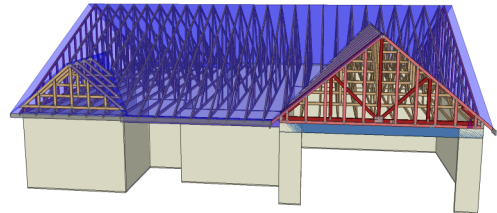




Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: Lamco Custom Builders LLC

**Model: Kristie A RH - 75 Blackberry
Manor**

THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

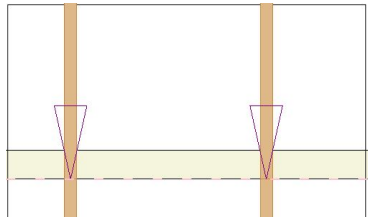
Date: _____

General Notes:

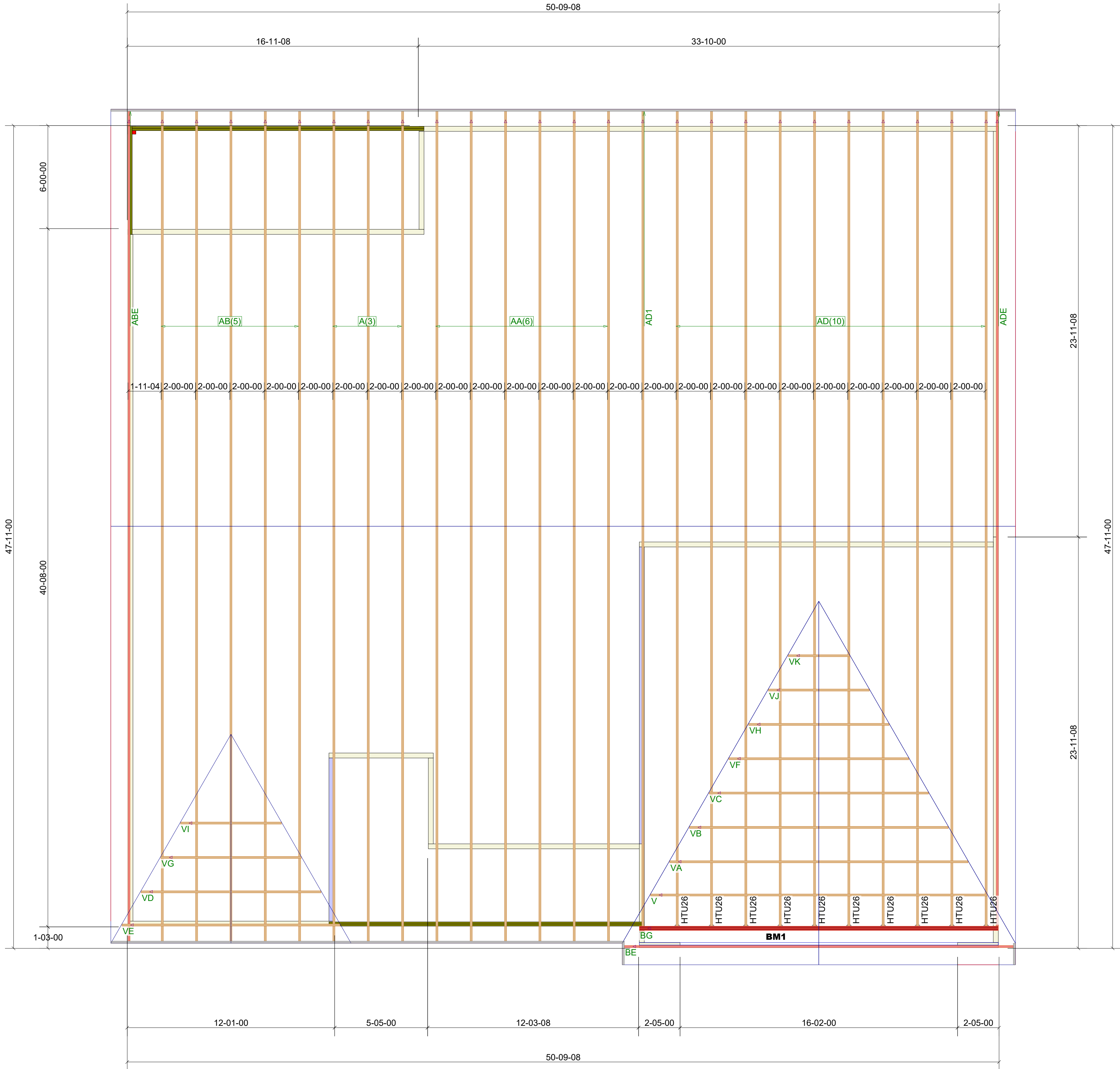
** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS. ** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT. ** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.



Truss Drawing Left End Indicator



Products					
PlotID	Length	Product	Piles	Net Qty	Fab Type
BM1	22'-05"-00	2.1 RigidLam SP LVL 1-3/4 x 11-7/8	2	2	FF

Truss Connector Total List		
Manuf.	Product	Qty
Simpson	HTU26	10
Simpson	One H2.5A	38

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS. ** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH. ** All uplift connectors shown within these documents are recommendations only. Per ANSII/TPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53179



Lamco Custom Builders LLC
75 Blackberry
Manor-Roof-Kristie A RH
ROOF PLACEMENT PLAN

Scale:	NTS
Date:	10/8/2025
Designer:	Mike Finch
Project Number:	25090182-01
Sheet Number:	

1/1

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.



Customer:
Job Name:
City:
Customer Ph...

Job Name: **01**
Level: **1st Floor**
Label: **BM1 - i45**
Type: **Beam**

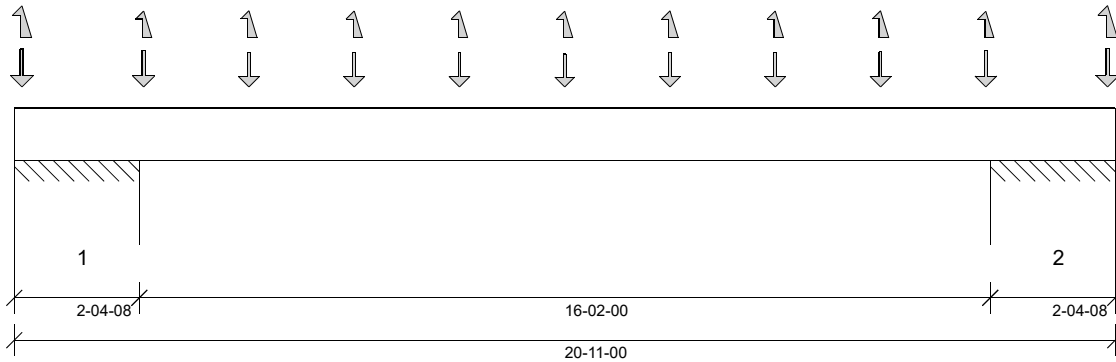
2 Ply Member
2.1 RigidLam SP LVL 1-3/4
x 11-7/8

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.7.3.303.Update13.26

Report Version: 2023.09.18 10/08/2025 14:29



DESIGN INFORMATION a

Building Code: IRC2015
Design Methodology: ASD
Risk Category: II (General Construction)
Residential
Service Condition: Dry
System Spacing: -
LL Deflection Limit: L/360, 0.75" (absolute)
TL Deflection Limit: L/240, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 20'- 11" Bottom: 20'- 11"

Bearing Stress of Support Material:

- 725 psi Wall @ 0'- 1 1/2"
- 725 psi Wall @ 2'- 3"
- 725 psi Wall @ 18'- 8"
- 725 psi Wall @ 20'- 9 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	10'- 5 1/2"	D + Lr	1.15	1087 lb ft	17352 lb ft	Passed - 6%
Max Neg. Moment:	18'- 8"	D + Lr	1.15	1693 lb ft	17352 lb ft	Passed - 10%
Max Shear:	17'- 6 5/8"	D + Lr	1.15	592 lb	9241 lb	Passed - 6%
Live Load (LL) Pos. Defl.:	10'- 5 1/2"	Lr		0.018"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	10'- 5 1/2"	D + Lr		0.037"	L/240	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	9-00	0.6D + 0.6W	1.60	63 lb		32870 lb	22838 lb	Passed - 0%
1	9-00	D + Lr	1.15		-717 lb	-	-	
1	1-06-00	D + Lr	1.15	1558 lb		47250 lb	45675 lb	Passed - 3%
1	1-06-00	0.6D + 0.6W	1.60		-247 lb	-	-	
2	1-06-00	D + Lr	1.15	1558 lb		47250 lb	45675 lb	Passed - 3%
2	1-06-00	0.6D + 0.6W	1.60		-245 lb	-	-	
2	9-00	0.6D + 0.6W	1.60	60 lb		32870 lb	22838 lb	Passed - 0%
2	9-00	D + Lr	1.15		-717 lb	-	-	

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	20'- 11"	Self Weight	Top	11 lb/ft	-	-	-	-
Point	0'- 1 3/4"	0'- 1 3/4"	BE(Cond01)	Top	66 lb	-	30 lb	92 lb	45/-171 lb
Point	2'- 5 1/2"	2'- 5 1/2"	BE(Cond01)	Top	79 lb	-	23 lb	81 lb	35/-117 lb
Point	4'- 5 1/2"	4'- 5 1/2"	BE(Cond01)	Top	68 lb	-	22 lb	76 lb	34/-120 lb
Point	6'- 5 1/2"	6'- 5 1/2"	BE(Cond01)	Top	71 lb	-	22 lb	72 lb	34/-118 lb
Point	8'- 5 1/2"	8'- 5 1/2"	BE(Cond01)	Top	70 lb	-	22 lb	71 lb	36/-119 lb
Point	10'- 5 1/2"	10'- 5 1/2"	BE(Cond01)	Top	62 lb	-	21 lb	79 lb	17/-116 lb
Point	12'- 5 1/2"	12'- 5 1/2"	BE(Cond01)	Top	70 lb	-	22 lb	71 lb	36/-119 lb
Point	14'- 5 1/2"	14'- 5 1/2"	BE(Cond01)	Top	71 lb	-	22 lb	72 lb	34/-118 lb
Point	16'- 5 1/2"	16'- 5 1/2"	BE(Cond01)	Top	69 lb	-	22 lb	77 lb	34/-120 lb
Point	18'- 5 1/2"	18'- 5 1/2"	BE(Cond01)	Top	77 lb	-	22 lb	81 lb	33/-114 lb
Point	20'- 9 1/4"	20'- 9 1/4"	BE(Cond01)	Top	67 lb	-	31 lb	93 lb	46/-174 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	2'- 4 1/2"	E8(i14)	848/-349 lb	-	201/-72 lb	787/-356 lb	275 lb/-807 lb
==>	0'- 1 1/2"	0'- 1 1/2"	E8(i14)	-349 lb	-	-72 lb	98/-350 lb	-
==>	2'- 3"	2'- 3"	E8(i14)	848 lb	-	201 lb	689/-6 lb	-
2	18'- 6 1/2"	20'- 11"	E7(i11)	852/-352 lb	-	202/-73 lb	784/-350 lb	275 lb/-807 lb
==>	18'- 8"	18'- 8"	E7(i11)	852 lb	-	202 lb	691 lb	-
==>	20'- 9 1/2"	20'- 9 1/2"	E7(i11)	-352 lb	-	-73 lb	93/-350 lb	-

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 0.54

	Customer:	Job Name: 01	2 Ply Member	Status:
	Job Name:	Level: 1st Floor	2.1 RigidLam SP LVL 1-3/4 x 11-7/8	Design Passed
	City:	Label: BM1 - i45		
	Customer Ph...	Type: Beam		

PLY TO PLY CONNECTION

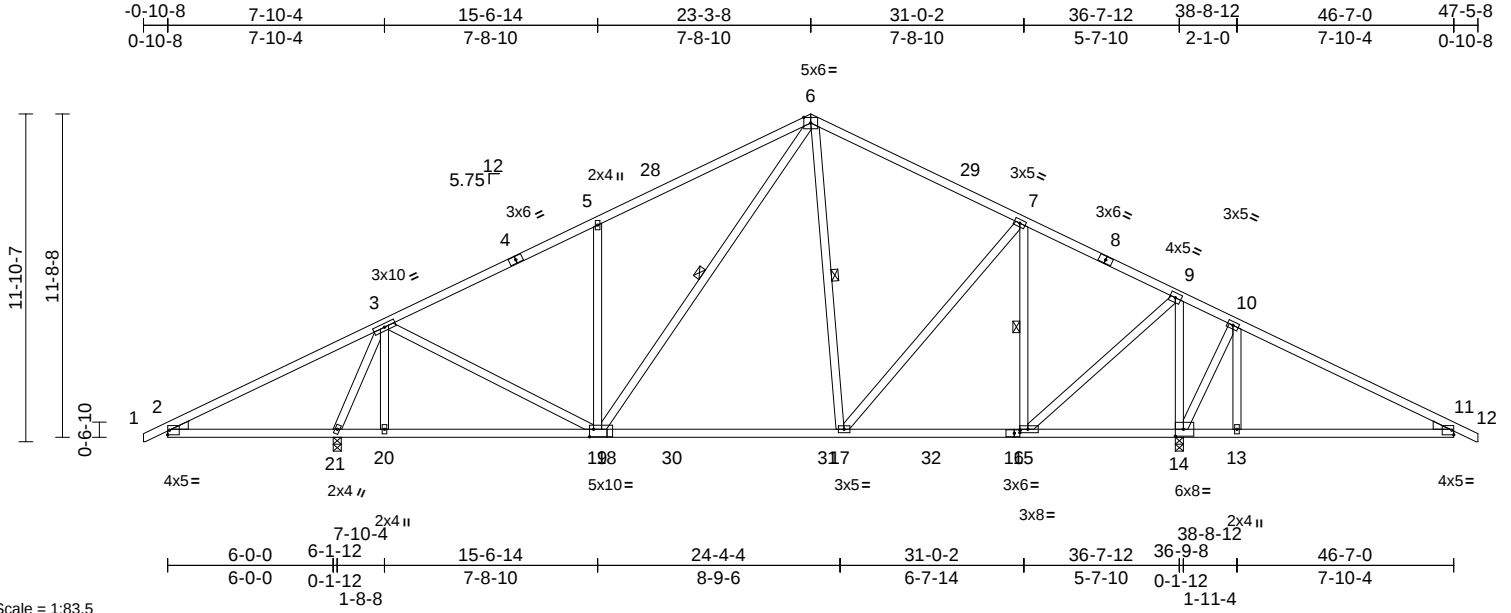
- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928219
25090182-01	A	Common	3	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:83.5

Plate Offsets (X, Y): [14:0-3-8,0-3-0], [15:0-3-8,0-1-8], [16:0-2-6,0-1-8], [19:0-1-12,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.77	Vert(LL)	-0.25	17-19	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.77	Vert(CT)	-0.37	17-19	>982	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(CT)	0.01	14	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 278 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 17-6,6-19:2x4 SP No.2
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-8-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-10 oc bracing.

WEBS 1 Row at midpt 7-15, 6-17, 6-19

REACTIONS (size) 14=0-3-8, 21=0-3-8
Max Horiz 21=114 (LC 15)
Max Uplift 14=-24 (LC 16), 21=-35 (LC 15)
Max Grav 14=2155 (LC 2), 21=1672 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/27, 2-3=-403/690, 3-5=-981/156, 5-6=-986/334, 6-7=-759/200, 7-9=-481/83, 9-10=-548/928, 10-11=-458/760, 11-12=0/27

BOT CHORD 2-21=-519/444, 20-21=-52/276, 19-20=-52/276, 17-19=0/658, 15-17=0/397, 14-15=-881/744, 13-14=-578/494, 11-13=-578/494

WEBS 5-19=-486/320, 7-15=-892/425, 9-14=-1579/557, 9-15=-536/1522, 6-17=-144/176, 6-19=-202/424, 3-20=-22/177, 3-19=-145/743, 7-17=-103/409, 3-21=-1577/409, 10-13=-252/225, 10-14=-657/541

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 .
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 21. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



October 9,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

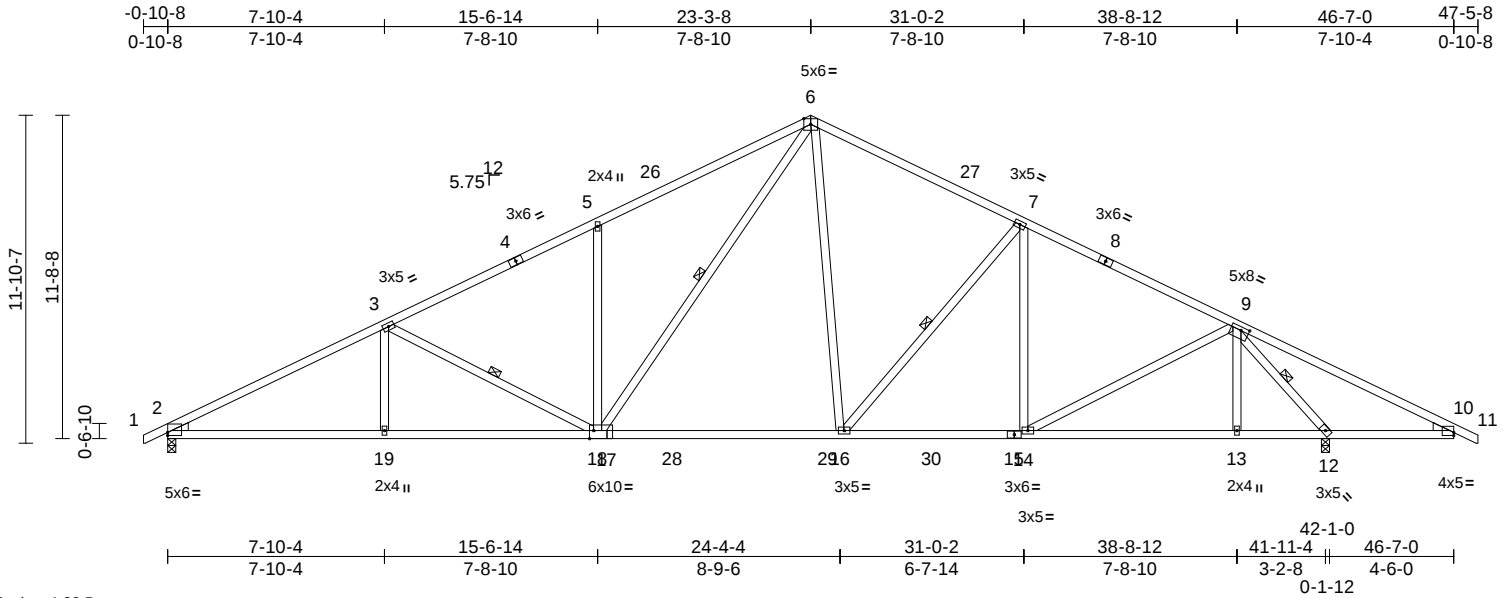
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928220
25090182-01	AA	Common	6	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:83.5

Plate Offsets (X, Y): [2:Edge,0-1-6], [9:0-3-9,0-1-8], [10:Edge,0-1-6], [18:0-1-12,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.78	Vert(LL)	-0.30	16-18	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.95	Vert(CT)	-0.55	16-18	>919	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(CT)	0.12	12	n/a	n/a	
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 268 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* 4-1,8-11:2x4 SP No.1
 BOT CHORD 2x4 SP No.1 *Except* 15-17:2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* 16-6,6-18:2x4 SP No.2
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
 WEBS 1 Row at midpt 9-12, 6-18, 3-18, 7-16

REACTIONS

(size) 2=0-3-8, 12=0-3-8
 Max Horiz 2=114 (LC 15)
 Max Grav 2=1755 (LC 2), 12=2177 (LC 2)

FORCES

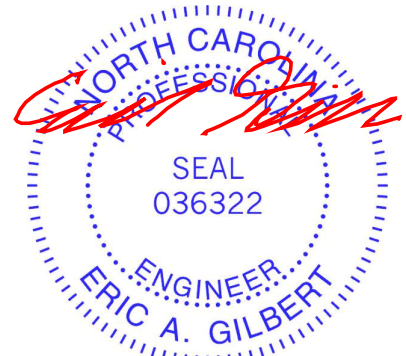
(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/28, 2-3=-3184/674, 3-5=-2547/616, 5-6=-2563/799, 6-7=-1872/558, 7-9=-2075/503, 9-10=-333/629, 10-11=0/28
 BOT CHORD 2-19=-491/2769, 18-19=-491/2769, 16-18=-62/1558, 14-16=-178/1785, 13-14=-85/1174, 12-13=-84/1174, 10-12=-465/383
 WEBS 9-12=-2551/663, 5-18=-527/321, 7-14=-167/138, 6-16=-38/570, 9-13=0/125, 6-18=-394/1212, 3-19=0/137, 3-18=-646/237, 9-14=-125/704, 7-16=-394/172

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.1 .
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



October 9,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
 A MiTek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928221
25090182-01	AB	Common	5	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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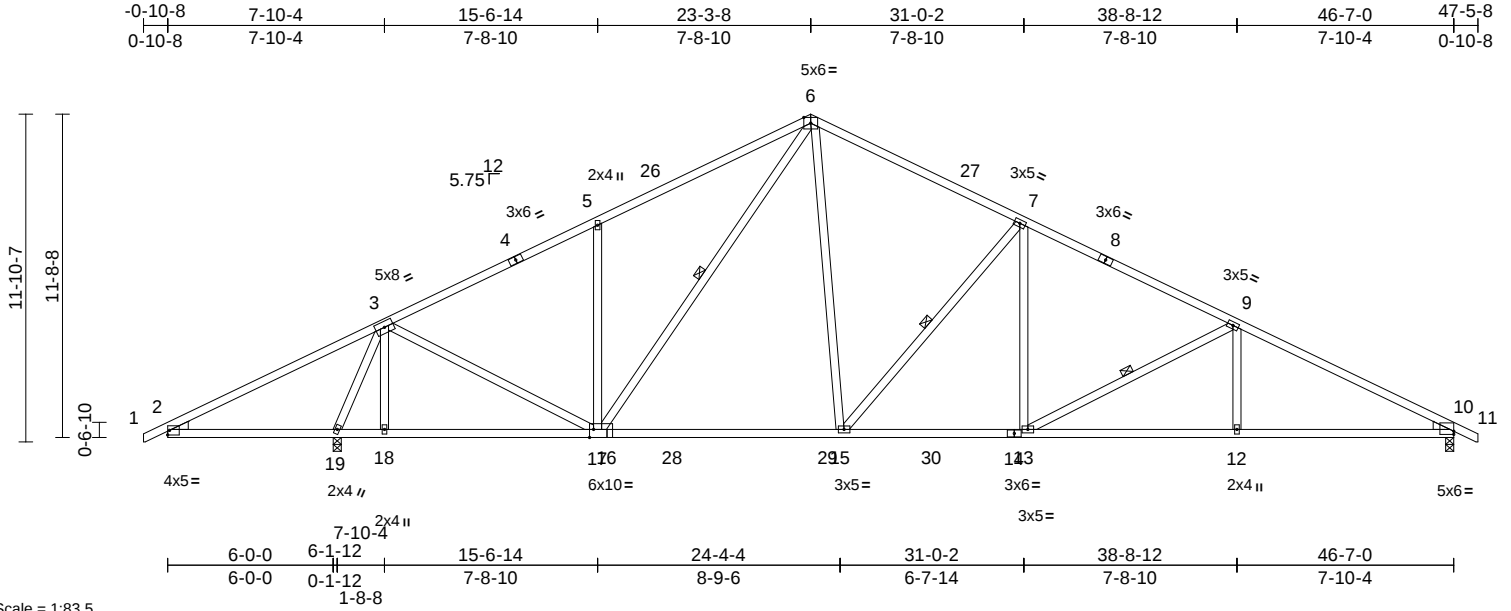


Plate Offsets (X, Y): [10:Edge,0-1-6], [17:0-1-12,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.28	15-17	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.49	15-17	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(CT)	0.11	10	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 266 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1 *Except* 4-1,8-11:2x4 SP No.2
BOT CHORD 2x4 SP No.1 *Except* 14-16:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 15-6,6-17:2x4 SP No.2
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 9-13, 6-17, 7-15

REACTIONS

(size) 10=0-3-8, 19=0-3-8
Max Horiz 19=114 (LC 15)
Max Grav 10=1671 (LC 2), 19=2262 (LC 2)

FORCES

TOP CHORD 1-2=0/28, 2-3=-388/701, 3-5=-1712/385,
5-6=-1717/564, 6-7=-1740/512,
7-9=-2352/563, 9-10=-3005/617, 10-11=0/28
BOT CHORD 2-19=-527/431, 18-19=-33/489,
17-18=-33/489, 15-17=-2/1402,
13-15=-227/2033, 12-13=-435/2609,
10-12=-435/2609
WEBS 5-17=-507/312, 7-13=-25/464,
9-13=-661/234, 9-12=0/147, 6-15=-152/954,
6-17=-131/281, 3-18=-44/183,
3-17=-283/1228, 7-15=-912/323,
3-19=-2185/548

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.1 .
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



October 9,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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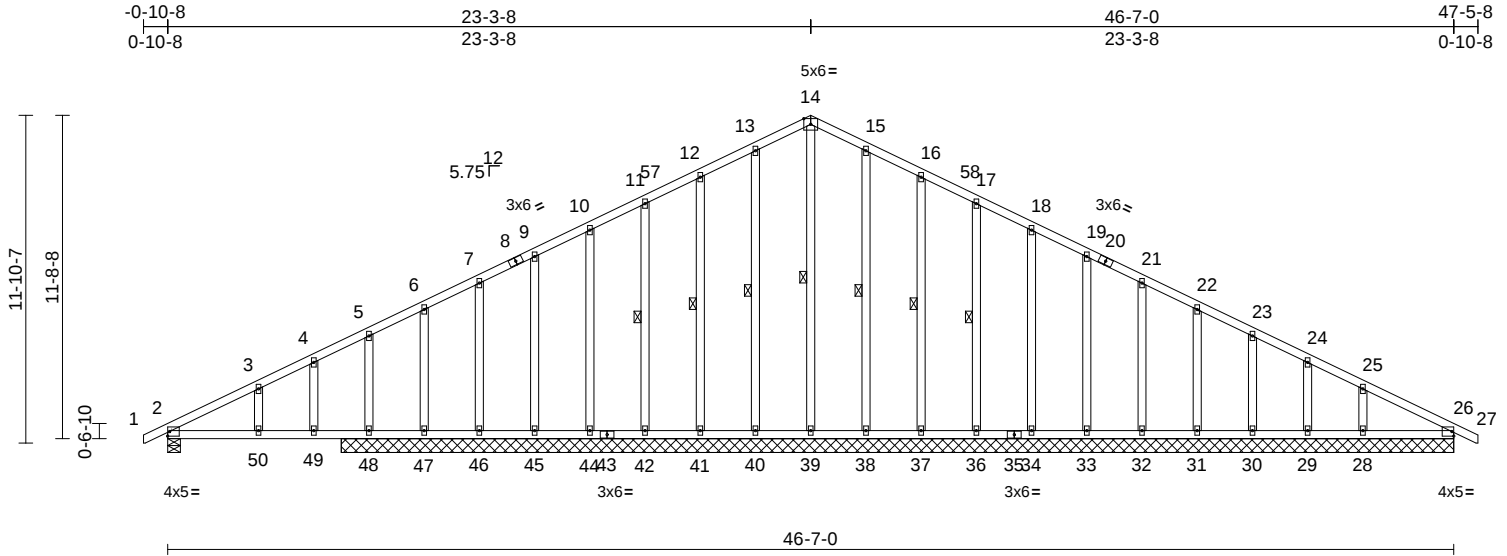
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH
25090182-01	ABE	Common Supported Gable	1	1	176928222
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:20
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Page: 1



Scale = 1:83.5

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.43	Vert(LL)	-0.08	50	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.51	Vert(CT)	-0.17	50	>519	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0											
Weight: 341 lb FT = 20%												

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3 *Except* 39-14,40-13,38-15:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 14-39, 13-40, 12-41, 11-42, 15-38, 16-37, 17-36

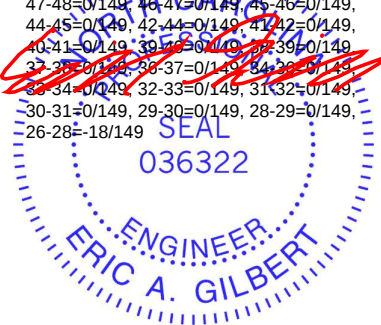
REACTIONS (size)
2=0-5-8, 26=40-3-8, 28=40-3-8, 29=40-3-8, 30=40-3-8, 31=40-3-8, 32=40-3-8, 33=40-3-8, 34=40-3-8, 36=40-3-8, 37=40-3-8, 38=40-3-8, 39=40-3-8, 40=40-3-8, 41=40-3-8, 42=40-3-8, 44=40-3-8, 45=40-3-8, 46=40-3-8, 47=40-3-8, 48=40-3-8
Max Horiz 2=111 (LC 15)
Max Uplift 28=44 (LC 16), 29=7 (LC 16), 30=20 (LC 16), 31=17 (LC 16), 32=18 (LC 16), 33=18 (LC 16), 34=18 (LC 16), 36=17 (LC 16), 37=21 (LC 16), 38=11 (LC 16), 40=12 (LC 15), 41=21 (LC 15), 42=17 (LC 15), 44=18 (LC 15), 45=15 (LC 15), 46=28 (LC 15), 47=136 (LC 33), 48=88 (LC 15)

Max Grav 2=337 (LC 2), 26=227 (LC 2), 28=239 (LC 34), 29=127 (LC 2), 30=163 (LC 34), 31=153 (LC 2), 32=155 (LC 34), 33=155 (LC 2), 34=155 (LC 2), 36=155 (LC 34), 37=193 (LC 23), 38=201 (LC 23), 39=158 (LC 16), 40=200 (LC 22), 41=194 (LC 22), 42=154 (LC 2), 44=159 (LC 33), 45=138 (LC 2), 46=226 (LC 33), 47=26 (LC 15), 48=642 (LC 33)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/27, 2-3=-215/20, 3-4=-156/42, 4-5=-125/58, 5-6=-196/100, 6-7=-135/122, 7-9=-152/171, 9-10=-157/214, 10-11=-172/258, 11-12=-187/301, 12-13=-204/347, 13-14=-216/385, 14-15=-216/385, 15-16=-204/347, 16-17=-187/301, 17-18=-172/258, 18-19=-157/214, 19-21=-150/170, 21-22=-150/126, 22-23=-151/82, 23-24=-149/37, 24-25=-157/16, 25-26=-181/29, 26-27=0/27
BOT CHORD 2-50=93/149, 49-50=0/149, 48-49=0/149, 47-48=0/149, 46-47=0/149, 45-46=0/149, 44-45=0/149, 42-44=0/149, 41-42=0/149, 40-41=0/149, 39-40=0/149, 38-39=0/149, 37-38=0/149, 36-37=0/149, 34-36=0/149, 33-34=0/149, 32-33=0/149, 31-32=0/149, 30-31=0/149, 29-30=0/149, 28-29=0/149, 26-28=-18/149

WEBS 14-39=-254/107, 13-40=-161/63, 12-41=-155/86, 11-42=-116/77, 10-44=-117/79, 9-45=-111/76, 7-46=-144/89, 6-47=0/30, 5-48=-339/165, 4-49=-44/49, 3-50=-55/69, 15-38=-162/63, 16-37=-155/86, 17-36=-116/77, 18-34=-116/79, 19-33=-116/78, 21-32=-116/79, 22-31=-115/78, 23-30=-121/82, 24-29=-99/66, 25-28=-170/118

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.



October 9,2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH
25090182-01	ABE	Common Supported Gable	1	1	I76928222
Job Reference (optional)					

- 4) TCLK: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard

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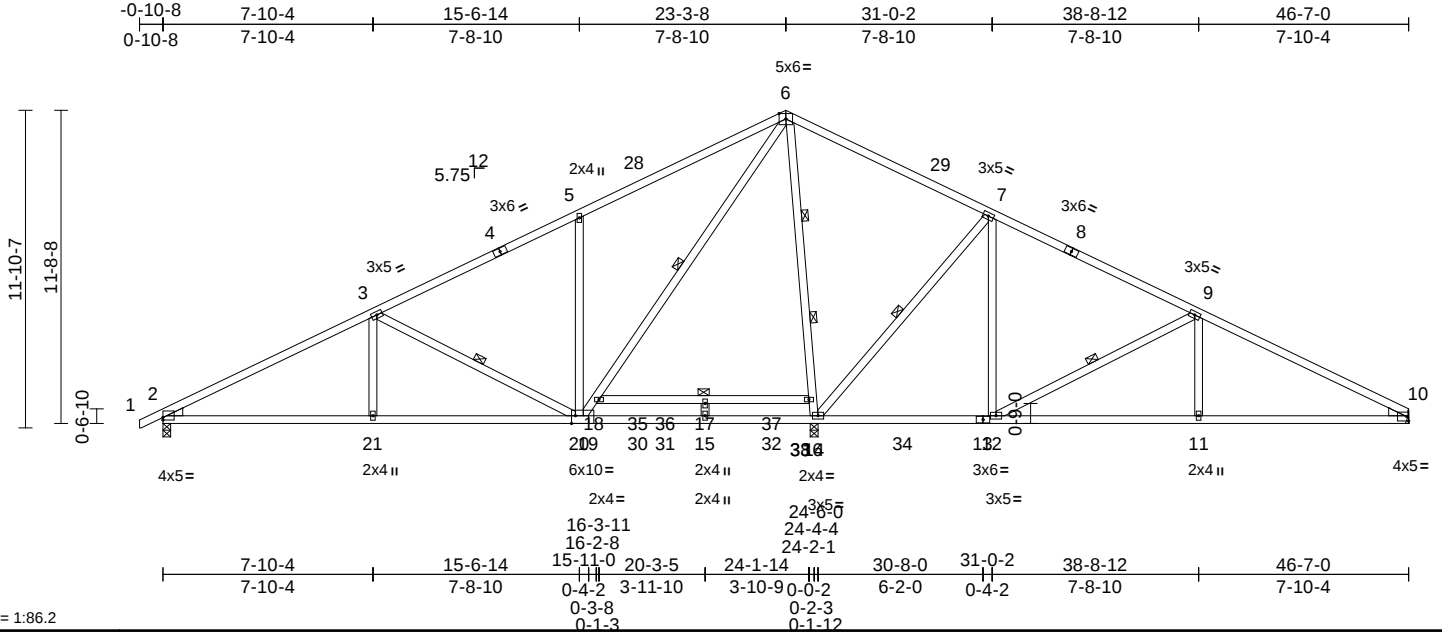
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928223
25090182-01	AD	Common	10	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:20

Page: 1

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.96	Vert(LL)	-0.29	15-20	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.94	Vert(CT)	-0.58	15-20	>501	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(CT)	0.02	14	n/a	n/a	
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 272 lb FT = 20%											

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 14-6,6-20:2x4 SP No.2
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 7-14, 9-12, 6-20, 3-20
WEBS 2 Rows at 1/3 pts 6-14

REACTIONS (size) 2=0-3-8, 10= Mechanical, 14=0-3-8
Max Horiz 2=119 (LC 15)
Max Uplift 10=59 (LC 16)
Max Grav 2=878 (LC 33), 10=663 (LC 34),
14=2814 (LC 3)

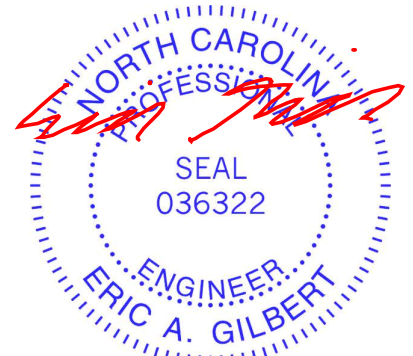
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/27, 2-3=-1312/233, 3-5=-631/122,
5-6=-646/304, 6-7=0/735, 7-9=-244/227,
9-10=-977/283
BOT CHORD 2-21=-242/1097, 20-21=-113/1097,
15-20=-433/315, 14-15=-433/315,
12-14=-175/131, 11-12=-159/797,
10-11=-230/797, 17-18=-30/13, 16-17=-30/13
WEBS 6-16=-1695/248, 14-16=-1776/189,
7-14=-884/358, 7-12=-55/461,
9-12=-761/262, 9-11=0/194,
18-20=-305/1350, 6-18=-242/1409,
5-20=-497/328, 3-20=-704/293, 3-21=0/149,
15-17=-47/16

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 20-3-5 from left end, supported at two points, 5-0-0 apart.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP No.2 , Joint 14 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



October 9,2025

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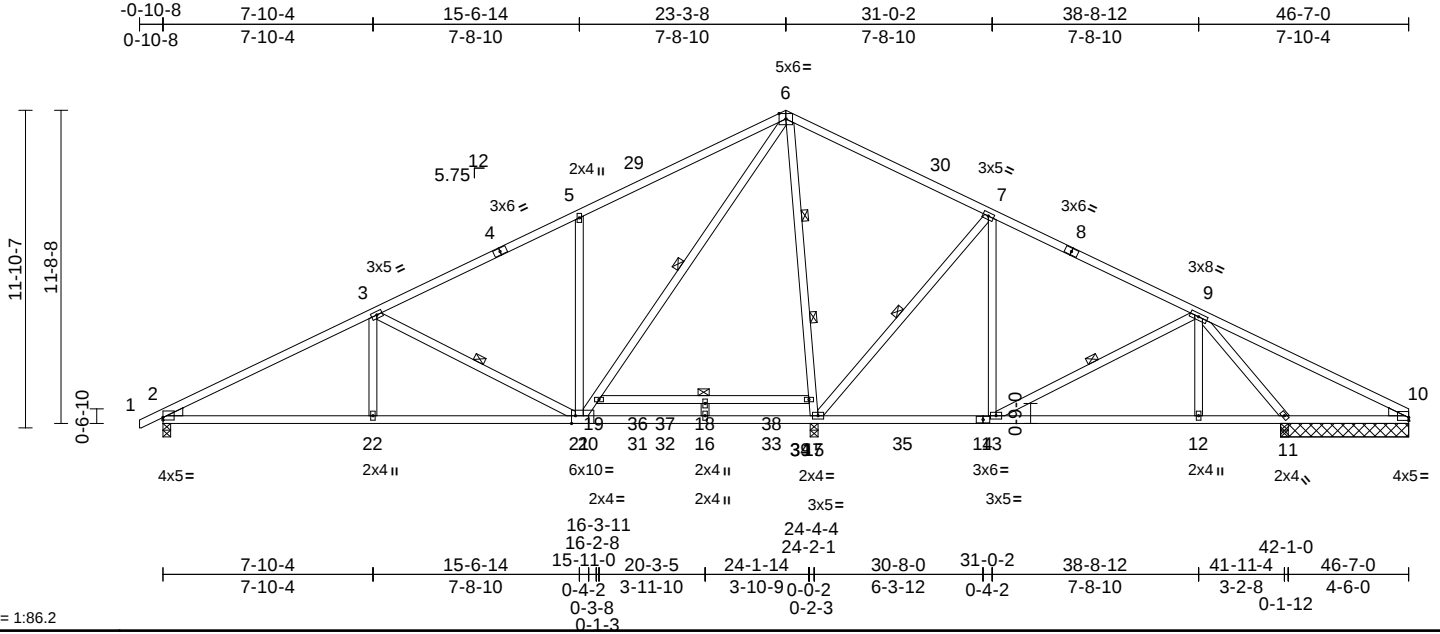
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928224
25090182-01	AD1	Common	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:21

Page: 1

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Scale = 1:86.2

Plate Offsets (X, Y): [2:Edge,0-1-2], [10:Edge,0-1-6], [21:0-1-12,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.29	16-21	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.94	Vert(CT)	-0.59	16-21	>498	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(CT)	0.03	23	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 279 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* 15-6,6-21:2x4 SP No.2
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
 WEBS 1 Row at midpt 9-13, 3-21, 6-21, 7-15
 WEBS 2 Rows at 1/3 pts 6-15

REACTIONS

(size) 2=0-3-8, 10=4-9-8, 11=0-3-8,
 15=0-3-8
 Max Horiz 2=119 (LC 15)
 Max Uplift 10=48 (LC 16), 11=5 (LC 16)
 Max Grav 2=900 (LC 33), 10=348 (LC 34),
 11=493 (LC 34), 15=2738 (LC 3)

FORCES

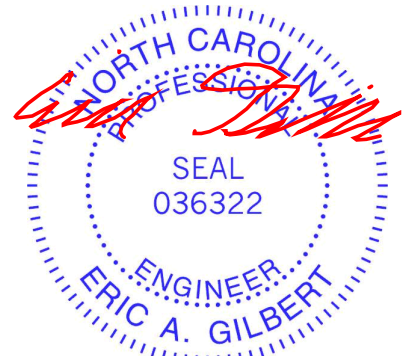
(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/28, 2-3=-1343/228, 3-5=-643/129,
 5-6=-658/311, 6-7=0/664, 7-9=-290/248,
 9-10=-391/328
 BOT CHORD 2-22=-236/1121, 21-22=-107/1121,
 16-21=-370/299, 15-16=-370/299,
 13-15=-191/169, 12-13=-118/556,
 11-12=-118/556, 10-11=-230/343,
 18-19=-29/14, 17-18=-29/14
 WEBS 7-13=-37/373, 9-13=-510/217, 9-12=0/134,
 5-21=-515/317, 3-21=-720/281, 3-22=0/147,
 6-17=-1665/213, 15-17=-1746/154,
 19-21=-285/1383, 6-19=-223/1442,
 7-15=-800/331, 16-18=-47/16, 9-11=-457/38

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) 200.0lb AC unit load placed on the bottom chord, 20-3-5 from left end, supported at two points, 5-0-0 apart.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2 .
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10, 15, and 11. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



October 9,2025

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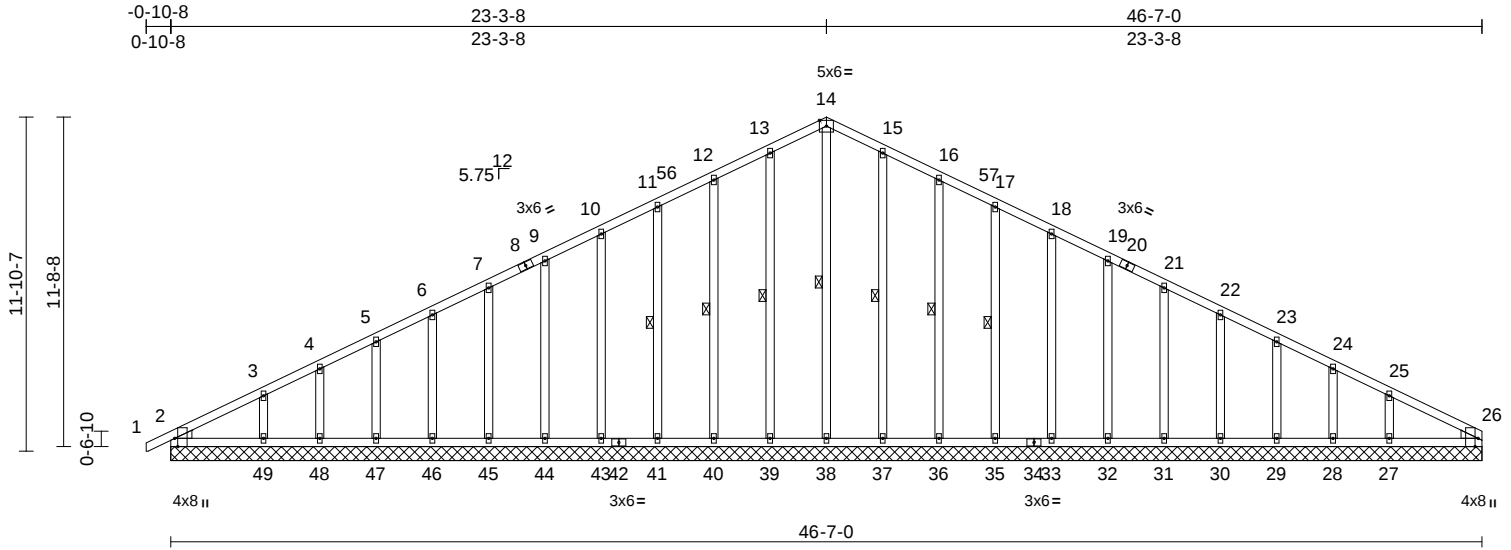
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928225
25090182-01	ADE	Common	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:21

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Scale = 1:81.9

Plate Offsets (X, Y): [2:0-3-8,Edge], [26:0-3-8,Edge]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.01	26	n/a	n/a	
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 341 lb FT = 20%											

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3 *Except* 38-14,39-13,37-15:2x4 SP No.2
WEDGE Left: 2x4 SP No.2
Right: 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS 1 Row at midpt 14-38, 13-39, 12-40, 11-41, 15-37, 16-36, 17-35

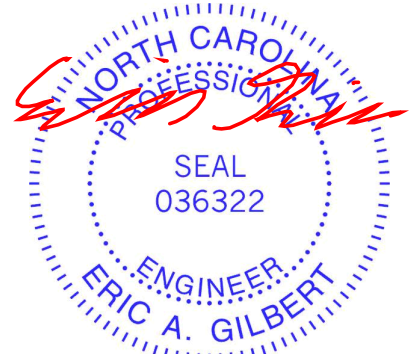
REACTIONS (size) 2=46-7-0, 26=46-7-0, 27=46-7-0, 28=46-7-0, 29=46-7-0, 30=46-7-0, 31=46-7-0, 32=46-7-0, 33=46-7-0, 35=46-7-0, 36=46-7-0, 37=46-7-0, 38=46-7-0, 39=46-7-0, 40=46-7-0, 41=46-7-0, 43=46-7-0, 44=46-7-0, 45=46-7-0, 46=46-7-0, 47=46-7-0, 48=46-7-0, 49=46-7-0

Max Horiz 2=115 (LC 15)
Max Uplift 27=-40 (LC 16), 28=-5 (LC 16), 29=-17 (LC 16), 30=-14 (LC 16), 31=-15 (LC 16), 32=-15 (LC 16), 33=-15 (LC 16), 35=-14 (LC 16), 36=-19 (LC 16), 37=-8 (LC 16), 39=-10 (LC 15), 40=-18 (LC 15), 41=-15 (LC 15), 43=-15 (LC 15), 44=-15 (LC 15), 45=-15 (LC 15), 46=-14 (LC 15), 47=-18 (LC 15), 48=-4 (LC 15), 49=-45 (LC 15)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/27, 2-3=-150/57, 3-4=-114/49, 4-5=-98/58, 5-6=-87/73, 6-7=-75/90, 7-9=-69/113, 9-10=-78/157, 10-11=-93/201, 11-12=-109/245, 12-13=-125/291, 13-14=-139/330, 14-15=-139/330, 15-16=-125/291, 16-17=-109/245, 17-18=-93/201, 18-19=-78/157, 19-21=-63/113, 21-22=-54/69, 22-23=-54/28, 23-24=-64/14, 24-25=-81/12, 25-26=-130/46
BOT CHORD 2-49=-37/147, 48-49=-37/147, 47-48=-37/147, 46-47=-37/147, 45-46=-37/147, 44-45=-37/147, 43-44=-37/147, 41-43=-37/147, 40-41=-37/147, 39-40=-37/147, 38-39=-37/147, 37-38=-37/147, 36-37=-37/147, 35-36=-37/147, 33-35=-37/147, 32-33=-37/147, 31-32=-37/147, 30-31=-37/147, 29-30=-37/147, 28-29=-37/147, 27-28=-37/147, 26-27=-37/147

WEBS 14-38=-211/39, 13-39=-168/63, 12-40=-155/82, 11-41=-121/75, 10-43=-120/76, 9-44=-120/76, 7-45=-121/76, 6-46=-120/75, 5-47=-125/78, 4-48=-103/66, 3-49=-170/108, 15-37=-170/63, 16-36=-158/82, 17-35=-121/75, 18-33=-120/76, 19-32=-120/76, 21-31=-121/76, 22-30=-119/75, 23-29=-125/79, 24-28=-101/64, 25-27=-176/110

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33



October 9,2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH
25090182-01	ADE	Common	1	1	I76928225
Job Reference (optional)					

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



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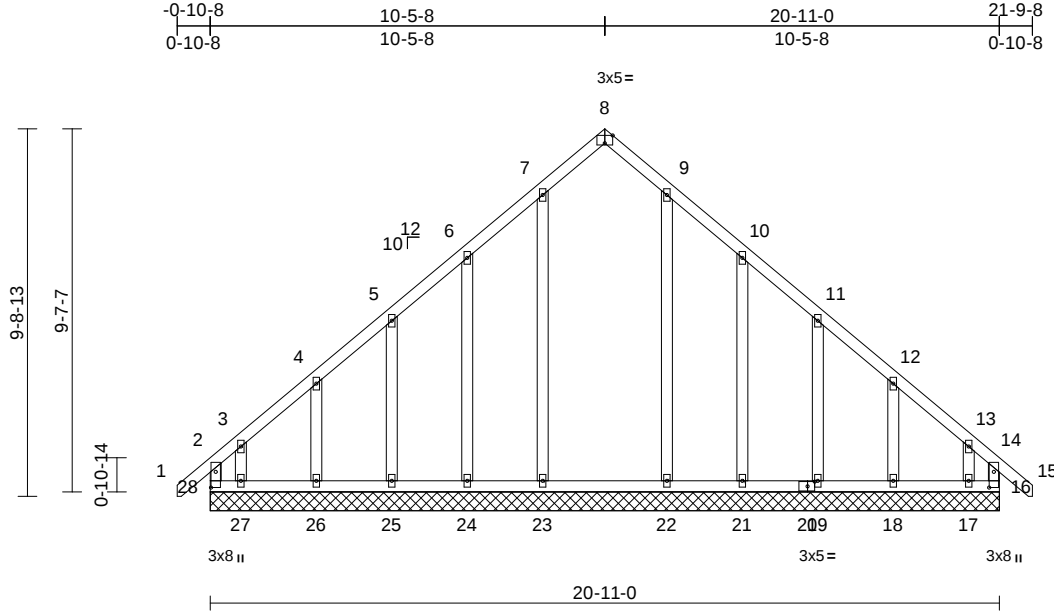
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928226
25090182-01	BE	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:21
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Page: 1



Scale = 1:61.1

Plate Offsets (X, Y): [8:0-2-8,Edge], [16:0-5-0,0-1-8], [20:0-2-4,0-1-8], [28:0-5-0,0-1-8]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.16	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.01	16	n/a	n/a	
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MR							
BCDL	10.0										
Weight: 142 lb FT = 20%											

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size)	16=20-11-0, 17=20-11-0, 18=20-11-0, 19=20-11-0, 21=20-11-0, 22=20-11-0, 23=20-11-0, 24=20-11-0, 25=20-11-0, 26=20-11-0, 27=20-11-0, 28=20-11-0
Max Horiz		28=199 (LC 11)
Max Uplift		16=-103 (LC 12), 17=-225 (LC 14), 18=-28 (LC 14), 19=-32 (LC 14), 21=-58 (LC 14), 24=-56 (LC 13), 25=-32 (LC 13), 26=-27 (LC 13), 27=-229 (LC 13), 28=-123 (LC 11)
Max Grav		16=321 (LC 14), 17=180 (LC 12), 18=180 (LC 26), 19=182 (LC 26), 21=158 (LC 30), 22=256 (LC 26), 23=265 (LC 25), 24=158 (LC 29), 25=183 (LC 25), 26=180 (LC 25), 27=192 (LC 11), 28=329 (LC 13)

FORCES

TOP CHORD	(lb) - Maximum Compression/Maximum Tension	2-28=-264/195, 1-2=0/49, 2-3=-333/246, 3-4=-208/156, 4-5=-132/90, 5-6=-119/52, 6-7=-106/56, 7-8=-107/79, 8-9=-107/79, 9-10=-95/56, 10-11=-110/39, 11-12=-131/90, 12-13=-208/156, 13-14=-332/253, 14-15=0/49, 14-16=-263/200
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BOT CHORD	27-28=-183/247, 26-27=-183/247, 25-26=-183/247, 24-25=-183/247, 23-24=-183/247, 22-23=-183/247, 21-22=-183/247, 19-21=-183/247, 18-19=-183/247, 17-18=-183/247, 16-17=-183/247
WEBS	7-23=-148/14, 9-22=-140/8, 6-24=-172/120, 5-25=-145/83, 4-26=-154/91, 3-27=-199/186, 10-21=-172/120, 11-19=-145/83, 12-18=-154/91, 13-17=-201/185

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 .
- N/A

- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



October 9,2025

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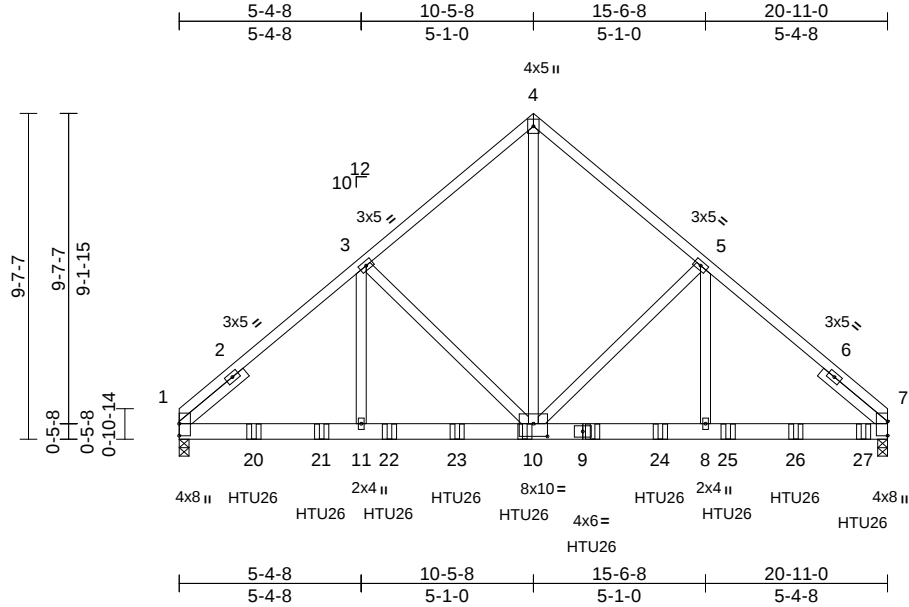
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928227
25090182-01	BG	Common Girder	1	2	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:21
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Page: 1



Scale = 1:68

Plate Offsets (X, Y): [10:0-5-0,0-4-8]

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	-0.08	10-11	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.80	Vert(CT)	-0.14	10-11	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.79	Horz(CT)	0.04	7	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 289 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=0-3-8, 7=0-3-8
Max Horiz 1=-167 (LC 28)
Max Uplift 1=-277 (LC 9), 7=-326 (LC 10)
Max Grav 1=3862 (LC 2), 7=4324 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-3=-4629/385, 3-4=-3365/348,
4-5=-3366/348, 5-7=-4665/388
BOT CHORD 1-11=-296/3483, 10-11=-296/3483,
8-10=-239/3515, 7-8=-239/3515
WEBS 4-10=-374/3835, 5-10=-1420/222,
5-8=-131/1578, 3-10=-1375/217,
3-11=-126/1533

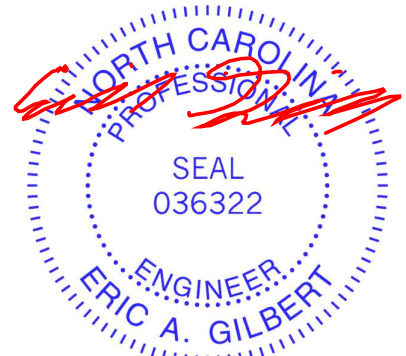
NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 7. This connection is for uplift only and does not consider lateral forces.
- Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-2-8 from the left end to 20-2-8 to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-52, 4-7=-52, 12-16=-19
Concentrated Loads (lb)
Vert: 9=-528 (B), 10=-528 (B), 20=-528 (B), 21=-528 (B), 22=-528 (B), 23=-528 (B), 24=-528 (B), 25=-528 (B), 26=-528 (B), 27=-531 (B)



October 9,2025

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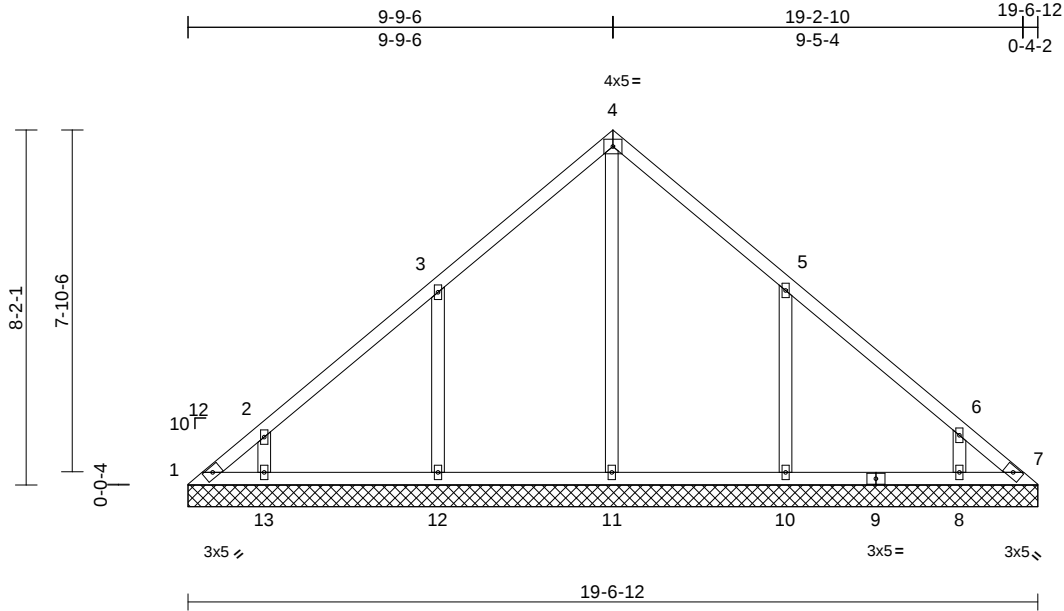
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928228
25090182-01	V	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:21

Page: 1

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Scale = 1:53

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.22	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.19	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.31	Horiz(TL)	0.00	7	n/a	n/a	
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 92 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=19-6-12, 7=19-6-12, 8=19-6-12, 10=19-6-12, 11=19-6-12, 12=19-6-12, 13=19-6-12
Max Horiz	1=158 (LC 10)
Max Uplift	1=-81 (LC 9), 8=-14 (LC 14), 10=-97 (LC 14), 12=-85 (LC 13), 13=-44 (LC 13)
Max Grav	1=110 (LC 12), 7=0 (LC 12), 8=299 (LC 2), 10=456 (LC 25), 11=482 (LC 27), 12=459 (LC 24), 13=285 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-183/192, 2-3=-183/176, 3-4=-251/264, 4-5=-252/264, 5-6=-101/93, 6-7=-49/71
BOT CHORD	1-13=-38/51, 12-13=-27/42, 11-12=-27/42, 10-11=-27/43, 8-10=-27/43, 7-8=-27/43
WEBS	4-11=-266/73, 3-12=-338/200, 2-13=-262/161, 5-10=-339/205, 6-8=-252/143

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



October 9,2025

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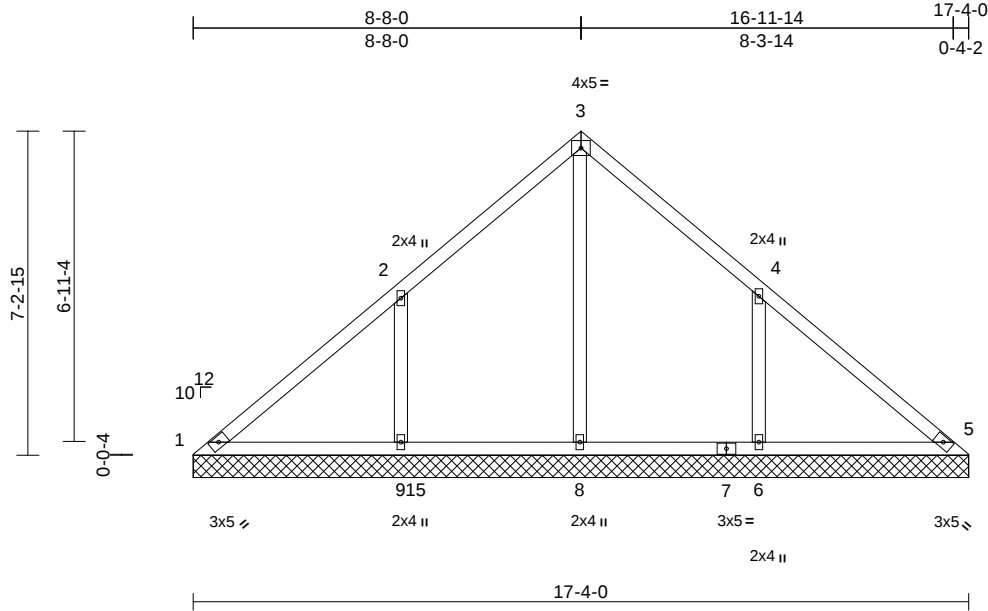
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928229
25090182-01	VA	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:21

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Scale = 1:51.5

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.18	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.47	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 78 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=17-4-0, 5=17-4-0, 6=17-4-0, 8=17-4-0, 9=17-4-0
Max Horiz	1=139 (LC 10)
Max Uplift	1=-48 (LC 9), 6=-88 (LC 14), 9=-93 (LC 13)
Max Grav	1=85 (LC 28), 5=1 (LC 25), 6=515 (LC 25), 8=672 (LC 24), 9=505 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-87/368, 2-3=0/299, 3-4=0/295, 4-5=-150/326
BOT CHORD	1-9=-200/72, 8-9=-200/72, 6-8=-199/72, 5-6=-199/72
WEBS	3-8=-510/0, 2-9=-361/205, 4-6=-359/203

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



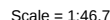
October 9,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Page: 1LOAD CASE(S) Standard

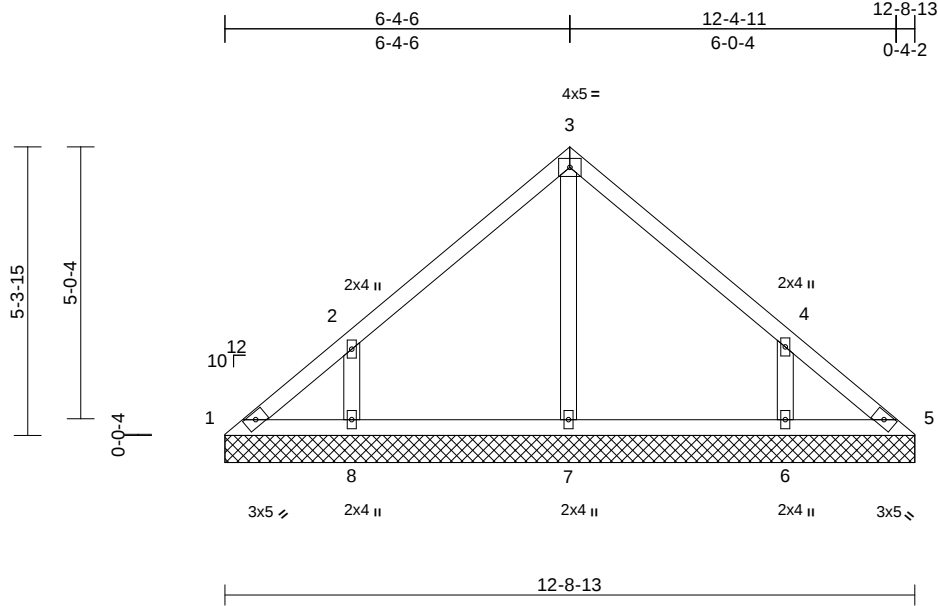
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	I76928231
25090182-01	VC	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:22

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 53 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	1=12-8-13, 5=12-8-13, 6=12-8-13, 7=12-8-13, 8=12-8-13
Max Horiz	1=-102 (LC 9)
Max Uplift	1=-19 (LC 9), 6=-64 (LC 14), 8=-67 (LC 13)
Max Grav	1=101 (LC 25), 5=86 (LC 24), 6=348 (LC 25), 7=270 (LC 2), 8=352 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension

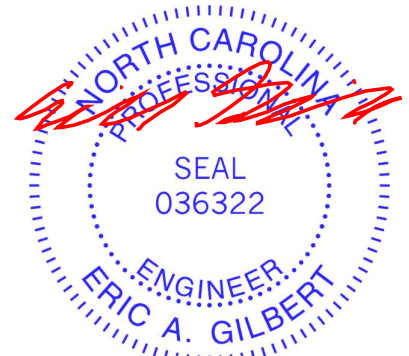
TOP CHORD	1-2=-125/95, 2-3=-146/110, 3-4=-137/111, 4-5=-106/60
BOT CHORD	1-8=-33/89, 7-8=-33/65, 6-7=-33/65, 5-6=-33/80
WEBS	3-7=-186/0, 2-8=-303/183, 4-6=-298/180

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.

LOAD CASE(S) Standard



October 9, 2025

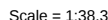
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ID:n9VjqRooqxhls16wd2D3TSzVDuc-RfC?PsB70Hg3NSqPqnL8w3u1tXBGKwRCDoi7J4zJC?f



LUMBER		4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
OTHERS	2x4 SP No.3	
BRACING		5) Gable requires continuous bottom chord bearing.
TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.	6) Gable studs spaced at 4-0-0 oc.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
REACTIONS	(size) 1=10-5-3, 3=10-5-3, 4=10-5-3	8) All bearings are assumed to be SP No.2 .
	Max Horiz 1=-83 (LC 11)	LOAD CASE(S) Standard
	Max Uplift 1=-39 (LC 29), 3=-37 (LC 28), 4=-3 (LC 13)	
	Max Grav 1=65 (LC 28), 3=68 (LC 29), 4=854 (LC 2)	
FORCES		
	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-110/379, 2-3=-109/374	
BOT CHORD	1-4=-300/160, 3-4=-296/158	
WEBS	2-4=-667/215	

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.



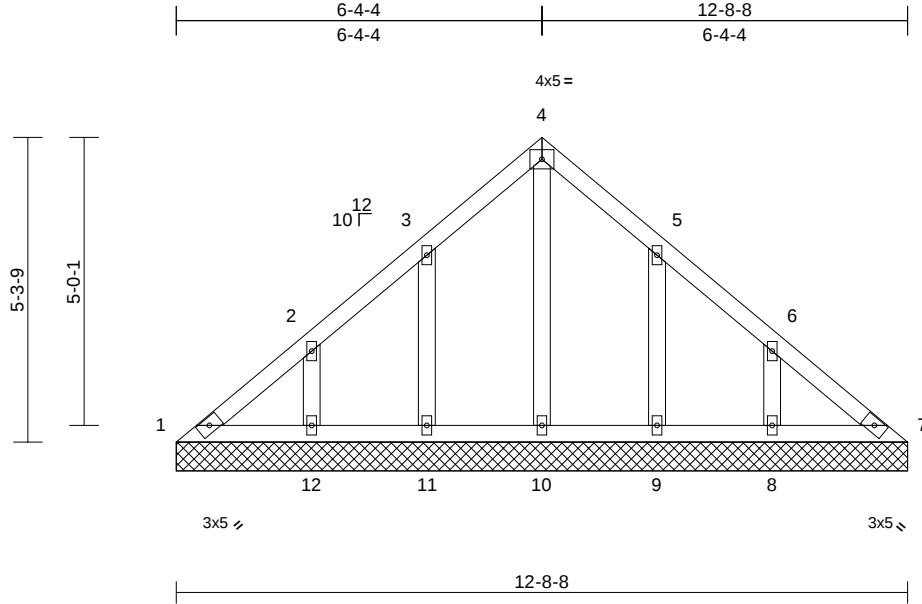
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928233
25090182-01	VE	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:22

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Scale = 1:40

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.07	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.06	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	7	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 62 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=12-8-8, 7=12-8-8, 8=12-8-8, 9=12-8-8, 10=12-8-8, 11=12-8-8, 12=12-8-8
Max Horiz	1=-101 (LC 11)
Max Uplift	1=-10 (LC 9), 8=-29 (LC 14), 9=-39 (LC 14), 11=-39 (LC 13), 12=-31 (LC 13)
Max Grav	1=95 (LC 25), 7=79 (LC 2), 8=219 (LC 25), 9=181 (LC 25), 10=170 (LC 2), 11=180 (LC 24), 12=222 (LC 24)

FORCES

(lb) - Maximum Compression/Maximum Tension

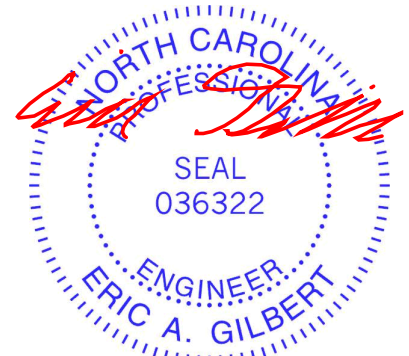
TOP CHORD	1-2=-113/101, 2-3=-70/75, 3-4=-92/108, 4-5=-92/108, 5-6=-46/51, 6-7=-89/83
BOT CHORD	1-12=-78/93, 11-12=-78/88, 10-11=-78/88, 9-10=-78/88, 8-9=-78/88, 7-8=-78/88
WEBS	4-10=-126/0, 3-11=-161/101, 2-12=-163/92, 5-9=-161/101, 6-8=-163/92

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



October 9,2025

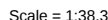
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ID:1bpNOez4nqFuqYHKc8x1DBzVDrp-RfC?PsB70Hg3NSqPanL8w3uITXbGKWkCDoi7J4zJC?f



LUMBER		4) TCELL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
OTHERS	2x4 SP No.3	
BRACING		
TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.	5) Gable requires continuous bottom chord bearing.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	6) Gable studs spaced at 4-0-0 oc.
REACTIONS	(size) 1=10-5-3, 3=10-5-3, 4=10-5-3	7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
	Max Horiz 1=-83 (LC 11)	
	Max Uplift 1=-39 (LC 29), 3=-37 (LC 28), 4=-3 (LC 13)	8) All bearings are assumed to be SP No.2 .
	Max Grav 1=65 (LC 28), 3=68 (LC 29), 4=855 (LC 2)	LOAD CASE(S) Standard
FORCES	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-110/379, 2-3=-109/374	
BOT CHORD	1-4=-300/160, 3-4=-296/158	
WEBS	2-4=-667/215	

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

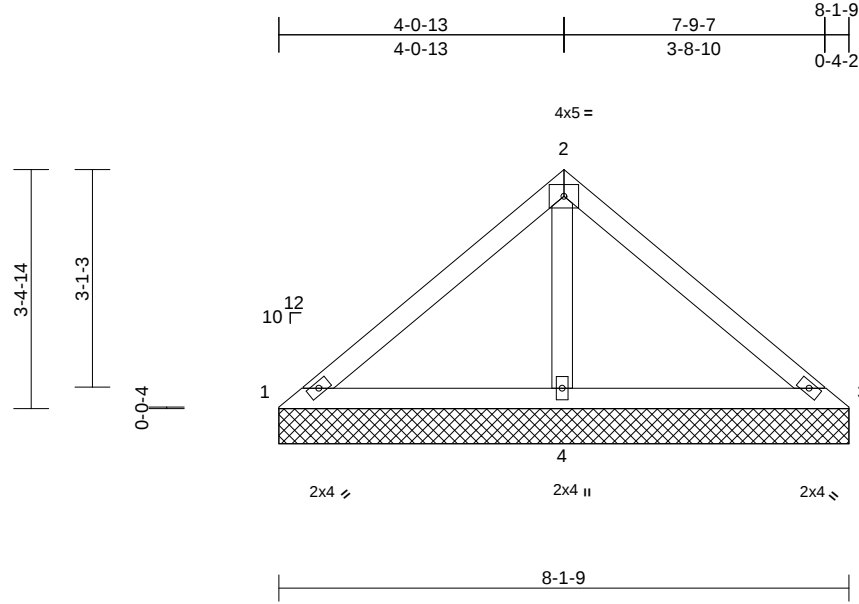


Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928235
25090182-01	VG	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:22
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Page: 1



Scale = 1:32.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.22	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 31 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 8-1-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=8-1-9, 3=8-1-9, 4=8-1-9
Max Horiz	1=-64 (LC 11)
Max Uplift	1=-17 (LC 29), 3=-15 (LC 28), 4=-2 (LC 13)
Max Grav	1=64 (LC 28), 3=67 (LC 29), 4=631 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-84/259, 2-3=-82/255
BOT CHORD	1-4=-230/133, 3-4=-226/132
WEBS	2-4=-464/153

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 4-0-0 oc.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 .
- LOAD CASE(S)** Standard



October 9,2025

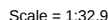
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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818 Soundside Road
Edenton, NC 27932

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:22 Page: 1
ID:1bpNOez4nqFuqYHKc8x1DBzVDrp-RfC?PsB70Hg3NSqPanL8w3uITXbGKWkCDoi7J4zJC?f



LUMBER		4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
OTHERS	2x4 SP No.3	
BRACING		5) Gable requires continuous bottom chord bearing.
TOP CHORD	Structural wood sheathing directly applied or 8-1-9 oc purlins.	6) Gable studs spaced at 4-0-0 oc.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
REACTIONS	(size) 1=8-1-9, 3=8-1-9, 4=8-1-9	8) All bearings are assumed to be SP No.2 .
	Max Horiz 1=-64 (LC 9)	LOAD CASE(S) Standard
	Max Uplift 1=-17 (LC 29), 3=-15 (LC 28), 4=-2 (LC 13)	
	Max Grav 1=64 (LC 28), 3=67 (LC 29), 4=632 (LC 2)	
FORCES	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-84/259, 2-3=-82/255	
BOT CHORD	1-4=-230/133, 3-4=-226/132	
WERS	2-4=-464/153	

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.



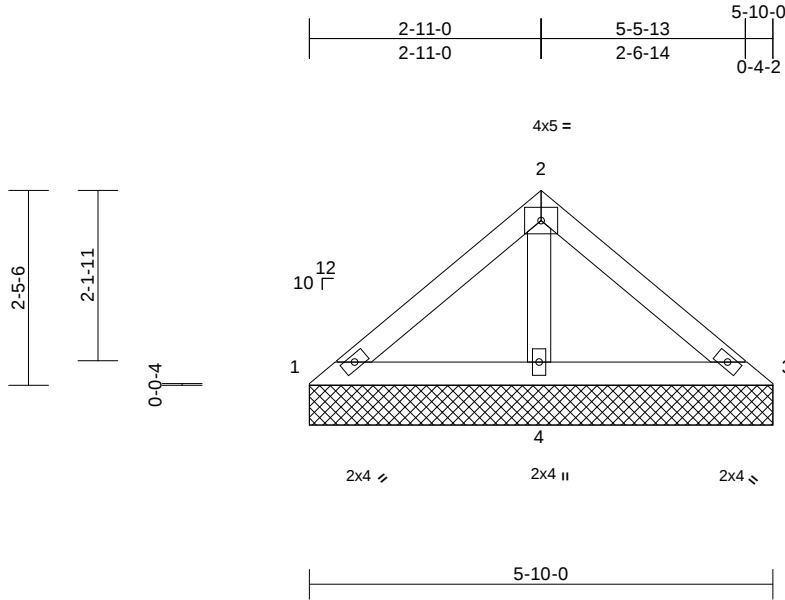
Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928237
25090182-01	VI	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:22

Page: 1

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Scale = 1:29

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.12	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	3	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 21 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 1=5-10-0, 3=5-10-0, 4=5-10-0
Max Horiz 1=-45 (LC 9)
Max Grav 1=67 (LC 28), 3=70 (LC 29), 4=394 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

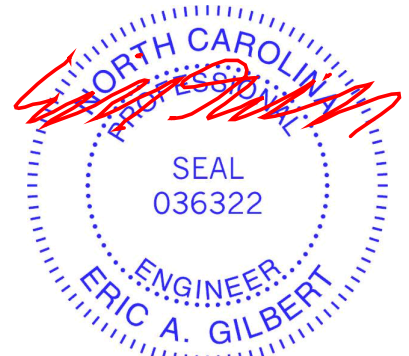
TOP CHORD 1-2=-57/136, 2-3=-61/132
BOT CHORD 1-4=-123/79, 3-4=-120/78
WEBS 2-4=-263/78

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- Gable requires continuous bottom chord bearing.

- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



October 9,2025

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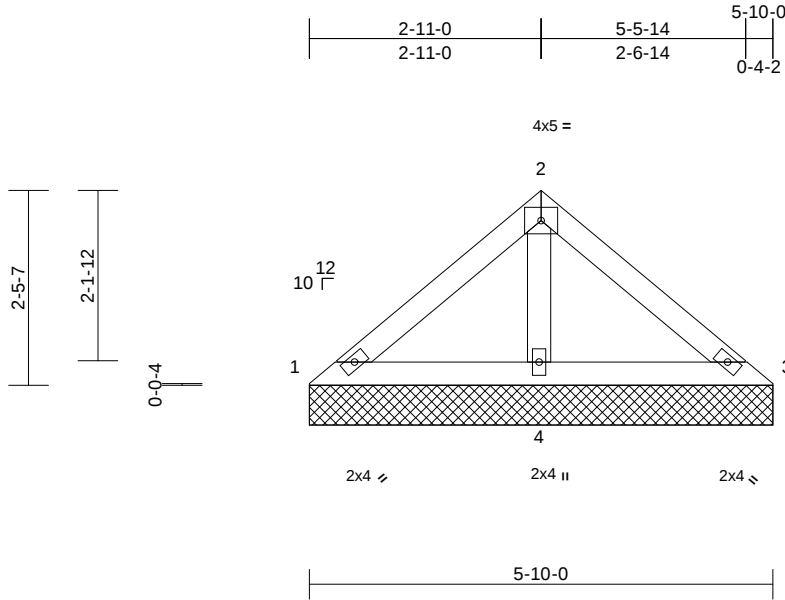
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928238
25090182-01	VJ	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:22
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Page: 1



Scale = 1:29

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 21 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-10-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS	(size)	1=5-10-0, 3=5-10-0, 4=5-10-0
	Max Horiz	1=-45 (LC 9)
	Max Uplift	4=-9 (LC 13)
	Max Grav	1=65 (LC 28), 3=67 (LC 29), 4=368 (LC 2)

FORCES

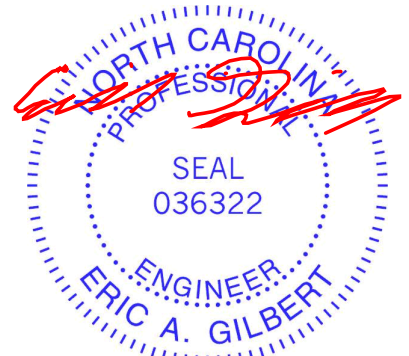
	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=-55/127, 2-3=-59/123
BOT CHORD	1-4=-118/82, 3-4=-115/81
WEBS	2-4=-243/90

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10

- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



October 9, 2025

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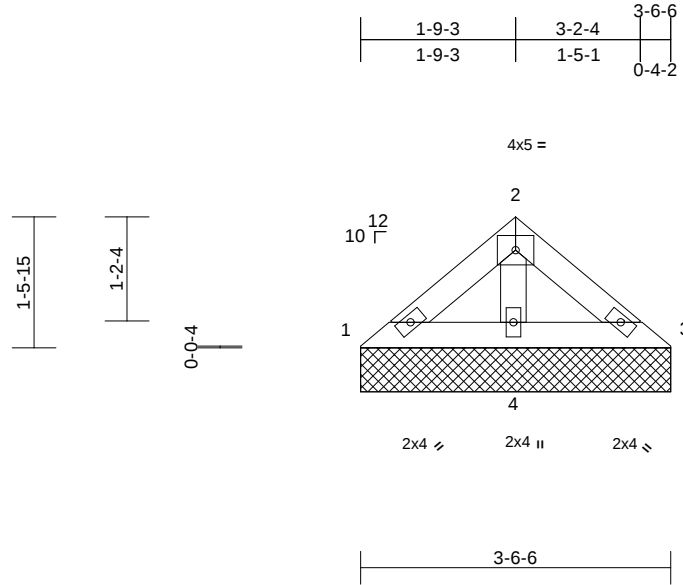
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	75 Blackberry Manor-Roof-Kristie A RH	176928239
25090182-01	VK	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Wed Oct 08 13:33:23
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Page: 1



Scale = 1:26.2

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.04	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 12 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-6-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=3-6-6, 3=3-6-6, 4=3-6-6
Max Horiz	1=-26 (LC 9)
Max Uplift	3=-2 (LC 14)
Max Grav	1=50 (LC 28), 3=52 (LC 29), 4=192 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

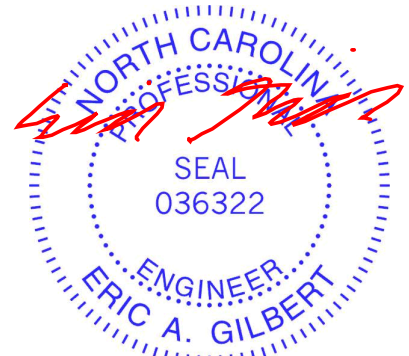
TOP CHORD	1-2=-42/49, 2-3=-46/45
BOT CHORD	1-4=-47/37, 3-4=-45/36
WEBS	2-4=-101/25

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10

- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .

LOAD CASE(S) Standard



October 9,2025

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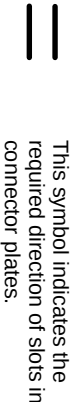
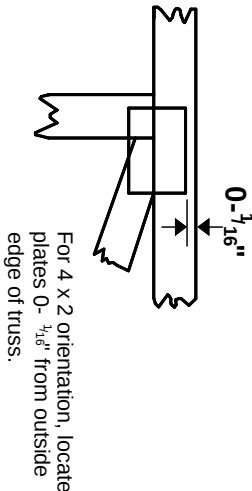
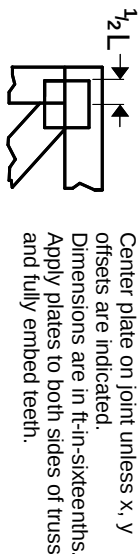
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Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

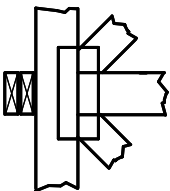
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

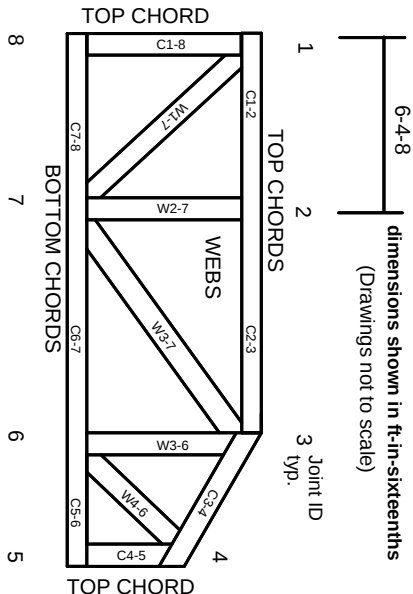


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1.1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023