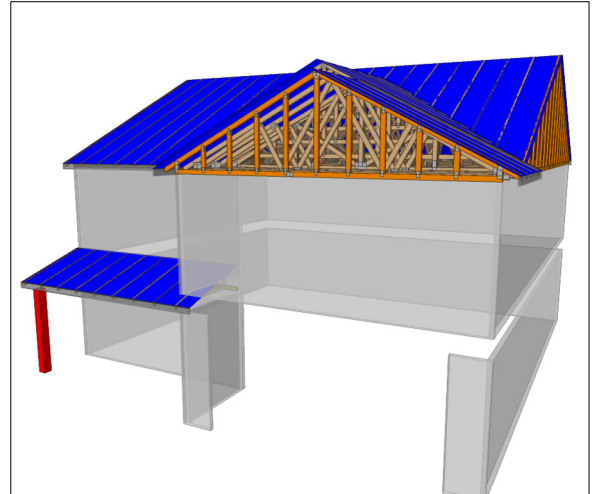




Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450

Builder: DR Horton Inc
Model: 5 Cross Creek -
Galen - F



THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

Date: _____

General Notes:

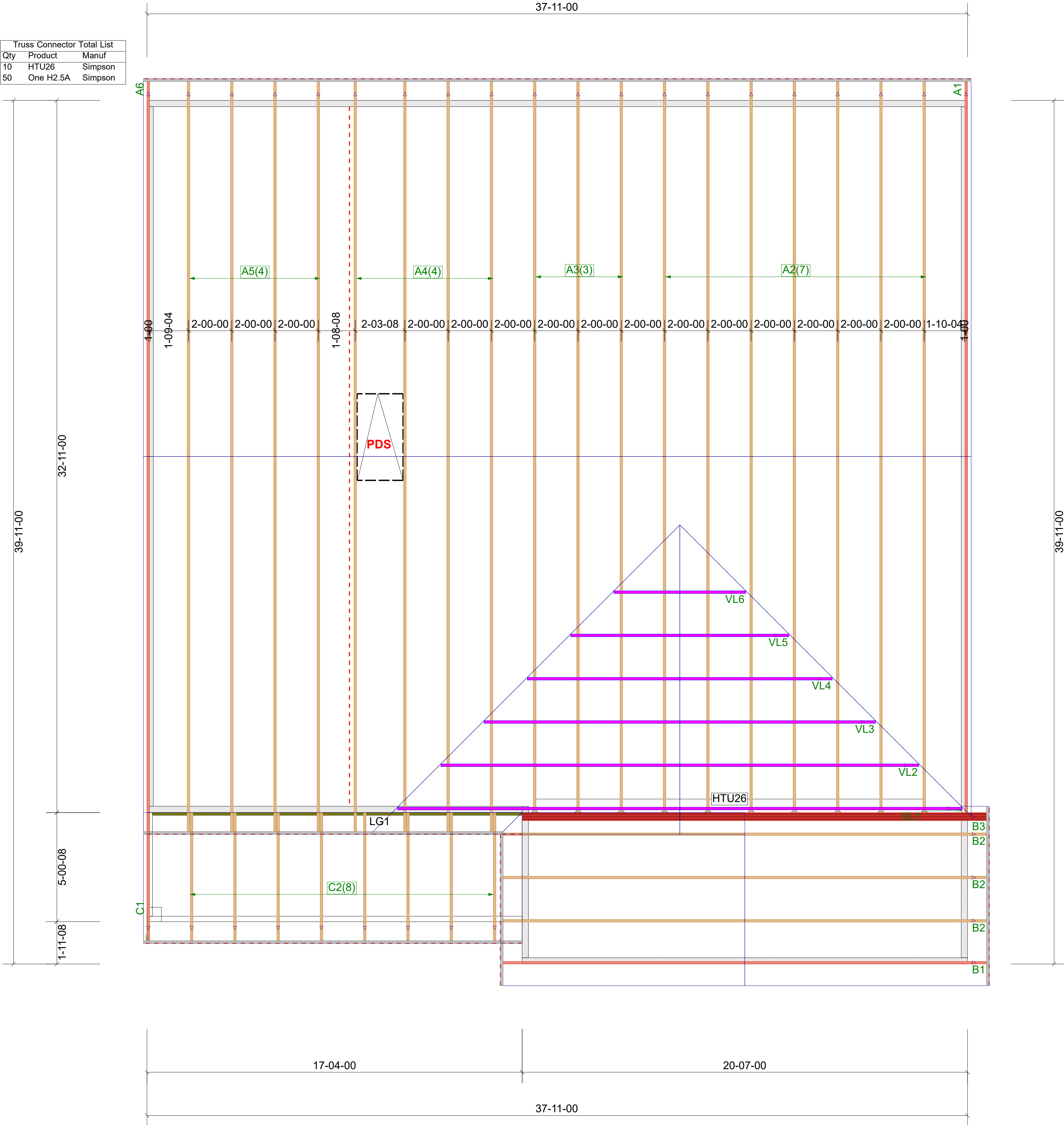
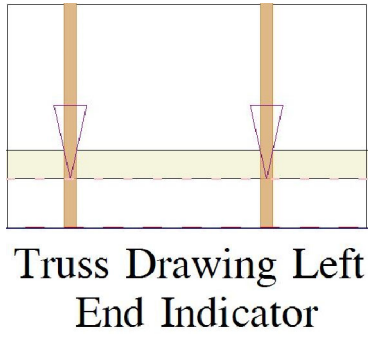
** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.

** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.



Truss Connector Total List		
Qty	Product	Manuf
10	HTU26	Simpson
50	One H2.5A	Simpson

ROOF PLACEMENT PLAN

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

** All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53179.



DR Horton Inc	5 Cross Creek - Galen - F	Roof Truss Layout
---------------	---------------------------	-------------------

Scale:	NTS
Date:	10/2/2025
Designer:	Nate Donaldson
Project Number:	25090186-C
Sheet Number:	

1/1

RE: 25090186
5 Cross Creek - Galen F - Roof

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Customer: DR Horton Inc Project Name: 25090186
Lot/Block: 5 Model: Galen F
Address: Subdivision: Cross Creek
City: State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Design Program: MiTek 20/20 8.7
Wind Code: ASCE 7-16 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 17 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	I69957936	A1	12/5/2024
2	I69957937	A2	12/5/2024
3	I69957938	A3	12/5/2024
4	I69957939	A4	12/5/2024
5	I69957940	A5	12/5/2024
6	I69957941	A6	12/5/2024
7	I69957942	B1	12/5/2024
8	I69957943	B2	12/5/2024
9	I69957944	B3	12/5/2024
10	I69957945	C1	12/5/2024
11	I69957946	C2	12/5/2024
12	I69957947	VL1	12/5/2024
13	I69957948	VL2	12/5/2024
14	I69957949	VL3	12/5/2024
15	I69957950	VL4	12/5/2024
16	I69957951	VL5	12/5/2024
17	I69957952	VL6	12/5/2024

The truss drawing(s) referenced above have been prepared by
Truss Engineering Co. under my direct supervision
based on the parameters provided by Carter Components (Sanford, NC)).

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



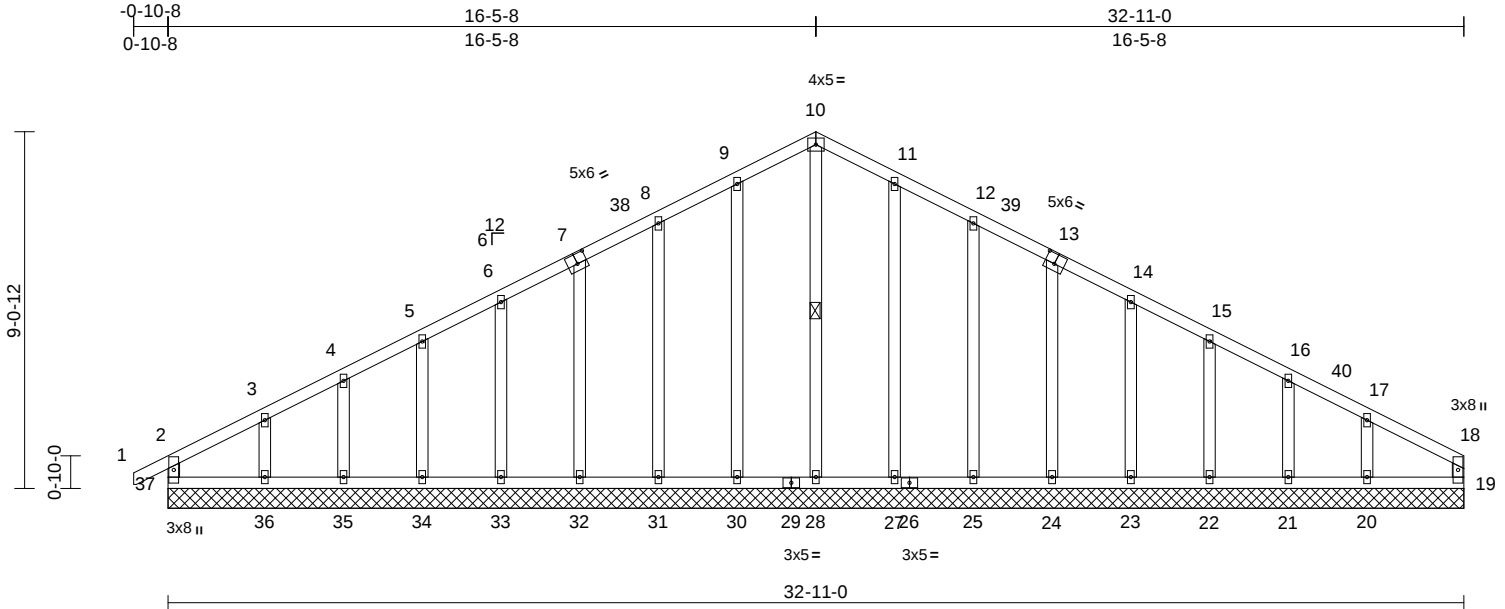
Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957936
25090186	A1	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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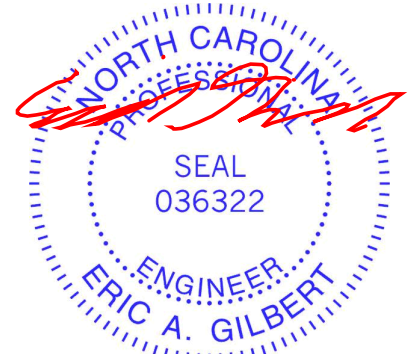
Page: 1

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Scale = 1:58.5		Plate Offsets (X, Y): [7:0-3-0,0-3-0], [13:0-3-0,0-3-0]	
Loading	(psf)	Spacing	2-0-0
TCLL (roof)	20.0	Plate Grip DOL	1.15
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15
TCDL	10.0	Rep Stress Incr	YES
BCLL	0.0*	Code	IRC2018/TPI2014
BCDL	10.0		
		CSI	
		TC	0.10
		BC	0.05
		WB	0.19
		Matrix-MR	
		DEFL	
		Vert(LL)	n/a
		Vert(CT)	n/a
		Horz(CT)	0.00
		PLATES	
		MT20	
		GRIP	
		244/190	
		Weight: 213 lb	FT = 20%

LUMBER		FORCES	(lb) - Maximum Compression/Maximum Tension	4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
TOP CHORD	2x4 SP No.2	TOP CHORD	2-37=-141/84, 1-2=0/33, 2-3=-93/59, 3-4=-76/56, 4-5=-70/78, 5-6=-70/125, 6-8=-91/211, 8-9=-107/260, 9-10=-124/301, 10-11=-124/301, 11-12=-107/260, 12-14=-91/211, 14-15=-70/125, 15-16=-57/78, 16-17=-56/36, 17-18=-71/34, 18-19=-68/16	5) Unbalanced snow loads have been considered for this design.
BOT CHORD	2x4 SP No.2	BOT CHORD	36-37=-29/70, 35-36=-29/70, 34-35=-29/70, 33-34=-29/70, 32-33=-29/70, 31-32=-29/72, 30-31=-29/72, 28-30=-29/72, 27-28=-29/72, 25-27=-29/72, 24-25=-29/72, 23-24=-28/70, 22-23=-28/70, 21-22=-28/70, 20-21=-28/70, 19-20=-28/70	6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
WEBS	2x4 SP No.3	WEBS	10-28=-198/48, 9-30=-165/67, 8-31=-144/85, 7-32=-126/76, 6-33=-120/73, 5-34=-129/81, 4-35=-123/70, 3-36=-144/127, 11-27=-164/67, 12-25=-144/85, 13-24=-126/77, 14-23=-120/73, 15-22=-130/80, 16-21=-119/69, 17-20=-155/155	7) All plates are 2x4 MT20 unless otherwise indicated.
OTHERS	2x4 SP No.3			8) Gable requires continuous bottom chord bearing.
BRACING				9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.			10) Gable studs spaced at 2-0-0 oc.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.			11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
WEBS	1 Row at midpt 10-28			12) All bearings are assumed to be SP No.2 .
REACTIONS (size)				
	19=32-11-0, 20=32-11-0, 21=32-11-0, 22=32-11-0, 23=32-11-0, 24=32-11-0, 25=32-11-0, 27=32-11-0, 28=32-11-0, 30=32-11-0, 31=32-11-0, 32=32-11-0, 33=32-11-0, 34=32-11-0, 35=32-11-0, 36=32-11-0, 37=32-11-0			
Max Horiz	37=106 (LC 14)			
Max Uplift	20=-44 (LC 16), 21=-7 (LC 16), 22=-18 (LC 16), 23=-13 (LC 16), 24=-16 (LC 16), 25=-20 (LC 16), 27=-11 (LC 16), 30=-12 (LC 15), 31=-19 (LC 15), 32=-16 (LC 15), 33=-13 (LC 15), 34=-19 (LC 15), 35=-5 (LC 15), 36=-52 (LC 15), 37=-24 (LC 11)			
Max Grav	19=91 (LC 33), 20=194 (LC 40), 21=150 (LC 2), 22=164 (LC 40), 23=154 (LC 2), 24=160 (LC 40), 25=184 (LC 23), 27=204 (LC 23), 28=157 (LC 36), 30=205 (LC 22), 31=184 (LC 22), 32=159 (LC 39), 33=154 (LC 2), 34=162 (LC 39), 35=156 (LC 2), 36=175 (LC 33), 37=163 (LC 2)			
		NOTES		
		1) Unbalanced roof live loads have been considered for this design.		
		2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-1 to 2-5-8, Exterior(2N) 2-5-8 to 16-5-8, Corner(3R) 16-5-8 to 19-8-10, Exterior(2N) 19-8-10 to 32-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33		
		3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.		



December 5,2024

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof
25090186	A1	Common Supported Gable	1	1	I69957936
Job Reference (optional)					

- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 37, 12 lb uplift at joint 30, 19 lb uplift at joint 31, 16 lb uplift at joint 32, 13 lb uplift at joint 33, 19 lb uplift at joint 34, 5 lb uplift at joint 35, 52 lb uplift at joint 36, 11 lb uplift at joint 27, 20 lb uplift at joint 25, 16 lb uplift at joint 24, 13 lb uplift at joint 23, 18 lb uplift at joint 22, 7 lb uplift at joint 21 and 44 lb uplift at joint 20.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road
Edenton, NC 27932

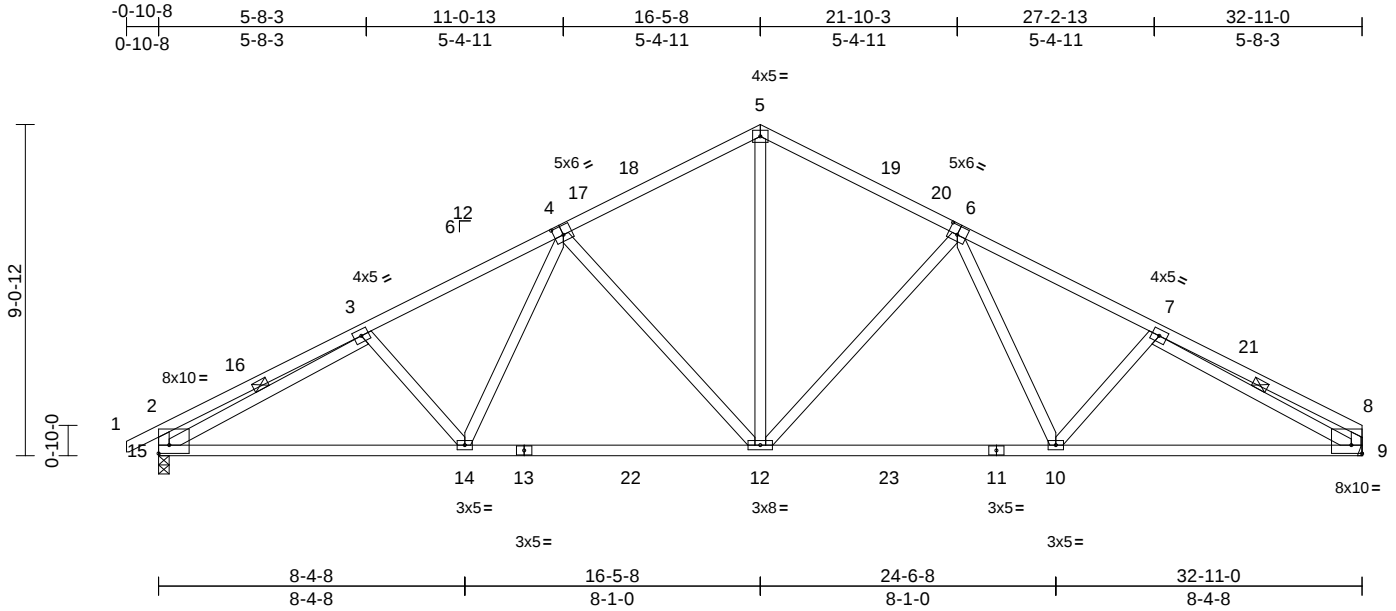
Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957937
25090186	A2	Common	7	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1

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Scale = 1:63

Plate Offsets (X, Y): [2:Edge,0-2-12], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [9:Edge,0-2-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.19	10-12	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.33	10-12	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(CT)	0.10	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 192 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-15, 7-9

REACTIONS

(size) 9= Mechanical, 15=0-3-8
Max Horiz 15=106 (LC 14)
Max Grav 9=1430 (LC 3), 15=1480 (LC 3)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-681/105, 3-5=-2471/238, 5-7=-2478/240, 7-8=-563/71, 2-15=-505/138, 8-9=-388/79
BOT CHORD 14-15=-153/2208, 12-14=-95/1948, 10-12=-84/1948, 9-10=-155/2217
WEBS 5-12=-85/1275, 6-12=-644/127, 6-10=0/462, 7-10=-175/120, 4-12=-644/127, 4-14=0/453, 3-14=-164/111, 3-15=-1963/124, 7-9=-2087/157

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-10-1 to 2-5-1, Interior (1) 2-5-1 to 16-5-8, Exterior(2R) 16-5-8 to 19-8-10, Interior (1) 19-8-10 to 32-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 15 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

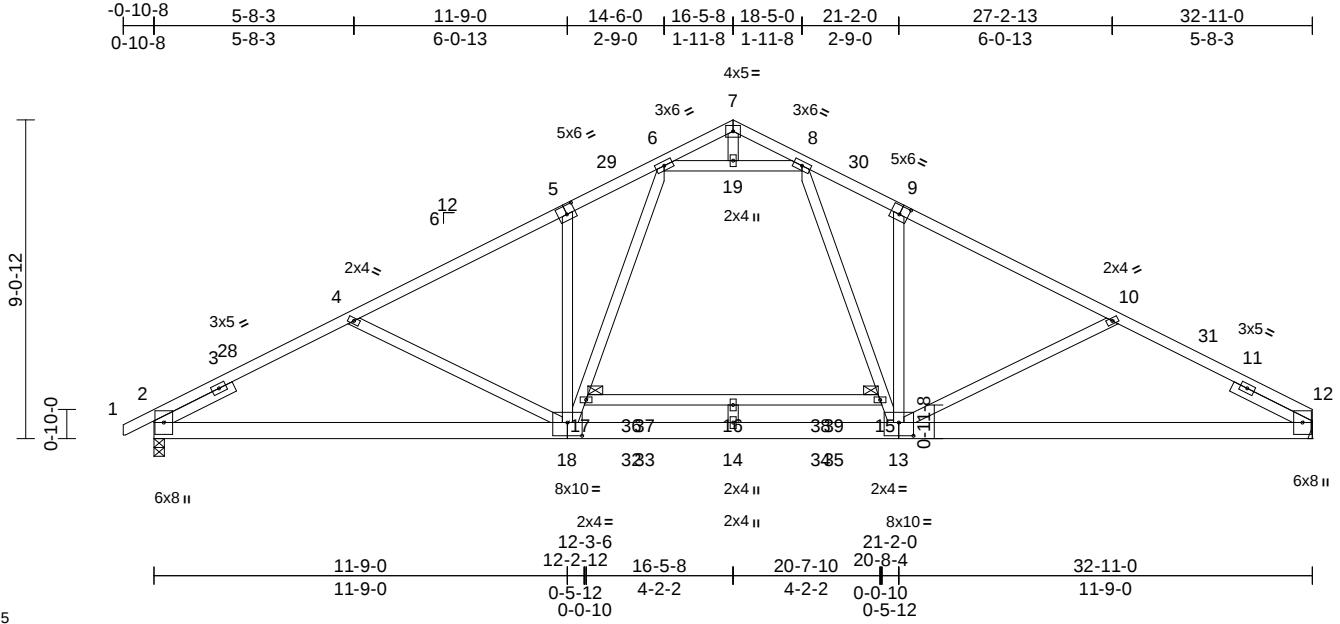
818 Soundside Road
Edenton, NC 27932

Job 25090186	Truss A3	Truss Type Common	Qty 3	Ply 1	5 Cross Creek - Galen F - Roof Job Reference (optional)	169957938
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Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:65.5

Plate Offsets (X, Y): [2:0-4-9,0-0-5], [5:0-3-0,0-3-0], [9:0-3-0,0-3-0], [12:0-4-9,0-0-5], [12:0-0-0,0-0-0], [13:0-5-0,0-4-8], [18:0-5-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.24	16	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.48	16	>821	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(CT)	0.08	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 222 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2 *Except* 0-0,17-15:2x4 SP No.2, 18-13:2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 12= Mechanical
Max Horiz 2=89 (LC 14)
Max Grav 2=1779 (LC 3), 12=1736 (LC 3)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/28, 2-4=-3234/0, 4-6=-3018/0, 6-7=-531/57, 7-8=-531/57, 8-10=-3019/0, 10-12=-3237/0
BOT CHORD 2-14=0/2820, 12-14=0/2823, 16-17=-5/66, 15-16=-5/66
WEBS 8-15=0/1252, 13-15=0/1182, 9-13=-432/139, 10-13=-298/165, 5-18=-432/138, 4-18=-295/161, 6-19=-1791/0, 8-19=-1791/0, 7-19=0/317, 17-18=0/1180, 6-17=0/1249, 14-16=-234/0

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-5-1, Interior (1) 2-5-1 to 16-5-8, Exterior(2R) 16-5-8 to 19-8-10, Interior (1) 19-8-10 to 32-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 16-5-8 from left end, supported at two points, 5-0-0 apart.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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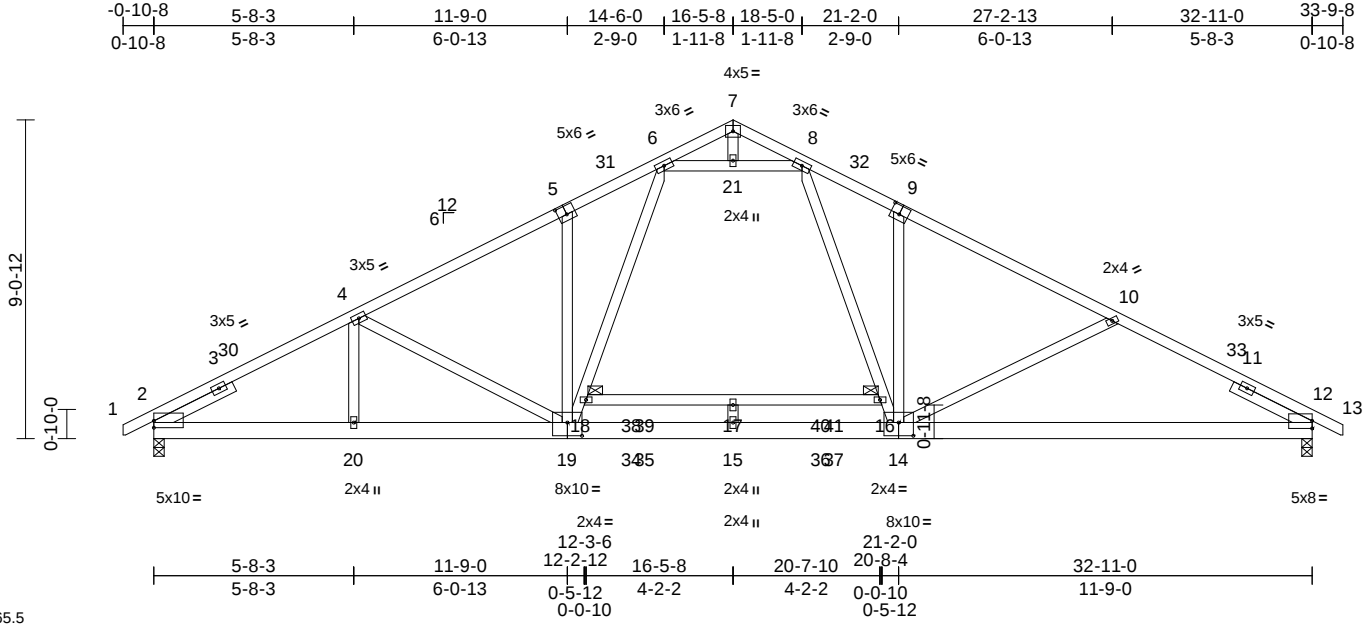
818 Soundside Road
Edenton, NC 27932

Job 25090186	Truss A4	Truss Type Common	Qty 4	Ply 1	5 Cross Creek - Galen F - Roof Job Reference (optional)	169957939
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:40
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Page: 1



Scale = 1:65.5

Plate Offsets (X, Y): [2:Edge,0-2-5], [5:0-3-0,0-3-0], [9:0-3-0,0-3-0], [12:Edge,0-2-9], [14:0-5-0,0-4-8], [19:0-5-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.22	17-18	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.46	Vert(CT)	-0.43	17-18	>921	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(CT)	0.06	12	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 228 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E *Except* 18-16:2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 12=0-3-8
Max Horiz 2=-88 (LC 13)
Max Grav 2=1779 (LC 3), 12=1779 (LC 3)

FORCES

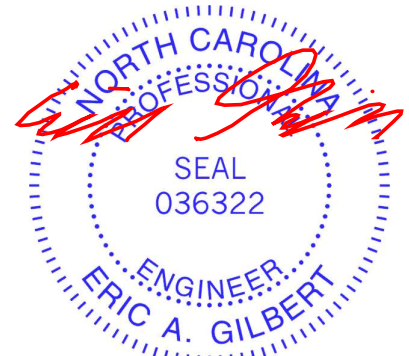
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/28, 2-4=-3155/0, 4-6=-3038/0, 6-7=-523/56, 7-8=-539/55, 8-10=-3014/0, 10-12=-3244/0, 12-13=0/28
BOT CHORD 2-20=0/2763, 15-20=0/2763, 12-15=0/2835, 17-18=-4/68, 16-17=-4/68
WEBS 5-19=-414/135, 18-19=0/1208, 6-18=0/1273, 4-19=-285/171, 8-16=0/1224, 14-16=0/1160, 9-14=-412/138, 10-14=-305/161, 6-21=-1804/0, 8-21=-1804/0, 7-21=0/319, 15-17=-242/0, 4-20=-126/3

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-5-7, Interior (1) 2-5-7 to 16-5-8, Exterior(2R) 16-5-8 to 19-9-0, Interior (1) 19-9-0 to 33-9-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 16-5-8 from left end, supported at two points, 5-0-0 apart.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP 2400F 2.0E .
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

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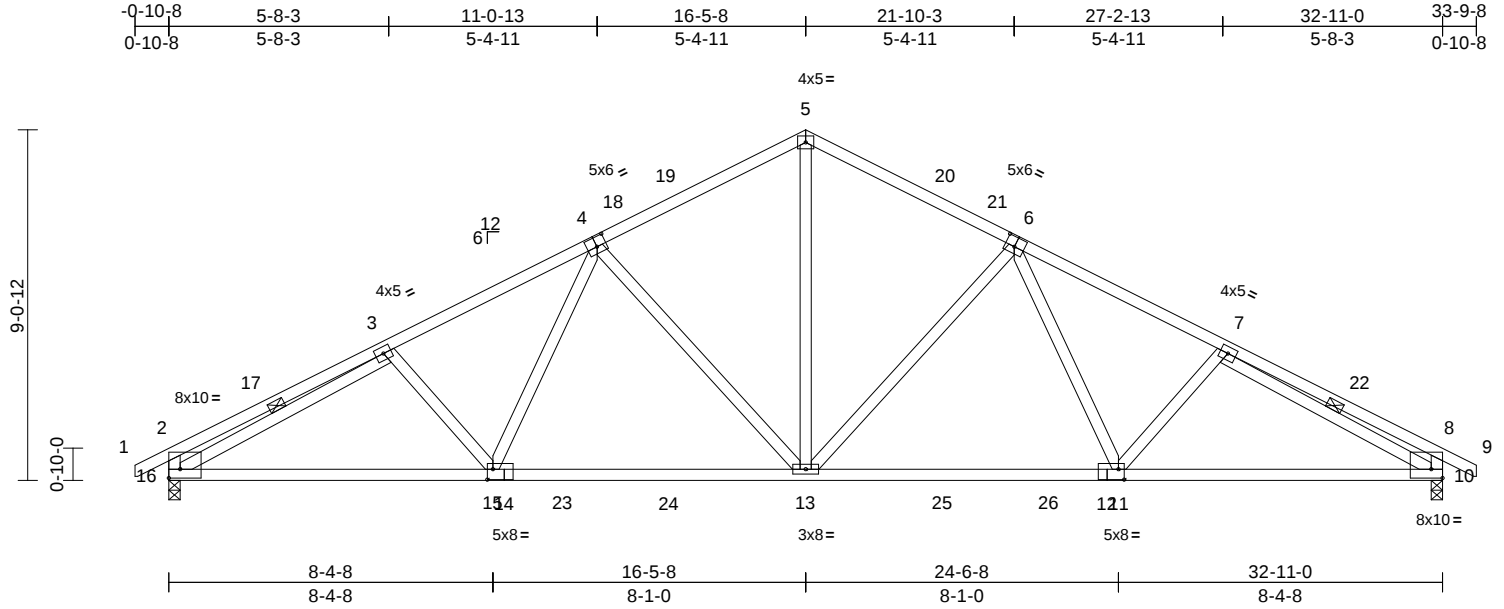
Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957940
25090186	A5	Common	4	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:41

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Scale = 1:59.5									
Plate Offsets (X, Y): [2:Edge,0-2-12], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [10:Edge,0-2-12], [11:0-1-12,0-3-4], [14:0-1-12,0-3-4]									
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.18	13-15	>999
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.92	Vert(CT)	-0.32	13-15	>999
TCDL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(CT)	0.10	10	n/a
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH					
BCDL	10.0								
Weight: 193 lb FT = 20%									

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-16, 7-10

REACTIONS
(size) 10=0-3-8, 16=0-3-8
Max Horiz 16=-105 (LC 13)
Max Grav 10=1477 (LC 3), 16=1477 (LC 3)

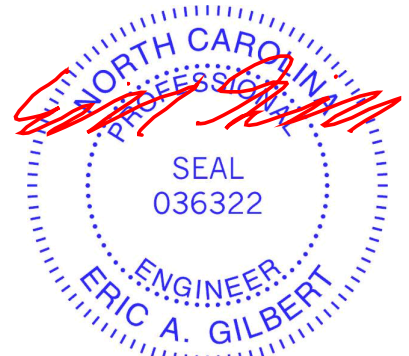
FORCES
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/33, 2-3=-681/105, 3-5=-2464/238, 5-7=-2464/238, 7-8=-681/105, 8-9=0/33, 2-16=-505/138, 8-10=-505/139
BOT CHORD 15-16=-126/2199, 13-15=-67/1940, 11-13=-55/1940, 10-11=-118/2196
WEBS 5-13=-84/1270, 6-13=-643/128, 6-11=0/450, 7-11=-164/111, 4-13=-643/128, 4-15=0/450, 3-15=-164/111, 3-16=-1956/124, 7-10=-1956/124

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 .
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-5-7, Interior (1) 2-5-7 to 16-5-8, Exterior(2R) 16-5-8 to 19-9-0, Interior (1) 19-9-0 to 33-9-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33



December 5, 2024

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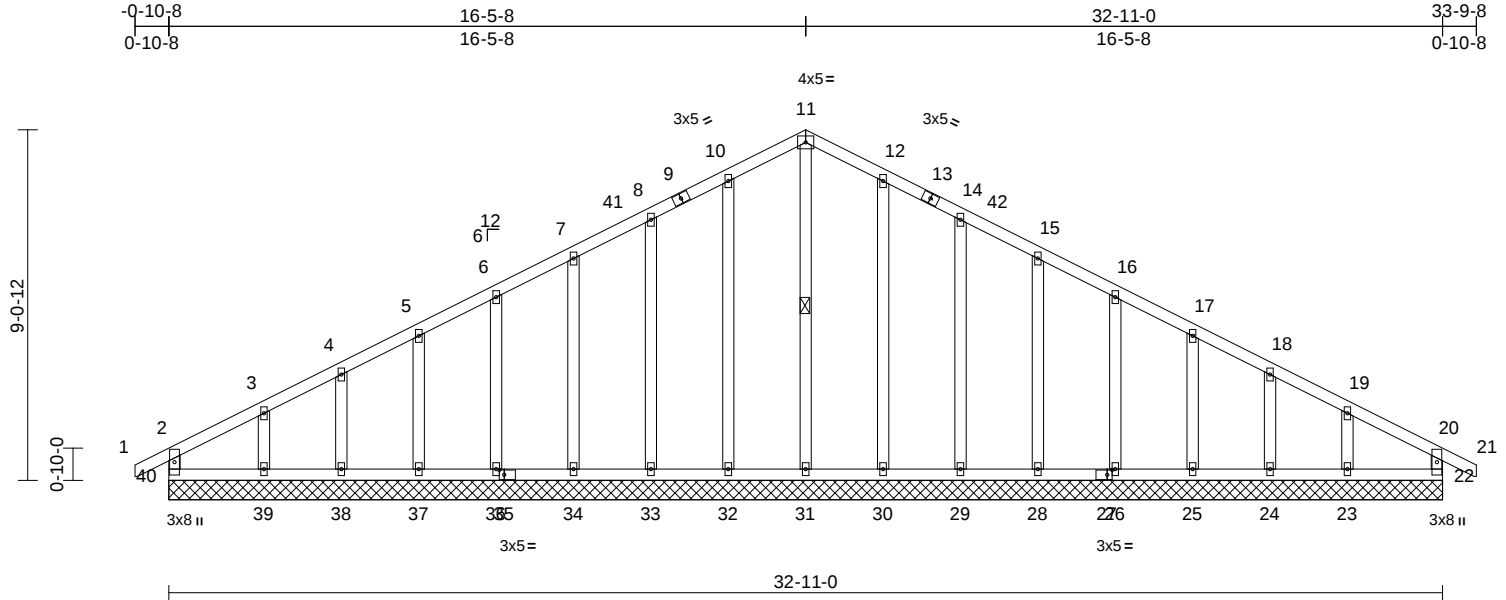
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957941
25090186	A6	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:59.5												
Plate Offsets (X, Y): [27:0-1-8,0-1-8], [35:0-1-8,0-1-8]												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.19	Horz(CT)	0.00	22	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR								
BCDL	10.0											
										Weight: 215 lb FT = 20%		

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 11-31

REACTIONS (size)
22=32-11-0, 23=32-11-0,
24=32-11-0, 25=32-11-0,
26=32-11-0, 28=32-11-0,
29=32-11-0, 30=32-11-0,
31=32-11-0, 32=32-11-0,
33=32-11-0, 34=32-11-0,
36=32-11-0, 37=32-11-0,
38=32-11-0, 39=32-11-0,
40=32-11-0
Max Horiz 40=105 (LC 13)
Max Uplift 22=7 (LC 12), 23=44 (LC 16),
24=7 (LC 16), 25=18 (LC 16),
26=15 (LC 16), 28=15 (LC 16),
29=18 (LC 16), 30=11 (LC 16),
32=12 (LC 15), 33=18 (LC 15),
34=16 (LC 15), 36=15 (LC 15),
37=18 (LC 15), 38=5 (LC 15),
39=51 (LC 15), 40=24 (LC 11)
Max Grav 22=157 (LC 2), 23=176 (LC 40),
24=156 (LC 2), 25=161 (LC 40),
26=160 (LC 2), 28=160 (LC 2),
29=179 (LC 23), 30=206 (LC 23),
31=160 (LC 36), 32=206 (LC 22),
33=179 (LC 22), 34=160 (LC 2),
36=160 (LC 2), 37=161 (LC 39),
38=156 (LC 2), 39=177 (LC 33),
40=157 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-40=-136/84, 1-2=0/33, 2-3=-85/61,
3-4=-69/56, 4-5=-63/78, 5-6=-62/125,
6-7=-74/170, 7-8=-88/215, 8-10=-107/262,
10-11=-124/303, 11-12=-124/303,
12-14=-107/262, 14-15=-88/215,
15-16=-74/170, 16-17=-62/125,
17-18=-50/79, 18-19=-50/38, 19-20=-70/37,
20-21=0/33, 20-22=-136/84
BOT CHORD 39-40=-31/87, 38-39=-31/87, 37-38=-31/87,
36-37=-31/87, 34-36=-31/87, 33-34=-31/87,
32-33=-31/87, 31-32=-31/87, 30-31=-31/87,
29-30=-31/87, 28-29=-31/87, 26-28=-31/87,
25-26=-31/87, 24-25=-31/87, 23-24=-31/87,
22-23=-31/87
WEBS 11-31=-200/49, 10-32=-166/67,
8-33=-139/82, 7-34=-126/76, 6-36=-126/77,
5-37=-128/80, 4-38=-123/70, 3-39=-146/127,
12-30=-166/67, 14-29=-139/82,
15-28=-126/76, 16-26=-126/77,
17-25=-128/80, 18-24=-123/70,
19-23=-144/127

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-1 to 2-5-8, Exterior(2N) 2-5-8 to 16-5-8, Corner(3R) 16-5-8 to 19-9-0, Exterior(2N) 19-9-0 to 33-9-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.



December 5, 2024

Continued on page 2

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof
25090186	A6	Common Supported Gable	1	1	I69957941
					Job Reference (optional)

- 12) All bearings are assumed to be SP No.2 .
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 40, 7 lb uplift at joint 22, 12 lb uplift at joint 32, 18 lb uplift at joint 33, 16 lb uplift at joint 34, 15 lb uplift at joint 36, 18 lb uplift at joint 37, 5 lb uplift at joint 38, 51 lb uplift at joint 39, 11 lb uplift at joint 30, 18 lb uplift at joint 29, 15 lb uplift at joint 28, 15 lb uplift at joint 26, 18 lb uplift at joint 25, 7 lb uplift at joint 24 and 44 lb uplift at joint 23.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

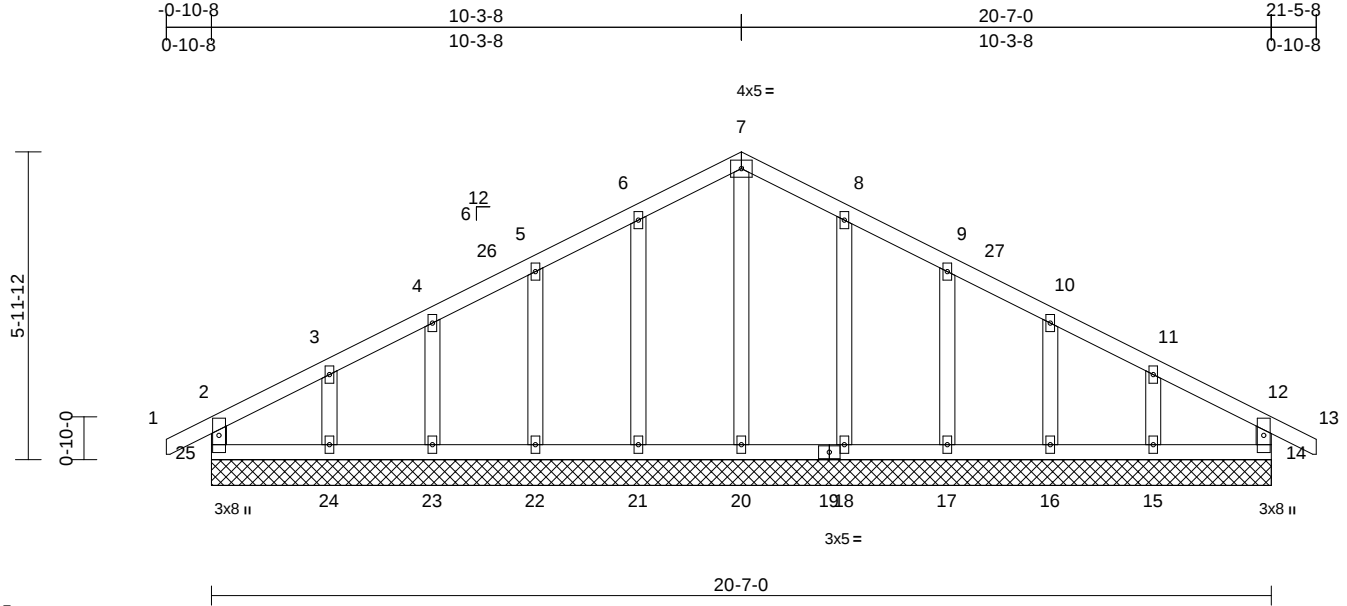
Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957942
25090186	B1	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:44.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.03	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	14	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MR							
BCDL	10.0										
										Weight: 112 lb FT = 20%	

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 14=20-7-0, 15=20-7-0, 16=20-7-0, 17=20-7-0, 18=20-7-0, 20=20-7-0, 21=20-7-0, 22=20-7-0, 23=20-7-0, 24=20-7-0, 25=20-7-0
Max Horiz 25=73 (LC 14)
Max Uplift 14=-8 (LC 12), 15=-32 (LC 16), 16=-11 (LC 16), 17=-18 (LC 16), 18=-15 (LC 16), 21=-15 (LC 15), 22=-18 (LC 15), 23=-10 (LC 15), 24=-35 (LC 15), 25=-18 (LC 11)
Max Grav 14=149 (LC 2), 15=165 (LC 40), 16=159 (LC 2), 17=180 (LC 23), 18=207 (LC 23), 20=144 (LC 2), 21=207 (LC 22), 22=180 (LC 22), 23=159 (LC 2), 24=165 (LC 39), 25=149 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-25=-130/103, 1-2=0/33, 2-3=-60/43, 3-4=-47/69, 4-5=-55/112, 5-6=-67/160, 6-7=-83/205, 7-8=-83/205, 8-9=-67/160, 9-10=-55/113, 10-11=-44/68, 11-12=-48/32, 12-13=0/33, 12-14=-130/102
BOT CHORD 24-25=-23/56, 23-24=-23/56, 22-23=-23/56, 21-22=-23/56, 20-21=-23/56, 18-20=-23/56, 17-18=-23/56, 16-17=-23/56, 15-16=-23/56, 14-15=-23/56

WEBS 7-20=-119/15, 6-21=-167/77, 5-22=-139/83, 4-23=-126/76, 3-24=-136/112, 8-18=-167/77, 9-17=-139/83, 10-16=-126/76, 11-15=-135/112

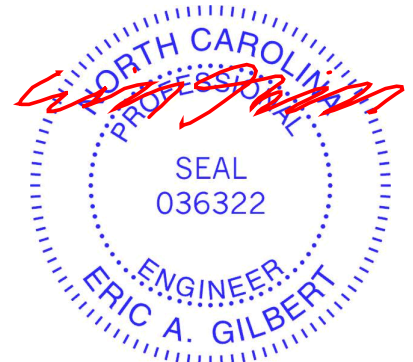
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-1 to 2-3-8, Exterior(2N) 2-3-8 to 10-3-8, Corner(3R) 10-3-8 to 13-3-8, Exterior(2N) 13-3-8 to 21-5-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 25, 8 lb uplift at joint 14, 15 lb uplift at joint 21, 18 lb uplift at joint 22, 10 lb uplift at joint 23, 35 lb uplift at joint 24, 15 lb uplift at joint 18, 18 lb uplift at joint 17, 11 lb uplift at joint 16 and 32 lb uplift at joint 15.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

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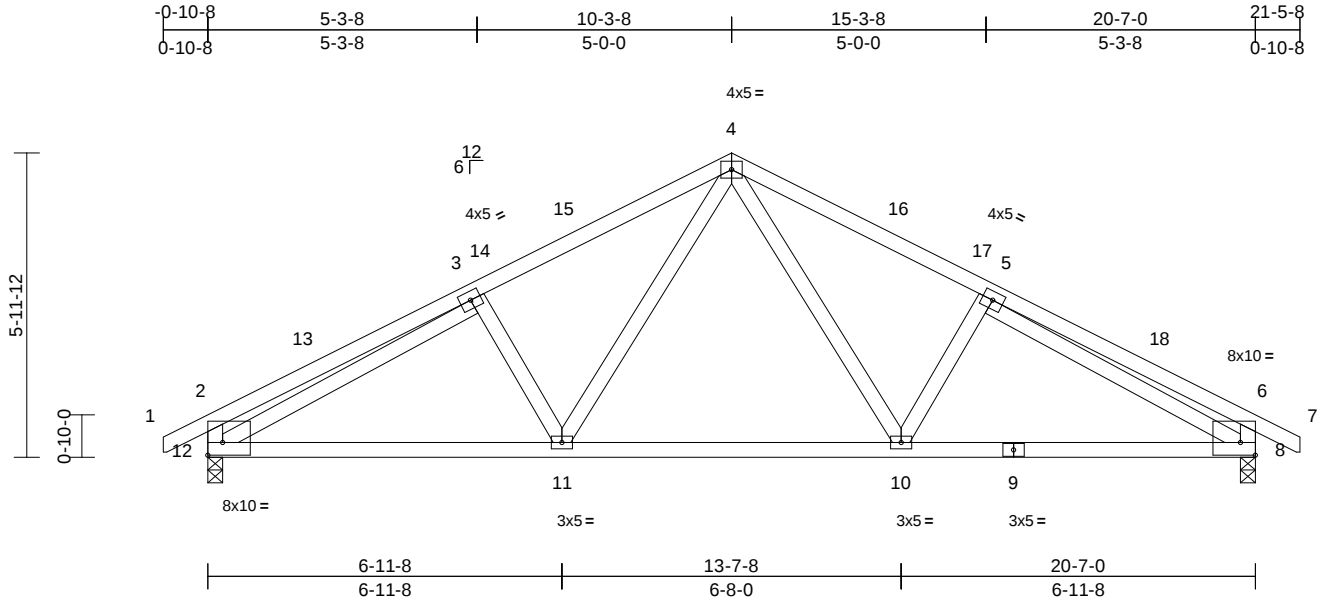
818 Soundside Road
Edenton, NC 27932

Job 25090186	Truss B2	Truss Type Common	Qty 3	Ply 1	5 Cross Creek - Galen F - Roof Job Reference (optional)	169957943
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:41
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Page: 1



Scale = 1:45.3

Plate Offsets (X, Y): [6:Edge,0-3-0], [12:Edge,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	Vert(LL)	-0.04	10-11	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.39	Vert(CT)	-0.09	10-11	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.03	8	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 115 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 8=0-3-8, 12=0-3-8
Max Horiz 12=73 (LC 14)
Max Grav 8=871 (LC 2), 12=871 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension
1-2=0/33, 2-3=-395/115, 3-4=-1117/240,
4-5=-1117/239, 5-6=-394/115, 6-7=0/33,
2-12=-352/148, 6-8=-353/148

11-12=-131/1022, 10-11=-28/728,
8-10=-119/1020

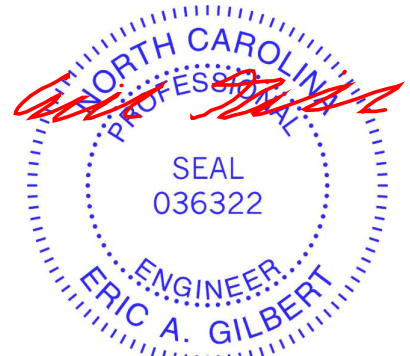
WEBS 4-10=-54/387, 5-10=-226/152, 4-11=-54/387,
3-11=-226/152, 3-12=-872/116, 5-8=-872/116

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-10-1 to 2-1-15, Interior (1) 2-1-15 to 10-3-8, Exterior(2R) 10-3-8 to 13-3-8, Interior (1) 13-3-8 to 21-5-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

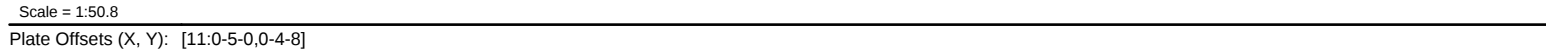
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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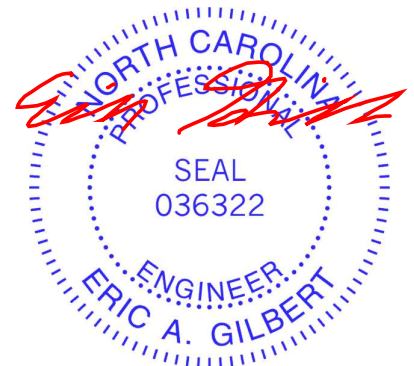
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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:41 Page: 1
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LUMBER	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP 2400F 2.0E
WEBS	2x4 SP No.3 *Except* 11-5:2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-10-12 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS	(size) 2=0-3-8, 8=0-3-8
	Max Horiz 2=58 (LC 10)
	Max Grav 2=7425 (LC 3), 8=9078 (LC 3)
FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/28, 2-4=-11626/0, 4-5=-8865/0, 5-6=-8866/0, 6-8=-12272/0
BOT CHORD	2-12=0/10288, 11-12=0/10288, 9-11=0/10872, 8-9=0/10872
WEBS	5-11=0/7521, 6-11=-3502/0, 6-9=0/3360, 4-11=-2815/0, 4-12=0/2727
NOTES	
1)	3-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 3 rows staggered at 0-5-0 oc. Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
2)	All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3)	Unbalanced roof live loads have been considered for this design.
4)	Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
5)	TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
6)	Unbalanced snow loads have been considered for this design.
7)	This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
8)	* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9)	All bearings are assumed to be SP 2400F 2.0E .
10)	This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11)	Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-0 from the left end to 20-0-0 to connect truss(es) to back face of bottom chord.
12)	Fill all nail holes where hanger is in contact with lumber.
LOAD CASE(S)	Standard
1)	Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (lb/ft) Vert: 1-5=-48, 5-8=-48, 13-17=-20 Concentrated Loads (lb) Vert: 10=-1084 (B), 11=-1084 (B), 15=-1285 (B), 23=-1084 (B), 24=-1084 (B), 25=-1084 (B), 26=-1084 (B), 27=-1084 (B), 28=-1281 (B), 29=-1281 (B)



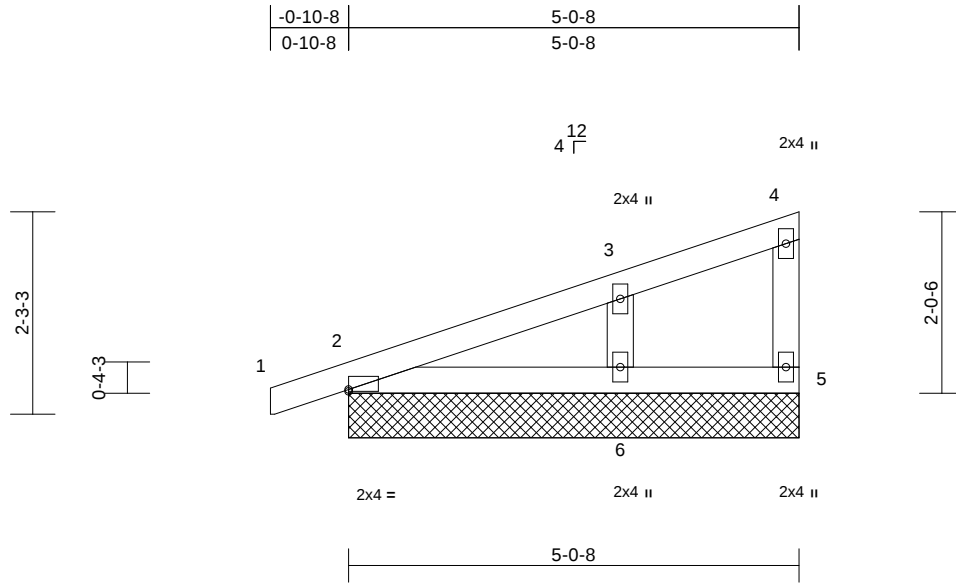
December 5, 2024

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	
25090186	C1	Monopitch Supported Gable	1	1	Job Reference (optional)	I69957945

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:25.8

Plate Offsets (X, Y): [2:Edge,0-0-4]

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.24	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 20 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-0-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size) 2=5-0-8, 5=5-0-8, 6=5-0-8, 7=5-0-8
Max Horiz	2=53 (LC 14), 7=53 (LC 14)
Max Uplift	2=-21 (LC 11), 7=-21 (LC 11)
Max Grav	2=184 (LC 22), 5=95 (LC 22), 6=290 (LC 22), 7=184 (LC 22)

FORCES

(lb) - Maximum Compression/Maximum Tension

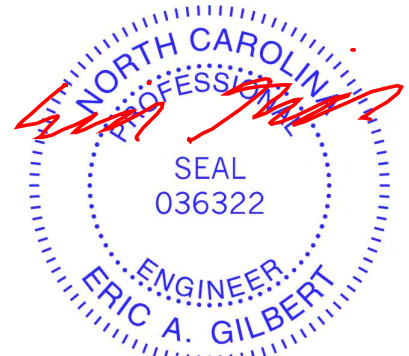
TOP CHORD	1-2=0/20, 2-3=-126/79, 3-4=-45/35, 4-5=-37/62
BOT CHORD	2-6=-46/64, 5-6=-28/38
WEBS	3-6=-231/231

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-3 to 2-1-13, Exterior(2N) 2-1-13 to 4-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 2 and 21 lb uplift at joint 2.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

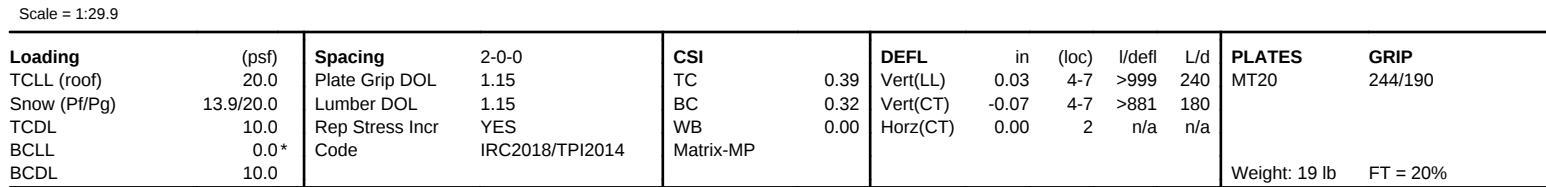
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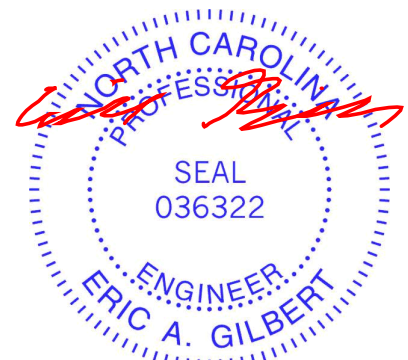
Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:41 Page: 1
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- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: Joint 2 SP No.2 , Joint 4 SP No.3 .
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2, 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2E) -0-10-3 to 2-1-13, Interior (1) 2-1-13 to 4-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) TCDL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.



December 5, 2024

 **WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

WARNING – Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-17-15169: 1/2/2023 BEFORE USE.

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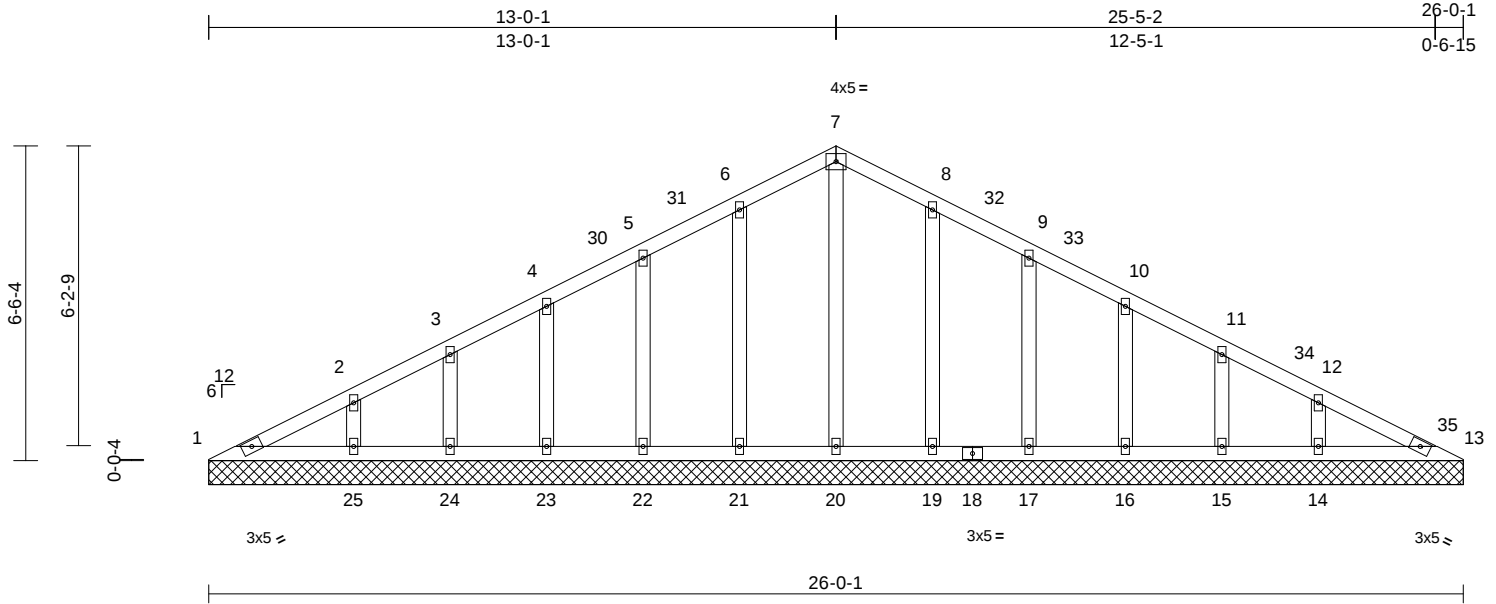
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957947
25090186	VL1	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	13	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0											
Weight: 133 lb FT = 20%												

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=26-0-1, 13=26-0-1, 14=26-0-1, 15=26-0-1, 16=26-0-1, 17=26-0-1, 19=26-0-1, 20=26-0-1, 21=26-0-1, 22=26-0-1, 23=26-0-1, 24=26-0-1, 25=26-0-1

Max Horiz 1=67 (LC 12)

Max Uplift 14=-12 (LC 16), 15=-18 (LC 16), 16=-15 (LC 16), 17=-17 (LC 16), 19=-15 (LC 16), 21=-16 (LC 15), 22=-17 (LC 15), 23=-15 (LC 15), 24=-17 (LC 15), 25=-14 (LC 15)

Max Grav 1=94 (LC 38), 13=64 (LC 39), 14=236 (LC 39), 15=134 (LC 2), 16=167 (LC 39), 17=178 (LC 22), 19=207 (LC 22), 20=164 (LC 2), 21=207 (LC 21), 22=177 (LC 21), 23=167 (LC 38), 24=132 (LC 2), 25=242 (LC 38)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-133/80, 2-3=-49/63, 3-4=-29/67, 4-5=-24/71, 5-6=-42/106, 6-7=-61/140, 7-8=-61/136, 8-9=-42/103, 9-10=-24/67, 10-11=-11/48, 11-12=-25/43, 12-13=-105/69

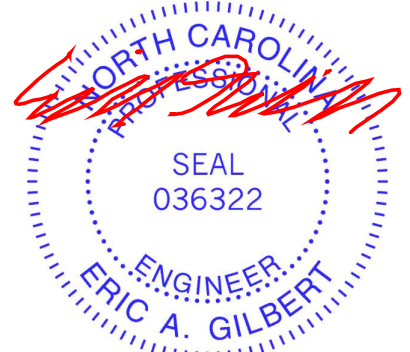
BOT CHORD 1-25=-43/116, 24-25=-43/68, 23-24=-43/68, 22-23=-43/68, 21-22=-43/68, 20-21=-43/68, 19-20=-43/68, 17-19=-43/68, 16-17=-43/68, 15-16=-43/68, 14-15=-43/68, 13-14=-43/94

WEBS 7-20=-125/0, 6-21=-167/75, 5-22=-138/57, 4-23=-129/53, 3-24=-115/54, 2-25=-162/60, 8-19=-167/75, 9-17=-138/57, 10-16=-129/53, 11-15=-115/54, 12-14=-159/61

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-8 to 3-0-9, Interior (1) 3-0-9 to 13-0-9, Exterior(2R) 13-0-9 to 16-0-9, Interior (1) 16-0-9 to 25-5-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 .

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 21, 17 lb uplift at joint 22, 15 lb uplift at joint 23, 17 lb uplift at joint 24, 14 lb uplift at joint 25, 15 lb uplift at joint 19, 17 lb uplift at joint 17, 15 lb uplift at joint 16, 18 lb uplift at joint 15 and 12 lb uplift at joint 14.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

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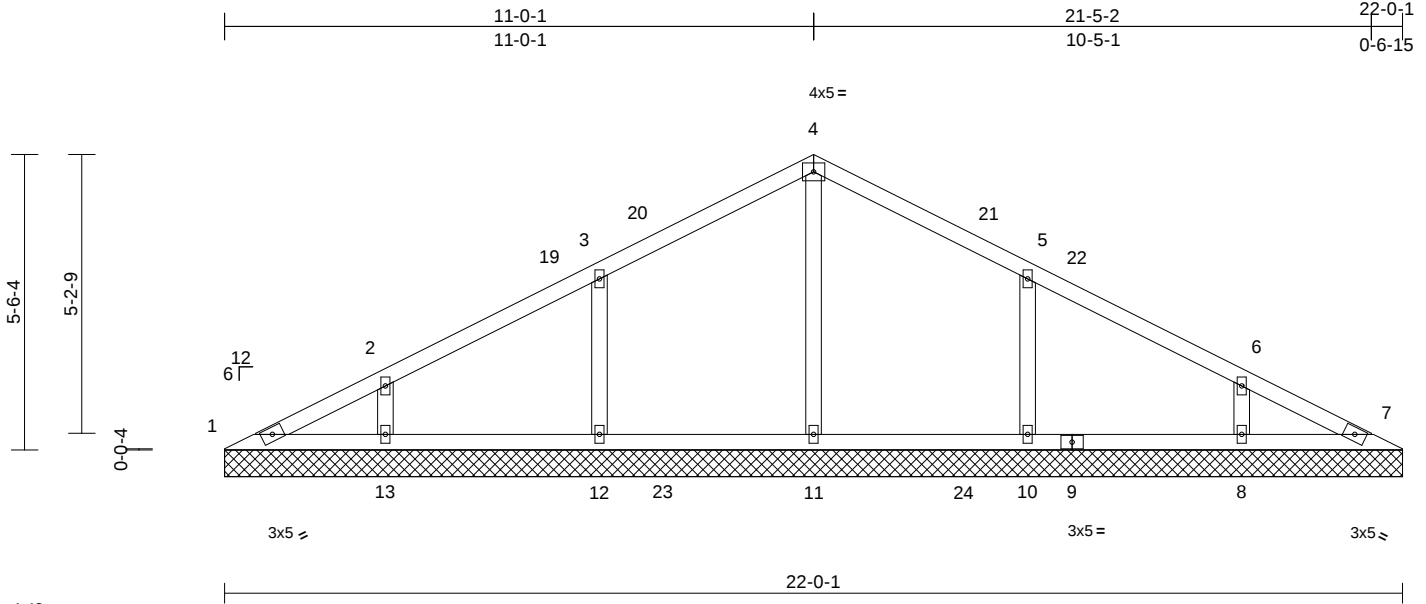
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957948
25090186	VL2	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.16	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.17	Horiz(TL)	0.00	7	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 86 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS	(size)	1=22-0-1, 7=22-0-1, 8=22-0-1, 10=22-0-1, 11=22-0-1, 12=22-0-1, 13=22-0-1, 18=22-0-1
	Max Horiz	1=56 (LC 12)
	Max Uplift	1=-7 (LC 16), 8=-20 (LC 16), 10=-42 (LC 16), 12=-39 (LC 15), 13=-25 (LC 15)
	Max Grav	1=71 (LC 38), 8=328 (LC 33), 10=391 (LC 6), 11=479 (LC 32), 12=400 (LC 5), 13=313 (LC 32)

FORCES

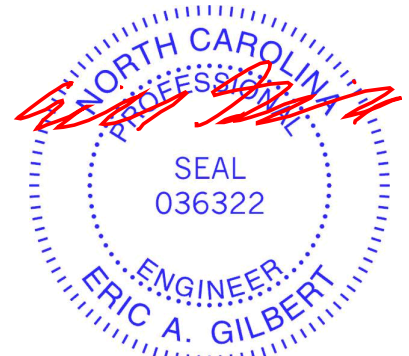
	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=-82/166, 2-3=-9/173, 3-4=-25/175, 4-5=-25/169, 5-6=0/161, 6-7=-36/154
BOT CHORD	1-13=-108/64, 12-13=-108/41, 11-12=-108/41, 10-11=-108/41, 8-10=-108/41, 7-8=-108/41
WEBS	4-11=-308/0, 3-12=-299/143, 2-13=-222/107, 5-10=-294/144, 6-8=-229/105

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-8 to 3-0-9, Interior (1) 3-0-9 to 11-0-9, Exterior(2R) 11-0-9 to 14-0-9, Interior (1) 14-0-9 to 22-0-9 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1, 39 lb uplift at joint 12, 25 lb uplift at joint 13, 42 lb uplift at joint 10 and 20 lb uplift at joint 8.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

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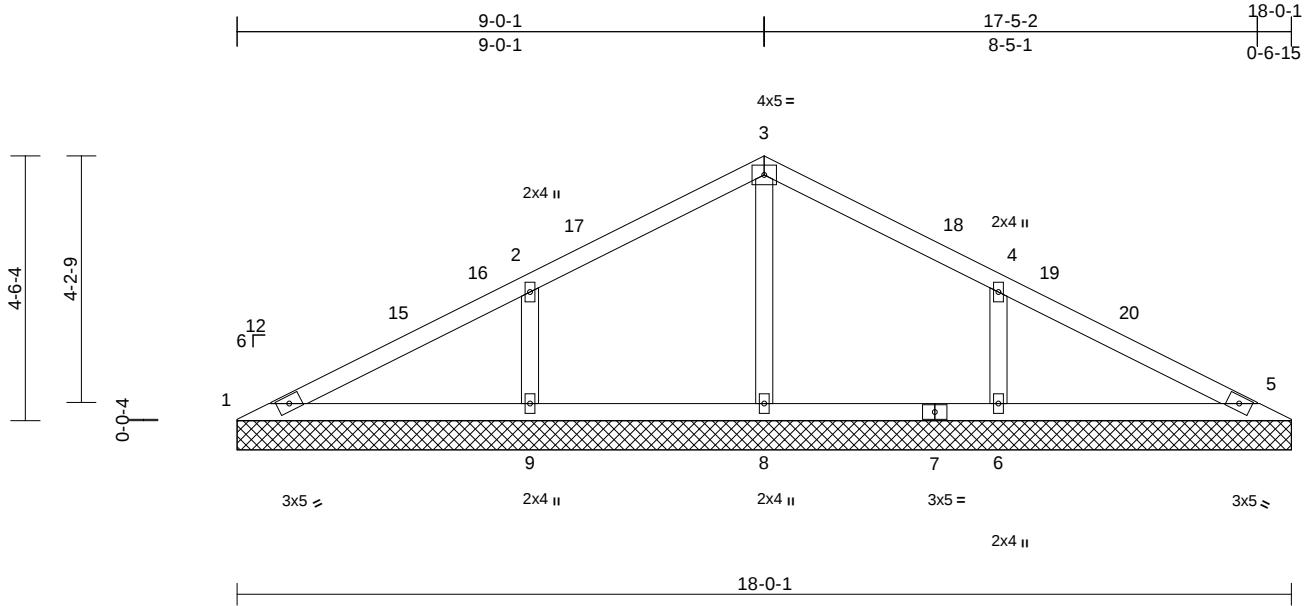
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957949
25090186	VL3	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:39.4

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.35	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.20	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.18	Horiz(TL)	-0.01	5	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 66 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=18-0-1, 5=18-0-1, 6=18-0-1, 8=18-0-1, 9=18-0-1, 14=18-0-1
Max Horiz	1=46 (LC 12)
Max Uplift	1=-34 (LC 39), 6=-43 (LC 16), 9=-46 (LC 15)
Max Grav	1=100 (LC 38), 5=0 (LC 2), 6=438 (LC 22), 8=520 (LC 2), 9=441 (LC 21), 14=0 (LC 2)

FORCES

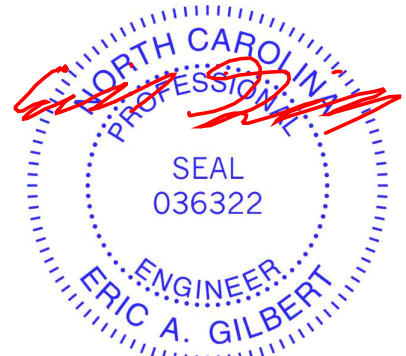
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-130/370, 2-3=0/347, 3-4=0/347, 4-5=-115/370
BOT CHORD	1-9=-283/111, 8-9=-283/91, 6-8=-283/91, 5-6=-283/91
WEBS	3-8=-467/70, 2-9=-326/165, 4-6=-325/166

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-8 to 3-0-8, Interior (1) 3-0-8 to 9-0-9, Exterior(2R) 9-0-9 to 12-0-9, Interior (1) 12-0-9 to 18-0-9 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1, 46 lb uplift at joint 9 and 43 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

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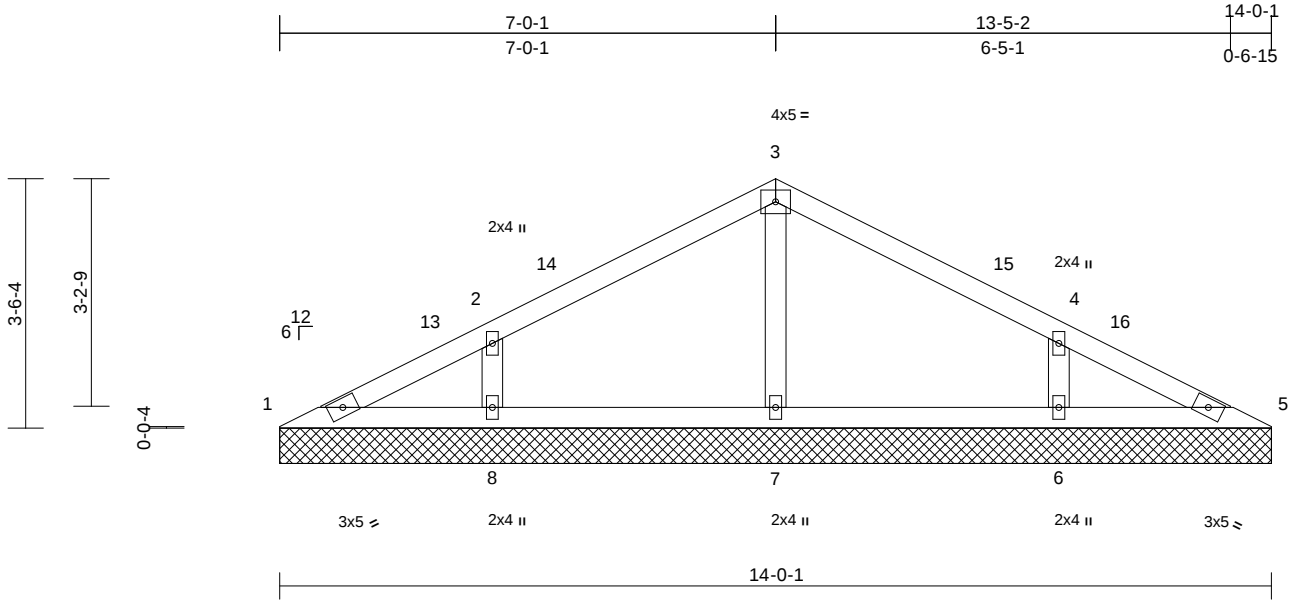
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957950
25090186	VL4	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:32.5												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.08	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 49 lb	FT = 20%

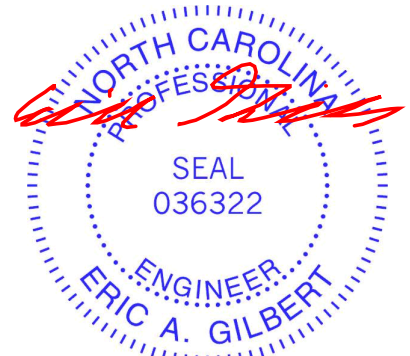
LUMBER		
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
OTHERS	2x4 SP No.3	
BRACING		
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.	
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	
REACTIONS		(size)
	1=14-0-1, 5=14-0-1, 6=14-0-1, 7=14-0-1, 8=14-0-1	
	Max Horiz	1=-34 (LC 13)
	Max Uplift	6=-34 (LC 16), 8=-35 (LC 15)
	Max Grav	1=81 (LC 2), 5=81 (LC 2), 6=367 (LC 22), 7=311 (LC 2), 8=367 (LC 21)
FORCES		(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=-104/68, 2-3=-78/91, 3-4=-78/86, 4-5=-104/61	
BOT CHORD	1-8=-26/86, 7-8=-26/42, 6-7=-26/42, 5-6=-26/84	
WEBS	3-7=-231/65, 2-8=-297/173, 4-6=-297/173	

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-8 to 3-0-9, Interior (1) 3-0-9 to 7-0-9, Exterior(2R) 7-0-9 to 10-0-9, Interior (1) 10-0-9 to 14-0-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 8 and 34 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

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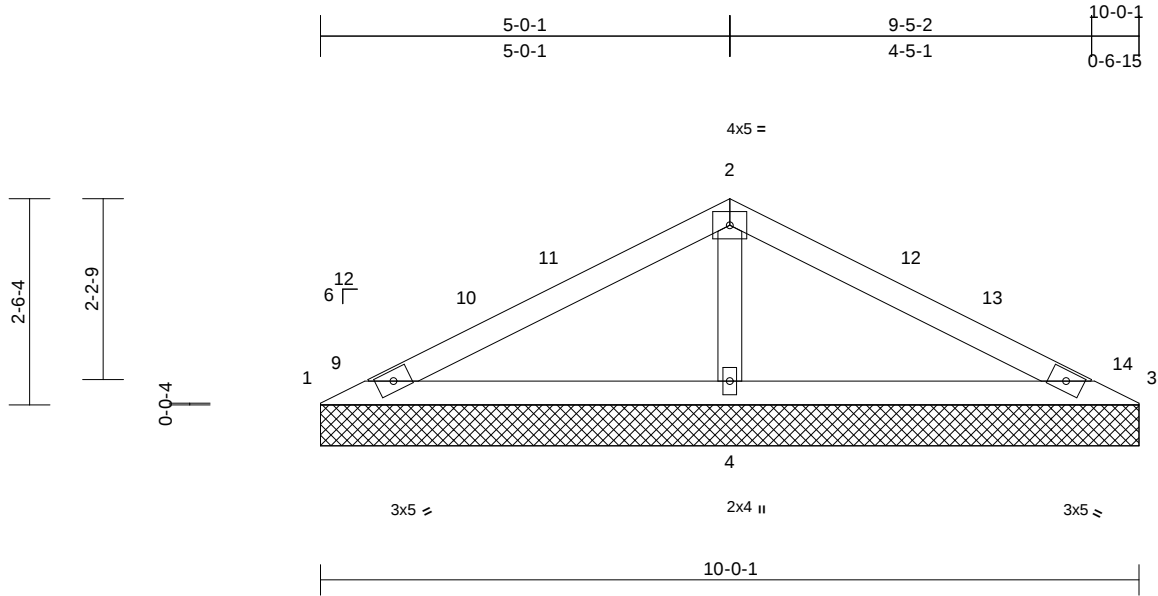
Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957951
25090186	VL5	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:42

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Scale = 1:28.2

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.33	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 32 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10'-0'-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6'-0'-0 oc bracing.

REACTIONS

(size)	1=10-0-1, 3=10-0-1, 4=10-0-1
Max Horiz	1=24 (LC 14)
Max Uplift	1=-17 (LC 22), 3=-17 (LC 21)
Max Grav	1=94 (LC 21), 3=94 (LC 22), 4=705 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

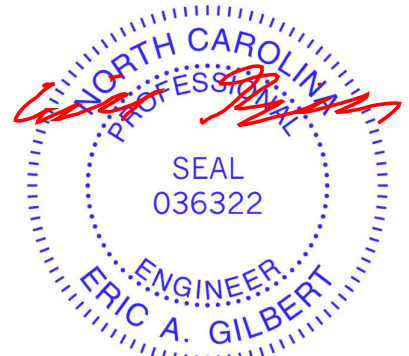
TOP CHORD	1-2=-137/370, 2-3=-130/370
BOT CHORD	1-4=-306/179, 3-4=-306/179
WEBS	2-4=-569/274

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-8 to 3-0-8, Interior (1) 3-0-8 to 5-0-9, Exterior(2R) 5-0-9 to 8-0-9, Interior (1) 8-0-9 to 10-0-9 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4'-0'-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0'-0 tall by 2'-0'-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1 and 17 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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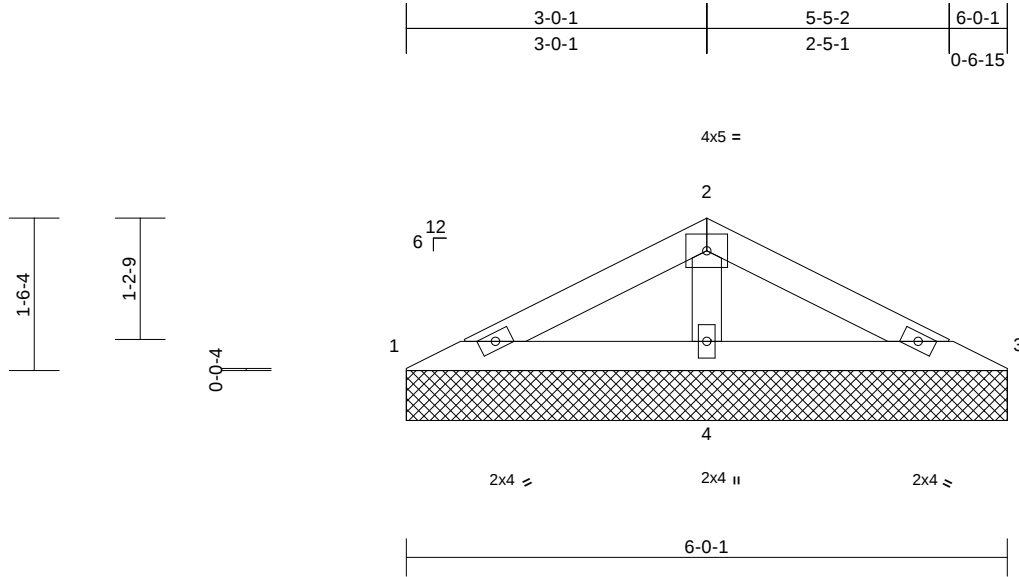
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	5 Cross Creek - Galen F - Roof	169957952
25090186	VL6	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Oct 31 2024 Print: 8.730 S Oct 31 2024 MiTek Industries, Inc. Tue Dec 03 13:56:42
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Page: 1



Scale = 1:23

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.12	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	4	n/a	n/a	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 18 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-1 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=6-0-1, 3=6-0-1, 4=6-0-1
Max Horiz	1=14 (LC 14)
Max Uplift	1=-2 (LC 15), 3=-4 (LC 16)
Max Grav	1=82 (LC 21), 3=82 (LC 22), 4=363 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

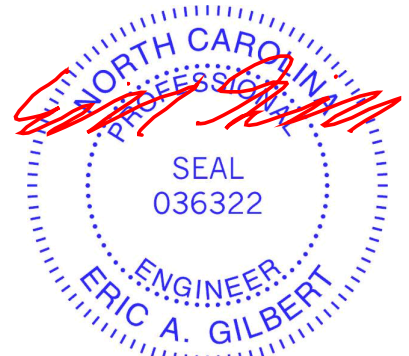
TOP CHORD	1-2=-88/157, 2-3=-88/157
BOT CHORD	1-4=-142/113, 3-4=-142/113
WEBS	2-4=-253/157

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 4 lb uplift at joint 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



December 5,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

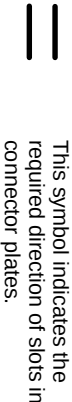
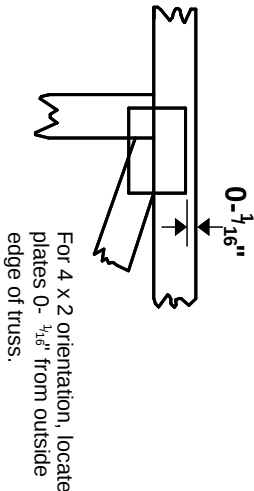
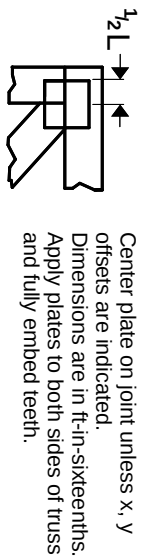
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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Symbols

PLATE LOCATION AND ORIENTATION



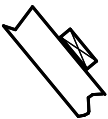
* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

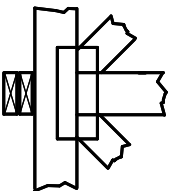
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

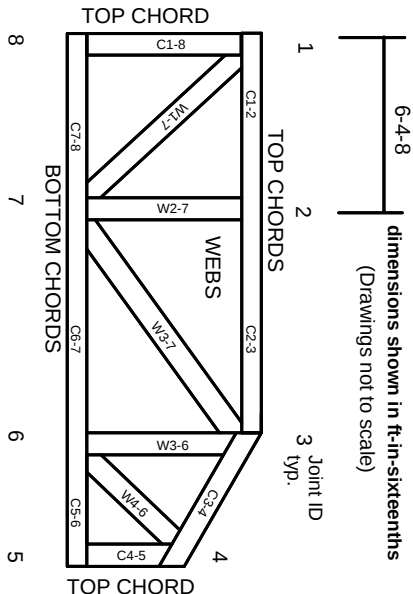
BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:
ANSI/TPI 1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:
ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.
Lumber design values are in accordance with ANSI/TPI 1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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MITek Engineering Reference Sheet: MIL-7473 rev. 1/2/2023