline{FIGURE\ N1101.7}\ \text{AND}\ \underline{TABLE\ N1101.7}.$ g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
 - h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 INSULATED SHEATHING. "15+3" MEANS R-15 CAVITY INSULATION. PLUS R-3 INSULATED SHEATHING. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR.
 - INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVITY
 - i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL.
 - j. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
 - k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
 - I. R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH
 - OF THE ATTIC ROOF DECK. m. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE. n. R-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED
 - AND INSTALLED IN A 2X4 WALL IS NOT DEEMED TO COMPLY o. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT	=	ALTERNATE	MAX	=	MAXIMUM
CANT	=	CANTILEVER	MIN	=	MINIMUM
CJ	=	CEILING JOIST	NOM	=	NOMINAL
CMU	=	CONCRETE MASONRY UNIT	O.C.	=	ON CENTER
COL	=	COLUMN	PL	=	POINT LOAD
CONC	=	CONCRETE	PT	=	PRESSURE TREATED
CONT	=	CONTINUOUS	REINF	=	REINFORCED
CT	=	COLLAR TIE	REQD	=	REQUIRED
DBL	=	DOUBLE	RJ	=	ROOF JOIST
DIA	=	DIAMETER	RS	=	ROOF SUPPORT
DJ	=	DOUBLE JOIST	SC	=	STUD COLUMN
DR	=	DOUBLE RAFTER	SCH	=	SCHEDULE
EA	=	EACH	SPEC	=	SPECIFIED
EE	=	EACH END	THK	=	THICK
FJ	=	FLOOR JOIST	TJ	=	TRIPLE JOIST
FND	=	FOUNDATION	TRTD	=	TREATED
FTG	=	FOOTING	TYP	=	TYPICAL
GALV	=	GALVANIZED	UNO	=	UNLESS NOTED OTHERWIS
HORIZ	=	HORIZONTAL	W	=	WIDE FLANGE BEAM
HT	=	HEIGHT	WWF	=	WELDED WIRE FABRIC
MANUF	=	MANUFACTURER	XJ	=	EXTRA JOIST

1) MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

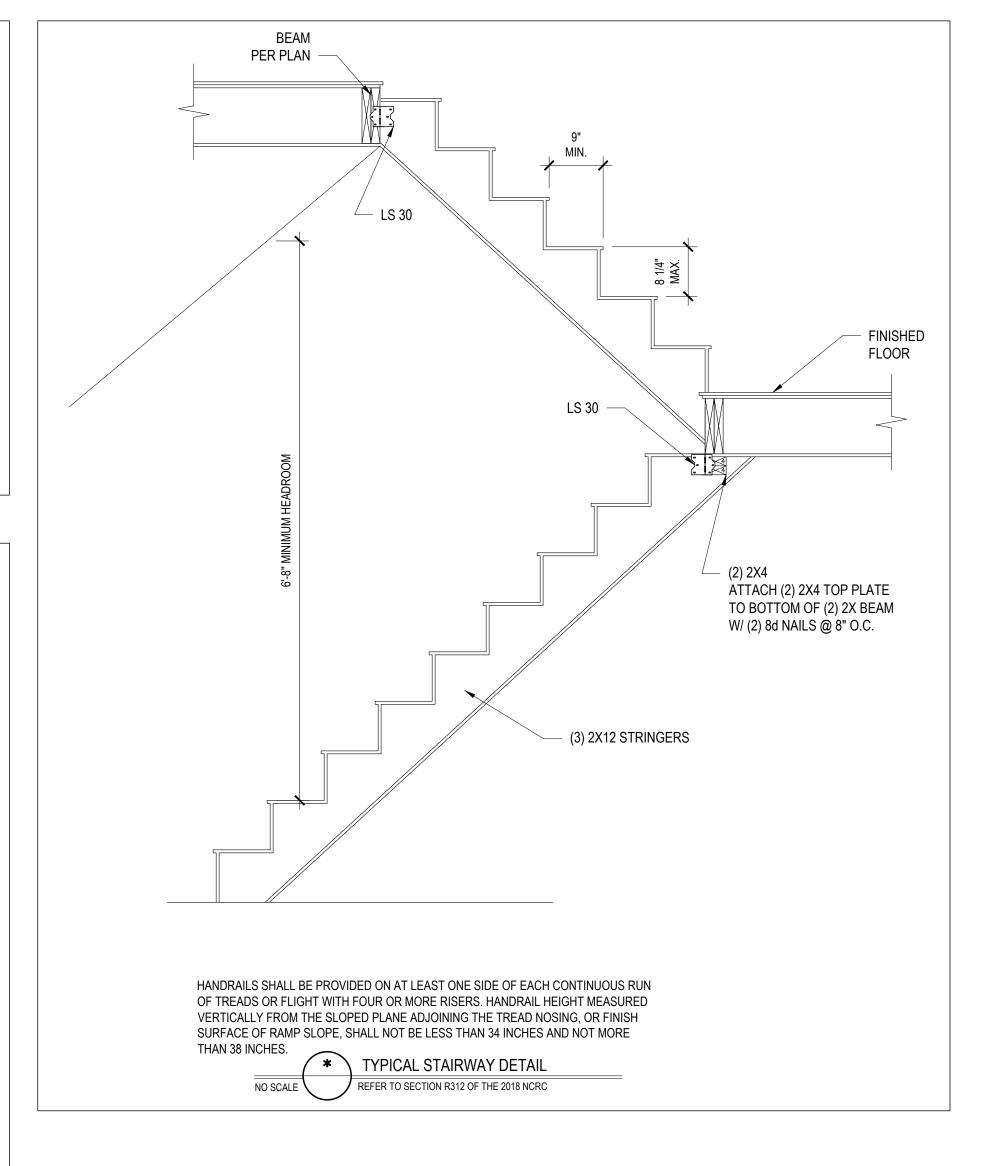
POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

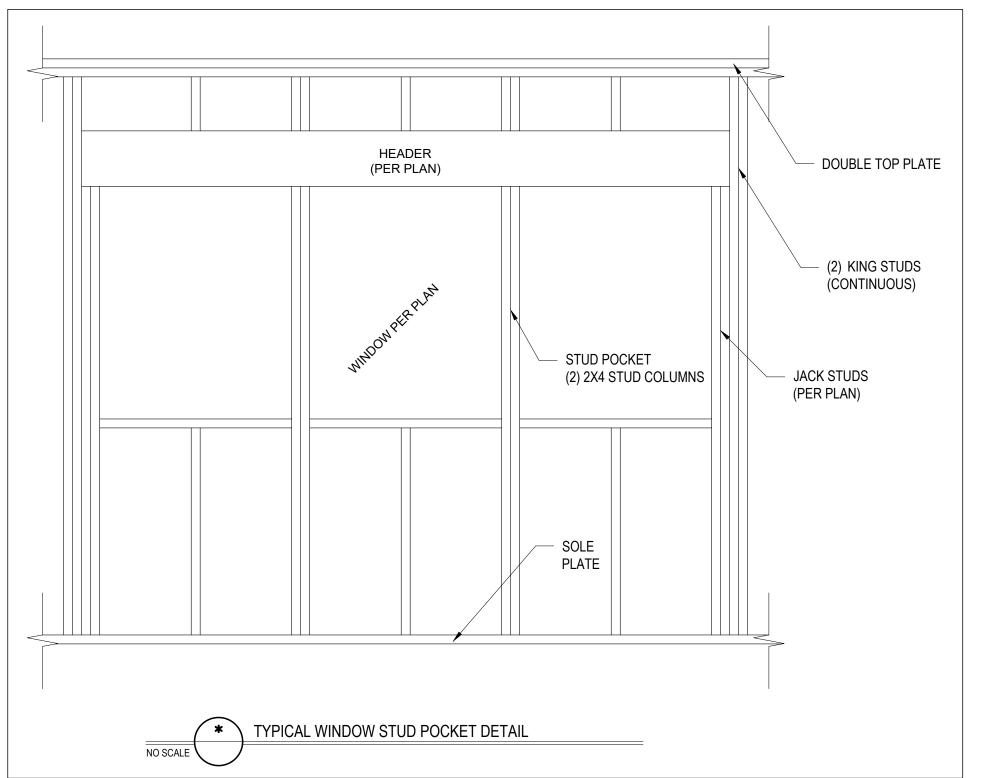
- * THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET
- WHICH MAY BE LOCATED AT DIFFERENT LEVELS. ** FROM TOP OF FOOTING TO BOTTOM OF GIRDER
- DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- 2) DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF
- THESE METHODS:
- A. THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4)
- ABOVE. LATERAL BRACING IS NOT REQUIRED. B. 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN
- 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED BOLT AT EACH END OF THE BRACE.
- C. FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER	
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"	
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"	

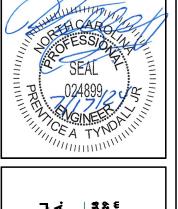
D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS.

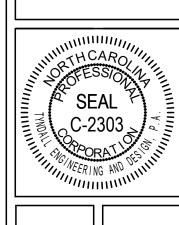
THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8"Ø HOT DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER. FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.









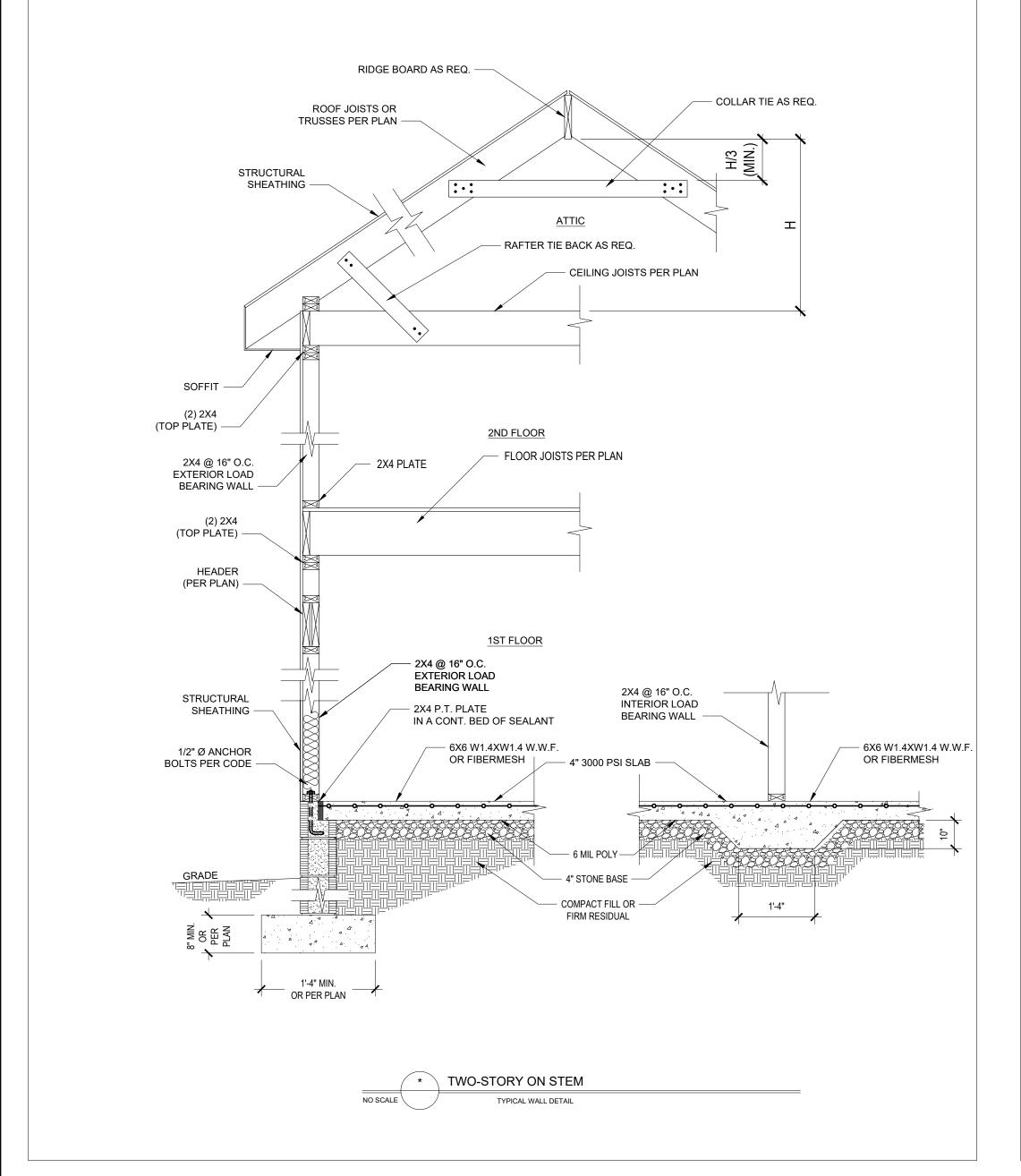


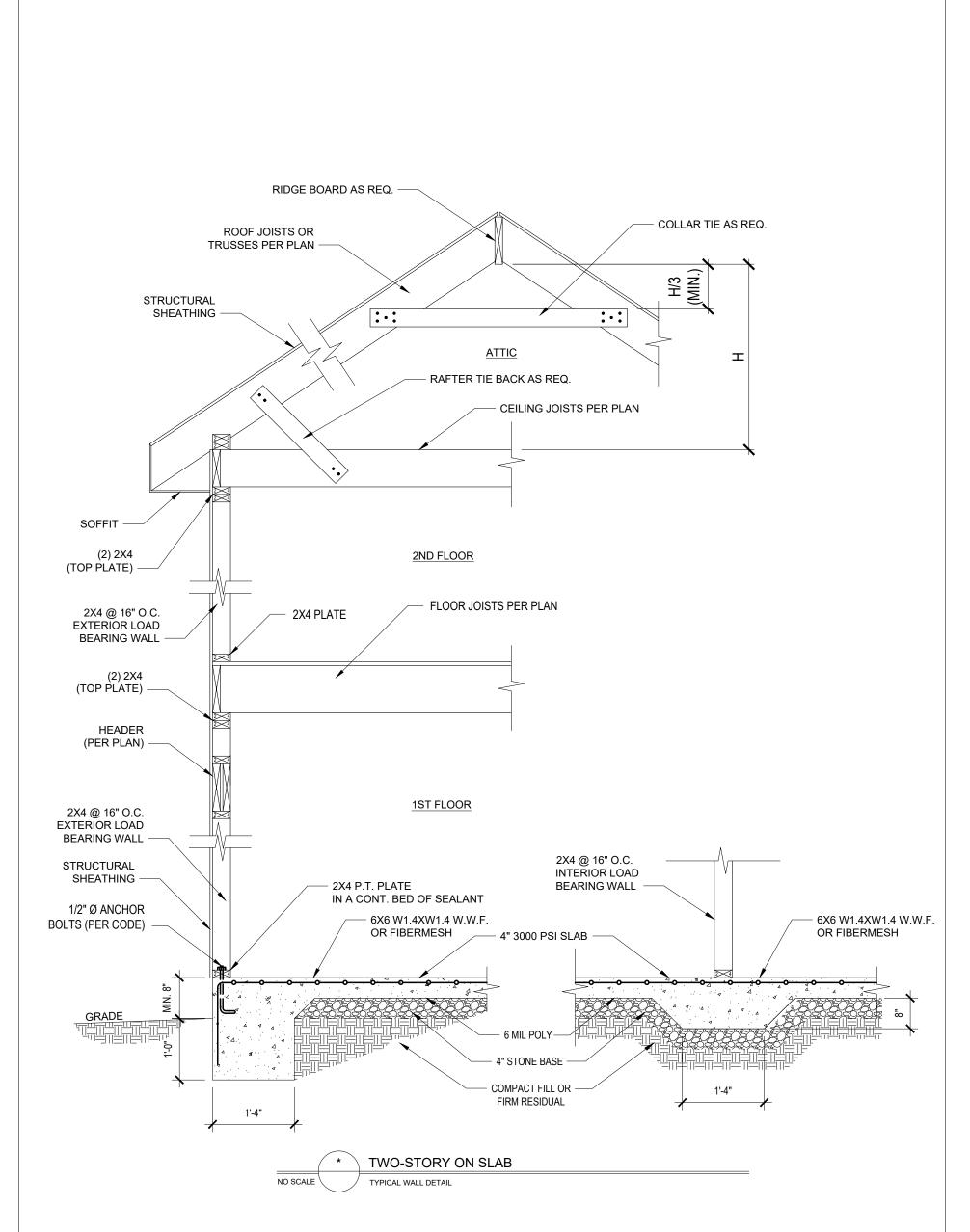
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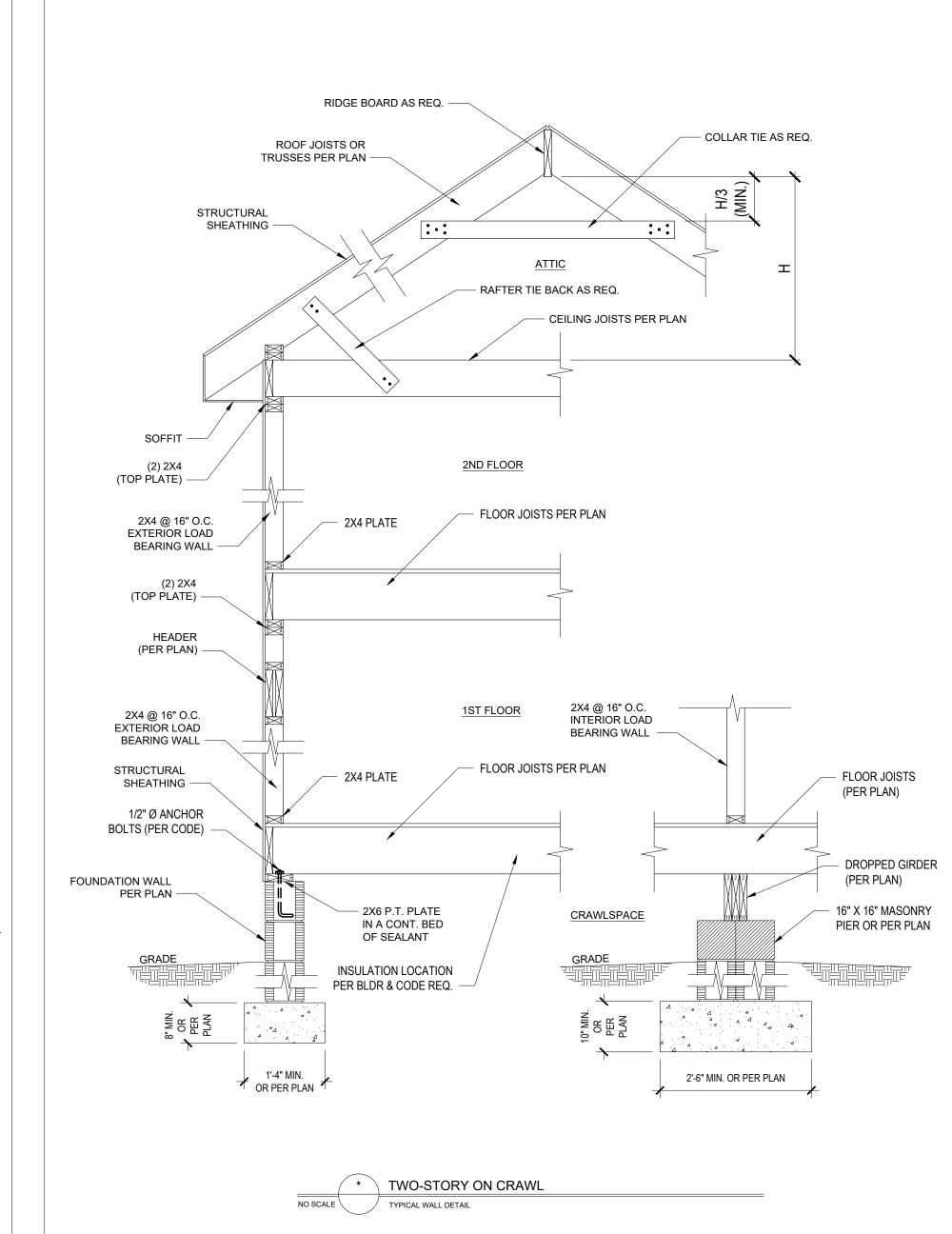
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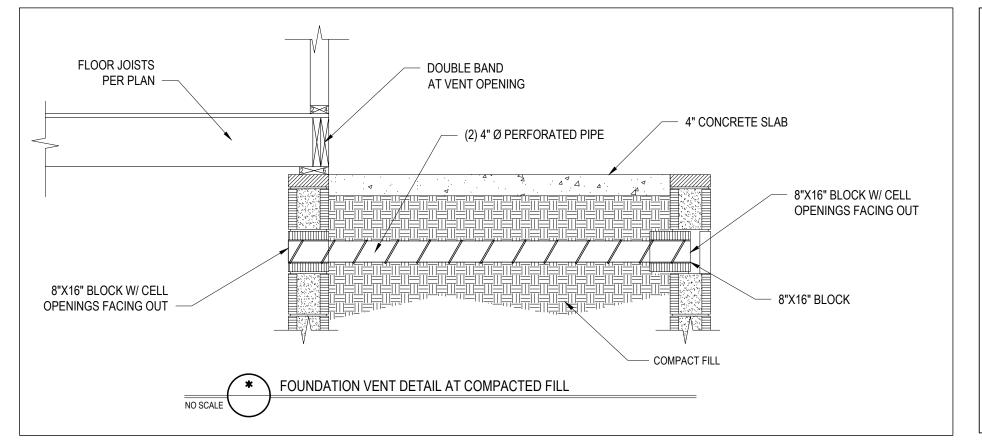
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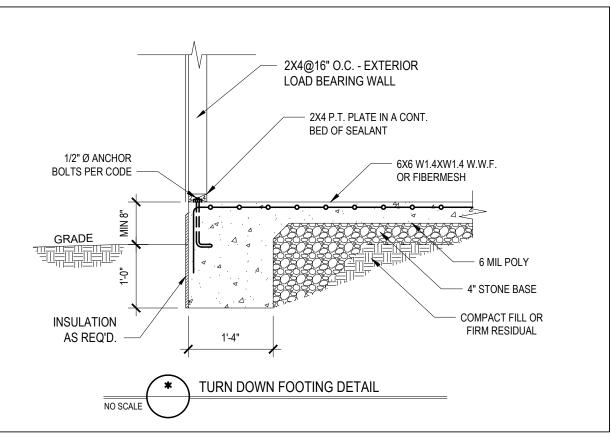
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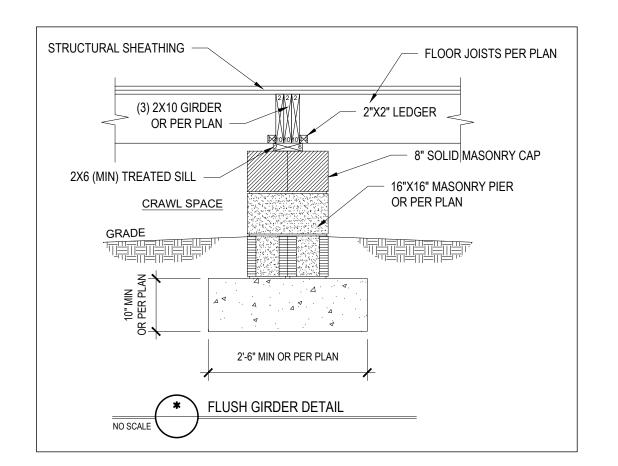


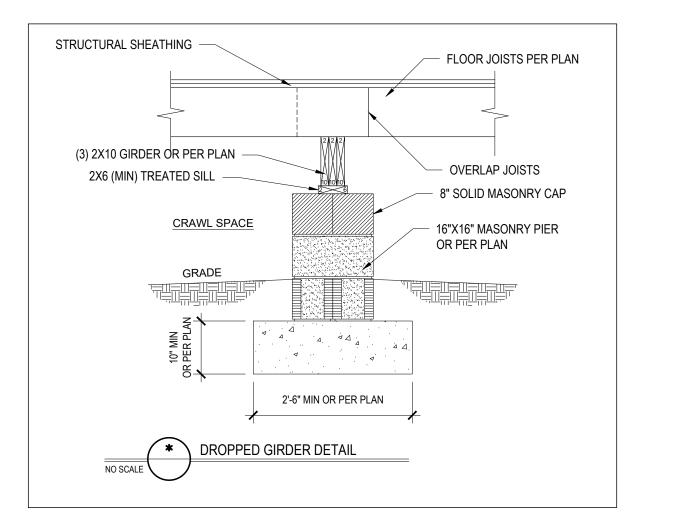


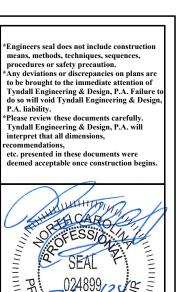


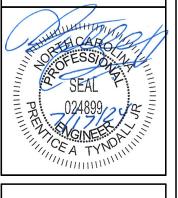


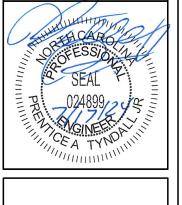


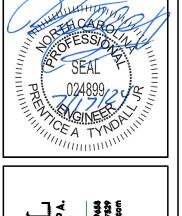


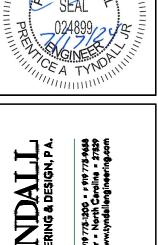
















William Seria	
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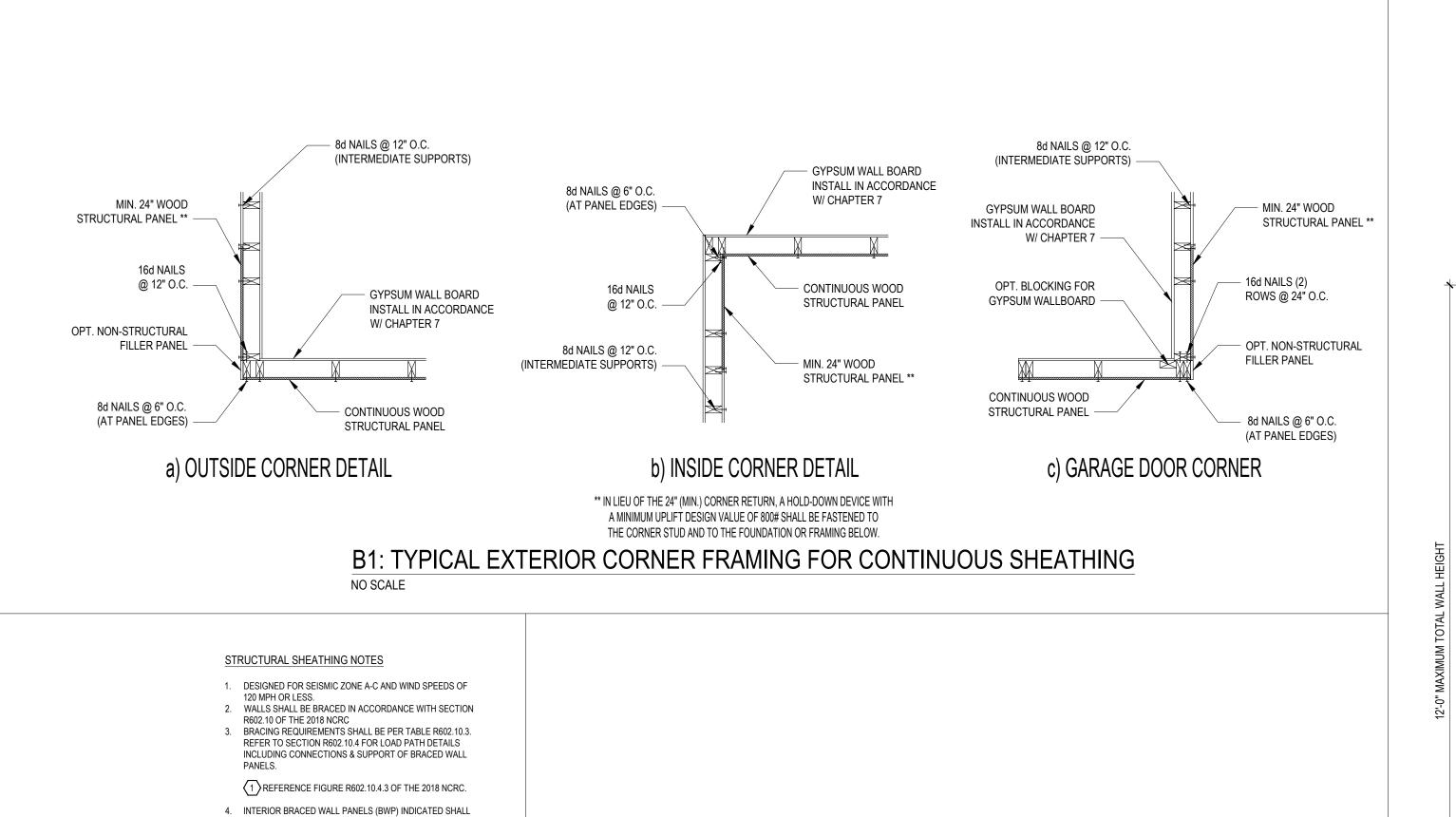
RIVER WILD 114 W. MAIN ST. CLAYTON, NC, 27520	,	THE FRANKLIN GARAGE RIGHT
	Plan:	

STANDARD DETAILS

Project#: DRB2301-0476B
Date: 05/13/2024
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REQUIRED BRACED WALL PANEL CONNECTIONS REQUIRED CONNECTION MATERIAL @ INTERMEDIATE SUPPORTS METHOD MIN. THICKNESS @ PANEL EDGES WOOD STRUCTURAL 6d COMMON NAILS 6d COMMON NAILS CS-WSP PANEL @ 12" O.C. @ 6" O.C. 5d COOLER NAIL** 5d COOLER NAIL** GYPSUM BOARD @ 7" O.C. @ 7" O.C. WOOD STRUCTURAL 6d COMMON NAILS 6d COMMON NAILS @ 12" O.C. @ 6" O.C.

**OR EQUIVALENT PER TABLE R702.3.5

B3: BRACE WALL PANEL CONNECTIONS

NO SCALE

BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR

WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)

2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS

3 3/8" WOOD STRUCTURAL PANEL)WSP) SECURE W/ 6d

AND 12" O.C. AT INTERMEDIATE SUPPORTS

CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)

GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED

WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6

O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT

'. MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP

- 24" ADJACENT TO OPENINGS NOT MORE THAN 67%

- 48" FOR OPENINGS GREATER THAN 85% OF WALL

PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH

ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH

WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE

CLOSEST TO THE CORNER AND TO THE FOUNDATION OR

FASTENED TO THE EDGE OF THE BRACED WALL PANEL

FIGURE R602.10.3 (4). IN LIEU OF A CORNER RETURN, EITHER A MINIMUM 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE

- 30" ADJACENT TO OPENINGS GREATER THAN 67% AND

INTERMEDIATE SUPPORTS.

OF WALL HEIGHT

METHOD SHALL BE AS FOLLOWS:

LESS THAN 85% OF WALL HEIGHT

 $\overline{\langle 4 \rangle}$ SHEATH INTERIOR AND EXTERIOR

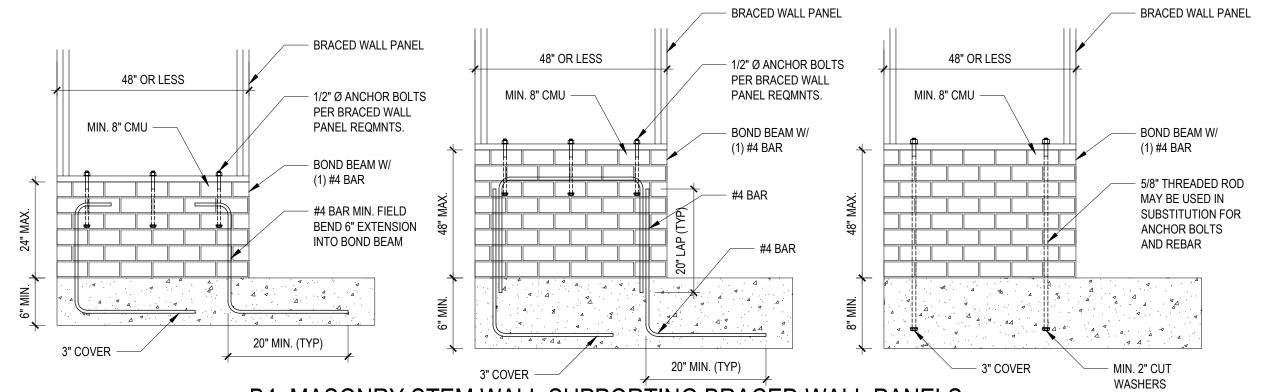
5 MINIMUM 800# HOLD-DOWN DEVICE

8. FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL

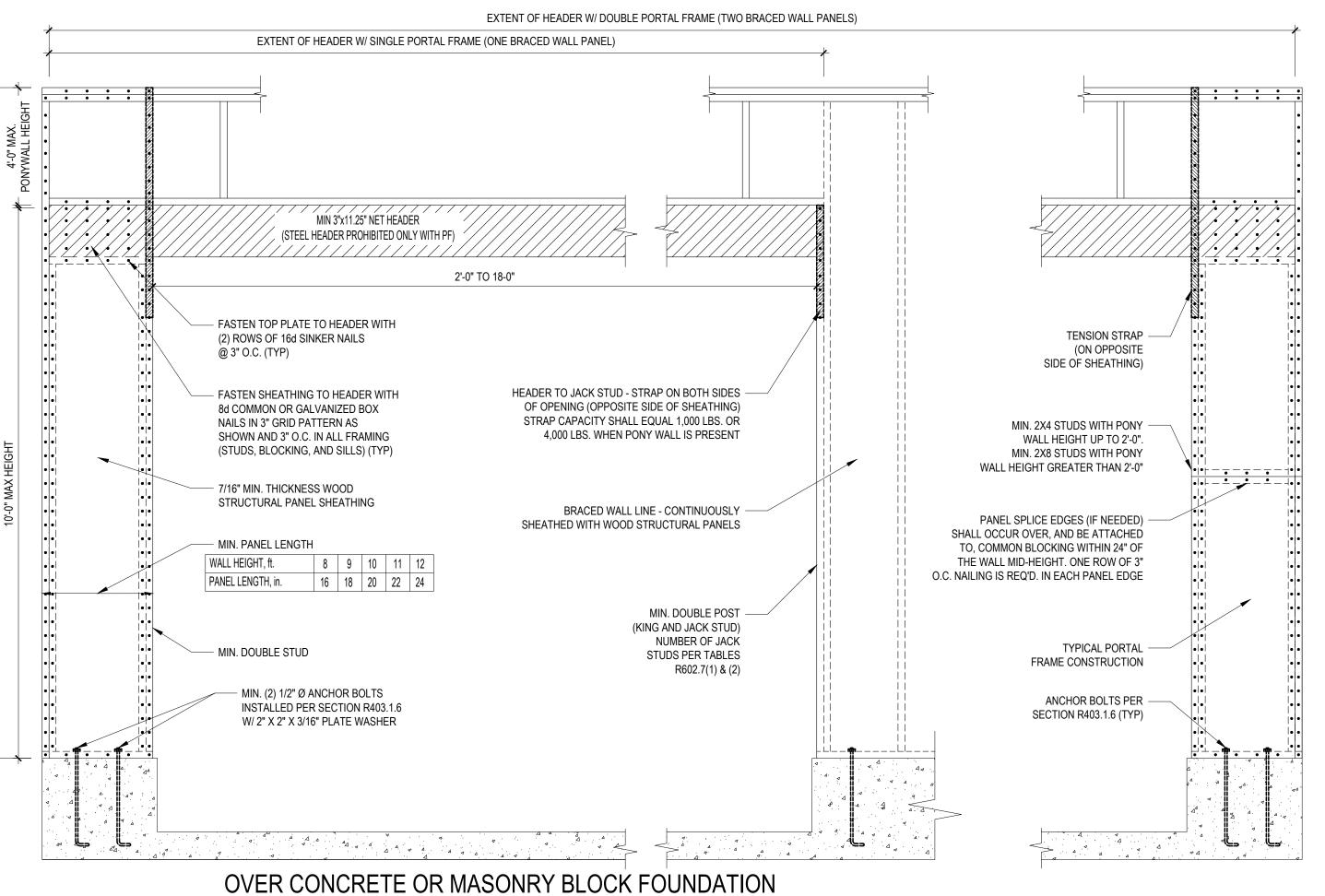
5. EXTERIOR BRACED WALL PANELS (BWP) SHALL BE

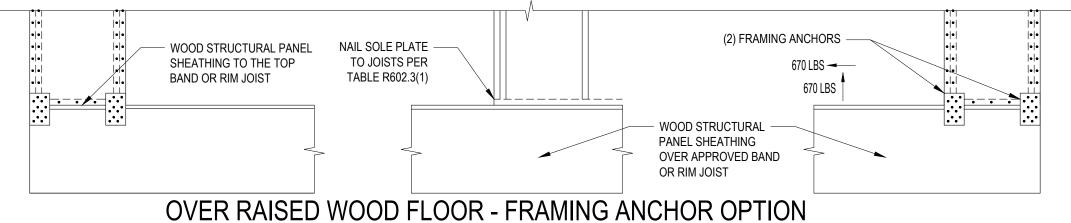
6. ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND

COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES



B4: MASONRY STEM WALL SUPPORTING BRACED WALL PANELS FIGURE R602.10.4.3 OF THE 2018 NCRC NOTE: GROUT BOND BEAMS AND ALL CELLS WHICH CONTAIN REBAR, THREADED RODS AND ANCHOR BOLTS





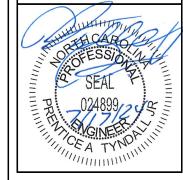
(WHEN PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST) ATTACH SHEATHING TO BAND NAIL SOLE PLATE WOOD STRUCTURAL PANEL OR RIM JOIST WITH 8d COMMON TO JOISTS PER SHEATHING TO THE TOP

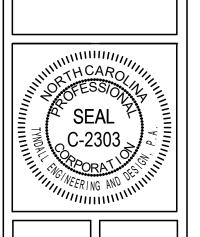
NAILS 3" O.C. TOP AND BOTTOM BAND OR RIM JOIST TABLE R602.3(1) WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST

> OVER RAISED WOOD FLOOR - OVERLAP OPTION (WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

B2: METHOD PF: PORTAL FRAME CONSTRUCTION FIGURE R602.10.1

ocedures or safety precaution. Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure do so will void Tyndall Engineering & Desig Please review these documents carefully Tyndall Engineering & Design, P.A. will interpret that all dimensions, etc. presented in these documents were





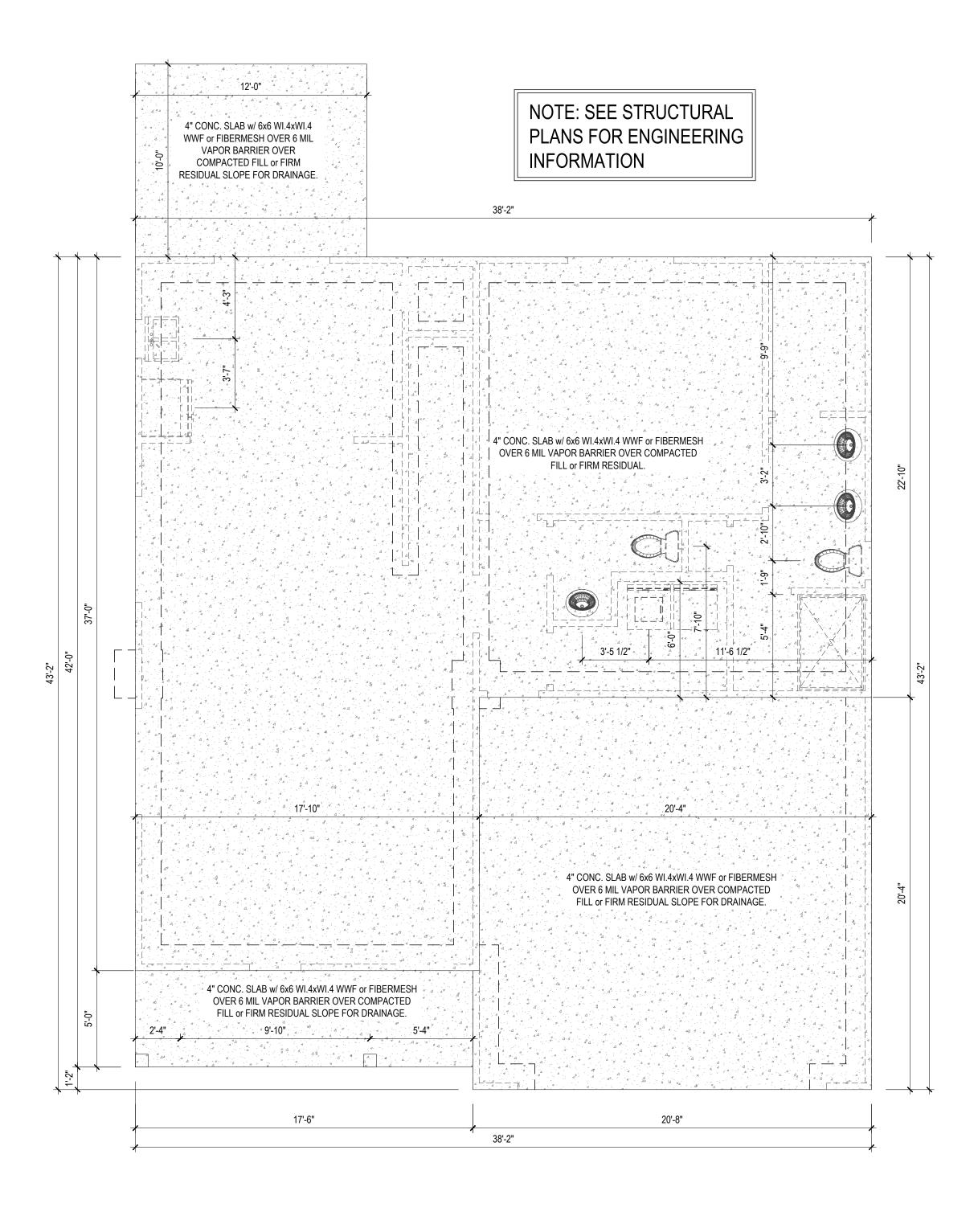
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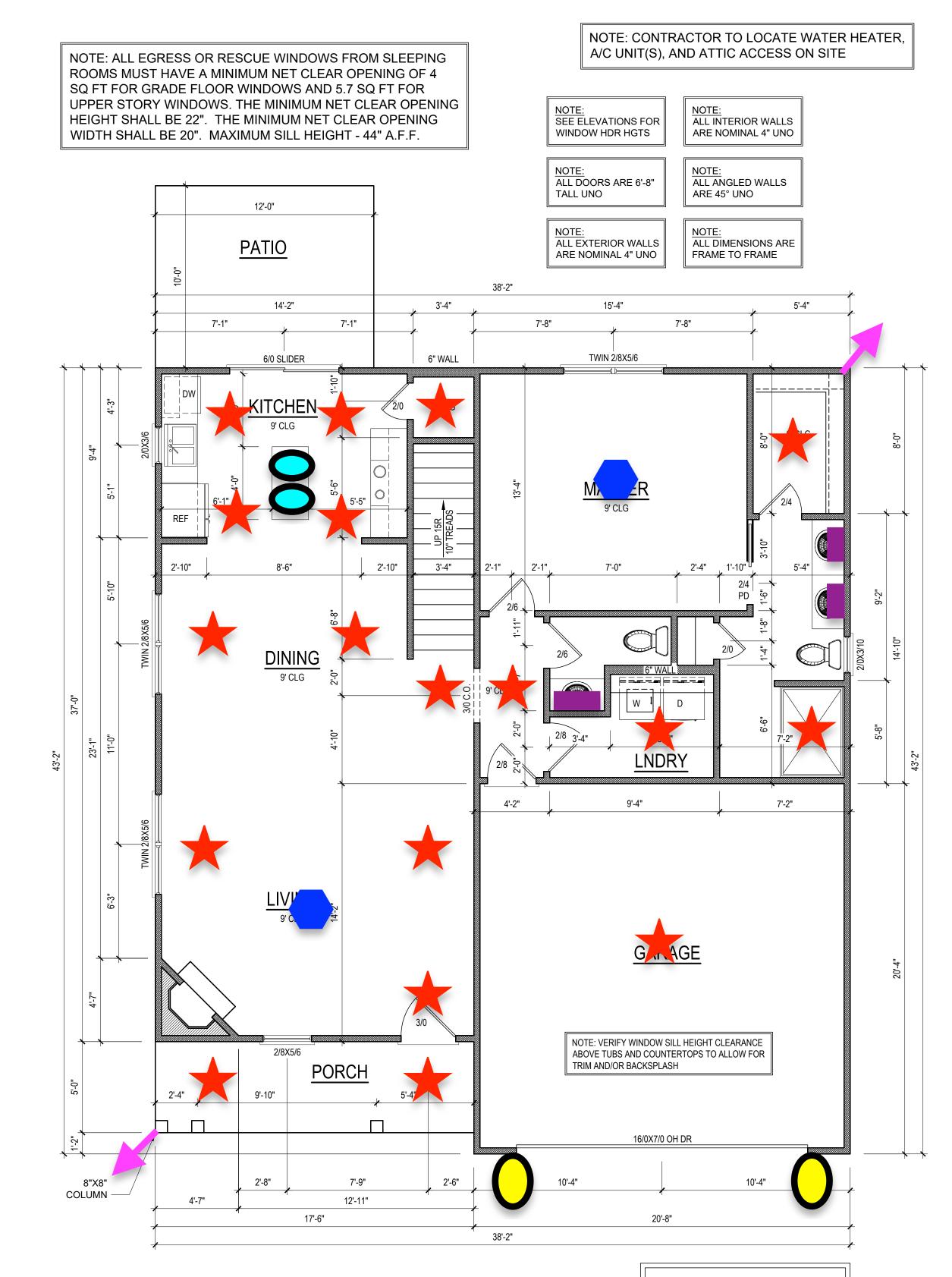
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DRB2301-0476B 05/13/2024 Engineered By: DWG. Checked By: AM

SEE PLAN REVISIONS Date:

Sheet Number





FOUNDATION PLAN - A&B 1/4" = 1'-0" **MONOLITHIC SLAB**

- DRB DESIGN assumes no liability for any home constructed from this plan.
- All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.
- Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required. Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.
- 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- Communication is imperfect and every contingency cannot be anticipated.
- Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.
- 8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences. Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB
- DESIGN of responsibility for any and all consequences arriving out of such changes. 10. Written dimensions on these plans always have precedence over scaled dimensions. 11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square
- footage errors once construction has begun. 12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.



FIRST FLOOR PLAN - A&B Flush Mount Fixture **Exterior Wall Mount** Flood Light

HEATED SQUARE FOOTAGE First Floor 1124 Second Floor 547 TOTAL HEATED 1671 UNHTD SQUARE FOOTAGE Front Porch 120 Patio 624 TOTAL UNHEATED 2295 TOTAL SQ FT Optional Rec

DRB2301-0476 B

05/03/2024

MMB

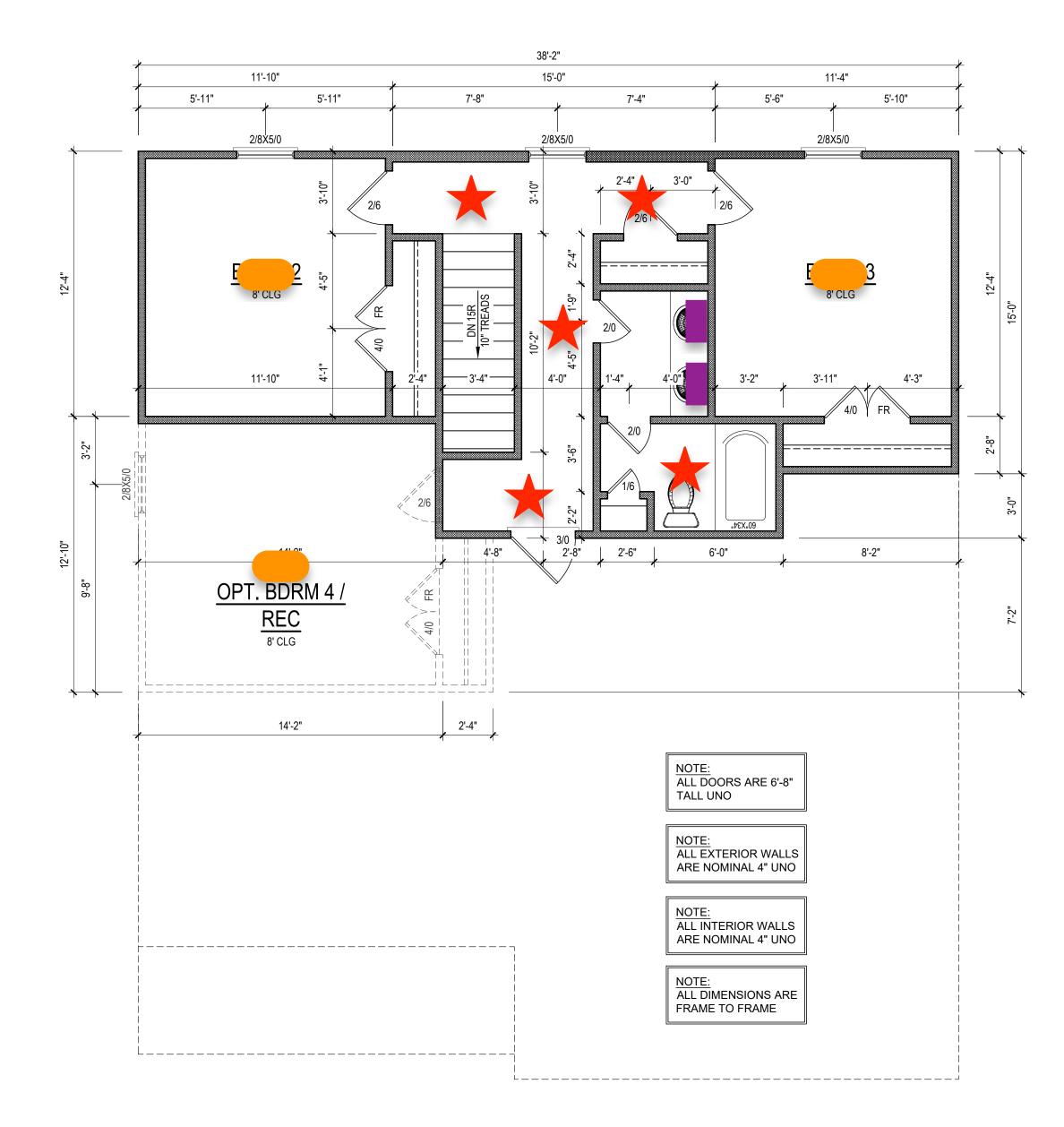
SCALE

CHECKED BY

1/4" = 1'-0"

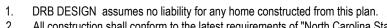
DRAWN/DESIGNED BY

SHEET NAME FND_1ST FL



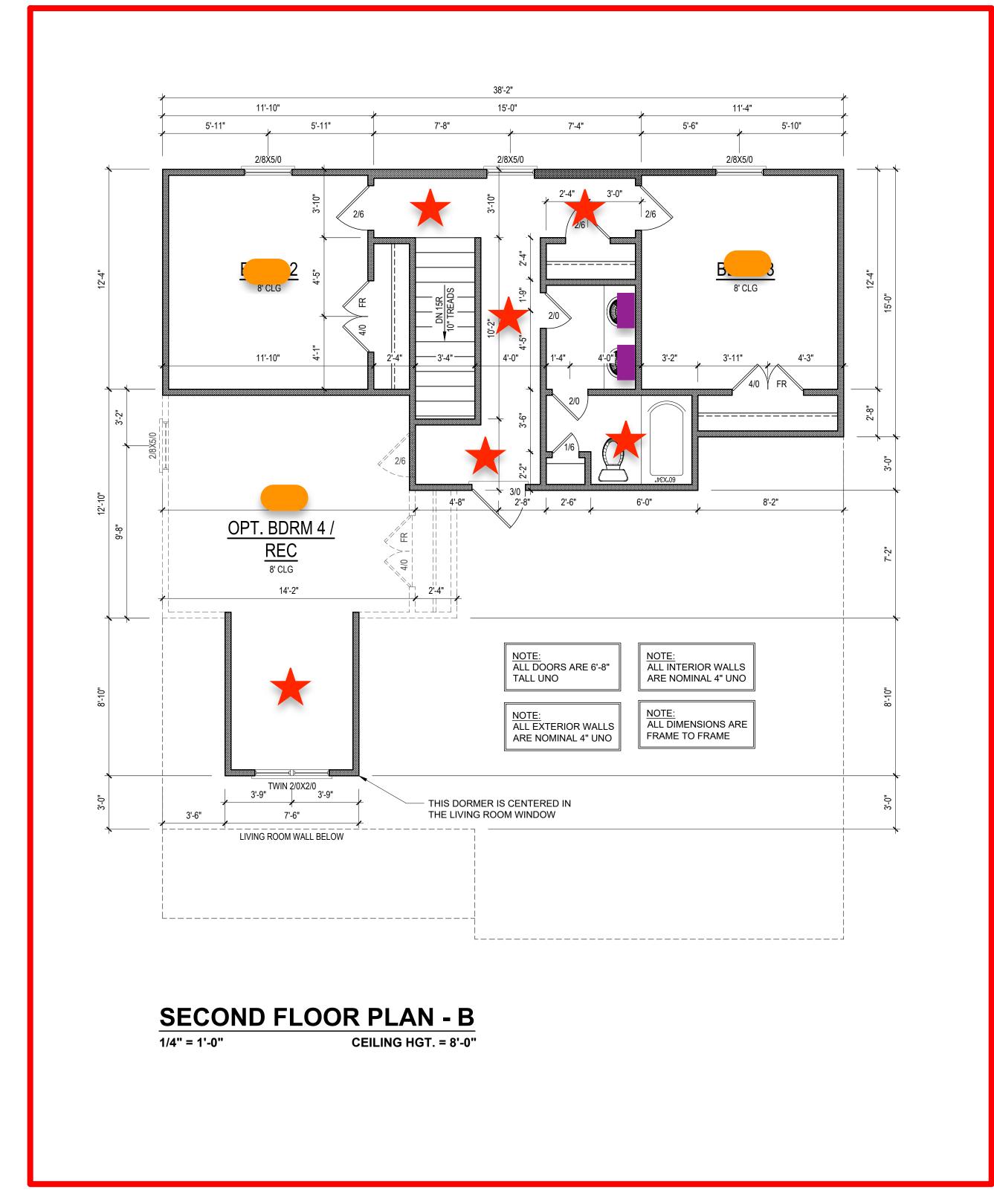
SECOND FLOOR PLAN - A

CEILING HGT. = 8'-0" 1/4" = 1'-0"



- 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code", in addition to all local codes and regulations.
- 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- 4. Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN. 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- Communication is imperfect and every contingency cannot be anticipated.
 Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB
- DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.
- 8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences.
- 9. Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB DESIGN of responsibility for any and all consequences arriving out of such changes.
- 10. Written dimensions on these plans always have precedence over scaled dimensions. 11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to
- footage errors once construction has begun. 12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

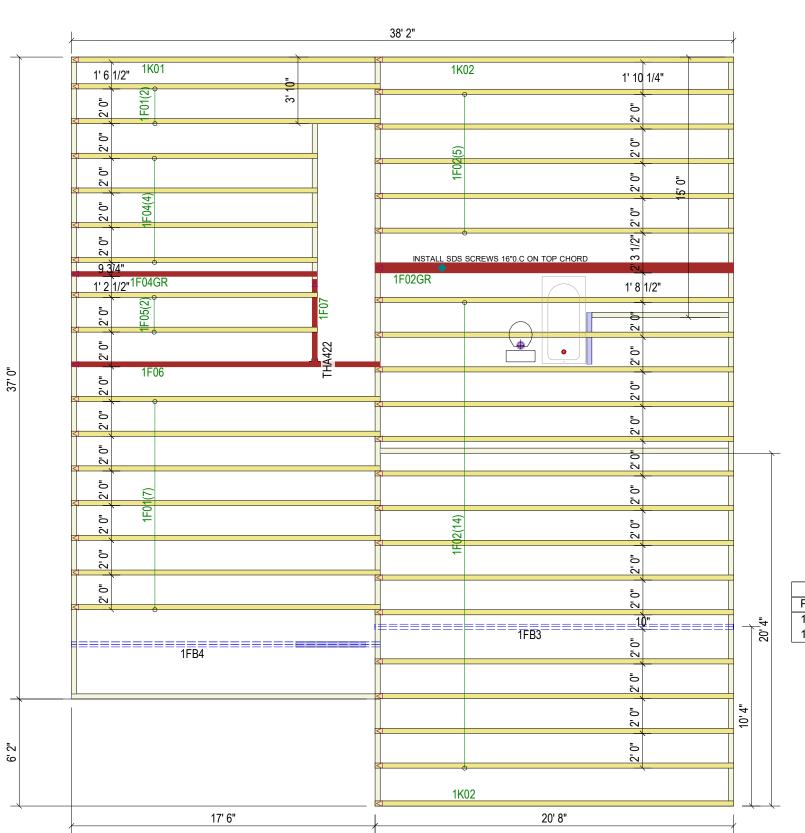
construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square



<u>PROJECT #</u> DRB2301-0476_B 05/03/2024

SHEET NAME

2ND_FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
1FB3	21' 0"	1 3/4" x 14" 2.0E Microllam® LVL	2	2
1FB4	18' 0"	1 3/4" x 14" 2.0E Microllam® LVL	2	2

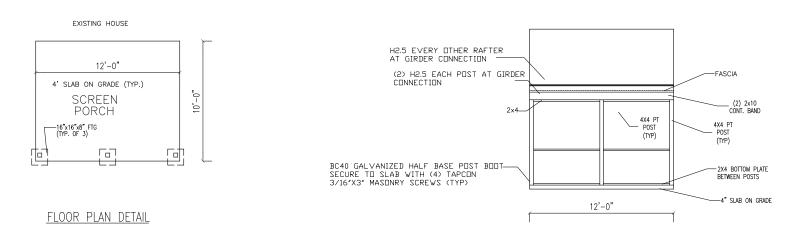
Truss Cor	nnector Tota	ıl List
Manuf	Product	Qty
Simpson	THA422	1

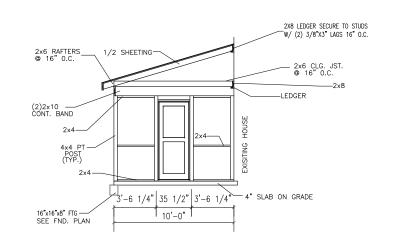
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THIS IS A TRUSS PLACEMENT DIAGRAM ONLY	
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SHOP DRAWING APPROVAL

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the root and floor system and for the overall structure. The design of the truss is structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult "Bracing of Wood Trusses" available from the Truss of the building designer.

Plan: FRANKLIN Backgrows. RIVERWILD Customer: RIVERWILD Site Address: Street Address: Street Address: Street Address: Street Address: Street Book of the roof and floor system and for the overall structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been structure. The design of the truss support have been supported by the trust of the politic structure. The design of the trust support have been supported by the trust supported
Designer: TW





24"X36" = 1/4"=1'-0' 11"X17" = 1/8"=1'-0'

Screen Porch Detail FILE

DESIGN

DRAWN ADS CHECKED

DATE 1/15/2013 SHEET