Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 61436 JOB: 25-7447-R01

JOB NAME: LOT 126 PROVIDENCE CREEK

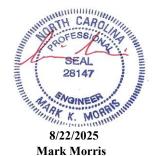
Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 35

These truss designs comply with IRC 2015 as well as IRC 2018.

19 Truss Design(s)

Trusses:

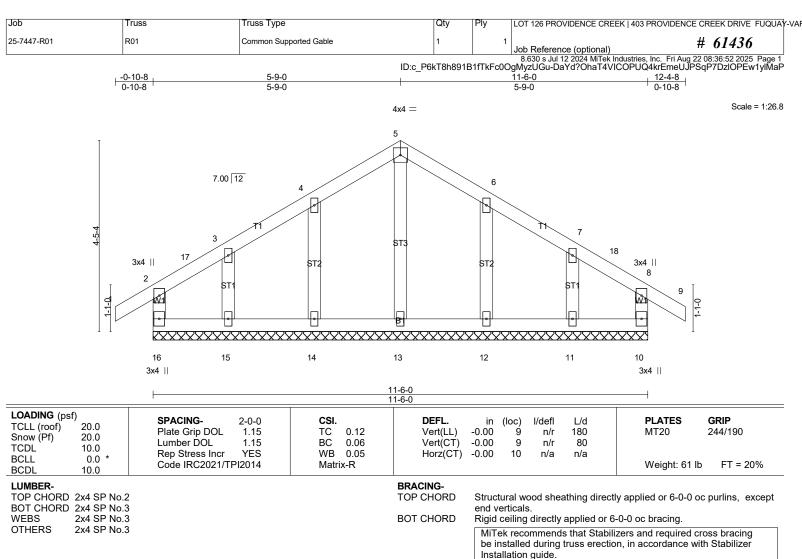
R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, SP01, SP02, VT01, VT02, VT03, VT04



My license renewal date for the state of North Carolina is 12/31/2025

Warning !--Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



REACTIONS. All bearings 11-6-0.

(lb) - Max Horz 16=-113(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 15, 11 except 14=255(LC 21), 12=255(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed: Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-9-0, Corner(3R) 3-9-0 to 7-9-0, Corner(3E) 7-9-0 to 12-4-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated
- 8) Gable requires continuous bottom chord bearing.

- 8) Gable required 19) Truss to be fully sheathed from one race 2.

 10) Gable studs spaced at 2-0-0 oc.

 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any 2...

 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-0-0 ten 2, fit between the bottom chord and any other members.

 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.

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Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an increase of an i of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE	CREEK DRIVE FUQUAY	VAI
25-7447-R01	R01	Common Supported Gable	1	1	Job Reference (optional)	# 61436	

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:52 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-DaYd?OhaT4VICOPUQ4krEmeUJPSqP7DzIOPEw1ylMaP

- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R02 DUAL RIDGE GABLE # 61436 ob Reference (optional) 8.630 s Juli 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:52 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-DaYd?OhaT4VICOPUQ4krEmePePO7P1gzlOPEw1ylMaF -0-10-8 0-10-8 10-0-0 20-0-0 20-10-8 0-10-8 4-3-0 10-0-0 Scale = 1:43.2 4x4 = 5 6 7.00 12 3x6 / 3 25 24 5x6 / 3x4 || 10 W W2 B1 🖟 • 20 18 19 3x6 =3x4 || 4x4 = 3x4 || 17 16 15 14 13 12 5-9-0 5-9-0 8-6-0 Plate Offsets (X,Y)-- [2:0-2-9,0-2-8] LOADING (psf) SPACING-DEFL. PLATES GRIP 2-0-0 CSI. in (loc) I/defl I/d TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.42 Vert(LL) -0.02 19-20 >999 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.30 Vert(CT) -0.05 19-20 >999 180 **TCDL** 10.0 WB 0.41 Rep Stress Incr YES Horz(CT) 0.01 12 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Matrix-SH Weight: 126 lb FT = 20%BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2x4 SP No.2 end verticals 2x4 SP No 3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. WFBS JOINTS 1 Brace at Jt(s): 21, 23 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer REACTIONS. All bearings 8-9-8 except (jt=length) 20=0-3-8, 17=0-3-8. (lb) - Max Horz 20=-167(LC 12) Max Uplift All uplift 100 lb or less at joint(s) 20, 16, 15, 14, 12 except 13=-127(LC 15) Max Grav All reactions 250 lb or less at joint(s) 16, 14, 13, 17 except 20=695(LC 21), 15=276(LC 22), 12=338(LC FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-24=-777/84, 3-24=-598/87, 3-4=-330/78, 6-7=-263/66, 9-10=-272/28, 2-20=-641/114, 10-12=-277/6 **BOT CHORD** 19-20=-162/286, 18-19=-69/600, 17-18=-69/600, 16-17=-69/600 3-23=-456/134, 21-23=-488/144, 21-22=-460/131, 16-22=-487/142, 2-19=0/440 WEBS NOTES-1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 5-2-6, Exterior(2R) 5-2-6 to 14-9-10, Interior(1) 14-9-10 to 16-0-0, Exterior(2E) 16-0-0 to 20-10-8 zone; cantilever left and right exposed; end OR OFESS! vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide w🎛 fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 16, 15, 14, 12 except (jt=lb) 13=127.

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MORPH HAMING Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an increase of an i of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE C	REEK DRIVE FUQUAY-\
25-7447-R01	R02	DUAL RIDGE GABLE	1	1	Job Reference (optional) #	61436

8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:36:53 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-hm6?DjiCEOd9qX_h_oF4nzBaOokM8Uw7_28oSTylMaO

- 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R03 Common 3 # 61436 lob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:53 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-hm6?DjiCEOd9qX_h_oF4nzBZ2oa68RQ7_28oSTylMaC -0-10-8 0-10-8 14-10-4 10-0-0 20-0-0 5-1-12 4-10-4 4-10-4 5-1-12 0-10-8 Scale = 1:41.4 4x4 = 4 7.00 12 4x4 // 4x4 < 5 3 3x4 II 3x4 || 6 ⊠ 10 9 8 5x8 = 3x6 = 3x6 = 10-0-0 10-0-0 Plate Offsets (X,Y)-- [9:0-4-0,0-3-4] LOADING (psf) CSI. DEFL. GRIP SPACING-2-0-0 in (loc) I/defl I/d PLATES TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.50 Vert(LL) -0.199-10 >999 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.96 Vert(CT) -0.38 9-10 >630 180 TCDL 10.0 WB 0.63 Rep Stress Incr YES Horz(CT) 0.03 8 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Weight: 114 lb Matrix-SH FT = 20%**BCDL** 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-3-13 oc purlins, except BOT CHORD 2x4 SP No.2 end verticals 2x4 SP No.3 BOT CHORD WFBS Rigid ceiling directly applied or 2-2-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. (lb/size) 10=850/0-3-8 (min. 0-1-8), 8=850/0-3-8 (min. 0-1-8) Max Horz 10=-167(LC 12)

Max Uplift10=-110(LC 14), 8=-110(LC 15) Max Grav 10=894(LC 21), 8=894(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-308/79. 3-4=-853/128. 4-5=-853/128. 5-6=-307/78. 2-10=-322/107. 6-8=-321/107 TOP CHORD

BOT CHORD 9-10=-134/854 8-9=-64/854

WFBS 4-9=-32/494, 5-9=-286/176, 3-9=-286/176, 3-10=-853/93, 5-8=-853/89

NOTES-(9-12)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 5-0-3, Exterior(2R) 5-0-3 to 14-11-13, Interior(1) 14-11-13 to 16-0-14, Exterior(2E) 16-0-14 to 20-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of the second s
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=110 8=110 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates Hat
- the member must be braced. 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING

MORPHER HANDER TO THE PARTY OF WATONS IDENTIFY DESCRIPTION OF DESCRIPTION OF THE PROPERTY OF of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive Madison WI 53719

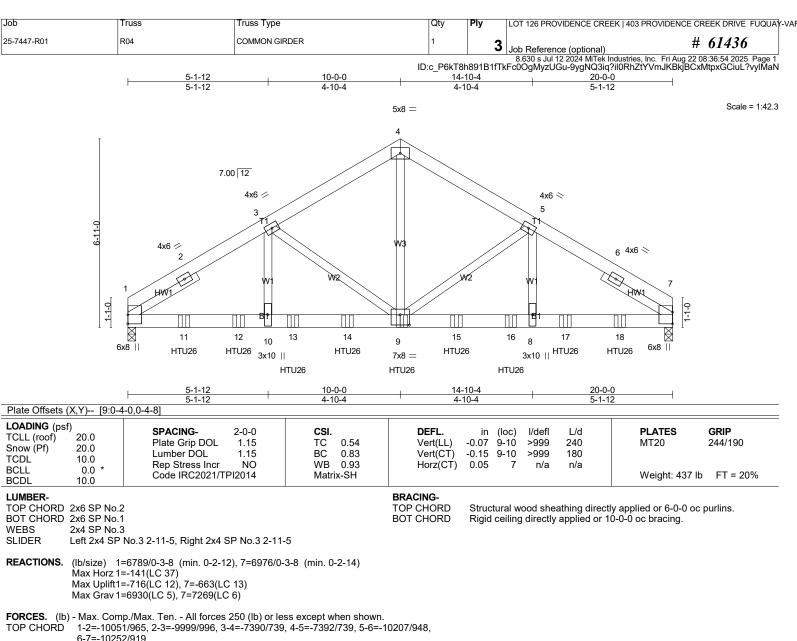
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Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE	CREEK DRIVE FUQUA	۷Y-VA
25-7447-R01	R03	Common	3	1	Job Reference (optional)	# 61436	

8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:36:53 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-hm6?DjiCEOd9qX_h_oF4nzBZ2oa68RQ7_28oSTylMaO

LOAD CASE(S) Standard





BOT CHORD

1-11=-847/8297, 11-12=-847/8297, 10-12=-847/8297, 10-13=-847/8297, 13-14=-847/8297, 9-14=-847/8297, 9-15=-713/8486, 15-16=-713/8486, 8-16=-713/8486, 8-17=-713/8486,

17-18=-713/8486, 7-18=-713/8486

WEBS 4-9=-620/6771, 5-9=-2595/333, 5-8=-244/3427, 3-9=-2406/385, 3-10=-297/3204

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-5-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

6) Unbalanced snow loads have been considered for this design

7) This true has been considered.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=716

9) Provide mechanical connection (by others) of truss to bearing plate capable of whitestraining 77–663.

10) Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the capable truss(es) R10 (1 ply 2x4 SP), R11 (1 ply 2x4 SP) to back face of bottom chord.

11) Fill all nail holes where hanger is in contact with lumber.

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MORRIS TO THE TOTAL TO THE TOTAL TOT Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for all more parameters and proper incorporation of component is responsibility of building designer — not truss designer or truss engineer. Bracing shown is for lateral support vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer — not truss designer or truss engineer. Bracing shown is for lateral support vertically. Additional permanent bracing of the overall structure is the of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 F	PROVIDENCE CREEK DRIVE FUQUAY-VA
25-7447-R01	R04	COMMON GIRDER	1	3	Job Reference (optional)	# 61436

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:54 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-9ygNQ3iq?il0RhZtYVmJKBkjBCxMtpxGCiuL?vylMaN

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 9=-1371(B) 11=-1286(B) 12=-1286(B) 13=-1371(B) 14=-1371(B) 15=-1371(B) 16=-1371(B) 17=-1371(B) 18=-1371(B)



.lob Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R05 GABLE # 61436 lob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:54 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-9ygNQ3iq?il0RhZtYVmJKBkitC0Yt1XGCiuL?vylMaN 0-10-80-10-8 5-0-0 Scale = 1:13.4 4.00 12 W1 0-6-5 3x4 = 2x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.63 BC 0.49 WB 0.00	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 1 n/r 180 Vert(CT) 0.01 1 n/r 80 Horz(CT) 0.00 4 n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2021/TPI2014	Matrix-P	, ,	Weight: 19 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 4=189/5-0-0 (min. 0-1-8), 2=251/5-0-0 (min. 0-1-8) Max Horz 2=70(LC 13) Max Uplift4=-40(LC 14), 2=-63(LC 10)

Max Grav 4=253(LC 21), 2=344(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.

LOAD CASE(S) Standard

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

To perform mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered.

13) Web bracing shown is for lateral.

- 12) Bearing syllinois also only stable.
 structural design of the truss to support the loads indicated.
 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED.
 AMINIMALIM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE CONSIDERATIONS.

8/22/2025

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Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

.lob Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R06 Monopitch 5 # 61436 ob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:55 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-d9EmdPjSm?tt3r836DHYsOGqGcl_cT_QRMduXLylMaM -0-10-8 0-10-8 13-0-0 6-1-12 6-10-4 Scale = 1:28.1 4.00 12 3 5 3x4 =3x4 =2x4 || 3x8 II 5-8-8 6-10-4 Plate Offsets (X,Y)-- [2:0-0-0,0-1-1], [2:0-2-10,Edge] LOADING (psf) SPACING-DEFL. GRIP 2-0-0 CSI. in (loc) I/defl I/d **PLATES** TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.78 Vert(LL) 0.08 2-6 >895 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.74 Vert(CT) -0.095-6 >856 180 **TCDL** 10.0 WB 0.12 Rep Stress Incr YES Horz(CT) -0.00n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Matrix-SH Weight: 62 lb FT = 20%**BCDL** 10.0

LUMBER-

WFBS

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=225/Mechanical, 6=595/0-3-8 (min. 0-1-8), 2=258/0-3-8 (min. 0-1-8)

Max Horz 2=170(LC 11)

Max Uplift5=-53(LC 14), 6=-168(LC 10), 2=-103(LC 10) Max Grav 5=310(LC 21), 6=689(LC 21), 2=258(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-6=-534/180

NOTES-(9-12)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 8-0-10, Exterior(2E) 8-0-10 to 12-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 6=168

 2=103.

 9) Graphical bracing representation does not depict the size, type or the prior to the member must be the memb
- the member must be braced.
- 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the
- 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling,
- Paraphical bracing representation does not depict the SIZE, type 5.
 he member must be braced.

 Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing and process of the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing and the structural design of the truss to support the loads indicated.

 Web Bracing SI IMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

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8/22/2025

LOADINGASE(S) rBiantisted parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R07 Monopitch 2 # 61436 ob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:55 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-d9EmdPjSm?tt3r836DHYsOGqGcNAcl7QRMduXLylMaM -0-10-8 0-10-8 12-10-8 6-1-12 6-8-12 Scale = 1:27.9 4.00 12 3x4 = 12 3 W3 6 2x4 || 3x4 =3x8 II 5-8-8 6-8-12 Plate Offsets (X,Y)-- [2:0-0-0,0-1-1], [2:0-2-10,Edge] GRIP DEFL. in (loc) I/defl I/d **PLATES**

LOADING (ps	f)	SPACING- 2-0-0	CSI.
TCLL (roof)	20.0		
` ,		Plate Grip DOL 1.15	TC 0.78
Snow (Pf)	20.0	Lumber DOL 1.15	BC 0.41
TCDL	10.0		20 0
BCLL	0.0 *	Rep Stress Incr YES	WB 0.81
BCDI	10.0	Code IRC2021/TPI2014	Matrix-AS

BRACING-

TOP CHORD BOT CHORD

Vert(LL)

Vert(CT)

Horz(CT)

0.11

-0.12

0.02

5-6

5-6

5

>999

>999

n/a

240

180

n/a

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

MT20

Weight: 61 lb

244/190

FT = 20%

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS

WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 5=487/Mechanical, 2=584/0-3-8 (min. 0-1-8)

Max Horz 2=170(LC 13)

Max Uplift5=-200(LC 10), 2=-223(LC 10) Max Grav 5=615(LC 21), 2=627(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-963/676, 4-5=-257/71 BOT CHORD 2-6=-580/875, 5-6=-580/875 WEBS 3-6=-267/251, 3-5=-873/663

NOTES-(10-13)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 7-11-2 Exterior(2E) 7-11-2 to 12-8-12 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- Neter to girder(s) for truss to truss connections.

 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=206 2=223.

 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the term.

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8/22/2025

Warning!—Verify design parameters and read notes before use. This design is based only upon parameters snown, and is not an individual outloing component to component of page 2 vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE CREEK DRIV	E FUQUAY-VA
25-7447-R01	R07	Monopitch	2	1	Job Reference (optional) # 6143	6

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:55 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-d9EmdPjSm?tt3r836DHYsOGqGcNAcl7QRMduXLyIMaM

- 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

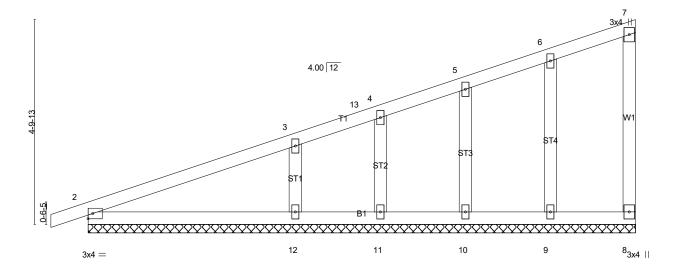


.lob Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R08 GABLE # 61436 lob Reference (optional)

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:56 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-6Lo8rlk4WJ?kh?jGfwonPcp710l7LwuZg0NS3oylMal

-0-10-8 0-10-8 12-10-8

Scale = 1:27.1



TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.26 BC 0.30 WB 0.07	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 1 n/r 180 Vert(CT) 0.01 1 n/r 80 Horz(CT) 0.00 8 n/a n/a	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDI 10.0	Code IRC2021/TPI2014	Matrix-SH	,	Weight: 63 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS

2x4 SP No.3 2x4 SP No 3 **OTHERS**

BRACING-

TOP CHORD BOT CHORD

end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer Installation guide.

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

REACTIONS. All bearings 12-10-8.

(lb) - Max Horz 2=169(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 8, 2, 9, 10, 11, 12

Max Grav All reactions 250 lb or less at joint(s) 8, 2, 9, 10, 11 except 12=397(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-12=-277/245

NOTES-(12-15)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 7-11-2, Corner(3E) 7-11-2 to 12-8-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the two mechanical connection (by others) of truss to bearing plate capable of withstanding 100 "

 12) Graphical bracing representation does not depict the trust the trust the trust that the trust is a fine trust to the trust the trust that the trust is a fine trust to the trust that the trust is a fine trust to the trust the trust trust trust that the trust is a fine trust trus

- 7) Gable required
 8) Gable studs spaced at 2-0-0 oc.
 9) This truss has been designed for a 10.0 psf bottom chord live load ...
 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas which fit between the bottom chord and any other members, with BCDL = 10.0psf.
 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2, 9, 10, 11, 12
 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 12 controlly graphical representations of a possible bearing condition. Bearing symbols are not considered in the member must be braced.
 13 controlly graphical representations of a possible bearing condition. Bearing symbols are not considered in the member must be braced.
 14 controlly graphical representations of a possible bearing condition. Bearing symbols are not considered in the member must be braced.
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MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING

MORRES Tand NOINEE K. MORR

8/22/2025

WATONS IDENCATION of Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded Continued on page vertically. Applies building of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive Madison WI 53719

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE CREEK DRIVE FUC	QUAY-VAI
25-7447-R01	R08	GABLE	1	1	Job Reference (optional) # 61436	

8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:36:56 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-6Lo8rlk4WJ?kh?jGfwonPcp710I7LwuZg0NS3oylMaL

LOAD CASE(S) Standard



Job Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R09 GABLE # 61436 lob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:57 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-aXMW25liHd7bJ9HSDeJ0xpMLyQ8G4LNjvg6?bEylMaK 16-8-0 33-0-8 16-8-0 16-4-8 Scale = 1:59.7 4x6 = 11 6.00 12 12 10 13 42 14 3x8 / 15 3x8 ≤ 16 6 9-4-0 ⁴³ 18 40 19 3x4 || 3x4 || 20 1-1-12 9-0-1 26 39 38 37 36 35 34 33 32 31 30 29 28 27 25 24 23 22 21 3x8 =3x4 || 4x8 = 3x4 II

Plate Offsets (X,Y)-- [27:0-3-8,0-1-4]

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.13 BC 0.11 WB 0.25	Vert(CT) -(in (loc) 0.00 1 0.00 1 0.00 21	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2021/TPI2014	Matrix-R	11012(01)	0.00 21	Π/α	II/a	Weight: 220 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 2x4 SP No 3 WFBS 2x4 SP No 3 OTHERS

BRACING-

WERS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 11-30

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

REACTIONS. All bearings 33-0-8

(lb) - Max Horz 39=126(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 39, 21, 31, 32, 33, 35, 36, 37, 29, 28, 27, 25, 24, 23, 22 except 38=-110(LC 14)

Max Grav All reactions 250 lb or less at joint(s) 39, 21, 33, 35, 36, 37, 38, 27, 25, 24, 23, 22 except

30=279(LC 27), 31=290(LC 5), 32=276(LC 5), 29=290(LC 6), 28=276(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 8-41=-95/253, 9-41=-87/261, 9-10=-114/306, 10-11=-131/343, 11-12=-131/343, TOP CHORD 12-13=-114/306, 13-42=-87/261, 14-42=-95/253

NOTES-(14-17)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 11-10-6, Corner(3R) 11-10-6 to 21-5-10, Exterior(2N) 21-5-10 to 28-1-2, Corner(3E) 28-1-2 to 32-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

MORRIGIANIA

122/2025

d and le

DUL=1.15); Is=1.0; Rough

Low psf or 2.00 times flat roof load of 20.0 psf on overhangs

Low psf or 2.00 times flat roof load of 20.0 psf on overhangs

Low psf or 2.00 times flat roof load of 20.0 psf on overhangs

Low psf or 2.00 times flat roof load of 20.0 psf on overhangs

Low rully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

10) Gable studs spaced at 2-0-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 39, 21, 31, 32, 33, 35, 36, 37, 29, 28, 27, 25, 24, 23, 22 except (jt=lb) 38=110.

Warning — Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual beautiful to findividual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the building designer. For general guidance regarding fabrication, qualification, qualifi Warning!—Verify design parameters and read notes before use. This design is based only upon parameters snown, and is not an individual outloing component to component of page 2 vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE CREEK DRIV	E FUQUAY-VA
25-7447-R01	R09	GABLE	1	1	Job Reference (optional) # 6143	6

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:57 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-aXMW25liHd7bJ9HSDeJ0xpMLyQ8G4LNjvg6?bEylMaK

- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R10 Common 2 # 61436 ob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:58 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-2kwuGRmL2wFSwJsenLqFU1uPVpG4pk4s7JsZ8gylMaJ 27-10-14 -0-10₇8 0-10-8 33-0-8 5-5-2 11-6-9 5-5-2 6-1-7 5-1-7 6-1-7 5-1-10 Scale = 1:61.3 4x6 = 6 19 6.00 12 4x4 🥢 4x4 < 5 18 J2 3x8 / 3x8 < 9-4-0 4x4 / 4x4 < W5 3 W2 3x4 || 3x4 < 10 1-1-12 1-0-0 R1 R1 16 15 20 21 22 13 12 14 4x4 = 3x8 =3x8 =4x4 =4x6 =4x6 =4x8 = 8-4-14 8-3-2 8-3-2 8-1-6 LOADING (psf) SPACING-GRIP CSI. **DEFL** PLATES 2-0-0 (loc) I/defl L/d TCLL (roof) 20.0 Plate Grip DOL 244/190 1.15 TC 0.52 Vert(LL) -0.22 12-14 >999 240 MT20 Snow (Pf) 20.0 Lumber DOL 1.15 BC 0.97 Vert(CT) -0.36 12-14 >999 180 **TCDL** 10.0 Rep Stress Incr YES WB 0.48 Horz(CT) 0.09 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Weight: 195 lb Matrix-SH FT = 20% BCDL 10.0 LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 3-9-9 oc purlins, except end verticals.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WFBS 2x4 SP No.3 *Except*

W1: 2x6 SP No.2

BOT CHORD

WFBS

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt 5-14, 7-14, 3-17, 9-11

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 17=1374/0-3-8 (min. 0-1-10), 11=1306/Mechanical

Max Hórz 17=127(LC 13)

Max Uplift17=-179(LC 14), 11=-156(LC 15)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-376/96, 3-4=-1942/298, 4-5=-1876/315, 5-18=-1467/304, 6-18=-1460/327,

6-19=-1459/327, 7-19=-1467/304, 7-8=-1852/315, 8-9=-1920/299, 2-17=-364/117

BOT CHORD 16-17=-292/1703, 15-16=-175/1546, 15-20=-175/1546, 20-21=-175/1546, 14-21=-175/1546,

14-22=-142/1535, 13-22=-142/1535, 12-13=-142/1535, 11-12=-193/1657

5-16=-28/341, 5-14=-593/205, 6-14=-153/1035, 7-14=-574/203, 7-12=-26/318,

3-17=-1758/250, 9-11=-1844/253

NOTES-(10-13)

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-10-6, Exterior(2R) 11-10-6 to 21-5-10, Interior(1) 21-5-10 to 28-0-2, Exterior(2E) 28-0-2 to 32-10-12 zone; cantilever left and right exposed;
- end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.63) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fil between the bottom chord and any other members, with BCDL = 10.0psf.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 17=139 11=156.

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8/22/2025

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an increase of an i of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE	CREEK DRIVE FUQUAY
25-7447-R01	R10	Common	2	1	Job Reference (optional)	# 61436

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:58 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-2kwuGRmL2wFSwJsenLqFU1uPVpG4pk4s7JsZ8gylMaJ

- 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

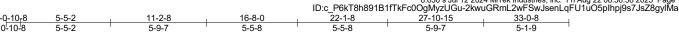
 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

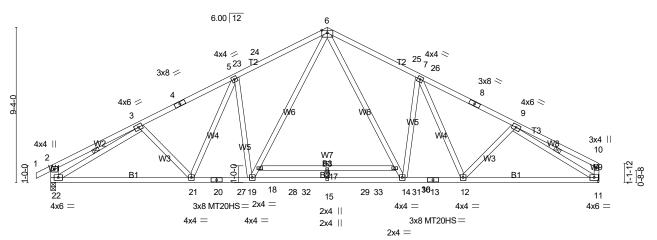




5x8 =



Scale = 1:69.4



	8-5-12 8-5-12	12-0-0 3-6-4 16-8-0 4-8-0	21-4-0 4-8-0	24-10-4 3-6-4	33-0-8 8-2-4	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.61 BC 0.87 WB 0.54 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/de -0.44 17 >89 -0.72 17 >54 0.09 11 n	97 240	PLATES GRIP MT20 244/190 MT20HS 187/143 Weight: 218 lb FT = 20%

LUMBER-TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

B2: 2x4 SP SS WFBS 2x4 SP No 3 *Except*

W1: 2x6 SP No.2

BRACING-TOP CHORD

WFBS

Structural wood sheathing directly applied or 3-3-4 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

6-0-0 oc bracing: 16-18 1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

3-22 9-11

REACTIONS. (lb/size) 22=1458/0-3-8 (min. 0-1-13), 11=1391/Mechanical

Max Horz 22=127(LC 11)

Max Uplift22=-137(LC 14), 11=-113(LC 15)

Max Grav 22=1536(LC 3), 11=1478(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-418/90, 3-4=-2270/224, 4-5=-2209/239, 5-23=-2230/285, 23-24=-2222/288,

6-24=-2155/310, 6-25=-2135/310, 25-26=-2199/287, 7-26=-2207/285, 7-8=-2168/241,

8-9=-2230/226, 9-10=-277/54, 2-22=-389/116

21-22=-229/1970, 20-21=-97/1968, 20-27=-97/1968, 19-27=-97/1968, 19-28=0/1476, **BOT CHORD**

15-28=0/1476, 15-29=0/1476, 29-30=0/1476, 14-30=0/1476, 14-31=-61/1946,

13-31=-61/1946, 12-13=-61/1946, 11-12=-132/1906

WEBS 5-19=-539/239, 18-19=-174/933, 6-18=-136/1033, 6-16=-134/993, 14-16=-171/893,

7-14=-517/237, 3-22=-1989/185, 9-11=-2077/189

NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-10-6, end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 a) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 a) Unbalanced snow loads have been considered for this design

Exterior(2R) 11-10-6 to 21-5-10, Interior(7, 2) and vertical left and right exposed; C-C for members and forces & MWFRS for reactions 5.10...., 2...

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rougu Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Unbalanced snow loads have been considered for this design.

This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

All plates are MT20 plates unless otherwise indicated.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit hetween the bottom chord and any other members, with BCDL = 10.0psf.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

6) All plates are MT20 plates unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) Refer to girder(s) for truss to truss connections

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 22=137 , 11=113.

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MORRES THE STATE OF THE STATE O Warning!—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE CREEK DRI	VE FUQUAY-VA
25-7447-R01	R11	Common	7	1	Job Reference (optional) # 6143	<i>36</i>

8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:36:59 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-WwTGTnmzpENJYSRqL2MU1ERZrDewYAP?Mzb6g7ylMal

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

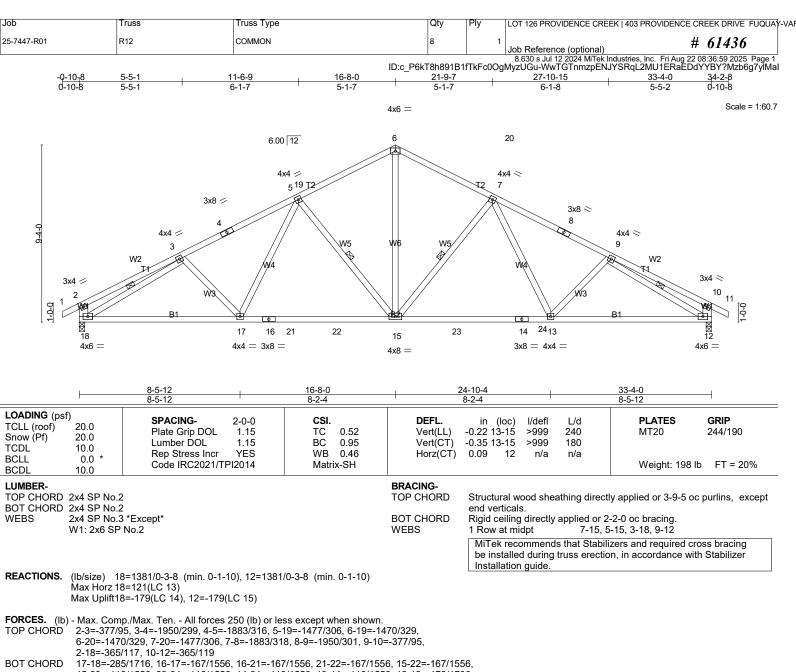
13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





15-23=-118/1556, 23-24=-118/1556, 14-24=-118/1556, 13-14=-118/1556, 12-13=-178/1706

6-15=-155/1046, 7-15=-594/205, 7-13=-27/340, 5-15=-594/205, 5-17=-27/340,

3-18=-1763/253, 9-12=-1763/233

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-10-6, Exterior(2R) 11-10-6 to 21-5-10, interior(1) 21-5-10 to 29-4-14, Exterior(2E) 29-4-14 to 34-2-8 zone; cantilever left and right exposed; COPESS TO THE CAROLING end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=159 12=179.

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Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an increase of an i of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 PROVIDENCE CREE	EK DRIVE FUQUAY-VA
25-7447-R01	R12	COMMON	8	1	Job Reference (optional) # 6	61436

8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:36:59 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-WwTGTnmzpENJYSRqL2MU1ERaEDdYYBY?Mzb6g7ylMal

- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job Truss Truss Type Qty LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF 25-7447-R01 R13 Common Supported Gable # 61436 lob Reference (optional) 8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:37:00 2025 Page 1 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-_61fh7nbaYV9Ac01umtjZS_sJdA?Hi79bdLfCZylMaH 33-4-0 16-8-0 16-8-0 0-10-8 Scale = 1:59.5 4x6 = 11 6.00 12 12 10 13 43 12 3x8 / 15 3x8 ≥ 16 6 17 18 44 19 3x4 || 3x4 | ²⁰ 21 1-0-0 40 39 38 37 36 35 34 33 32 31 30 29 28 27 26 25 24 23 22 3x8 = 3x8 =3x4 || 3x4 || 33-4-0LOADING (psf) SPACING-GRIP CSI. DEFL. PLATES 2-0-0 I/defl L/d (loc) TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.12 Vert(LL) -0.00 20 n/r 180 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 BC 0.11 Vert(CT) -0.00 21 n/r 80 **TCDL** 10.0 Rep Stress Incr YES WB 0.25 Horz(CT) 0.00 22 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Weight: 223 lb Matrix-R FT = 20% BCDL 10.0 BRACING-LUMBER-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3 2x4 SP No.3 OTHERS

BOT CHORD WFBS

end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 11-31

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 33-4-0.

(lb) - Max Horz 40=120(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 40, 22, 32, 33, 34, 36, 37, 38, 30, 29, 28, 26, 25, 24, 23 except

Max Grav All reactions 250 lb or less at joint(s) 40, 22, 34, 36, 37, 38, 39, 28, 26, 25, 24, 23 except 31=278(LC 27), 32=290(LC 5), 33=276(LC 5), 30=290(LC 6), 29=276(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 9-42=-87/256, 9-10=-114/301, 10-11=-131/339, 11-12=-131/339, 12-13=-114/301, 13-43=-87/256

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 11-10-6, Corner(3R) 11-10-6 to 21-5-10, Exterior(2N) 21-5-10 to 29-4-14, Corner(3E) 29-4-14 to 34-2-8 zone; cantilever left and right exposed end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1

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Warning !— Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual "

Warning!— Verify design parameters and proper incorporation of component is responsibility of the building designer. For general guidance regarding fabrication.

Warning !— Verify design parameters and proper incorporation of component is responsibility of be building designer. For general guidance regarding fabrication.

Plate Connected Wood Truss Construction and BCSI 1-03 General guidance regarding fabrication. Warning!—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is not all the restaurable of page 2. Online on page 2. Online of page 3. Online of page 4. Onli of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583

Job	Truss	Truss Type	Qty	Ply	LOT 126 PROVIDENCE CREEK 403 P	ROVIDENCE CREEK DRIVE FUQUAY-V	/AF
25-7447-R01	R13	Common Supported Gable	1	1	Job Reference (optional)	# 61436	

8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:37:00 2025 Page 2 ID:c_P6kT8h891B1fTkFc0OgMyzUGu-_61fh7nbaYV9Ac01umtjZS_sJdA?Hi79bdLfcZylMaH

- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the
- 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

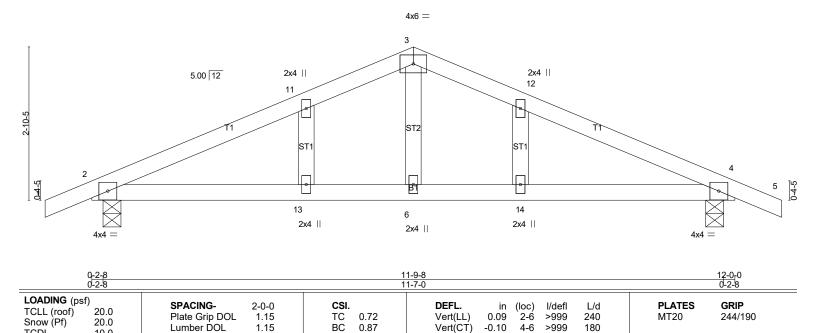
 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF Job Truss Truss Type Qty 25-7447-R01 SP01 COMMON SUPPORTED GAB # 61436 lob Reference (optional) 8.630 s Jul 12 2024 MTek Industries, Inc. Fri Aug 22 08:37:01 2025 Page 1 ID:I8799bTYYbxotYa75q2Yk9yieyH-SJb1uToDLre0nmbDSTOy6fWsk1JF0BclpH4Dk?ylMaG 12-0-0 12-10-8 0-10-8 6-0-0 6-0-0 0-10-8

Scale = 1:21.5



LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

10.0

10.0

0.0

BRACING-

Horz(CT)

0.01

n/a

n/a

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-1-5 oc purlins. Rigid ceiling directly applied or 6-6-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Weight: 48 lb

FT = 20%

REACTIONS. (lb/size) 2=529/0-4-0 (min. 0-1-8), 4=529/0-4-0 (min. 0-1-8)

Rep Stress Incr

Code IRC2021/TPI2014

YES

Max Horz 2=43(LC 18)

Max Uplift2=-148(LC 10), 4=-148(LC 11) Max Grav 2=623(LC 21), 4=623(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-11=-786/862, 3-11=-650/874, 3-12=-650/874, 4-12=-786/862 TOP CHORD 2-13=-704/619, 6-13=-704/619, 6-14=-704/619, 4-14=-704/619 **BOT CHORD**

WEBS 3-6=-418/278

NOTES-(11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Corner(3R) 3-11-2 to 8-0-14, Corner(3E) 8-0-14 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB 0.11

Matrix-SH

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

- o) Tries truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will at between the bottom chord and any other members.

 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 is 100 i

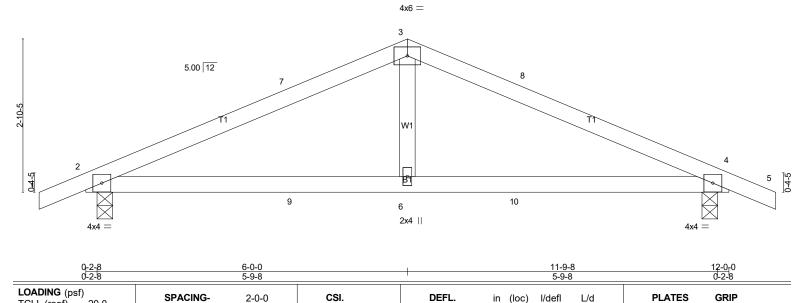
LOAD CASE(S) Standard

8/22/2025

VOINEE

MORRIS HAMINATOR OF THE PARTY O Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

LOT 126 PROVIDENCE CREEK | 403 PROVIDENCE CREEK DRIVE FUQUAY-VAF Job Truss Truss Type Qty 25-7447-R01 SP02 COMMON 5 # 61436 lob Reference (optional) 8.630 s Jul 12 2024 MiTek Industries, Inc. Fri Aug 22 08:37:01 2025 Page 1 ID:l8799bTYYbxotYa75q2Yk9yieyH-SJb1uToDLre0nmbDSTOy6fWsd1Kv0BclpH4Dk?ylMaG 12-0-0 12-10-8 0-10-8 6-0-0 6-0-0 0-10-8 Scale = 1:21.5



LUMBER-

TCLL (roof)

Snow (Pf)

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WFBS 2x4 SP No.3

20.0

20.0

10.0

10.0

0.0

BRACING-

Vert(LL)

Vert(CT)

Horz(CT)

0.09

-0.10

0.01

4-6

4-6

>999

>999

n/a

240

180

n/a

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins. Rigid ceiling directly applied or 6-8-9 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

MT20

Weight: 44 lb

244/190

FT = 20%

REACTIONS. (lb/size) 2=530/0-3-8 (min. 0-1-8), 4=530/0-3-8 (min. 0-1-8)

Plate Grip DOL

Rep Stress Incr

Code IRC2021/TPI2014

Lumber DOL

1.15

1.15

YES

Max Horz 2=43(LC 14)

Max Uplift2=-148(LC 10), 4=-148(LC 11) Max Grav 2=624(LC 21), 4=624(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-7=-791/824, 3-7=-655/836, 3-8=-655/836, 4-8=-791/824 **BOT CHORD** 2-9=-681/625, 6-9=-681/625, 6-10=-681/625, 4-10=-681/625

WEBS 3-6=-407/280

NOTES-(9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Exterior(2R) 3-11-2 to 8-0-14, Exterior(2E) 8-0-14 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TC

BC

WB 0.11

Matrix-SH

0.72

0.77

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

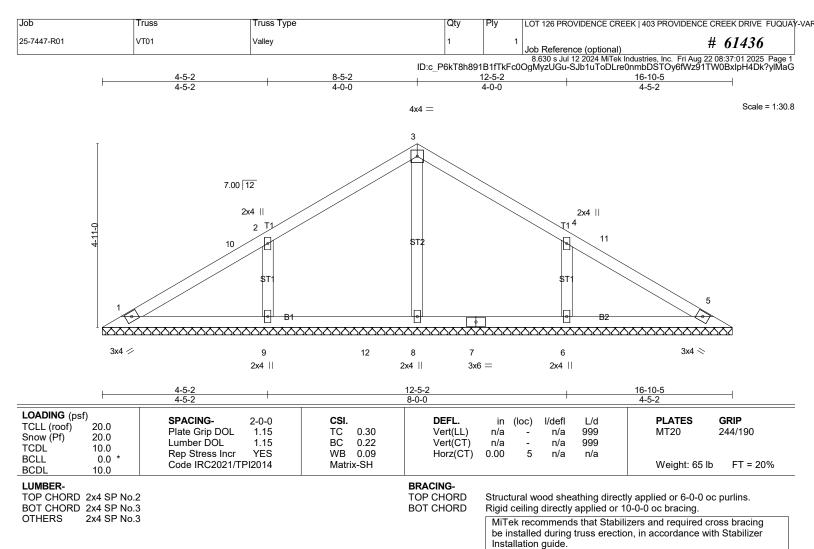
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=148. 4=148.

LOAD CASE(S) Standard

SEAL 28147 MORRIS INTERIOR OF THE PARTY OF

8/22/2025

Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



All bearings 16-10-5. REACTIONS.

(lb) - Max Horz 1=101(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-127(LC 14), 6=-127(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=358(LC 23), 9=486(LC 20), 6=486(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-9=-389/160, 4-6=-389/160

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 11-6-3, Exterior(2E) 11-6-3 to 16-3-13 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

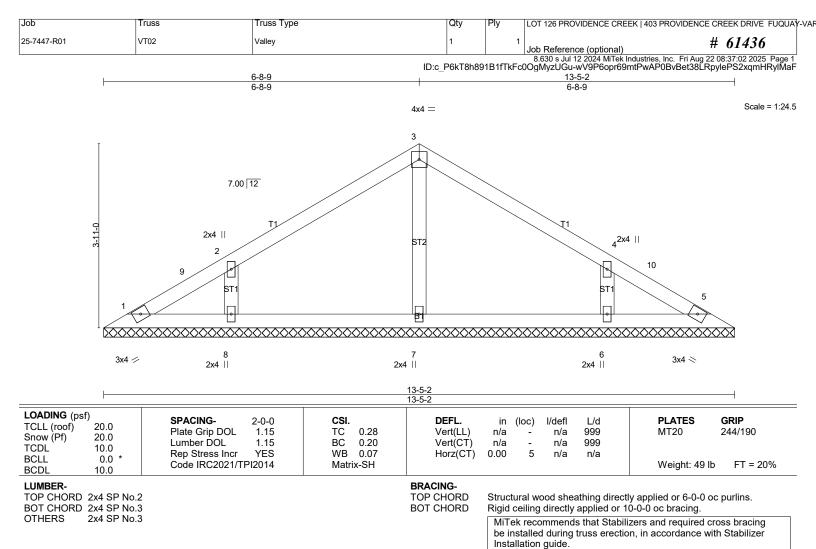
 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines in additional bracing guidelines. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb)

- 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

NONE K. MORR 8/22/2025

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All bearings 13-5-2. REACTIONS.

(lb) - Max Horz 1=-79(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-105(LC 14), 6=-105(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=292(LC 20), 8=422(LC 20), 6=422(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-361/140, 4-6=-361/140

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 8-1-1, Exterior(2E) 8-1-1 to 12-10-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- b=105.

 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines in studies. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=105,

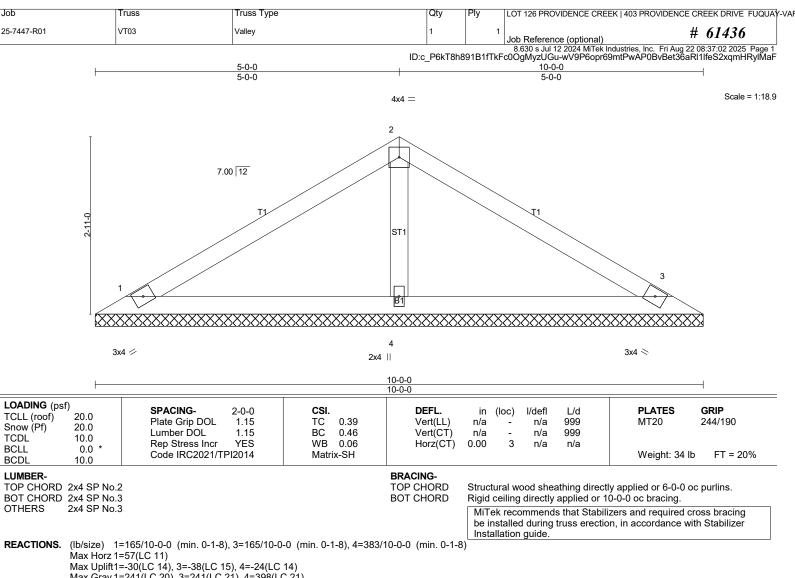
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LOAD CASE(S) Standard

NONE K. MORR 8/22/2025

72/2025

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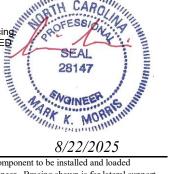
Max Grav 1=241(LC 20), 3=241(LC 21), 4=398(LC 21)

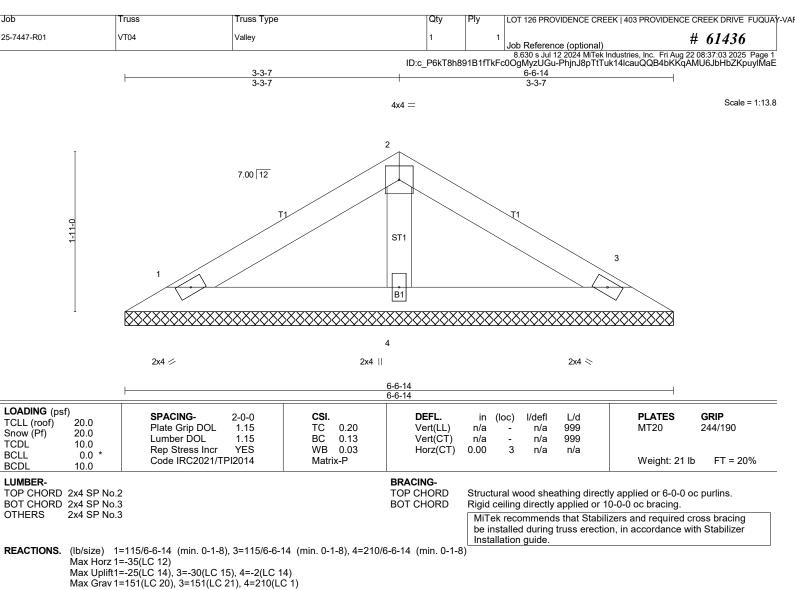
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed: Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced
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LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed: Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
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- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced
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LOAD CASE(S) Standard

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