

MODEL HANOVER-LH

Cross Creek I ot 27 Spring Lake, NC 28390

NO: DATE: REVISION: /ı\ | 01.28.25 INITIAL RELEASE <u>2</u> 02.17.25 ADDED FOUNDATIONS FRONT DOOR REV. 05.02.25 4 05.16.25 CLIENT REVISIONS

PROFESSIONAL SEAL:

PROJECT TITLE:

PLAN CHANGES: 40' Series DATE: DESCRIPTION: INITIAL RELEASE OF PLANS *0*1.28.25 02.17.25 ADDED FOUNDATIONS

CONSULTANTS:

BUILDER: LOCAL JURISDICTION:

STRUCTURAL ENGINEER: DESIGNER:

CLIENTS NAME:

GENERAL NOTES DESIGNER NORTH CAROLINA:

THESE DOCUMENTS ARE THE PROPERTY OF THE DESIGNER AND SHALL NOT BE COPIED. PROVIDE BLOCKING AND/OR BACKING AT ALL TOWEL BAR. TOWEL RING AND/OR DUPLICATED, ALTERED, MODIFIED OR REVISED IN ANY WAY WITHOUT THE EXPRESSED WRITTEN APPROVAL OF THE DESIGNER.

CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE AND ALL INCONSISTENCES SHALL BE BROUGHT TO THE ATTENTION OF THE DEVELOPER AND THE DESIGNER BEFORE PROCEEDING WITH WORK.

ANY ERRORS OR OMISSIONS FOUND IN THESE DRAWINGS SHALL BE BROUGHT TO DEVELOPERS AND DESIGNERS ATTENTION IMMEDIATELY.

DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED

ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS OTHERWISE NOTED.

ALL TRUSS DRAWINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO ISSUANCE OF BUILDING PERMIT.

ALL OR EQUAL SUBSTITUTIONS MUST BE SUBMITTED TO AND APPROVED BY CITY BUILDING OFFICIAL PRIOR TO INSTALLATION.

ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO

ALL ANGLED PARTITIONS ARE 45 DEGREES UNLESS OTHERWISE NOTED. PROVIDE FIREBLOCKING. (PER LOCAL CODES.)

RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY

TOILET PAPER HOLDER LOCATIONS, AS SHOWN PER PLAN. TYPICAL AT ALL BATHROOMS AND POWDER ROOMS. VERIFY LOCATIONS AT FRAMING WALK.

ELASTOMERIC SHEET WATERPROOFING: FURNISH AND INSTALL ALL WATERPROOFING COMPLETE. A 40 MIL. SELF-ADHERING MEMBRANE OF RUBBERIZED ASPHALT INTEGRALLY BONDED TO POLYETHYLENE SHEETING, OR EQUAL. INSTALL PER MANUFACTURE'S AND TRADE ASSOCIATION'S PRINTED INSTALLATION INSTRUCTIONS. 6" MINIMUM LAP AT ALL ADJACENT WALL SURFACES.

TO THE BEST OF THE DESIGNER'S KNOWLEDGE THESE DOCUMENTS ARE IN CONFORMANCE WITH THE REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY.

SHOP DRAWING REVIEW AND DISTRIBUSTION, ALONG WITH PRODUCT SUBMITTALS, REQUESTED IN THE CONSTRUCTION DOCUMENTS, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR, UNLESS DIRECTED OTHERWISE UNDER A SEPARATE AGREEMENT.

DEVIATIONS FROM THESE DOCUMENTS IN THE CONSTRUCTION PHASE SHALL BE REVIEWED BY THE DESIGNER AND THE OWNER PRIOR TO THE START OF WORK IN QUESTION. ANY DEVIATIONS FROM THESE DOCUMENTS WITHOUT PRIOR REVIEW, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS REPRESENTED ON THESE DOCUMENTS INCLUDING THE WORK AND MATERIALS FURNISHED BY SUBCONTRACTORS AND VENDORS.

GEOTECHNICAL ENGINEER (SOILS REPORT), ON THE STUDY OF THE PROPOSED SITE, TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR. IN THE EVENT THE GEOTECHNICAL REPORTS DO NOT EXIST, THE SOILS CONDITION SHALL BE ASSUMED TO BE A MINIMUM DESIGN SOIL PRESSURE STATED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PURPOSE OF STRUCTURAL DESIGN. GENERAL CONTRACTOR SHALL ASSURE THE SOIL CONDITIONS MEET OR EXCEED

THE BUILDER SHALL FURNISH ANY AND ALL REPORTS RECEIVED FROM THE

ALL WORK PERFORMED BY THE GENERAL CONTRACTOR SHALL COMPLY AND CONFORM WITH LOCAL AND STATE BUILDING CODES, ORDINANCES AND REGULATIONS, ALONG WITH ALL OTHER AUTHORITIES HAVING JURISDICTION. THE GENERAL CONTROATOR IS RESPONSIBLE TO BE AWARE OF THESE REQUIREMENTS

PROVIDE AN APPROVED WASHER DRAIN PAN AT SECOND FLOOR ONLY THAT DRAINS TO EXTERIOR.

AND GOVERNING REGULATIONS

LOCAL CODES.

WINDOW SUPPLIER TO VERIFY AT LEAST ONE WINDOW IN ALL BEDROOMS TO HAVE A CLEAR OPENABLE AREA OF 4.0 SQ FT. THE MINIMUM NET CLEAR OPENING HEIGHT SHALL BE 22" AND THE MINIMUM NET CLEAR OPENING WIDTH SHALL BE 20". GLAZING TOTAL AREA OF NOT LESS THAN 5.0 SQ FT IN THE CASE OF A GROUND WINDOW AND NOT LESS THAN 5.7 SQ FT IN THE CASE OF AN UPPER STORY WINDOW. (PER NCRC SECTION R310.1.1)

ALL HANDRAIL BALLUSTERS TO BE SPACED SUCH THAT A 4" SPHERE CANNOT PASS BETWEEN BALLUSTERS. (PER LOCAL CODES.) PROVIDE STAIR HANDRAILS AND GUARDRAILS PER

BUILDER SET:

THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BUILDER'S SET" OF CONSTRUCTION DOCUMENTS AND GENERAL NOTES HEREINAFTER REFERRED TO AS "PLANS". THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT; HOWEVER, ALL MATERIALS AND METHODS OF CONSTRUCTION NECESSARY TO COMPLETE THE PROJECT ARE NOT NECESSARILY DESCRIBED. THE PLANS DELINEATE AND DESCRIBE ONLY LOCATIONS, DIMENSIONS, TYPES OF MATERIALS, AND GENERAL METHODS OF ASSEMBLING OR FASTENING. THEY ARE NOT INTENDED TO SPECIFY PARTICULAR PRODUCTS OR OTHER METHODS OF ANY SPECIFIC MATERIALS, PRODUCT OR METHOD. THE IMPLEMENTATION OF THE PLANS REQUIRES A CLIENT / CONTRACTOR THOROUGHLY KNOWLEDGEABLE WITH THE APPLICABLE BUILDING CODES AND METHODS OF CONSTRUCTION SPECIFIC TO THIS PRODUCT TYPE AND TYPE OF CONSTRUCTION.

CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY; WHERE QUALITY LEVEL IS NOT INDICATED, PROVIDE WORK OF QUALITY CUSTOMARY IN SIMILAR TYPES OF WORK. WHERE THE PLANS AND SPECIFICATIONS, CODES, LAWS, REGULATIONS, MANUFACTURER'S RECOMMENDATIONS OR INDUSTRY STANDARDS REQUIRE WORK OF HIGHER QUALITY OR PERFORMANCE, PROVIDE WORK COMPLYING WITH THOSE REQUIREMENTS AND QUALITY. WHERE TWO OR MORE QUALITY PROVISIONS OF THOSE REQUIREMENTS CONFLICT WITH THE MOST STRINGENT REQUIREMENT; WHERE REQUIREMENTS ARE DIFFERENT BUT APPARENTLY EQUAL, AND WHERE IT IS UNCERTAIN WHICH REQUIREMENT IS MOST STRINGENT, OBTAIN CLARIFICATION FROM THE GMD DESIGN GROUP BEFORE PROCEEDING.

AREA CALCULATIONS:

MODEL 'HANOVER' SQUARE FOOTAGES

AKEA	ELEV A		
Ist FLOOR	1222 SF		
2nd FLOOR	1582 SF		
TOTAL LIVING	2804 SF		
GARAGE	415 SF		
PORCH	71 SF		
OPT. BASEMENT	II56 SF		
BASEMENT AREA IS TAKEN TO INSIDE OF CONCRETE WALL			

PROJECT NO: GMD17049

TITLE SHEET









Front Elevation 'D-2'

SCALE: 1/4"=1'-0" AT 22"X34" LAYOUT 1/8"=1'-0" AT 11"XIT" LAYOUT

NO: DATE: REVISION:

Ol.28.25 INITIAL RELEASE

O2.17.25 ADDED FOUNDATIONS

O5.02.25 FRONT DOOR REV.

O5.16.25 CLIENT REVISIONS

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTIO

CLIENTS NAME:



PROJECT NO: GMD17049

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TITLE SHEET

January 28, 2025

SHEET NO:

0.1

GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN. BUILDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS. - WINDOW HEAD HEIGHTS: IST FLOOR = 6'-8" U.N.O. ON ELEVATIONS. 2ND FLOOR = 7'-0" U.N.O. ON ELEVATIONS. - ROOFING: PITCHED SHINGLES PER DEVELOPER. · WINDOWS: MANUFACTURER PER DEVELOPER. DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS - ENTRY DOOR: AS SELECTED BY DEVELOPER. GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN. ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. PROTECTION AGAINST DECAY: (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLUDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.) INSULATION: PER TABLE NIIO2.I.2. R-15 BATTS MINIMUM. VERIFY EXTERIOR WALLS: CEILING WITH ATTIC ABOVE: R-38 BATTS MINIMUM. VERIFY FLOOR OVER GARAGE: R-19 BATTS MINIMUM. VERIFY ATTIC KNEEWALL: R-19 BATTS MINIMUM. VERIFY CRAWL SPACE FLOORING: R-19 BATTS MINIMUM. VERIFY KEY NOTES: MASONRY: ADHERED STONE VENEER AS SELECTED BY DEVELOPER. HEIGHT AS NOTED. 2 MASONRY FULL BRICK AS SELECTED BY DEVELOPER. HEIGHT AS NOTED. 3 MASONRY FULL STONE AS SELECTED BY DEVELOPER. HEIGHT AS NOTED. 4 8" SOLDIER COURSE. 5 ROWLOCK COURSE 6 N/A TYPICALS: 7 CORROSION RESISTANT SCREEN LOUVERED VENTS, SIZE AS NOTED. 8 CODE APPROVED TERMINATION CHIMNEY CAP. GORROSION RESISTANT ROOF TO WALL FLASHING. CODE COMPLIANT FLASHING PER NCRC R905.2.8.3 O STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS. II DECORATIVE WROUGHT IRON, SEE DETAILS. 12 VINYL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT SHAKE SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.) 13 VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT LAP SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.) 14 VINYL WAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT WAYY SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.) 15 VINYL BOARD AND BATT SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT PANEL SIDING W/ IX3 BATTS AT 12" O.C. PER DEVELOPER W/ IX4 CORNER TRIM BOARD.) 16 VINYL TRIM SIZE AS NOTED (AT SPECIFIC LOCATIONS: IX FIBER CEMENT TRIM OR EQUAL, U.N.O. SIZE AS NOTED FYPON SHUTTERS, TYPE AS SHOWN. SIZE AS NOTED. $\frac{171}{1}$ (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN. SIZE AS NOTED.)

ALL WINDOWS WHOSE OPENING IS LESS THAN 24" ABOVE THE FINISH FLOOR AND WHOSE OPENING IS GREATER THAN 72" ABOVE THE OUTSIDE WALKING SURFACE MUST HAVE WINDOW OPENING LIMITING DEVICES COMPLYING WITH THE

NCRC SECTION R312.2.1 AND R312.2.2.



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PROFESSIONAL SEAL:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

'HANOVER' - LH **EXTERIOR ELEVATIONS** '4EGF-A'

ATTIC VENT CALCULATION FOR PLAN 'HANOVER': 1:150 RATIO.

THE NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/150 OF THE AREA OF THE SPACE VENTILATED, PROVIDED THAT AT LEAST 50 PERCENT AND NOT MORE THAN 80 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED AT LEAST 3 FEET ABOVE THE EAVE OR CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS.

EXCEPTIONS:

I. EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN I SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS SOFFIT VENTILATION ONLY.

2. ENCLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONLY.

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

(PER SECTION R806.2)

I SQUARE INCH VENT FOR EVERY 150 SQUARE INCHES OF CEILING *144 SQ. IN. = 1 SQ. FT.

BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / I50 = SQ. IN. OF VENT REQUIRED

ROOF AREA I:= 1637 SF 1637 SQ. FT. X 144 = 235728 SQ. IN. 235728 SQ. IN. / 150 = 1571.52 SQ. IN. OF VENT REQ'D

ROOF AREA 2:= 71 SF 71 SQ. FT. X 144 = 10224 SQ. IN. 10224 SQ. IN. / 150 = 68.16 SQ. IN. OF VENT REQ'D

- ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY.

- DASHED LINES INDICATE WALL BELOW. - LOCATE GUTTER AND DOWNSPOUTS PER BUILDER.

- PITCHED ROOFS AS NOTED.

- TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DRAWINGS TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT

FOR REVIEW PRIOR TO FABRICATIONS. - ALL PLUMBING VENTS SHALL BE COMBINED INTO A MINIMUM AMOUNT OF ROOF TO THE REAR OF THE MAIN RIDGE.

ATTIC VENT CALCULATION FOR PLAN 'HANOVER': 1:300 RATIO.

AS AN ALTERNATE TO THE 1/150 RATIO LISTED ABOVE, THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO 1/300 WHEN A CLASS I OR II VAPOR RETARDER IS INSTALLED ON THE WARM - IN - WINTER SIDE OF THE CEILING.

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

(PER SECTION R806.2)

I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING

*144 SQ. IN. = 1 SQ. FT.

BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / 300 = SQ. IN. OF VENT REQUIRED

SQ. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW.

REA I: =1637 SF

1637 SQ. FT. X 144 = 235728 SQ. IN.

235728 SQ. FT. / 300 = 785.76 SQ. IN. OF VENT REQ'D

785.76 SQ. IN. / 2 = 392.88 SQ. IN.

392.88 SQ. IN. OF VENT AT HIGH & 392.88 SQ. IN. OF VENT AT LOW REQUIRED.

ROOF AREA 2: =

TI SF

TI SQ. FT. X 144 = 10224 SQ. IN.

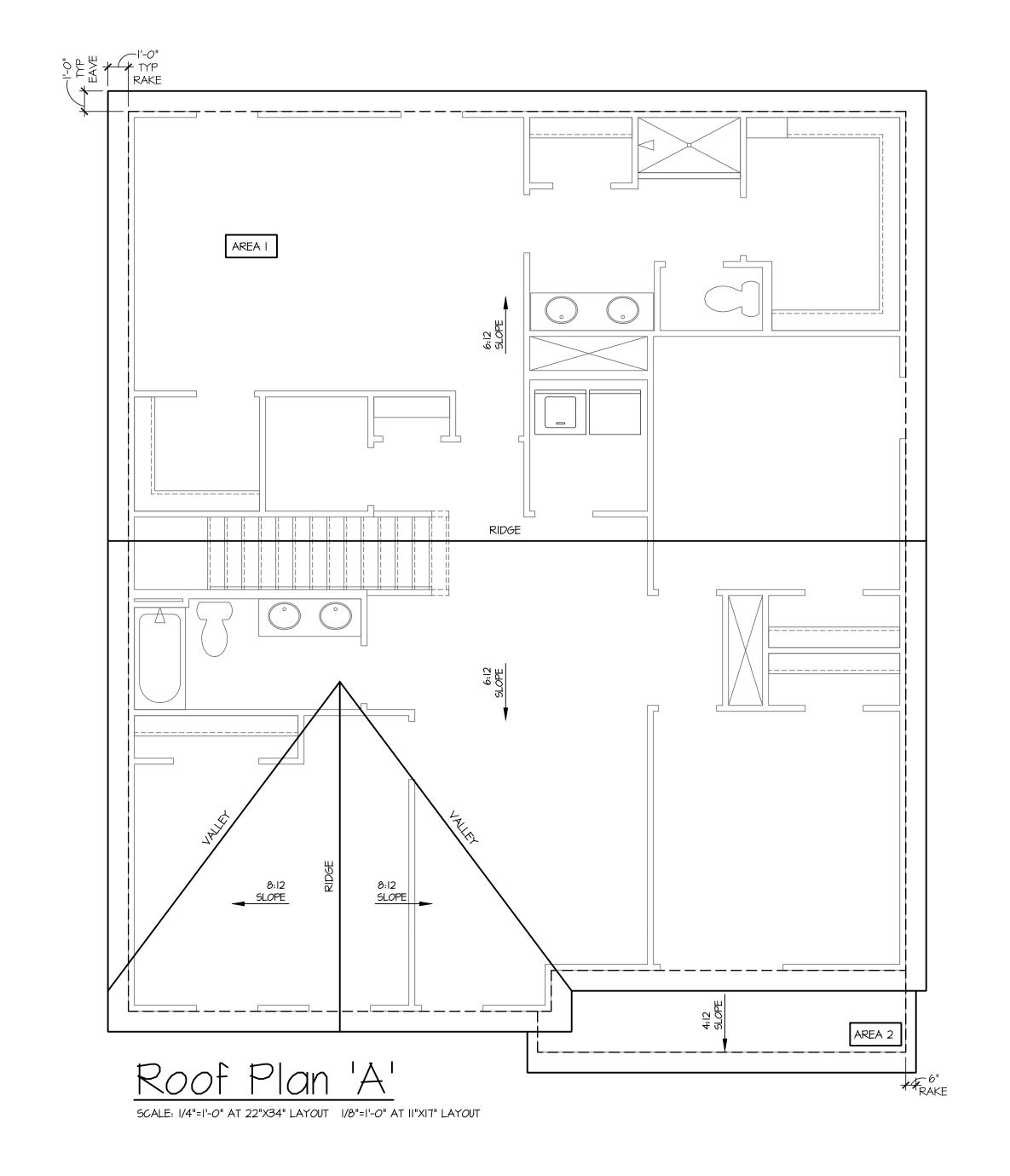
10224 SQ. FT. / 300 = 34.08 SQ. IN. OF VENT REQ'D

34.08 SQ. IN. / 2 = 17.04 SQ. IN.

17.04 SQ. IN. OF VENT AT HIGH & 17.04 SQ. IN. OF VENT AT LOW REQUIRED.

BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12

> AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)



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PROFESSIONAL SEAL:

40' Series

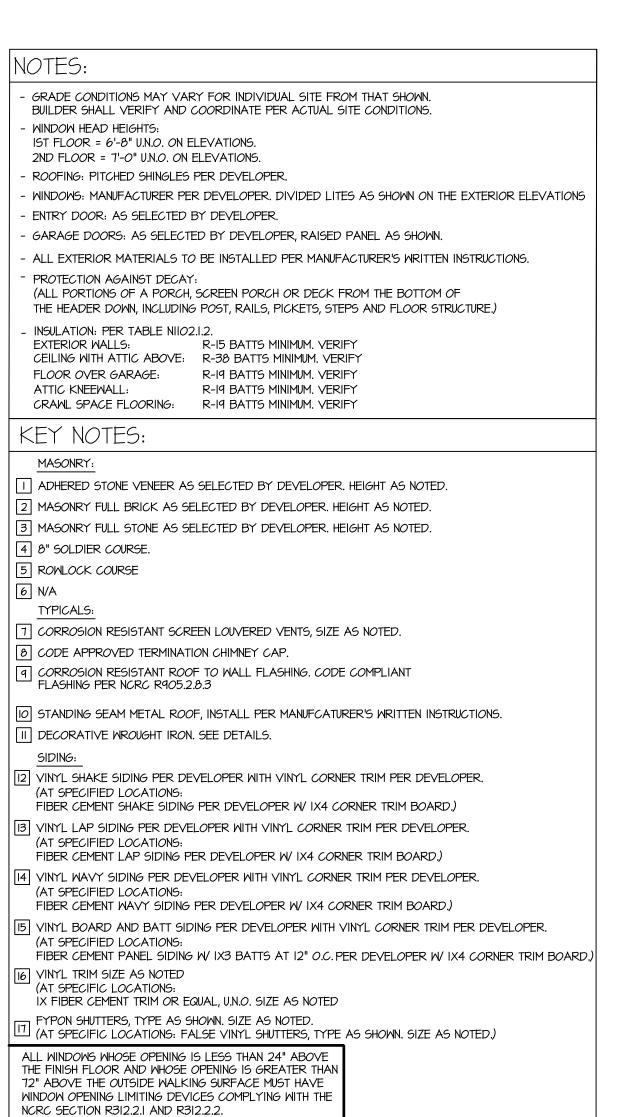
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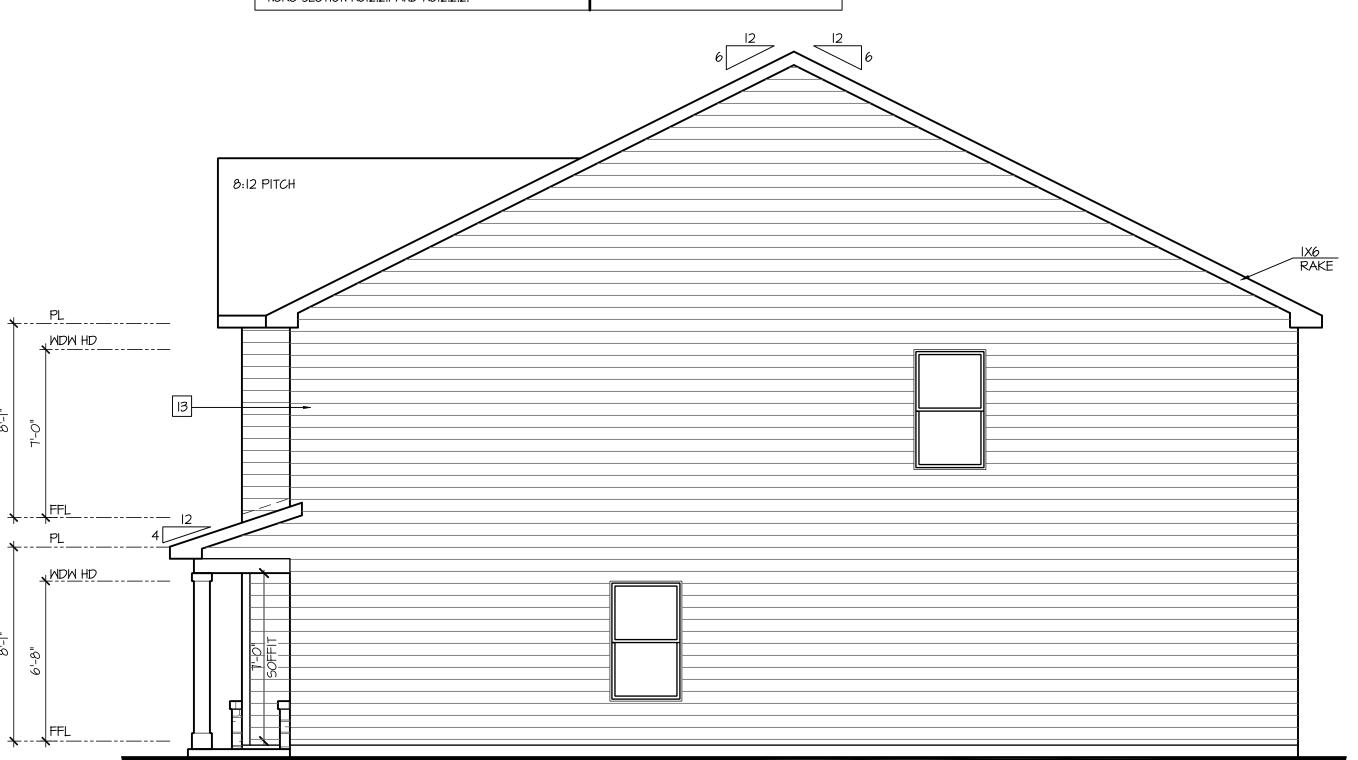
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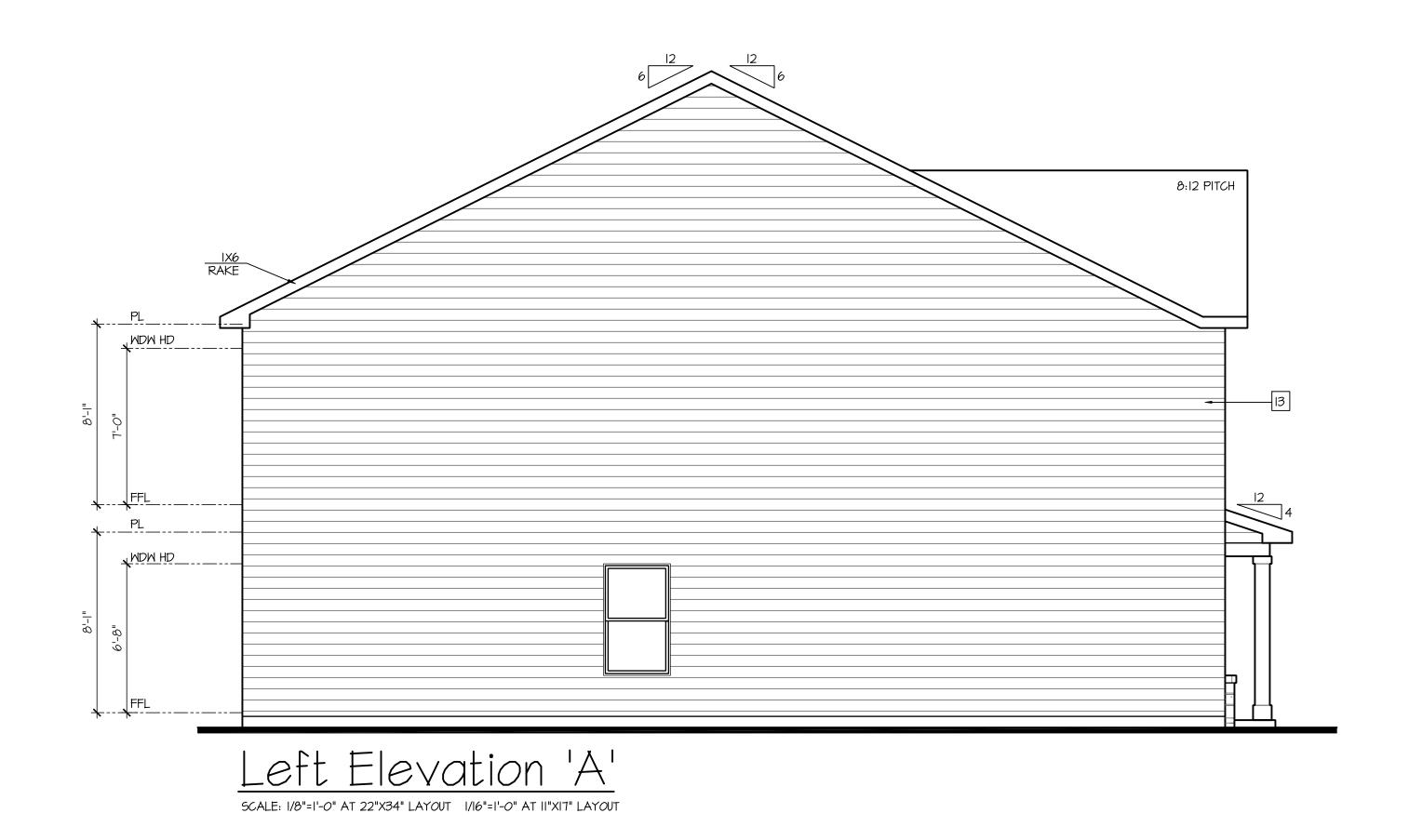
PROJECT NO: GMD17049

'HANOVER' - LH **ROOF PLAN '4EGF-A'**





Right Elevation 'A'





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PROJECT TITL

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

SHEET TITLE:

'HANOVER' - LH

EXTERIOR

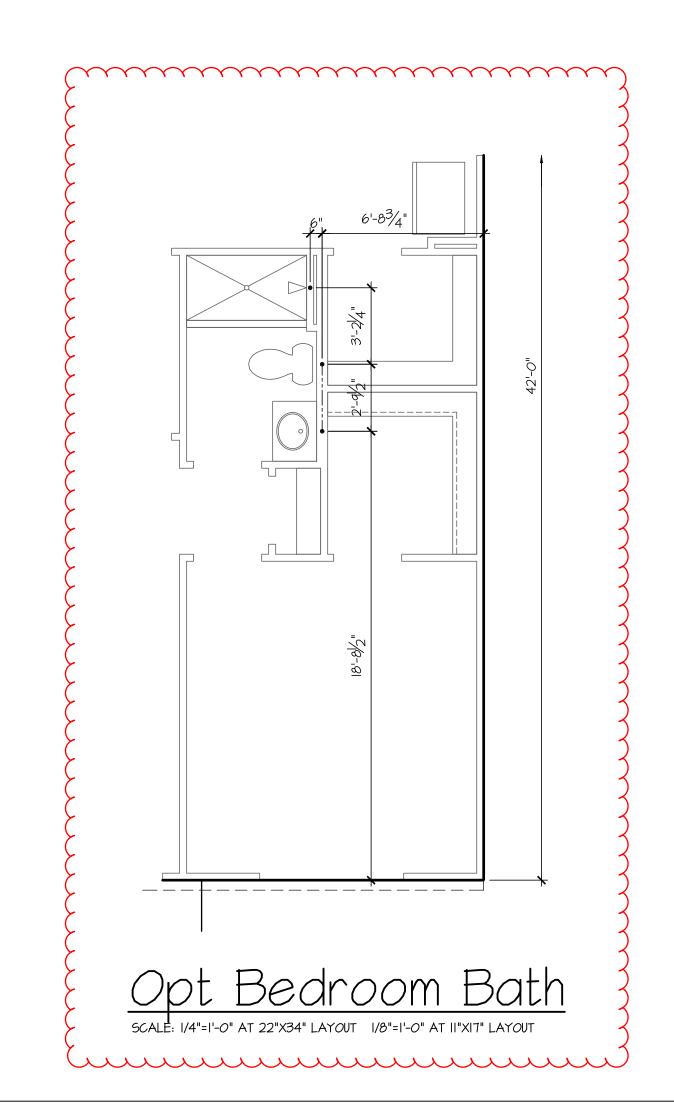
ELEVATIONS

PRINT DATE:

January 28, 2025

'4EGF-A'

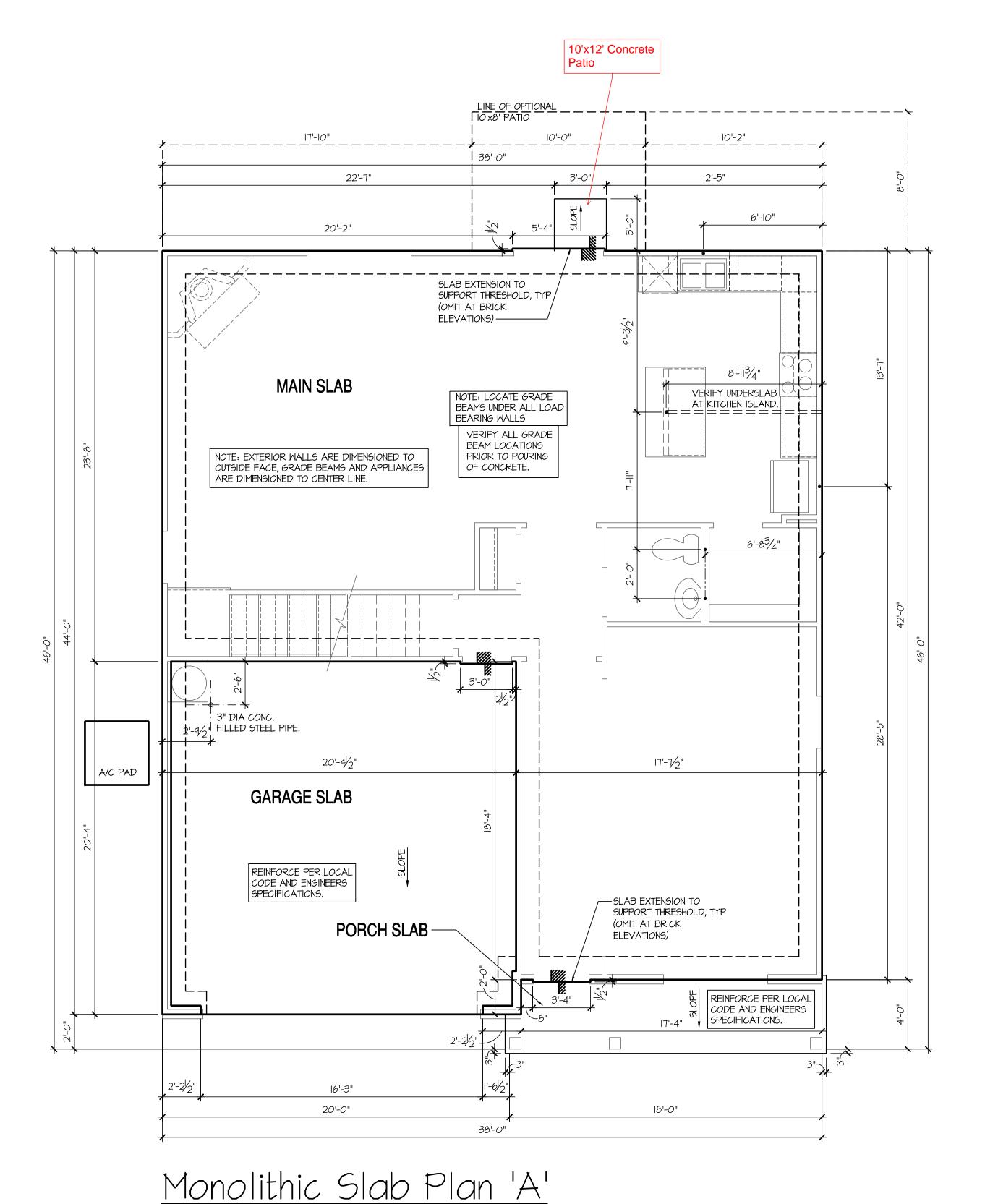
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- IRRIGATION SYSTEM SHALL BE DESIGNED TO PREVENT THE SATURATION OF SOIL ADJACENT TO BUILDING.
- THIS PERIMETER DIMENSION PLAN IS FOR DIMENSIONAL INFORMATION ONLY.
- SLOPE ALL STOOPS AND HARDSCAPE MATERIAL AWAY FROM BUILDING TYPICAL. - SLOPE GARAGE FLOOR I/8" PER FOOT TO GARAGE DOOR OPENING.
- VERIFY CURB CUT BLOCKOUT WITH GARAGE DOOR MANUFACTURER.
- REFER TO CIVIL DRAWINGS FOR FINISH SURFACE ELEVATIONS.
- FINISH GRADE SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING. REFER TO SOILS REPORT FOR ANY SPECIFIC REQUIREMENTS.
- REFER TO STRUCTURAL DRAWINGS FOR HOLDDOWNS, FOOTING DETAILS, CURB THICKNESS, AND
- INFORMATION NOT SHOWN ON THIS PLAN.
- PLUMBING FIXTURES, VENT LOCATIONS, ETC. ARE APPROXIMATE. CONTRACTOR TO VERIFY COUNT AND LOCATION. VERIFY THE SUPPLY FOR SEPARATE CONDUITS TO ANY ISLAND FOR GAS, WATER OR ELECTRIC.
- VERIFY ALL DOOR THRESHOLD HEIGHTS TO HARD SURFACES. 8 I/4" MAX AT INSWING DOORS. (PER NCRC SECTION R3II.3.I.)
- TYP STOOP AT INSWING/SLIDER DOORS: 36" DEEP BY THE WIDTH OF THE DOOR SERVED, MINIMUM. (PER NCRC SECTION R3II.3.) PROVIDE A SLIP-RESISTANT FINISH.
- FOR THE USE OF EXPOSED GAS WATER HEATERS IN THE GARAGE, PROTECT THE WATER HEATER WITH 3" DIA CONCRETE FILLED STEEL PIPE EMBEDDED INTO CONCRETÉ FOOTING.
- APPROVED BRAND OF TERMITE PROTECTION TO BE APPLIED TO FRAMING PER PRODUCT SPECIFICATIONS. (PROVIDE CHEMICAL TREATMENT FOR PROTECTION FROM TERMITE INVESTATION
- ACCORDING TO THE STANDARDS OF THE NC DEPT OF AGRICULTURE.)
- SEPARATION TO GRADE SHALL BE PRESSURE TREATED OR FOUNDATION GRADE

WOOD CONTACTING CONCRETE OR MASONRY OR LESS THAN CODE REQUIRED REDWOOD. SET ALL EXTERIOR WALL SILLS IN MASTIC.



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40' Series

FOR CONSTRUCTION

CLIENTS NAME:



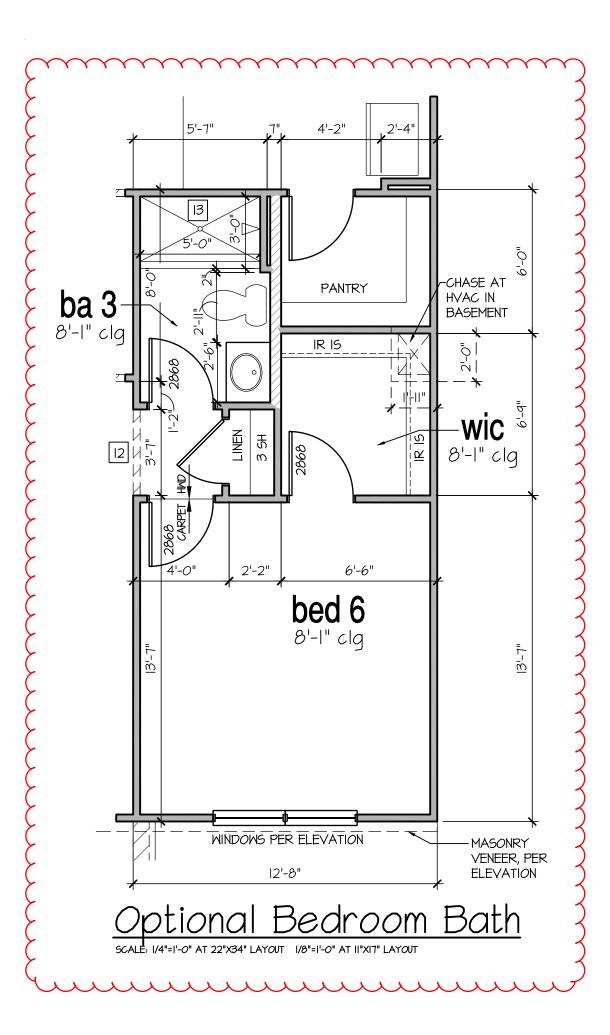
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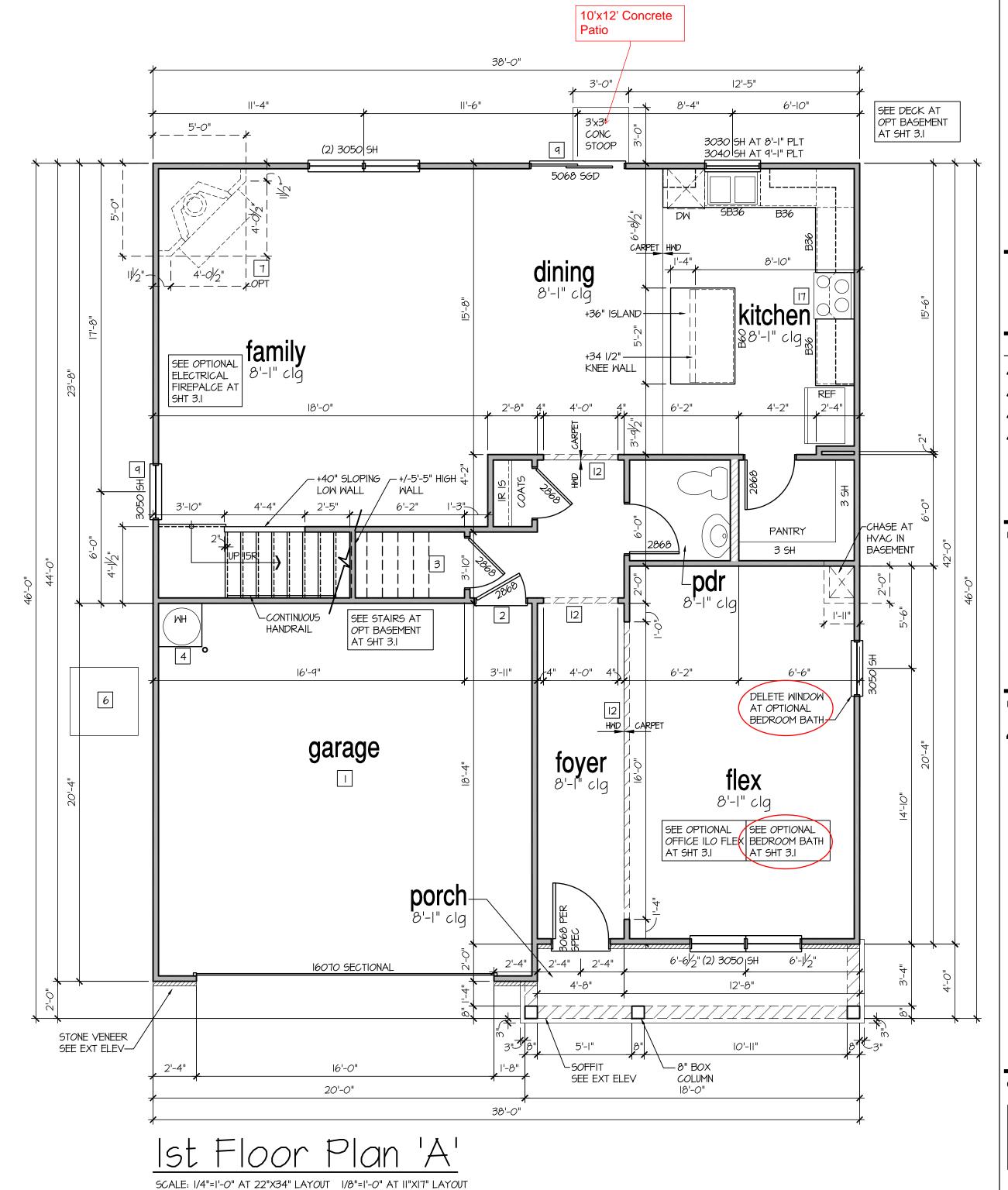
'HANOVER' - LH **MONOLITHIC** SLAB

PLAN '4EGF-A'

PRINT DATE: January 28, 2025

3 MS A





8'-9 1/2" STAIR NOTE: (USE 14" TJI WITH 3/4" PLYWOOD SUBFLOOR) 15 TREADS AT IO" EACH VERIFY

16 RISERS AT +/- 7.5" = 120 1/4" TOTAL RISE VERIFY

9'-1" STAIR NOTE: (USE 14" TJI WITH 3/4" PLYWOOD SUBFLOOR) 16 TREADS AT 10" EACH VERIFY 17 RISERS AT +/- 7.28" = 123 3/4" TOTAL

RISE VERIFY

FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS. | KEY NOTES: IRC WINDOW HEAD HEIGHTS: IST FLOOR = 6'-8" U.N.O. ON ELEVATIONS. 2ND FLOOR = 7'-0" U.N.O. ON ELEVATIONS. ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE.

NALL LEGEND:

FULL HEIGHT 2X4 WOOD STUD PARTITION

BRICK / STONE VENEER

LOW GYPSUM BOARD WALL

HEIGHT AND STUD SIZE AS NOTED

FULL HEIGHT 2X6 WOOD STUD PARTITION

STUD WALL BELOW HEIGHT AND STUD SIZE AS NOTED

DRYWALL OPENING. HEIGHT

AS NOTED ON PLAN.

FIRE PROTECTION:

I HOUSE TO GARAGE FIRE SEPARATION. GARAGE/HOUSE SEPARATION AT VERTICAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER I/2" GYPSUM BOARD. (PER IRC SECTION R302.6) GARAGE/HOUSE SEPARATION AT HORIZONTIAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER 5/8" TYPE 'X' GYPSUM BOARD. (PER IRC SECTION R302.6)

- 2 HOUSE TO GARAGE DOOR SEPARATION. PROVIDE I-3/8" SOLID CORE DOOR OR APPROVED 20 MINUTE RATED DOOR. (PER IRC SECTION R302.5.I)
- 3 BENEATH STAIRS AND LANDINGS. I/2" GYPSUM BOARD ON WALLS AND CEILING OF ENCLOSED ACCESSIBLE AREAS. (PER IRC SECTION R302.7)
- 4 GAS WATER HEATER ON 18" HIGH PLATFORM. (PER IRC SECTION P2801.6) (AT SPECIFIC LOCATIONS: ELECTRIC WATER HEATER PER LOCAL CODES)

- 6 A/C CONDENSER PAD. (VERIFY)
- 7 PRE-FABRICATED METAL FIREPLACE.
 INSTALL PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 8 ATTIC ACCESS LARGE ENOUGH TO REMOVE LARGEST PIECE OF EQUIPMENT BUT NOT LESS THAN 30"x22". FIRE RATED ACCESS AS NOTED. (PER IRC SECTION MI305.1.3) ATTIC ACCESS LADDER, VERIFY LOCATION AND SIZE WITH TRUSSES. (25 I/2" X 54" SIZE.)
- TYPICALS: TEMPERED SAFETY GLASS. (PER IRC SECTION R308.4)
- 10 PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER. HEIGHT AS NOTED.
- III HALF WALL, HEIGHT AS NOTED.

- 12 INTERIOR SOFFITS: FFL = 7'-1" U.N.O. SFL = 7'-6" U.N.O.
- 36"x60" ACRYLIC SHOWER PAN
- 14 30"x60" TUB/SHOWER PAN
- 16 42"x60" ACRYLIC ALCOVE TUB
- [17] 30" SLIDE-IN ELECTRICAL RANGE W/ HOOD VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 18 30" GAS COOKTOP AND HOOD. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS. 19 ELECTRIC OVEN WITH MICROWAVE OVEN.

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FOR CONSTRUCTION

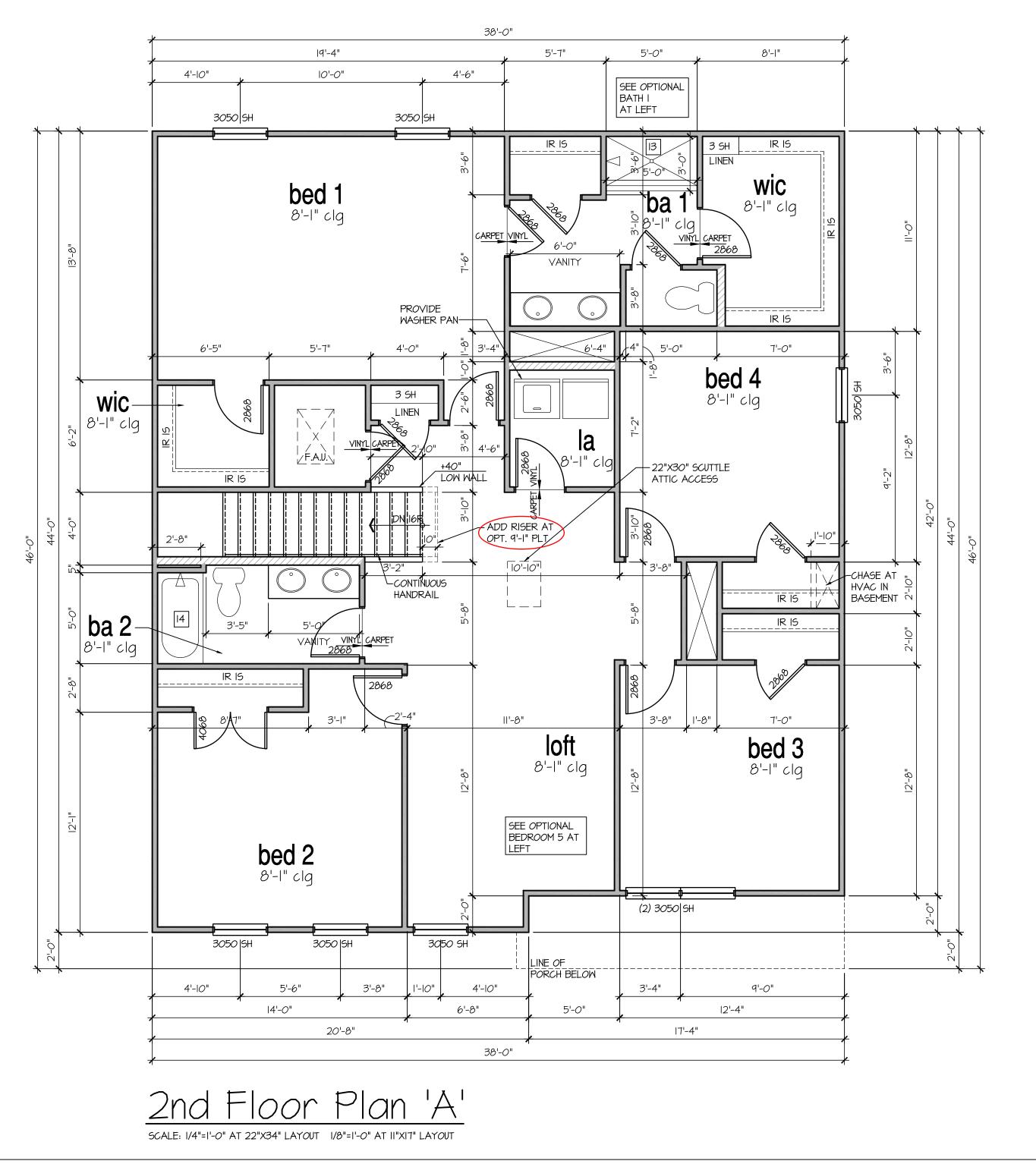
CLIENTS NAME:



PROJECT NO: GMD17049

'HANOVER' - LH 1st FLOOR PLAN '4EGF-A'

PRINT DATE: January 28, 2025



8'-9 1/2" STAIR NOTE: (USE 14" TJI WITH 3/4" PLYWOOD SUBFLOOR)

15 TREADS AT 10" EACH VERIFY 16 RISERS AT +/- 7.5" = 120 1/4" TOTAL RISE VERIFY

9'-1" STAIR NOTE:

(USE 14" TJI WITH 3/4" PLYWOOD SUBFLOOR) 16 TREADS AT IO" EACH VERIFY 17 RISERS AT +/- 7.28" = 123 3/4" TOTAL RISE VERIFY

2ND FLOOR = 7'-0" U.N.O. ON ELEVATIONS.

- FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS. KEY NOTES: IRC WINDOW HEAD HEIGHTS: IST FLOOR = 6'-8" U.N.O. ON ELEVATIONS.

ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE. WALL LEGEND:

-----FULL HEIGHT 2X4 WOOD STUD PARTITION

FULL HEIGHT 2X6 WOOD STUD PARTITION

BRICK / STONE VENEER

STUD WALL BELOW HEIGHT AND STUD SIZE AS NOTED

LOW GYPSUM BOARD WALL DRYWALL OPENING. HEIGHT HEIGHT AND STUD SIZE AS NOTED AS NOTED ON PLAN.

FIRE PROTECTION:

HOUSE TO GARAGE FIRE SEPARATION. GARAGE/HOUSE SEPARATION AT VERTICAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER I/2" GYPSUM BOARD. (PER IRC SECTION R302.6) GARAGE/HOUSE SEPARATION AT HORIZONTIAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER 5/8" TYPE 'X' GYPSUM BOARD. (PER IRC SECTION R302.6)

- 12 HOUSE TO GARAGE DOOR SEPARATION. PROVIDE I-3/8" SOLID CORE DOOR OR APPROVED 20 MINUTE RATED DOOR. (PER IRC SECTION R302.5.I)
- 3 BENEATH STAIRS AND LANDINGS. I/2" GYPSUM BOARD ON WALLS AND CEILING OF ENCLOSED ACCESSIBLE AREAS. (PER IRC SECTION R302.7)
- 4 GAS WATER HEATER ON 18" HIGH PLATFORM. (PER IRC SECTION P2801.6) (AT SPECIFIC LOCATIONS: ELECTRIC WATER HEATER PER LOCAL CODES)

5 N/A

- 6 A/C CONDENSER PAD. (VERIFY)
- 7 PRE-FABRICATED METAL FIREPLACE. INSTALL PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 8 ATTIC ACCESS LARGE ENOUGH TO REMOVE LARGEST PIECE OF EQUIPMENT BUT NOT LESS THAN 30"x22". FIRE RATED ACCESS AS NOTED. (PER IRC SECTION MI305.1.3) ATTIC ACCESS LADDER, VERIFY LOCATION AND SIZE WITH TRUSSES. (25 1/2" X 54" SIZE.)
- TYPICALS: TEMPERED SAFETY GLASS. (PER IRC SECTION R308.4)
- O PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER. HEIGHT AS NOTED.
- II) HALF WALL, HEIGHT AS NOTED.

12 INTERIOR SOFFITS: FFL = 7'-1" U.N.O. SFL = 7'-6" U.N.O. BATHS:

- 13 36"x60" ACRYLIC SHOWER PAN
- 14 30"x60" TUB/SHOWER PAN 15 N/A
- 16 42"x60" ACRYLIC ALCOVE TUB KITCHEN:

19 ELECTRIC OVEN WITH MICROWAVE OVEN.

- [17] 30" SLIDE-IN ELECTRICAL RANGE W/ HOOD VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- [18] 30" GAS COOKTOP AND HOOD. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

PROJECT NO: GMD17049

CLIENTS NAME:

FOR CONSTRUCTION

REVISION:

OI.28.25 INITIAL RELEASE

O2.17.25 ADDED FOUNDATIONS

05.02.25 FRONT DOOR REV.

05.16.25 CLIENT REVISIONS

PROFESSIONAL SEAL:

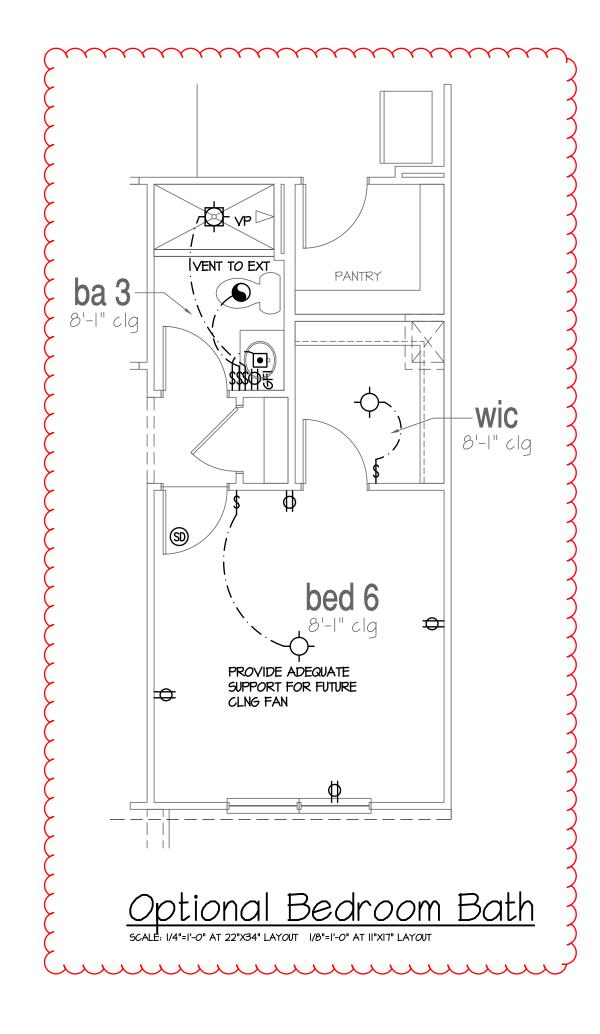
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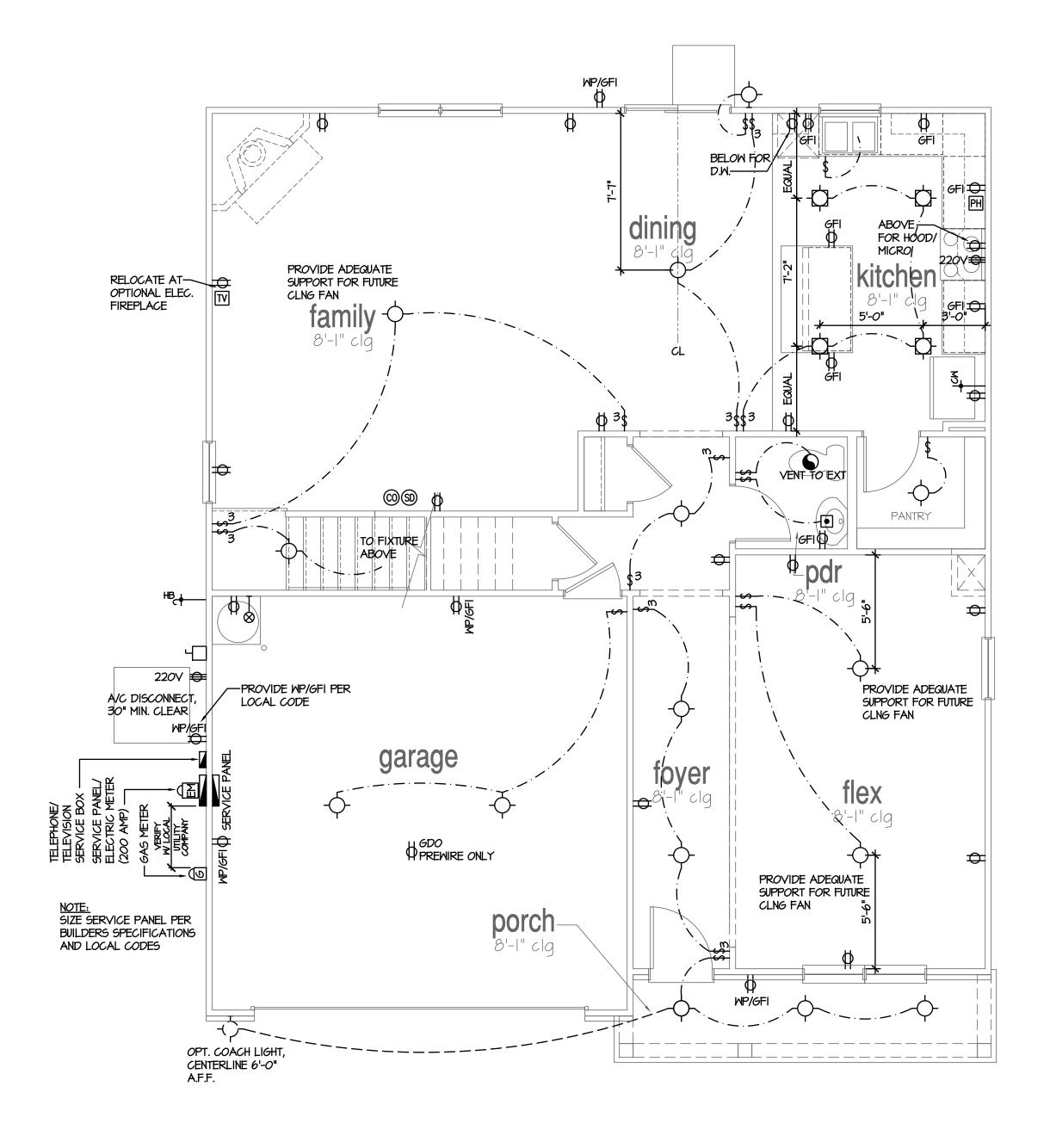
40' Series

'HANOVER' - LH 2nd FLOOR PLAN '4EGF-A'

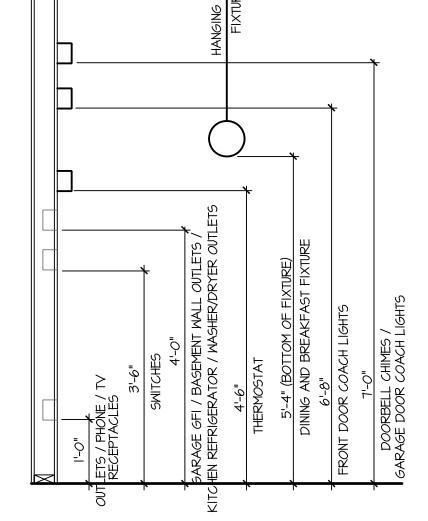
PRINT DATE: January 28, 2025

5 A

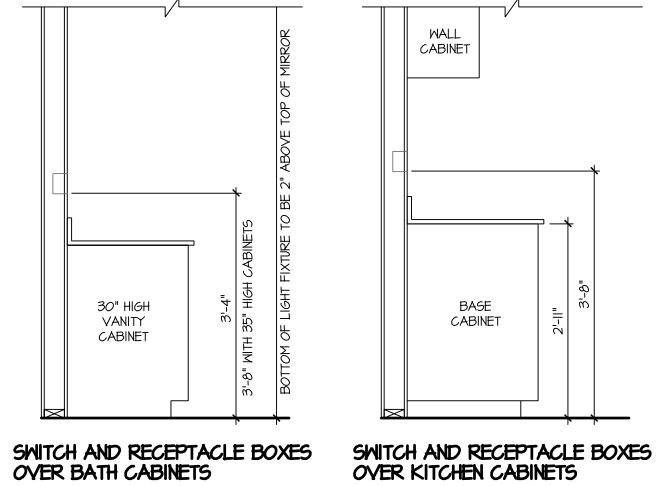


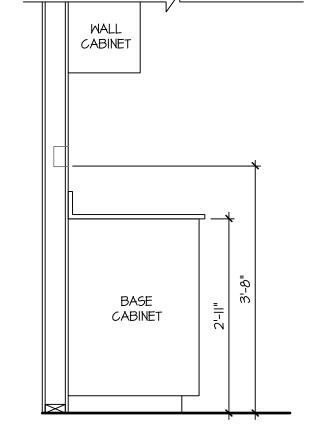


WALL SCONCE



STANDARD ELECTRICAL BOX HEIGHTS





- PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

	_			
NOTES:	LEGEND:			
- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL	φ DUPLEX OUTLET		CH CHIMES	
CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. - ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.	\$\text{\$\psi\$ WP/GFI WEATHERPROOF GFI DUPLEX OUTLET}\$	-H- HANGING FIXTURE	PUSHBUTTON SWITCH	<u> </u>
- FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS." - ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE	Ø GFI GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET	FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	(SD) IIOV SMOKE DETECTOR W/ BATTERY BACKUP	CEILING FAN (PROVIDE ADEQUATE SUPPORT)
CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT. - PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY	HALF-SWITCHED DUPLEX OUTLET	2 2-LIGHT VANITY FIXTURE	© CO2 DETECTOR	<u> </u>
NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL	\$\psi 220V 220 \text{ \text{OUTLET}}	2-LIGHT VANITY FIXTURE	THERMOSTAT	GAS SUPPLY WITH VALVE
CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.	① REINFORCED JUNCTION BOX	- 3 3-LIGHT VANITY FIXTURE	PH TELEPHONE	HD HOSE BIBB
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.	\$ WALL SMITCH	-4 4-LIGHT VANITY FIXTURE	TELEVISION	
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.	\$ 3 THREE-WAY SWITCH	- WALL MOUNT FIXTURE	ELECTRIC METER	CW I/4" WATER STUB OUT
DIS WITTER SOLIT, AND TWILE TEXTERS / THE SUBJECT TO RECOVER DUE TO THEED COMMITTIONS.			ELECTRIC PANEL	И

\$4 FOUR-WAY SWITCH

Ist Floor Plan 'A'

EXHAUST FAN (VENT TO EXTERIOR)

NO:	DATE:	REVISION:
	OI.28.25	INITIAL RELEASE
$\sqrt{2}$	02.17.25	ADDED FOUNDATIONS
<u>\$</u>	05.02.25	FRONT DOOR REV.
4	05.16.25	CLIENT REVISIONS
l	ı	l

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

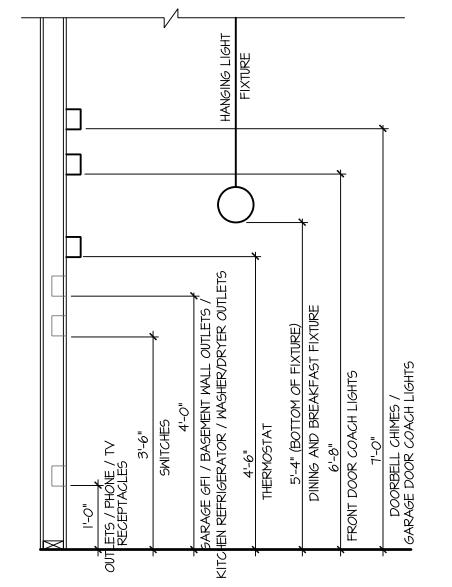
FOR CONSTRUCTION

CLIENTS NAME:

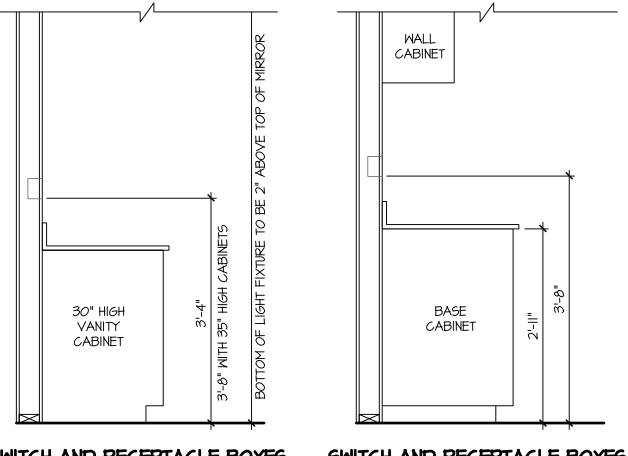


PROJECT NO: GMD17049

'HANOVER' - LH 1st FLOOR **UTILITY PLAN**



STANDARD ELECTRICAL BOX HEIGHTS

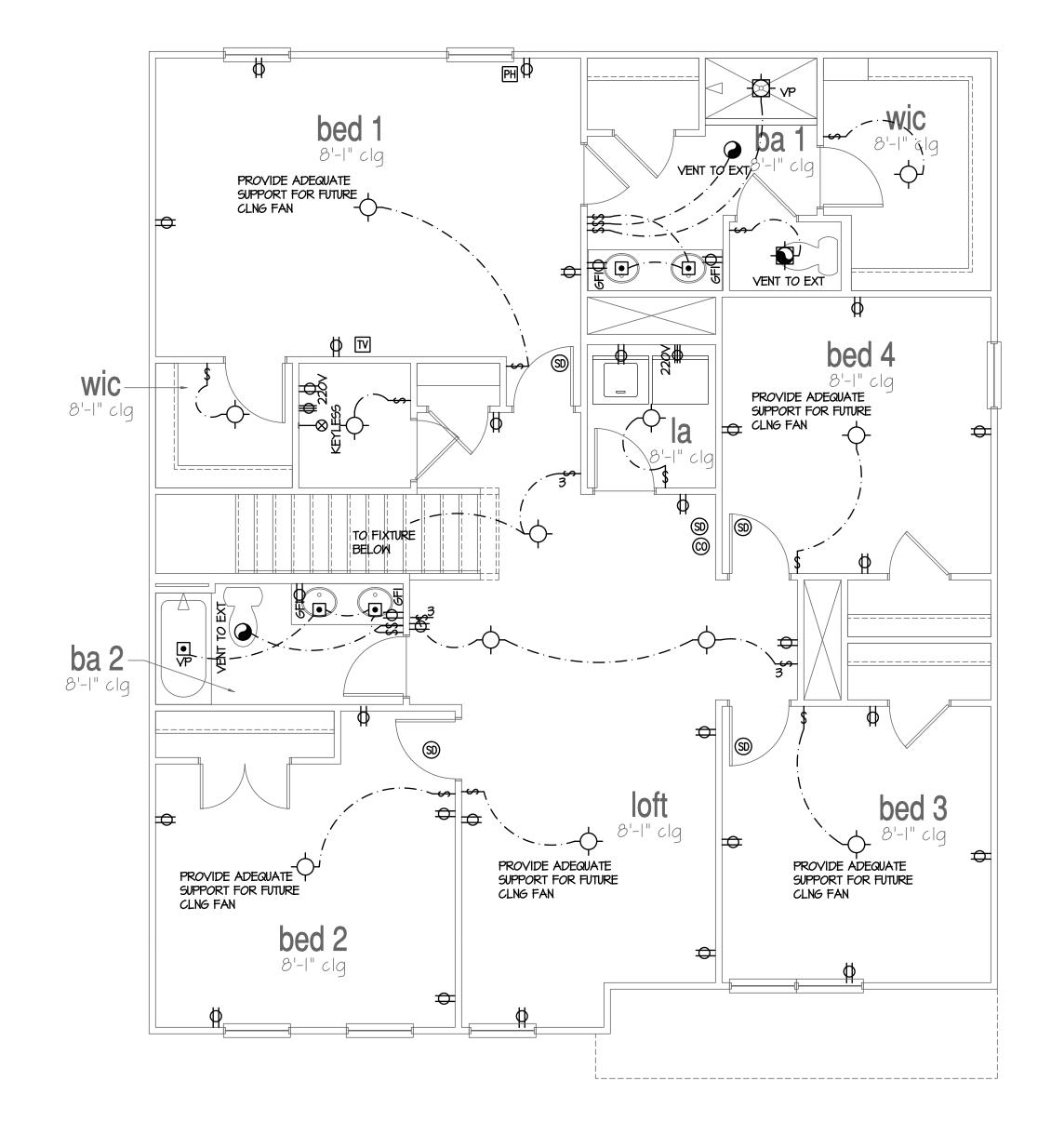


SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

NOTES:	LEGEND:			
- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL	φ DUPLEX OUTLET		CH CHIMES	
CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. - ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.	ØMP/GFI WEATHERPROOF GFI DUP	LEX OUTLET -HANGING FIXTURE	PUSHBUTTON SMITCH	
- FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS." - ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE	GFI GROUND-FAULT CIRCUIT-I	NTERRUPTER FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	® IIOV SMOKE DETECTOR W BATTERY BACKUP	CEILING FAN (PROVIDE ADEQUATE SUPPORT)
CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY	HALF-SWITCHED DUPLEX		© CO2 DETECTOR	
NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. - PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL	\$\psi 220 \times 220 \times 00\text{UTLET}	<u> </u>		→ → GAS SUPPLY WITH VALVE
CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. - ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.	① REINFORCED JUNCTION B	OX -3 3-LIGHT VANITY FIXTURE	PH TELEPHONE	HB HOSE BIBB
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOUR-UPS/CUTOFFS. - HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.	\$ WALL SWITCH	-4 4-LIGHT VANITY FIXTURE	TELEVISION	
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.	\$ 3 THREE-WAY SMITCH	- WALL MOUNT FIXTURE	ELECTRIC METER FI FCTRIC PANFI	CM 1/4 MAILE SIOD COI

\$4 FOUR-WAY SMITCH

- PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.



WALL SCONCE

2nd Floor Plan 'A'

DISCONNECT SWITCH

SCALE: |/4"=|'-0" AT 22"X34" LAYOUT |/8"=|'-0" AT ||"X|7" LAYOUT

EXHAUST FAN (VENT TO EXTERIOR)

NO:	DATE:	REVISION:
$\overline{\triangle}$	OI.28.25	INITIAL RELEASE
2	02.17.25	ADDED FOUNDATION
3	05.02.25	FRONT DOOR REV.
4	05.16.25	CLIENT REVISIONS

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

'HANOVER' - LH 2nd FLOOR

UTILITY PLAN

DESIGN SPECIFICATIONS:

Construction Type: Commerical \square Residential \boxtimes

Applicable Building Codes:

2018 North Carolina Residential Building Code with All Local Amendments
 ASCE 7—10: Minimum Design Loads for Buildings and Other Structures

Design Loads:

Ü	1	Roof Live Loads		
	١.			
		1.1. Conventional 2x	20	PS
		1.2. Truss	20	PS
		1.2.1. Attic Truss	60	PS
	2.	Roof Dead Loads		
		2.1. Conventional 2x	10	PS
		2.2. Truss	20	PS
	3.	Snow		
		3.1. Importance Factor	1.0	
	4.	Floor Live Loads		
		4.1. Typ. Dwelling		
		4.2. Sleeping Areas	30	PS
		4.3. Decks	40	PS

	10. 50000		. 01	
	4.4. Passenger Garage	50	PSF	
5.	Floor Dead Loads			
	5.1. Conventional 2x	10	PSF	
	5.2. I-Joist	15	PSF	
	5.3. Floor Truss	15	PSF	
6.	Ultimate Design Wind Speed (3 sec. gust) 130 MPH			
	6.1. Exposure			В
	6.2. Importance Factor	1.0		

	6.3	.1.	Vx =			
	6.3	.2.	Vy =			
7.	Component	and	Cladding	(in	PSF)	

6.3. Wind Base Shear

ı	<i>J</i> (,		
MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.6,-18.9	18.3,-19.7	18.8,-20.2
ZONE 2	16.7,-21.0	17.6,-22.1	18.3,-22.9	18.8,-23.6
ZONE 3	16.7,-21.0	17.6,-22.1	18.3,-22.9	18.8,-23.6
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.8	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.2	20.4,-26.9

8. Seismic

0010111	10	
8.1.	Site Class D	
8.2.	Design Category C	
8.3.	Importance Factor 1.0	
8.4.	Seismic Use Group 1	
8.5.	Spectral Response Acceleration	
	8.5.1. Sms = %g	
	8.5.2. Sm1 = $\%g$	
8.6.	Seismic Base Shear	
	8.6.1. Vx =	
	8.6.2. Vy =	
8.7.	Basic Structural System (check one)	

 Bearing Wall
 Building Frame
 Building Frame
 The state of the st ☐ Moment Frame □ Dual w/ Special Moment Frame □ Dual w/ Intermediate R/C or Special Steel ☐ Inverted Pendulum

8.8. Arch/Mech Components Anchored .. 8.9. Lateral Design Control: Seismic □ Wind ⊠ 9. Assumed Soil Bearing Capacity 2000psf



UES PROFESSIONAL SOLUTIONS 29, INC

FORMERLY SUMMIT ENGINEERING, LABORATORY, & **TESTING INC.**

STRUCTURAL PLANS PREPARED FOR:

HANOVER PLAN LH (NC)

PROJECT ADDRESS:

DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

DESIGNER: DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify UES Professional Solutions 29, Inc. (UES) before construction begins.

<u>PLAN ABBREVIATIONS:</u>

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
EW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
OC	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to UES Professional Solutions 29, Inc. (UES) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton, Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify UES immediately.

SHEET LIST:

REVISION LIST:

Revision

No.

Date | Project No.|

1 3.10.25 A25117.00 321.000

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
S1.0m	Monolithic Slab Foundation
S1.0s	Stem Wall Foundation
S1.0c	Crawl Space Foundation
S1.0b	Basement Foundation
S2.0	Basement Framing Plan
S3.0	First Floor Framing Plan
S4.0	Second Floor Framing Plan
\$5.0	Roof Framing Plan
S6.0	Basement Bracing Plan
\$7.0	First Floor Bracing Plan
S8.0	Second Floor Bracing Plan

Description

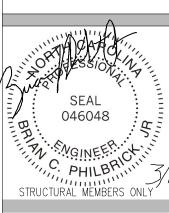
Added Basement Plan

DR HORTON PROJECT	T_SIGN-OFF:
Manager	Signature
Operations	
Operations System	
Operations Product Development	

			L
I	l	l	







DATE: 03/10/2025 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A20117.00091.000 DRAWN BY: MGC CHECKED BY: BCP



GENERAL STRUCTURAL NOTES:

- . The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of UES Professional Solutions 29, Inc. (UES) or the SER. For the purposes of these construction documents the SER and UES shall be considered the same entity.
- 2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to UES for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field
- conditions, is not the responsibility of the SER or UES. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any
- discrepancies to UES before construction begins. 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings. . This structure and all construction shall conform to all
- applicable sections of the international residential code. 8. This structure and all construction shall conform to all applicable sections of local building codes.
- 9. All structural assemblies are to meet or exceed to requirements of the current local building code.
- 10. The Structural Engineer of Record's (SER) seal applies to structural components only. The SER's seal does not certify dimensional accuracy or architectural layout, including roof geometry. UES Professional Solutions 29, Inc. (UES) nor the SER assumes no liability for changes made to sealed drawings by others, construction methods, or any deviation from these plans. The SER shall be notified prior to construction if any
- discrepancies are noted on the plans. 1. All sealed structural drawings shall have a signed and dated seal to be valid and are limited to the following
- A. If these structural drawings are issued as part of a master-plan set intended to be used for mass development, these drawings shall be valid for a period of two (2) years from the date on the seal, or if any code required updates are placed in effect by the governing jurisdiction.
- B. If these structural drawings are not issued as part of a master plan set intended for mass development, these drawings are valid for a conditional one time use for the lot of the address specified within the title block.
- 12. UES Professional Solutions 29, Inc. (UES) as its option, may create a set of standard details for a client that are referenced within our drawings. Any details created by UES whether specific to one plan or as part of a "Standard Detail" package are only valid with use of drawings created by UES Professional Solutions 29, Inc. (UES) and shall not be used with any other drawings or for any other construction purposes.

- 1. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- 3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer.
- 4. The resulting soil shall be compacted to a minimum of 95% maximum dry density. 5. Excavations of footings shall be lined temporarily with a

6 mil polyethylene membrane if placement of concrete

does not occur within 24 hours of excavation. 6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

STRUCTURAL STEEL:

- 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest
- 2. Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- 3. All steel shall have a minimum yield stress (F_v) of 36 ksi unless otherwise noted.
- 4. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes and consumables for both shop and field welding shall be 70ksi. All welding shall be performed by a certified welder per the above standards.

- 1. Concrete shall be nominal weight concrete with no aggregate larger than 3/4" and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%
- Exterior Slabs: 5% 4. No admixtures shall be added to any structural
- concrete without written permission of the SER. 5. Concrete slabs—on—grade shall be constructed in accordance with the latest version of ACI 302.1: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab—on—grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance
- with the above assumptions. Control or saw cut joints shall be spaced in interior slabs—on—grade at a maximum of 15'—0" O.C. and in exterior slabs—on—grade at a maximum of 10'—0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- 9. Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut

10. All welded wire fabric (W.W.F.) for concrete slabs—on—grade shall be placed at mid—depth of slab. The W.W.F. shall be securely supported during the

CONCRETE REINFORCEMENT:

- I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs—on—grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- 3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- 4. Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- 5. Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- 7. Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted. WOOD FRAMING:
- 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine
- 2. LVL or PSL engineered wood shall have the following minimum design values:
 - E = 1.900,000 psi2.2. Fb = 2600 psi
 - 2.3. Fv = 285 psi2.4. Fc = 700 psi
- 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be
- treated in accordance with AWPA standard C-2 4. Nails shall be common wire nails unless otherwise
- 5. Lag screws shall conform to ANSI/ASME standard B18.2.1—1981. Lead holes for lag screws shall be in
- accordance with NDS specifications. 6. All beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.

- 8. Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.
- 9. Multi-ply beams shall have each ply attached with (3) 12d nails @ 12" O.C.
- 10. Four and five ply beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise.
- 11. All fasteners that will be exposed to the elements shall be hot dipped galvanized.

- 1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7—16), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction

WOOD STRUCTURAL PANELS:

- 1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial." and all other applicable APA standards.
- 2. All structurally required wood sheathing shall bear the
- mark of the APA. 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.

- 4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)—8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the
- sheathing as required by the state Building Code. 5. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)—8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the
- sheathing as required by the state Building Code. 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

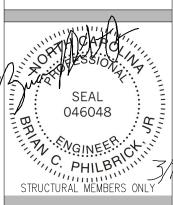
STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- 2. All structurally required fiberboard sheathing shall bear
- the mark of the AFA.
- 3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- 4. Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.



10121 Pineville Distribution St Pineville, NC 28134 Office: 704.504.1717 Fax: 704.504.1125





DATE: 03/10/2025 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A20117.00091.000 DRAWN BY: MGC CHECKED BY: BCP

ORIGINAL INFORMATION PROJECT # DATE A20117.00091.000 02/26/2025

FOUNDATION NOTES:

- 1. FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- 2. STRUCTURAL CONCRETE TO BE $F_c = 3000$ PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.
- 3. FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.
- 4. FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE
- SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION. 5. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF MASONRY.
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE. 7. PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
- 8. PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO
- OUTLET AS REQUIRED BY SITE CONDITIONS. 9. PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE.
- 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEERS. 11. CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
- 12. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.1.6. MINIMUM 1/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A 7" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION AND (1) LOCATED NOT MORE THAN 12" FROM THE CORNER. ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- 13. ABBREVIATIONS:
 - DJ = DOUBLE JOIST SJ = SINGLE JOISTGT = GIRDER TRUSS FT = FLOOR TRUSSSC = STUD COLUMNDR = DOUBLE RAFTER
 - EE = EACH ENDTR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTERPL = POINT LOADCL = CENTER LINE
- 14. ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x16" MASONRY,
- 15. WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
- 16. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, UES PROFESSIONAL SOLUTIONS 29, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- 17. ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLD-DOWNS. ADDITIONAL INFORMATION PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND <u>NOT</u> BRICK VENEER, UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP PER TABLE R405.1

NOTE: FOUNDATION ANCHORAGE HAS BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>02/17/2025</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS 29, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES PROFESSIONAL SOLUTIONS 29, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF UES PROFESSIONAL SOLUTIONS 29, INC. (UES). FAILURE TO DO SO WILL VOID UES LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

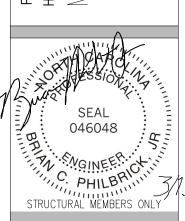
MONOLITHIC SLAB FOUNDATION PLAN SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

AMENDMENTS.





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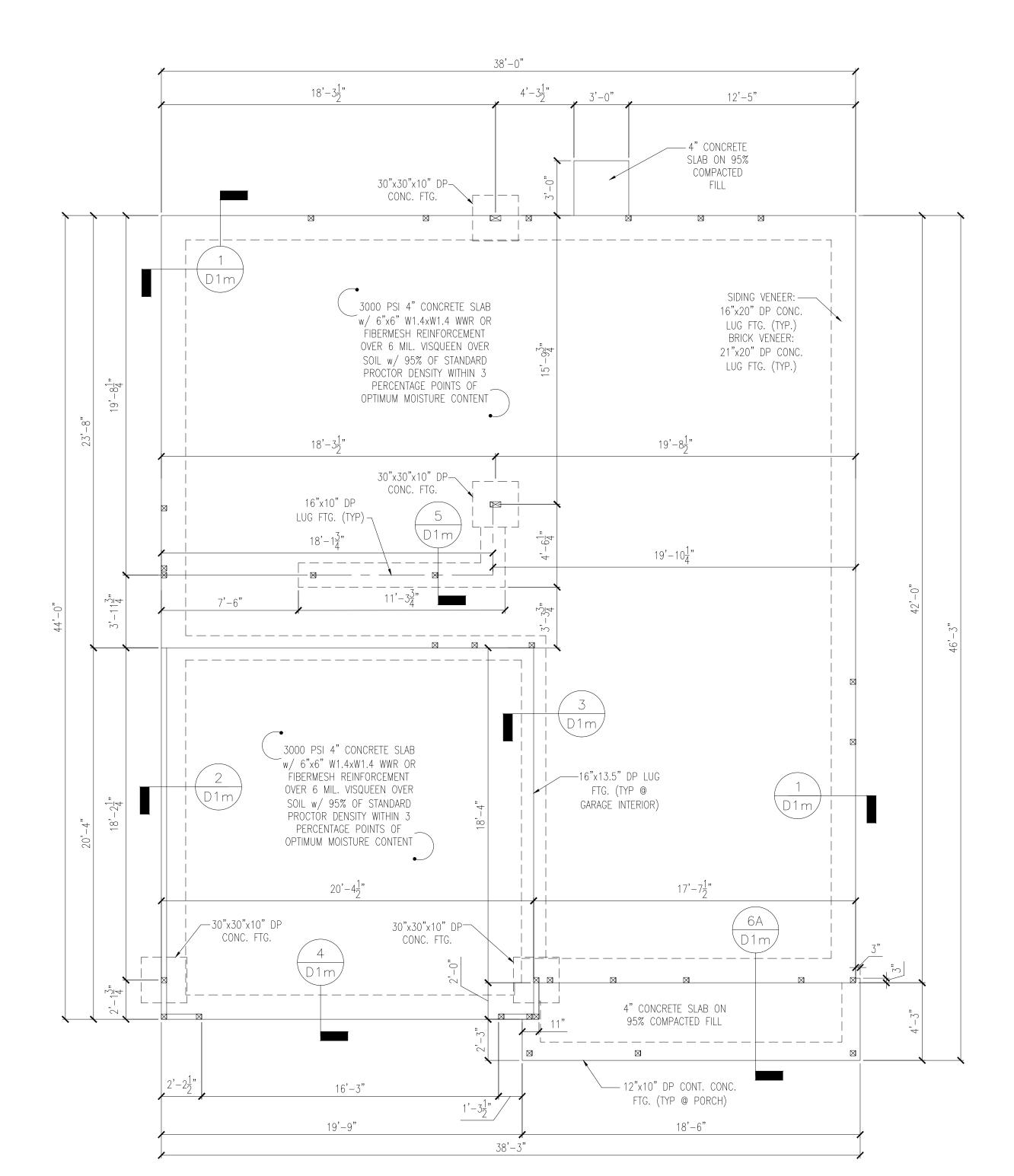


DATE: 03/10/2025 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A20117.00091.000 DRAWN BY: MGC CHECKED BY: BCP

ORIGINAL INFORMATION PROJECT # DATE A20117.00091.000 02/26/2025

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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ELEVATION A

REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	A A TEDIAL	NUM THIOMATECO	REQUIRED (CONNECTION
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6"O.C.	6d COMMON NAILS @ 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** @ 7" O.C.	5d COOLER NAILS** @ 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1
**OR EQUIVALENT PER TABLE R702.3.5				

BRACED WALL NOTES:

- 1. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS.
- 2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE DESIGN WIND SPEEDS UP TO 130 MPH.
- 3. REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES. 4. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE
- WITH TABLE R602.10.1 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD
- 8. FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END
- 9. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE.
- 11. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4 13. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5
- 14. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.104.6
- 15. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1
- 16. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- 17. ABBREVIATIONS:

GB = GYPSUM BOARD WSP = WOOD STRUCTURAL PANEL CS-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION PF = PORTAL FRAME ENG-PF = ENG. PORTAL FRAME

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT DAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

HEADER SCHEDULE						
TAG	SIZE	JACKS (EACH END)				
А	(2) 2x6	(1)				
В	(2) 2x8	(2)				
С	(2) 2x10	(2)				
D	(2) 2x12	(2)				
Е	(2) 9-1/4" LSL/LVL	(3)				
F	(3) 2x6	(1)				
G	(3) 2x8	(2)				
Н	(3) 2x10	(2)				
	(3) 2x12	(2)				
NOTES:						

1. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (U.N.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (U.N.O.).

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>02/17/2025</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS 29, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES PROFESSIONAL SOLUTIONS 29, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF UES PROFESSIONAL SOLUTIONS 29, INC. (UES). FAILURE TO DO SO WILL VOID UES LIABILITY.

STRUCTURAL. ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

FIRST F	FLOOR BRACIN	NG (FT)
CONTINUOUS S	SHEATHING METHOD -	- ELEVATION A
	REQUIRED	PROVIDED
BWL 1-1	13.0	20.6
BWL 1-A	11.4	36.5
BWL 1-2	13.0	23.8
BWL 1-B	11.4	39.0

BWL	1-A I	-В
BWL 1-2-	8'-4" CS-WSP CS-WSP CS-WSP B B	→ BWL 1-2
16'-2" CS-WSP	14" FLOOR JOISTS PER MANUF.	25'-8" CS-WSP
SEE TWO STORY WALL NOTE	DOUBLE JOIST W/ (2) S.C.E.E. (2) 14" FLOOR JOISTS PER MANUF.	A
20'-4" CS-WSP	14" FLOOR JOISTS PER MANUF. 14" FLOOR JOISTS PER MANUF. NOTE: FRAMING PLAN DOES NOT CHANGE WITH OPTIONAL OFFICE	13'-4" CS-WSP
BWL 1-1	PF: SHEATHE INTERIOR AND EXTERIOR 2'-2½" (PF) MIN. 4" P.T. POSTS OR COL. RATED FOR 2000# (MIN, TYP) ATTACH POSTS TO HEADER W/ (3) S.C.E.E. FRAME PORTAL WALL PER DETAIL 1/D1f 3.5"x7" 1.8E PSL COL PF: SHEATHE INTERIOR AND EXTERIOR 2.5"x7" 1.8E PSL COL PF: SHEATHE INTERIOR AND EXTERIOR (2) 4'-3½" (2) 22x8 P.T. DROPPED HEADER (MIN., TYP.) MIN. 4" P.T. POSTS OR COL. RATED FOR 2000# (MIN, TYP) ATTACH POSTS TO HEADER W/ SST CS16 STRAPS AND ATTACH POSTS TO FOUNDATION W/ SST ABA44 POST BASE OR EQUIV. (TYP)	——BWL 1-1
 BWL	I I I BWL 1	-B

ELEVATION A

KING STUD	SCHEDULE
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
3'-0"	(1)
4'-0"	(2)
8'-0"	(3)
12'-0"	(5)
16'-0"	(6)

WALL STUD SCHEDULE (10FT HEIGHT MAX.					
STUD SIZE	STUD SPACING (O.C.)				
	ROOF ONLY	ROOF & 1 FLOOR	ROOF & 2 FLOORS	NON-LOAI BEARING	
2x4	24"	16"	12"	24"	
2x6	24"	24"	16"	24"	
NOTES:					

1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 16" O.C.

3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ CROSS BRACING @ 6'-0" O.C. VERTICALLY.

	LINTEL SCHEDULE					
	TAG	SIZE	OPENING SIZE			
	1)	L3x3x1/4"	LESS THAN 6'-0"			
	2	L5x3x1/4"	6'-0" TO 10'-0"			
	3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"			
	4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS			
	SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR ③) ALL HEADERS WHERE BRICK IS USED, TO BE: ① (UNO)					

GENERAL STRUCTURAL NOTES:

- 1. CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
- 2. CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION. 4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS: MICROLLAM (LVL): $F_b = 2600 \text{ PSI}$, $F_v = 285 \text{ PSI}$, $E = 2.0 \text{x} 10^6 \text{ PSI}$

3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING

- PARALLAM (PSL): $F_b = 2900$ PSI, $F_v = 290$ PSI, $E = 1.25 \times 10^6$ PSI 5. ALL WOOD MEMBERS SHALL BE #2 SYP UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE #2 SYP (UNO).
- 6. ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 #2 SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERWISE.
- 7. ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".
- 8. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.1.6. MINIMUM 1/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A 7" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION AND (1) LOCATED NOT MORE THAN 12" FROM THE CORNER. ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- 9. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- 11. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP #2, DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP #2, DROPPED. (UNLESS NOTED OTHERWISE)
- 12. ABBREVIATIONS:

DJ = DOUBLE JOIST SJ = SINGLE JOISTGT = GIRDER TRUSS FT = FLOOR TRUSS SC = STUD COLUMN DR = DOUBLE RAFTER EE = EACH ENDTR = TRIPLE RAFTER TJ = TRIPLE JOIST OC = ON CENTERCL = CENTER LINE PL = POINT LOAD

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

DECK JOISTS SHALL BE SPACED AT A MAX. 12" O.C. WHEN DECK BOARDS ARE INSTALLED DIAGONALLY.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

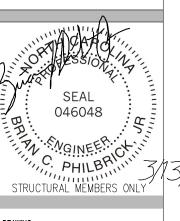
== DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.





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DATE: 03/10/2025 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A20117.00091.000 DRAWN BY: MGC CHECKED BY: BCP

ORIGINAL INFORMATION PROJECT # DATE A20117.00091.000 02/26/2025

REQUIRED BRACED WALL PANEL CONNECTIONS					
METHOD	NA TEDIAL	MINI THOUNECO	REQUIRED (CONNECTION	
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS	
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6"O.C.	6d COMMON NAILS @ 12"O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** @ 7"O.C.	5d COOLER NAILS** @ 7" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6"O.C.	6d COMMON NAILS @ 12"O.C.	
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1	

**OR EQUIVALENT PER TABLE R702.3.5

BRACED WALL NOTES:

- 1. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE
- 2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE DESIGN WIND SPEEDS UP TO 130 MPH.
- 3. REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES.
- 4. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- 6. MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1.
- 7. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD
- 8. FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END
- 9. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE.
- 11. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4 13. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5
- 14. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.104.6
- 15. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1
- 16. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS. 17. ABBREVIATIONS:
- GB = GYPSUM BOARD WSP = WOOD STRUCTURAL PANEL

CS-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION ENG-PF = ENG. PORTAL FRAMEPF = PORTAL FRAME

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

HEADER SCHEDULE					
TAG	SIZE	JACKS (EACH END)			
А	(2) 2x6	(1)			
В	(2) 2x8	(2)			
С	(2) 2x10	(2)			
D	(2) 2x12	(2)			
E	(2) 9-1/4" LSL/LVL	(3)			
F	(3) 2x6	(1)			
G	(3) 2x8	(2)			
Н	(3) 2x10	(2)			
	(3) 2x12	(2)			
NOTEC					

1. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (U.N.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS

LISTED ABOVE (U.N.O.).

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>02/17/2025</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS 29, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES PROFESSIONAL SOLUTIONS 29, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED

STRUCTURAL MEMBERS ONLY

DIFFERENTLY THAN THE DATE LISTED ABOVE.

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF UES PROFESSIONAL SOLUTIONS 29, INC. (UES). FAILURE TO DO SO WILL VOID UES LIABILITY.

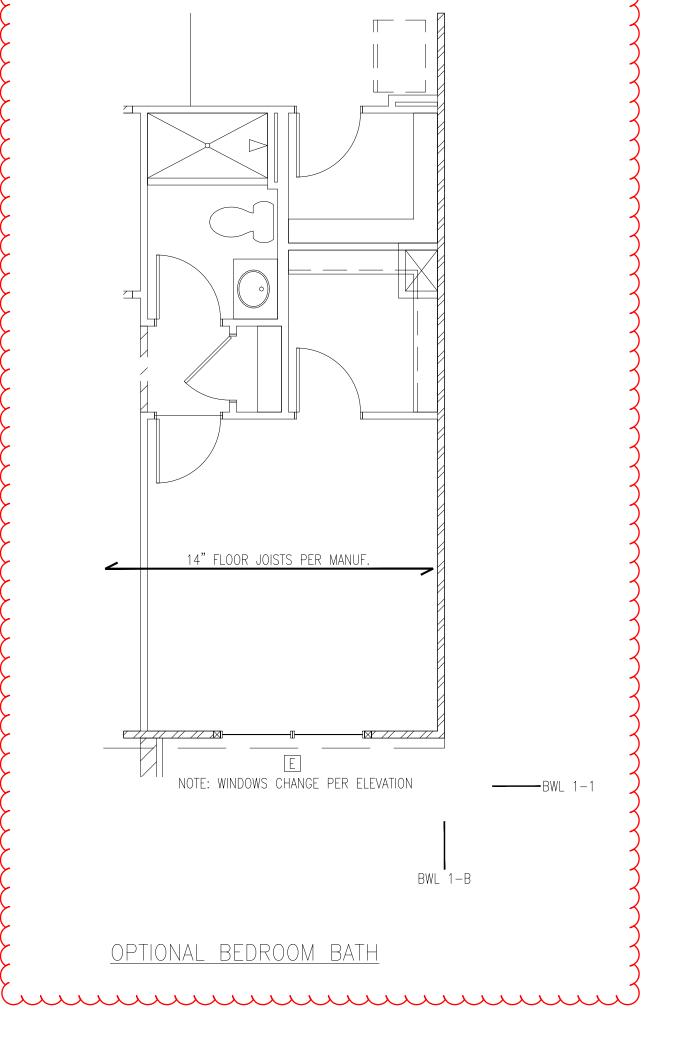
STRUCTURAL. ANALYSIS BASED ON 2018 NCRC.

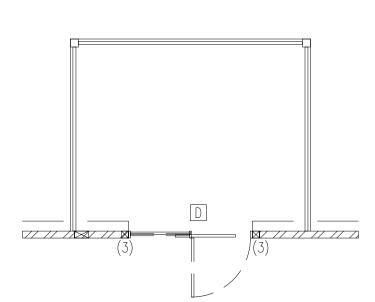
FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

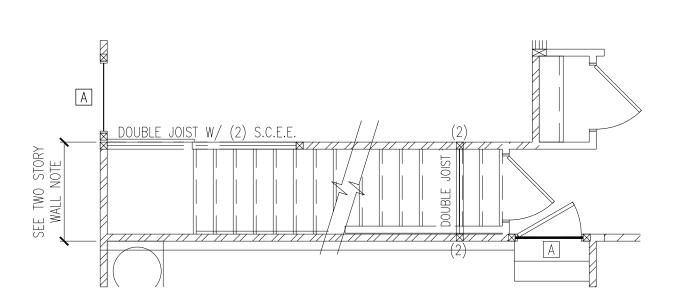
FIRST F	FLOOR BRACIN	NG (FT)
CONTINUOUS SHEA	THING METHOD - OF	T. BEDROOM BATH
	REQUIRED	PROVIDED
BWL 1-1	13.0	PER ELEV.
BWL 1-A	11.4	36.5
BWL 1-2	13.0	23.8
BWL 1-B	11.4	42.0

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DECK AT OPT. BASEMENT



STAIRS AT OPT. BASEMENT

KING STUD	SCHEDULE
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
3'-0"	(1)
4'-0"	(2)
8'-0"	(3)
12'-0"	(5)
16'-0"	(6)

WALL STUD SCHEDULE (10FT HEIGHT MA				
STUD SIZE STUD SPACING (O.C.)				
	ROOF ONLY	ROOF & 1 FLOOR	ROOF & 2 FLOORS	NON-LO
2x4	24"	16"	12"	24"
2x6	24"	24"	16"	24"
<u>NOTES:</u>				

1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 16" O.C.

3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ CROSS BRACING @ 6'-0" O.C. VERTICALLY.

LINTEL SCHEDULE			
TAG SIZE		OPENING SIZE	
	L3x3x1/4"	LESS THAN 6'-0"	
2	L5x3x1/4"	6'-0" TO 10'-0"	
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"	
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS	
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3)			
ALL HEADERS WHERE BRICK IS USED, TO BE: 1 (UNO)			

GENERAL STRUCTURAL NOTES:

1. CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL

BUILDING CODE WITH ALL LOCAL AMENDMENTS.

2. CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.

3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.

4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS: MICROLLAM (LVL): $F_b = 2600 \text{ PSI}$, $F_v = 285 \text{ PSI}$, $E = 2.0 \text{x} 10^6 \text{ PSI}$

PARALLAM (PSL): $F_b = 2900$ PSI, $F_v = 290$ PSI, $E = 1.25 \times 10^6$ PSI 5. ALL WOOD MEMBERS SHALL BE #2 SYP UNLESS NOTED ON PLAN. ALL STUD

COLUMNS AND JOISTS SHALL BE #2 SYP (UNO). 6. ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 #2 SYP STUD COLUMN

AT EACH END UNLESS NOTED OTHERWISE. 7. ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM

A615 AND SHALL HAVE A MINIMUM COVER OF 3". 8. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.1.6. MINIMUM 1/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A 7" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION AND (1) LOCATED NOT MORE THAN 12" FROM THE CORNER. ANCHOR BOLTS

SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. 9. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.

10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.

11. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP #2, DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP #2, DROPPED. (UNLESS NOTED OTHERWISE)

12. ABBREVIATIONS:

DJ = DOUBLE JOIST SJ = SINGLE JOISTGT = GIRDER TRUSS FT = FLOOR TRUSSSC = STUD COLUMN DR = DOUBLE RAFTER EE = EACH END TR = TRIPLE RAFTERTJ = TRIPLE JOIST OC = ON CENTERCL = CENTER LINE PL = POINT LOAD

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

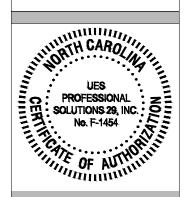
DECK JOISTS SHALL BE SPACED AT A MAX. 12" O.C. WHEN DECK BOARDS ARE INSTALLED DIAGONALLY.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

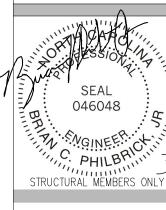
DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.





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DATE: 03/10/2025 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A20117.00091.000 DRAWN BY: MGC CHECKED BY: BCP

ORIGINAL INFORMATION PROJECT # DATE A20117.00091.000 02/26/2025

	REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	NAA TEDIAI	AUNI THOUNIECC	REQUIRED (CONNECTION	
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS	
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6"O.C.	6d COMMON NAILS @ 12" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** @ 7" O.C.	5d COOLER NAILS** @ 7" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6"O.C.	6d COMMON NAILS @ 12" O.C.	
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1	
**OR EQUIVALENT PER TABLE R702.3.5					

BRACED WALL NOTES:

- 1. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE
- 2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE DESIGN WIND SPEEDS UP TO 130 MPH.
- 3. REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES. 4. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- 6. MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1. 7. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD
- 8. FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END
- 9. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE.
- 11. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4 13. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN
- ACCORDANCE WITH SECTION R602.10.4.5 14. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN
- ACCORDANCE WITH SECTION R602.104.6 15. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1
- 16. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- 17. ABBREVIATIONS:

WSP = WOOD STRUCTURAL PANEL GB = GYPSUM BOARD CS-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION ENG-PF = ENG. PORTAL FRAMEPF = PORTAL FRAME

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

HEADER SCHEDULE			
	TAG	SIZE	JACKS (EACH END)
	А	(2) 2x6	(1)
	В	(2) 2x8	(2)
	С	(2) 2x10	(2)
	D	(2) 2x12	(2)
	E	(2) 9-1/4" LSL/LVL	(3)
	F	(3) 2x6	(1)
	G	(3) 2x8	(2)
	Н	(3) 2x10	(2)
		(3) 2x12	(2)
	MOTEC.	·	·

1. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (U.N.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (U.N.O.).

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>02/17/2025</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS 29, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES PROFESSIONAL SOLUTIONS 29, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

SECOND FLOOR BRACING (FT

CONTINUOUS SHEATHING METHOD - ELEVATION

5.9

5.9

5.1

5.1

BWL 2-1

BWL 2-2

BWL 2-A

BWL 2-B

PROVIDE

32.0

39.7

39.0

STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

<u>SECOND FLO</u>OR FRAMING PLAN SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"

DWI 2 2	B	B	03-W3I	DWI 2 2
BWL 2-2 				#0-21 #0-21
SEE TWO STORY WALL NOTE	CROOF TRUSSES PER MANUF.		ROOF TRUSSES PER MANUF.	26'-0" CS-WSP
20'-2" CS-WSP		DOES NO- OPTIONAI	TRAMING PLAN T CHANGE WITH L BEDROOM 5	
BWL 2-1	ROOF TRUS A 2'-6"	3.5"x7" 1.8E PSL COL USS PER MANUF. SSES PER MANUF. A A A 3.5"x7" 1.8E PSL COL 3.5"x7" 1.8E	E	BWL 2-1
BWL	CS-WSP CS-WSP	CS-WSP CS-		CS-WSP BWL 2-B

ELEVATION A

KING STUD	SCHEDULE
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
3'-0"	(1)
4'-0"	(2)
8'-0"	(3)
12'-0"	(5)
16'-0"	(6)

WALL STUD SCHEDULE	(10FT HEIGHT	MAX.)
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,				
STUD SIZE	STUD SPACING (O.C.)			
	ROOF ONLY	ROOF & 1 FLOOR	ROOF & 2 FLOORS	NON-LOA BEARING
2x4	24"	16"	12"	24"
2x6	24"	24"	16"	24"
NOTES:				

- 1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 16" O.C.
- 3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ CROSS BRACING @ 6'-0" O.C. VERTICALLY.

		ULE		
	TAG	SIZE	OPENING SIZE	
		L3x3x1/4"	LESS THAN 6'-0"	
	2	L5x3x1/4"	3x1/4" 6'-0" TO 10'-0"	
	3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"	
	4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS	
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3) ALL HEADERS WHERE BRICK IS USED, TO BE: 1 (UNO)				
			O BE: (1) (UNO)	

GENERAL STRUCTURAL NOTES:

BWL 2-B

1. CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL

BUILDING CODE WITH ALL LOCAL AMENDMENTS.

2. CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.

- 3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
- 4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS: MICROLLAM (LVL): $F_b = 2600 \text{ PSI}$, $F_v = 285 \text{ PSI}$, $E = 2.0 \text{x} 10^6 \text{ PSI}$ PARALLAM (PSL): $F_b = 2900$ PSI, $F_v = 290$ PSI, $E = 1.25 \times 10^6$ PSI
- 5. ALL WOOD MEMBERS SHALL BE #2 SYP UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE #2 SYP (UNO).
- 6. ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 #2 SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERWISE.
- 7. ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".
- 8. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.1.6. MINIMUM 1/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A 7" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION AND (1) LOCATED NOT MORE THAN 12" FROM THE CORNER ANCHOR BOLTS. SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- 9. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- 11. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP #2, DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP #2, DROPPED. (UNLESS NOTED OTHERWISE)
- 12. ABBREVIATIONS:

DJ = DOUBLE JOIST	SJ = SINGLE JOIST
GT = GIRDER TRUSS	FT = FLOOR TRUSS
SC = STUD COLUMN	DR = DOUBLE RAFTER
EE = EACH END	TR = TRIPLE RAFTER
TJ = TRIPLE JOIST	OC = ON CENTER
CL = CENTER LINE	PL = POINT LOAD

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

DECK JOISTS SHALL BE SPACED AT A MAX. 12" O.C. WHEN DECK BOARDS ARE INSTALLED DIAGONALLY.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

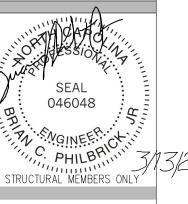
== DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.





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DATE: 03/10/2025 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A20117.00091.000 DRAWN BY: MGC CHECKED BY: BCP

ORIGINAL INFORMATION PROJECT # DATE A20117.00091.000 02/26/2025

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S4.0

TRUSS UPLIFT CONNECTOR SCHEDULE				
MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND	
600 LBS	H2.5A	PER WALL SHEATHIN	G & FASTENERS	
1200 LBS	(2) H2.5A	CS16 (END = 11")	DTT2Z	
1450 LBS	HTS20	CS16 (END = 11")	DTT2Z	
2000 LBS	(2) MTS20	(2) CS16 (END = 11")	DTT2Z	
2900 LBS	(2) HTS20	(2) CS16 (END = 11")	HTT4	
3685 LBS	LGT3-SDS2.5	MSTC52	HTT4	

1. ALL PRODUCTS LISTED ARE SIMPSON STRONG—TIE. EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.
2. UPLIFT VALUES LISTED ARE FOR SYP #2 GRADE MEMBERS.
3. REFER TO TRUSS LAYOUT PER MANUF. FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.
4. CONTACT UES FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

REFER TO DETAIL 5/D3f FOR EYEBROW, RETURN OR SHED ROOF FRAMING REQUIREMENTS. (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION R802.11.1.1. WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC. REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>02/17/2025</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS 29, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES PROFESSIONAL SOLUTIONS 29, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

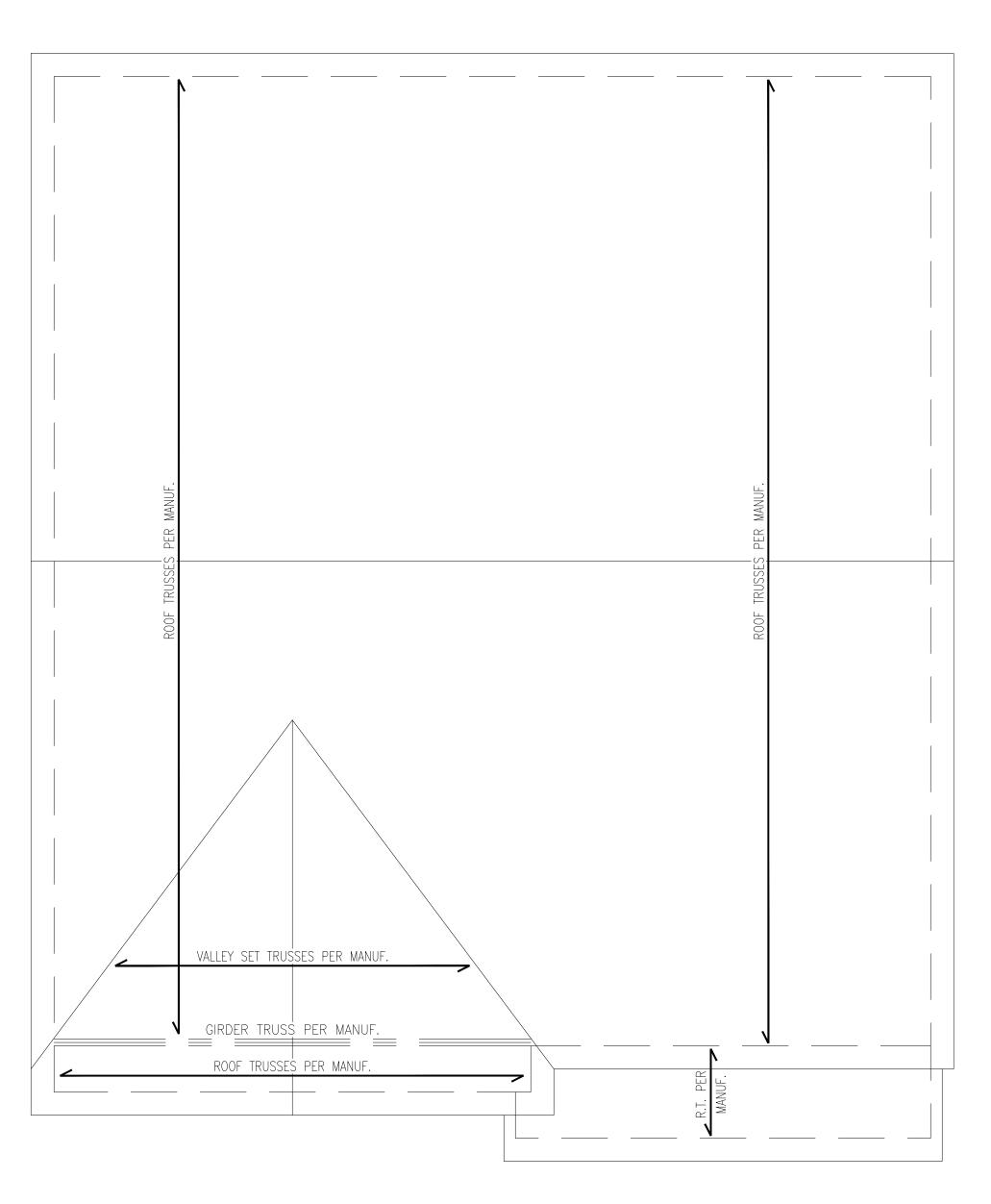
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



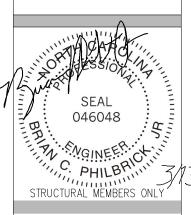






CLIENT:
DR Horton, Inc.
8001 Arrowridge Blvd.
Charlotte, NC, 28273

Hanover Plan LH (NC) Roof Framing Pla



DATE: 03/10/2025

SCALE: 22x34 1/4"=1'-0"
11x17 1/8"=1'-0"

PROJECT #: A20117.00091.000

DRAWN BY: MGC

CHECKED BY: BCP

ORIGINAL INFORMATION

PROJECT # DATE
A20117.00091.000 02/26/2025

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S5.0

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

Design Loads:

1.	Roof Live Loads			
	1.1. Conventional 2x	20	PSF	
	1.2. Truss	20	PSF	
	1.2.1.Attic Truss	60	PSF	
2.	Roof Dead Loads			
	2.1. Conventional 2x	10	PSF	
	2.2. Truss	20	PSF	
3.	Snow			
	3.1. Importance Factor	1.0		

4. Floor Live Loads 4.1. Typ. Dwelling 40 PSF 30 PSF 4.2. Sleeping Areas . 40 PSF 4.3. Decks 50 PSF 4.4. Passenger Garage 5. Floor Dead Loads 10 PSF 5.1. Conventional 2x 5.2. I-Joist 15 PSF

7.1. Importance Factor.. 8. Component and Cladding (in PSF)

6. Ultimate Wind Speed (3 sec. gust) .

5.3. Floor Truss.

7. Exposure

•	,	*		
MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

9. Seismic

9.1. Site Class . 9.2. Design Category . 9.3. Importance Factor

9.4. Seismic Use Group. 9.5. Basic Structural System (check one) ⊠ Bearing Wall

> ☐ Building Frame ☐ Moment Frame □ Dual w/ Special Moment Frame

□ Dual w/ Intermediate R/C or Special Steel ☐ Inverted Pendulum

9.6. Arch/Mech Components Anchored. 9.7. Lateral Design Control: Seismic ☐ Wind⊠ 10. Assumed Soil Bearing Capacity 2000psf



UES PROFESSIONAL SOLUTIONS 29, INC

FORMERLY SUMMIT ENGINEERING, LABORATORY, & TESTING INC.

STRUCTURAL PLANS PREPARED FOR:

STANDARD DETAILS (OX-IS)

PROJECT ADDRESS:

. 15 PSF

. PER PLAN

DR Horton Carolinas Division 8001 Arrowridge Blvd Charlotte, NC 28273

ARCHITECT/DESIGNER: GMD Design Group 1845 Satellite Blvd. Duluth, GA 30097

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify UES Professional Solutions 29, Inc. (UES) before construction begins.

PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
OC	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to UES Professional Solutions 29, Inc. (UES) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton</u>, <u>Inc</u>. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify UES immediately.

SHEET LIST:

CS1 Cover Sheet, Specifications, Revisions D1m Monolithic Slab Foundation Details D1s Stem Wall Foundation Details D1c Crawl Space Foundation Details D1b Basement Foundation Details D1f Framing Details	Sheet No.	Description
D1s Stem Wall Foundation Details D1c Crawl Space Foundation Details D1b Basement Foundation Details	CS1	Cover Sheet, Specifications, Revisions
D1c Crawl Space Foundation Details D1b Basement Foundation Details	D1m	Monolithic Slab Foundation Details
D1b Basement Foundation Details	D1s	Stem Wall Foundation Details
Business Foundation Bottom	D1c	Crawl Space Foundation Details
D1f Framing Datails	D1b	Basement Foundation Details
Truming Details	D1f	Framing Details

DR HORTON PROJECT SIGN-OFF:

Manager	Signature
Operations	
Operations System	
Operations Product Development	

SOLUTIONS 29, INC.

10121 Pineville Distribution St Pineville, NC 28134 Office: 704.504.1717 Fax: 704.504.1125

<u>REVISION LIST:</u>

Revision No.	Date	Project No.	Description
1	5.11.17		Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem waand crawl space foundations
2	7.12.17		Revised stem wall insulation note.
3	2.15.18		Revised garage door detail, NC only
4	2.28.18		Added high—wind foundation details
5	12.19.18		Revised per 2018 NCRC
6	2.19.19		Revised per Mecklenburg County Comments
7	3.1.19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3.6.19		Corrected dimensions at perimeter footings
9	3.2.20		Added tall turndown detail
10	3.18.20		Added balloon framing detail
11	10.20.20		Added alternate two—pour detail for slab and added note for crawl girder above grade
12	3.1.21		Added OX—IS Standard Details
13	5.18.21		Updated OX—IS Standard Details
14	02.14.23		Added 4/D2m — Tall Slab Detail w/ Siding
15	08.10.23		Updated (Hit HY150 Adhesive) for HY200 Adhesive
16	04.01.24		Added Hilti Kwik Bolt KBI 1/2—5 TO Wall Anchor Schedule
17	4.26.24		Update Wall Anchor Schedule
18	5.06.24		Update Wall Anchor Schedule

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- GENERAL STRUCTURAL NOTES: The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of UES Professional Solutions 29, Inc. (UES) or the SER. For the purposes of these construction documents the SER and UES shall be considered the
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- 3. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents. should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to UES for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or UES.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to UES before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- . This structure and all construction shall conform to all applicable sections of the international residential code. 8. This structure and all construction shall conform to all applicable
- sections of local building codes. 9. All structural assemblies are to meet or exceed to requirements of the current local building code.

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below
- 3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. 4. The resulting soil shall be compacted to a minimum of 95%
- maximum dry density. 5. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur

within 24 hours of excavation.

6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

STRUCTURAL STEEL:

- . Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- 2. Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- 3. All steel shall have a minimum yield stress (F_v) of 36 ksi unless otherwise noted.
- 4. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shop and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.

- . Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:

3.1. Footings: 5%

3.2. Exterior Slabs: 5% 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- 5. Concrete slabs—on—grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab—on—grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs—on—grade at a maximum of 15'—0" O.C. and in exterior slabs—on—grade at a maximum of 10'—0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished 9. Reinforcing steel may not extend through a control joint.
- Reinforcing steel may extend through a saw cut joint. 10. All welded wire fabric (W.W.F.) for concrete slabs—on—grade shall be placed at mid-depth of slab. The W.W.F. shall be securely

CONCRETE REINFORCEMENT:

supported during the concrete pour.

- 1. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs—on—grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- 3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard) 4. Fibermesh shall comply with ASTM C1116, any local building code

requirements, and shall meet or exceed the current industry

- 5. Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures" 7. Horizontal footing and wall reinforcement shall be continuous and
- as the horizontal reinforcement with a class B tension splice. 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

shall have 90° bends, or corner bars with the same size/spacing

- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted. 11. Unless otherwise specified, concrete reinforcing is not required.

1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be

- Spruce-Yellow-Pine (SYP) #2. 2. LVL or PSL engineered wood shall have the following minimum design values:
 - 2.1. E = 1,900,000 psi
 - 2.2. $F_{b} = 2600 \text{ psi}$ 2.3. $F_v = 285 \text{ psi}$ 2.4. $F_c = 700 \text{ psi}$
- 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted. 5. Lag screws shall conform to ANSI/ASME standard B18.2.1—1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- 6. All beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- 8. Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.
- 9. Multi-ply beams shall have each ply attached with (3) 10d nails @ 24" O.C.

10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 24" O.C. per schedule unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam.

WOOD TRUSSES:

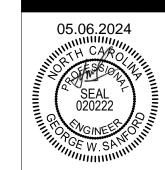
- 1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping,
- and architectural fixtures attached to the trusses. 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB—91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

- 1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- 2. All structurally required wood sheathing shall bear the mark of
- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- 4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)—8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 5. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)—8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.



STRUCTURAL MEMBERS ONLY

DATE: 05/06/2024 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A21117.00066.000 DRAWN BY: MGC

CHECKED BY: GWS

ORIGINAL INFORMATION PROJECT # DATE 1/31/2017





6A COVERED PATIO DETAIL

6 PATIO SLAB DETAIL

WUES FORMERLY SUMMIT ENGINEERING, LABORATORY, & TESTING, INC.







CLIENT: DR Horton Carolina Div 8001 Arrowridge Blvd. Charlotte, NC 28273

Detai

05.06.2024

STRUCTURAL MEMBERS ONLY

SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0"

PROJECT #: A21117.00066.000

PROJECT # DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D1m

DATE: 05/06/2024

DRAWN BY: MGC

CHECKED BY: GWS

ORIGINAL INFORMATION

DRAWING

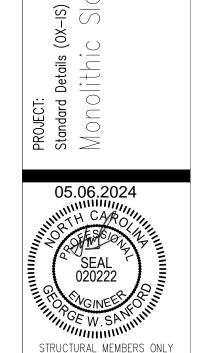
CONNECTIONS

5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL

6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE

ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

AMENDMENTS AND REQUIREMENTS NOT SHOWN



DRAWING

DATE: 05/06/2024

DRAWN BY: MGC

CHECKED BY: GWS

ORIGINAL INFORMATION

SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0"

PROJECT #: A21117.00066.000

PROJECT # DATE 1/31/2017

NOTES:

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET

(2) #4 REBAR -

4 TALL SLAB DETAIL W/ SIDING

CONTINUOUS

PER PLAN

FOR ADDITIONAL INFORMATION.

2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS—ON—GRADE.

3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS

5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN

6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

UES PROFESSIONAL SOLUTIONS 29, INC.

FORMERLY SUMMIT ENGINEERING, LABORATORY, & TESTING, INC.

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www.feamues.com

UES
PROFESSIONAL
No. F-1454
OF AUTHORITI

D2m

<u>SECTION</u>



OVER RAISED WOOD FLOOR - OVERLAP OPTION

FRONT ELEVATION





CLIENI: DR Horton Carolina Divis 8001 Arrowridge Blvd. Charlotte, NC 28273

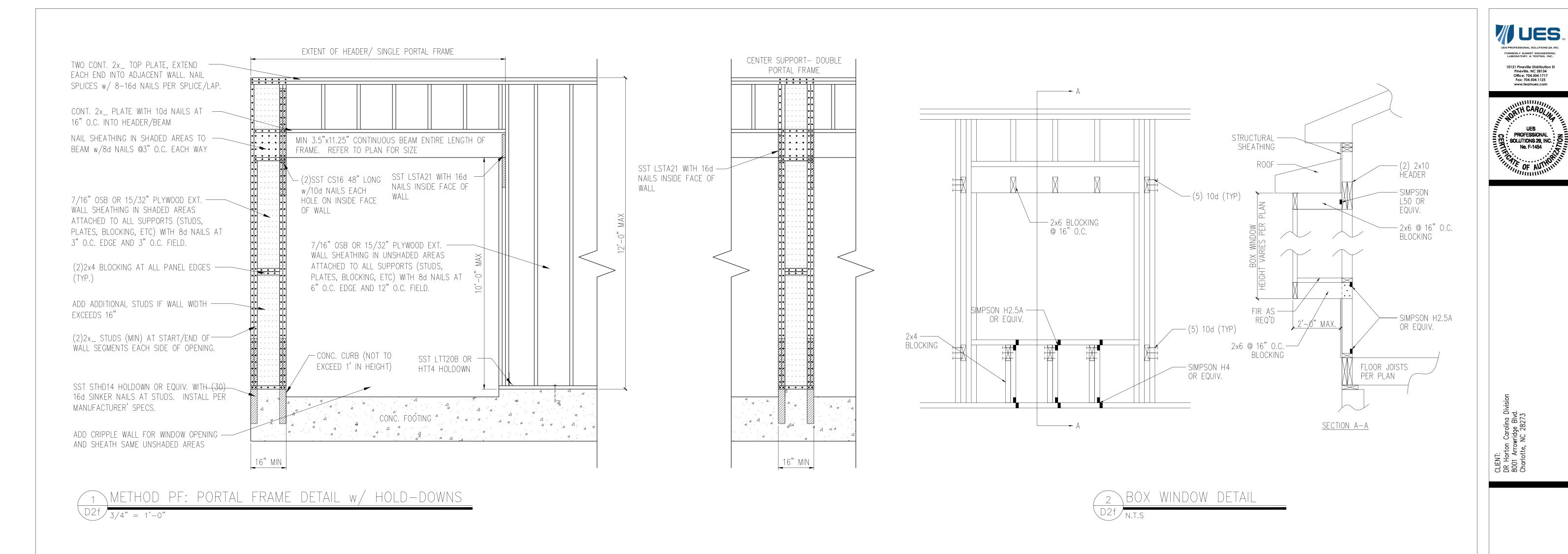
Standard Details (OX-IS)
Framing Details

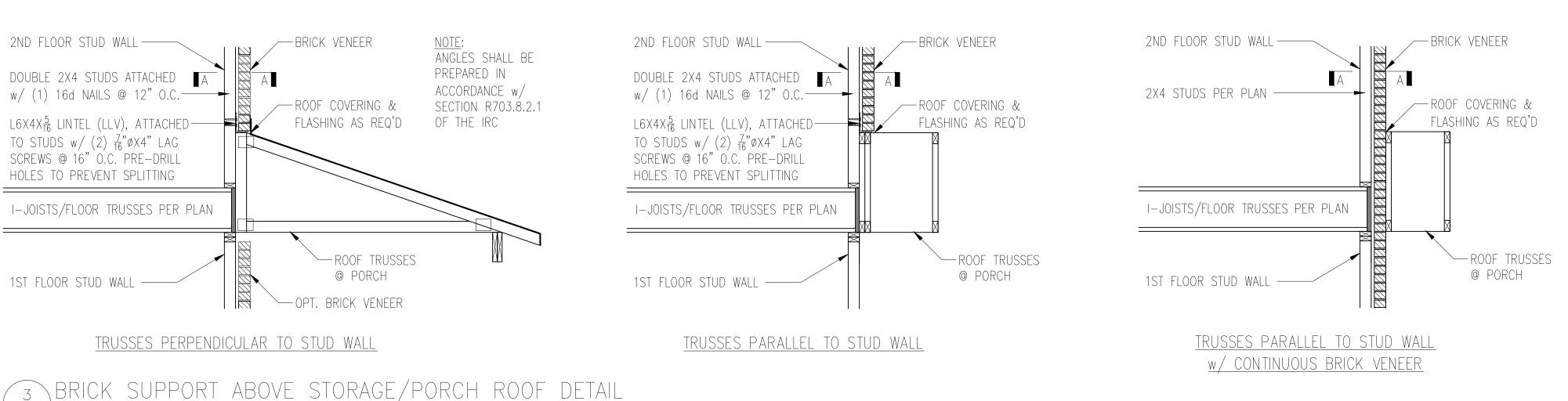


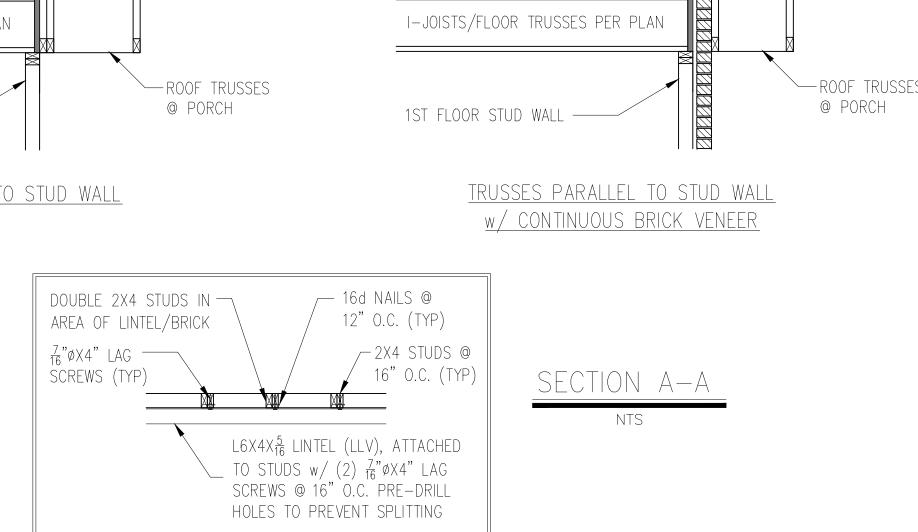
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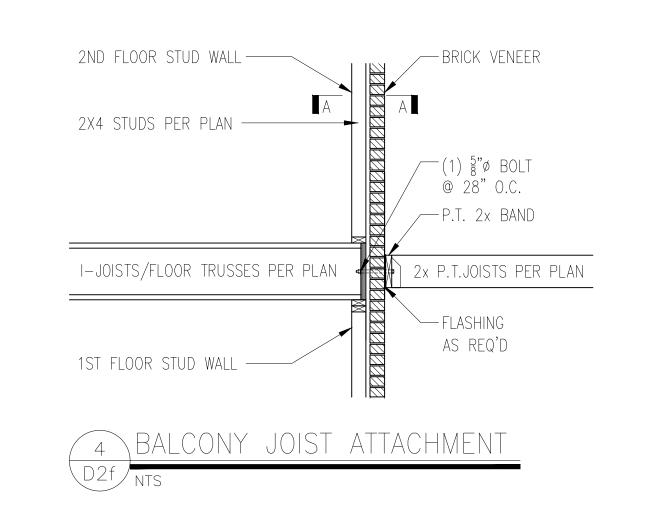
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PROJECT # DATE
1/31/2017











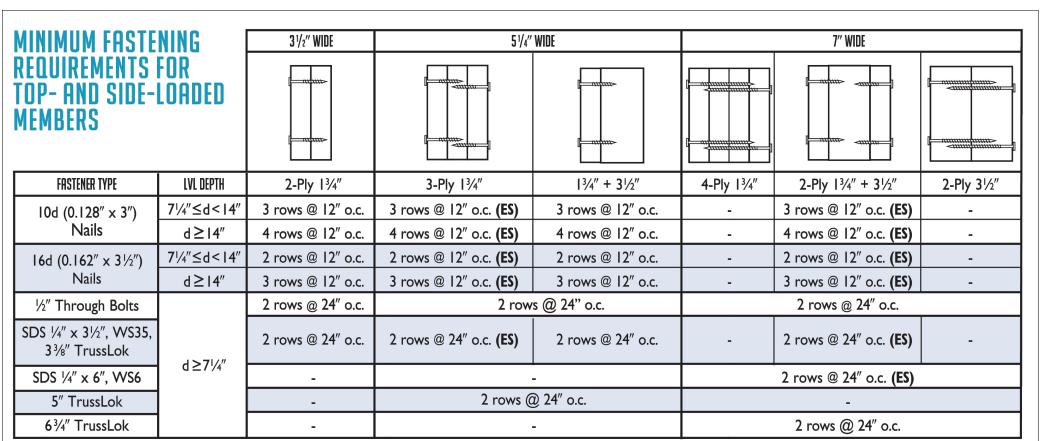
(ox-IS) Details

STRUCTURAL MEMBERS ONLY

DATE: 05/06/2024 SCALE: 22x34 1/4"=1'-0" 11x17 1/8"=1'-0" PROJECT #: A21117.00066.000 DRAWN BY: MGC CHECKED BY: GWS

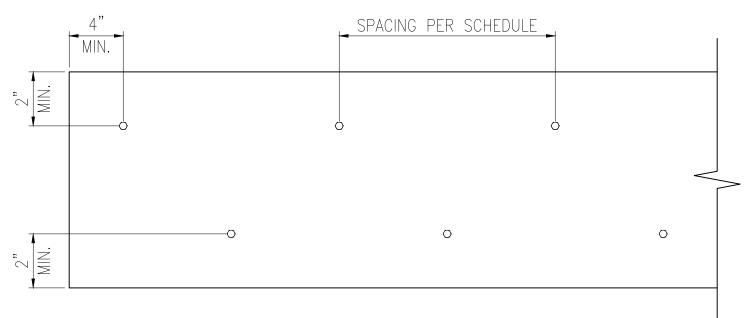
ORIGINAL INFORMATION

PROJECT # DATE 1/31/2017 REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS



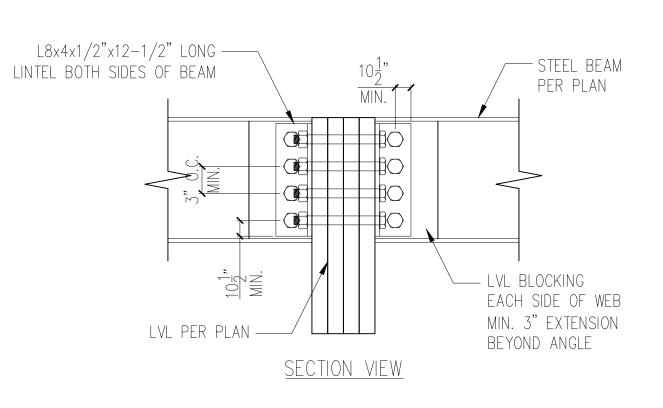
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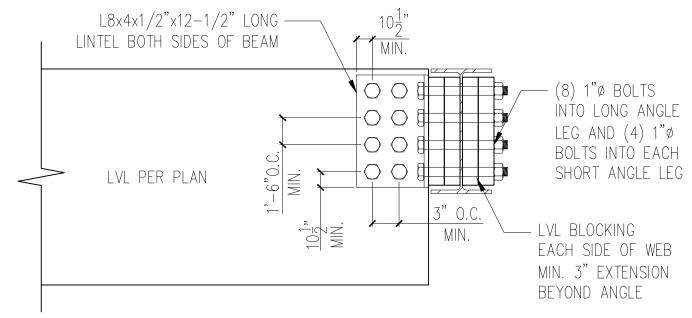
- I.All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- 2. Minimum fastening requirements for depths less than $7\frac{1}{4}$ " require special consideration. Please contact your technical representative.
- 3. Three general rules for staggering or offsetting for a certain fastener schedule:
- (1) if staggering or offsetting is not referenced, then none is required;(2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener
- clearances above; and
 (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



ELEVATION VIEW

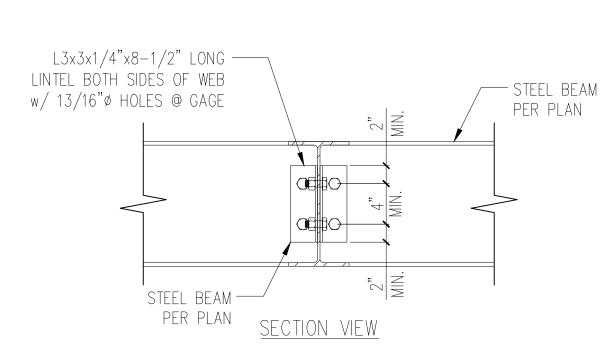
1 MULTI-PLY BEAM CONNECTION DETAIL

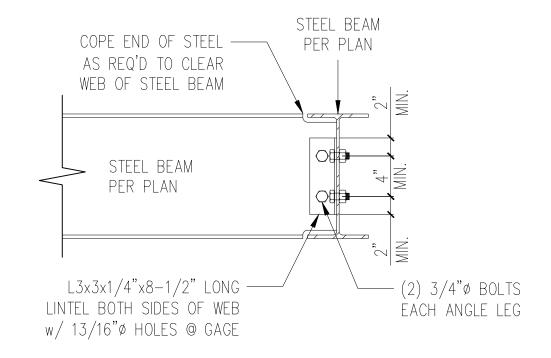




ELEVATION VIEW

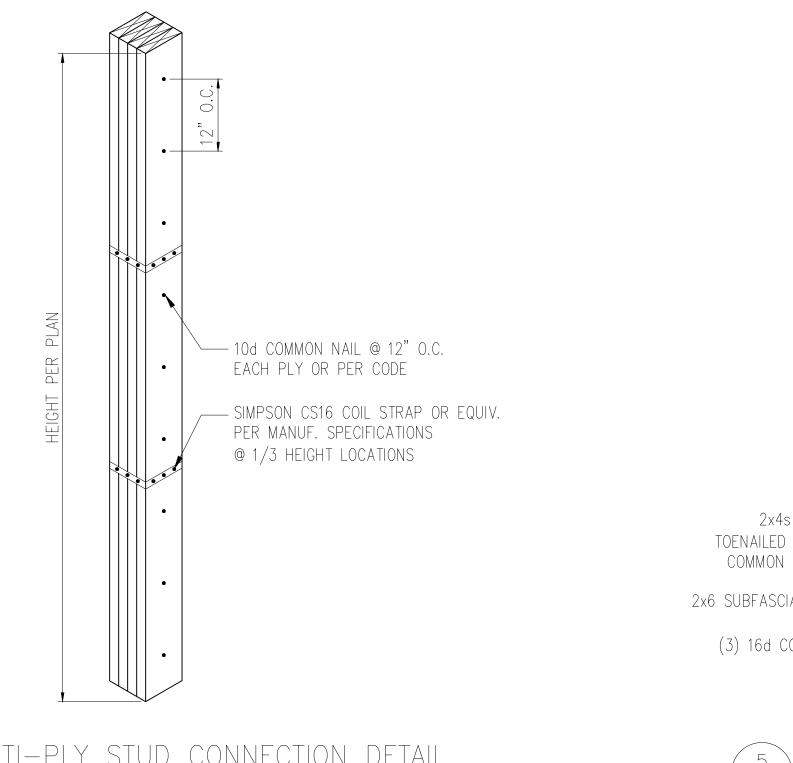




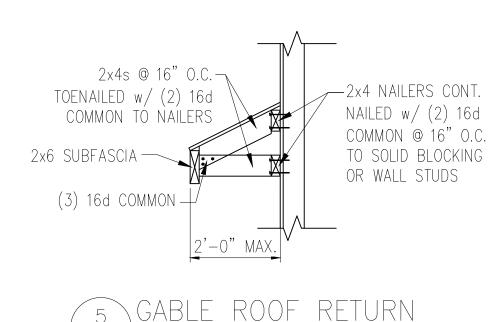


<u>ELEVATION VIEW</u>

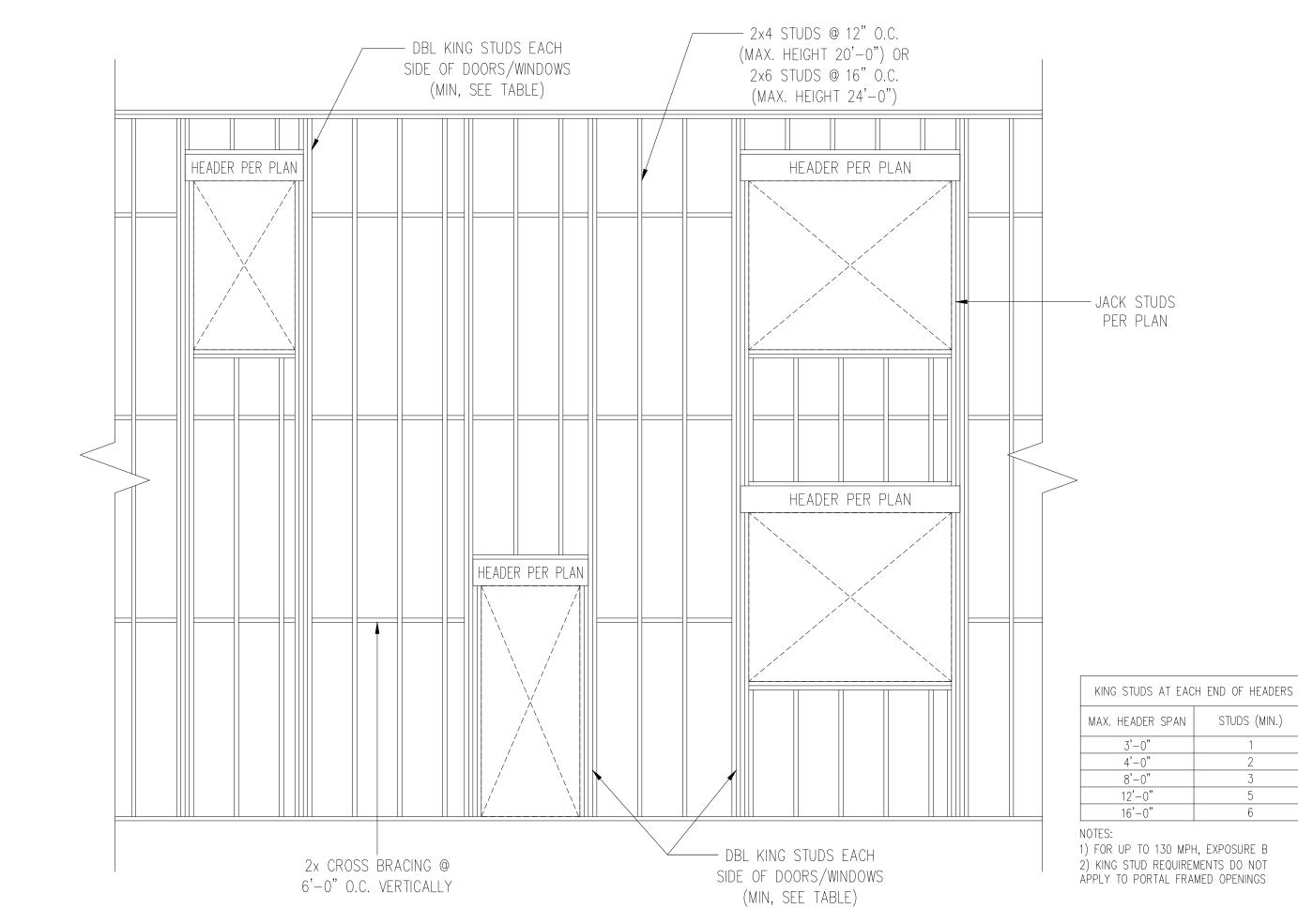








5 GABLE ROOF RETURN
D3f



	6	TYP.	BALLOON	FRAMING	DETAIL
--	---	------	---------	---------	--------

D3f N.T.S





CLIENT: DR Horton Carolina Divisio 8001 Arrowridge BIvd.

Standard Details (0X-1S)
Framing Details



DRAWING

DATE: 05/06/2024

SCALE: 22x34 1/4"=1'-0"
11x17 1/8"=1'-0"

PROJECT #: A21117.00066.000

DRAWN BY: MGC
CHECKED BY: GWS

ORIGINAL INFORMATION
PROJECT # DATE
1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

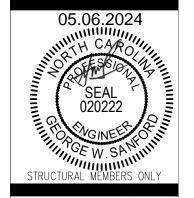
D3f

YP. EXTERIOR CORNER ATTACHMENT

WUES FORMERLY SUMMIT ENGINEERING, LABORATORY, & TESTING, INC. 10121 Pineville Distribution St Pineville, NC 28134 Office: 704.504.1717 Fax: 704.504.1125 www.teamues.com



Details (0X-IS)

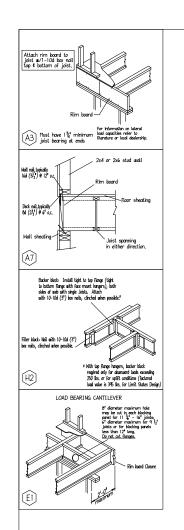


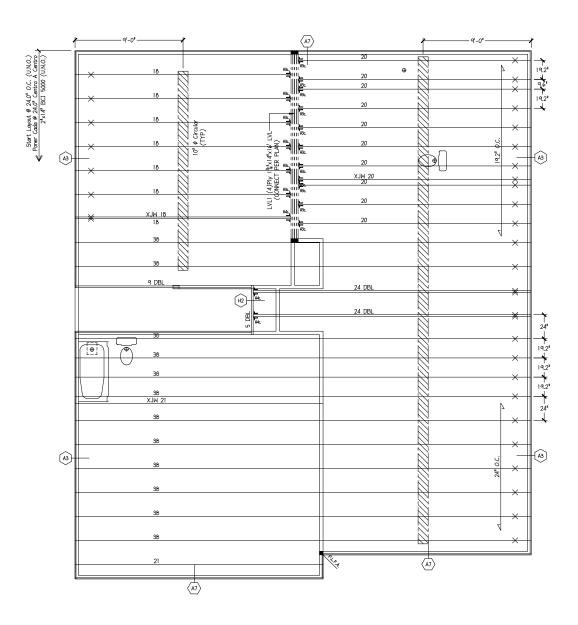
DATE: 05/06/2024 DRAWN BY: MGC CHECKED BY: GWS

> ORIGINAL INFORMATION PROJECT # DATE 1/31/2017

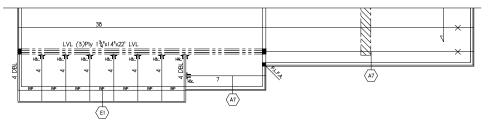
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D4f

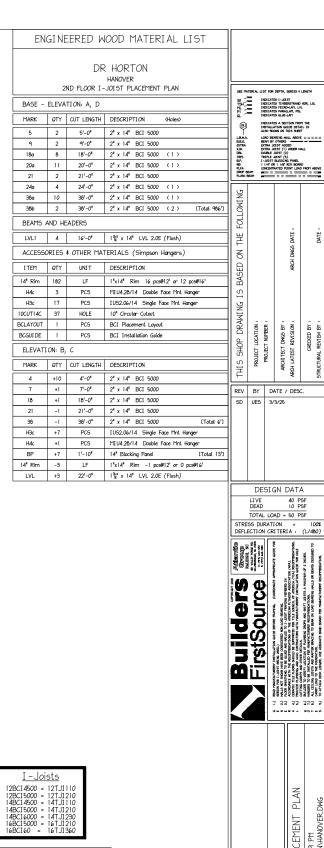




ELEVATIONS: A, D 2ND FLOOR I-JOIST PLACEMENT PLAN DR HORTON - HANOVER



ELEVATIONS: B, C



POINT LOAD FROM ABOVE (P.L.F.A.) USE SOLID BLOCKING TO TRANSFER LOAD FROM ABOVE TO FOUNDATION

DO NOT CUT ORANGE END NO CORTE EL COLOR NARANJA

"X" INDICATES ORANGE END "X" INDICAR EL COLOR NARANJA

