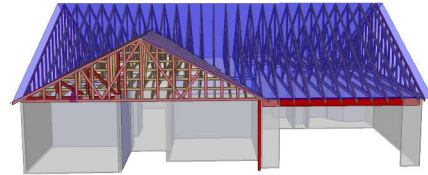




Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: Lamco Custom Builders LLC

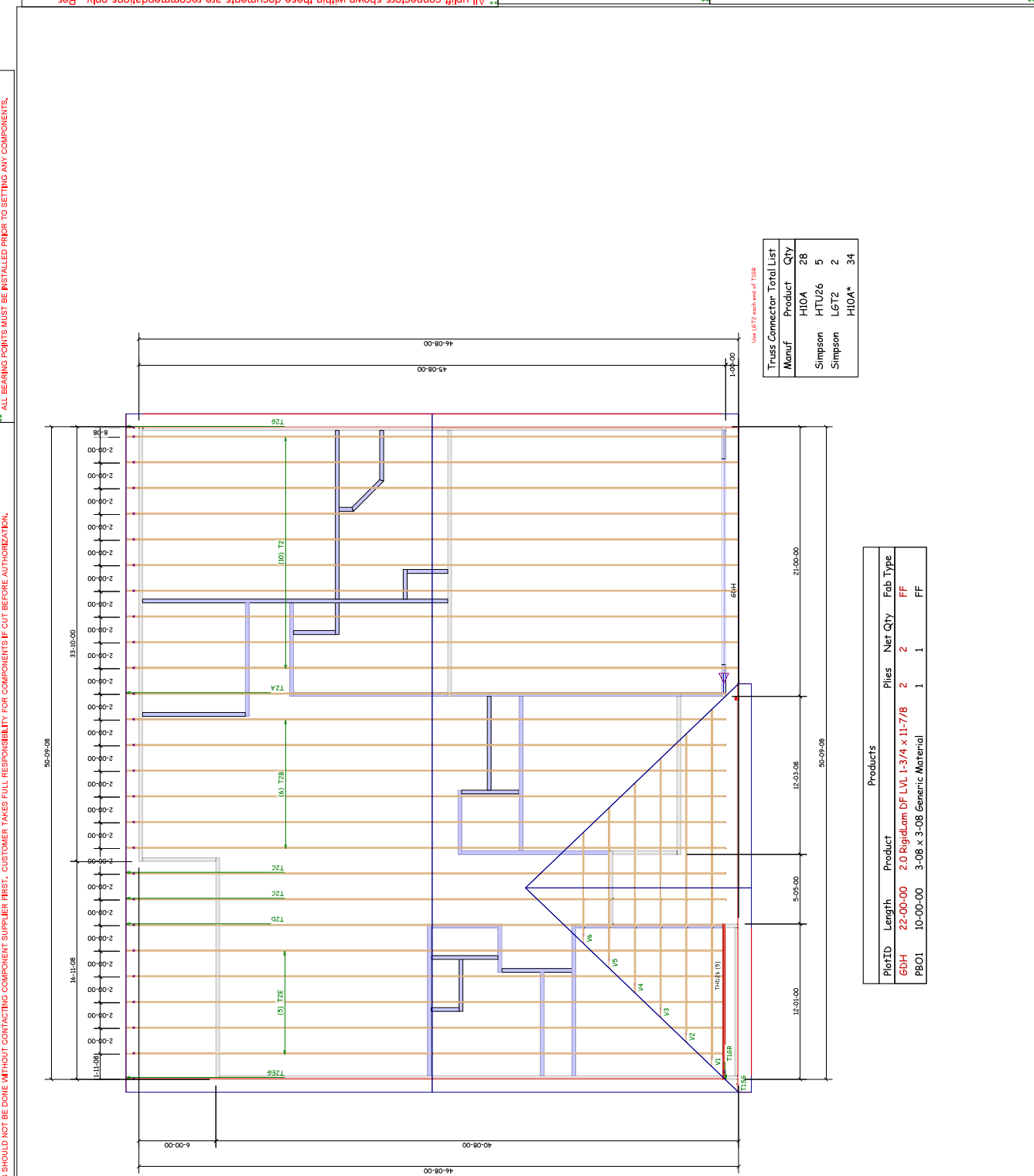
**Model: Yarborough A RH - 77
Blackberry**

THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

Date: _____





Customer:
Job Name:
City:
Customer Ph...

Job Name: **01**
Level: **0**
Label: **GDH - i47**
Type: **Beam**

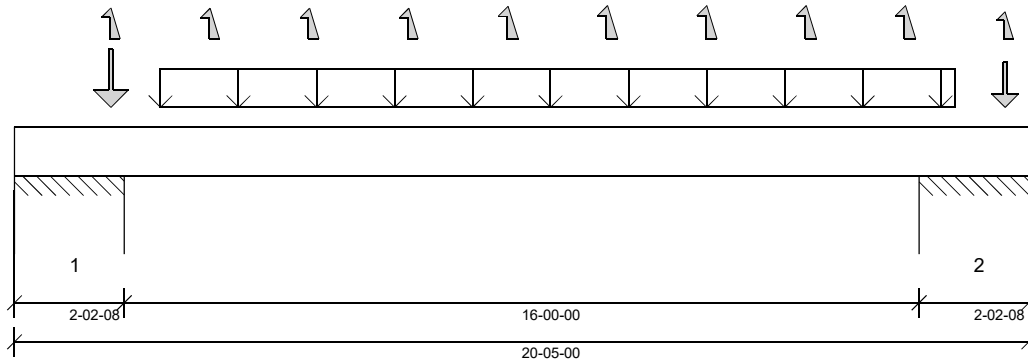
2 Ply Member
2.0 RigidLam DF LVL 1-3/4
x 11-7/8

Status:
Design
Passed

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version
8.7.3.303.Update13.26

Report Version: 2023.09.18 09/22/2025 08:21



DESIGN INFORMATION a

Building Code: IRC 2012
Design Methodology: ASD
Risk Category: II (General Construction)
Residential
Service Condition: Dry
System Spacing: -
LL Deflection Limit: L/480, 0.75" (absolute)
TL Deflection Limit: L/360, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 1'- 10 1/2" Bottom: 20'- 5"

Bearing Stress of Support Material:

- 875 psi Wall @ 0'- 1 1/2"
- 875 psi Wall @ 2'- 1"
- 875 psi Wall @ 18'- 4"
- 875 psi Wall @ 20'- 3 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	9'- 11 1/4"	D + Lr	1.15	4143 lb ft	24296 lb ft	Passed - 17%
Max Neg. Moment:	18'- 4"	D + Lr	1.15	6544 lb ft	17642 lb ft	Passed - 37%
Max Shear:	17'- 2 5/8"	D + Lr	1.15	2333 lb	9241 lb	Passed - 25%
Live Load (LL) Pos. Defl.:	10'- 2 7/16"	0.75(L + Lr + 0.6W)		0.082"	L/480	Passed - L/999
Total Load (TL) Pos. Defl.:	10'- 2 7/16"	D + 0.75(L + Lr + 0.6W)		0.135"	L/360	Passed - L/999

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	9-00	0.6D + 0.6W	1.60	463 lb		32870 lb	27563 lb	Passed - 2%
1	9-00	D + Lr	1.15		-3281 lb	-	-	
1	1-05-08	D + Lr	1.15	6253 lb		45938 lb	53594 lb	Passed - 14%
1	1-05-08	0.6D + 0.6W	1.60		-885 lb	-	-	
2	1-05-12	D + 0.75(L + Lr + 0.6W)	1.60	6311 lb		46594 lb	54359 lb	Passed - 14%
2	1-05-12	0.6D + 0.6W	1.60		-1083 lb	-	-	
2	8-12	0.6D + 0.6W	1.60	502 lb		31957 lb	26797 lb	Passed - 2%
2	8-12	D + Lr	1.15		-3174 lb	-	-	

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	20'- 5"	Self Weight	Top	11 lb/ft	-	-	-	-
Uniform	2'- 11 1/4"	18'- 11 1/4"	Smoothed Load	Top	116 lb/ft	-	112 lb/ft	194 lb/ft	77 lb/ft
Point	1'- 11 1/4"	1'- 11 1/4"	T2(c07)	Top	237 lb	-	230 lb	397/-43 lb	156/-343 lb
Point	3'- 11 1/4"	3'- 11 1/4"	T2(c04)	Top	-	-	-	-53 lb	-324 lb
Point	5'- 11 1/4"	5'- 11 1/4"	T2(c01)	Top	-	-	-	-53 lb	-324 lb
Point	7'- 11 1/4"	7'- 11 1/4"	T2(c06)	Top	-	-	-	-53 lb	-324 lb
Point	9'- 11 1/4"	9'- 11 1/4"	T2(c09)	Top	-	-	-	-53 lb	-371 lb
Point	11'- 11 1/4"	11'- 11 1/4"	T2(c03)	Top	-	-	-	-53 lb	-388 lb
Point	13'- 11 1/4"	13'- 11 1/4"	T2(c05)	Top	-	-	-	-53 lb	-388 lb
Point	15'- 11 1/4"	15'- 11 1/4"	T2(c08)	Top	-	-	-	-53 lb	-388 lb
Point	17'- 11 1/4"	17'- 11 1/4"	T2(c10)	Top	-	-	-	-53 lb	-388 lb
Point	19'- 11 1/4"	19'- 11 1/4"	T2(c02)	Top	186 lb	-	151 lb	262/-36 lb	103/-262 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	2'- 2 1/2"	W43(i45)	969 lb	-	840 lb	1958/-814 lb	1091 lb/-1899 lb
==>	0'- 1 1/2"	0'- 1 1/2"	W43(i45)	-	-	-	502 lb	-
==>	2'- 1"	2'- 1"	W43(i45)	969 lb	-	840 lb	1456/-814 lb	-
2	18'- 2 1/2"	20'- 5"	W5(i9)	3286/-1743 lb	-	2874/-1549 lb	5233/-3414 lb	1091 lb/-1899 lb
==>	18'- 4"	18'- 4"	W5(i9)	3286 lb	-	2874 lb	4987/-514 lb	-
==>	20'- 3 1/2"	20'- 3 1/2"	W5(i9)	-1743 lb	-	-1549 lb	246/-2900 lb	-

DESIGN NOTES

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Customer:	Job Name: 01	2 Ply Member	Status:
Job Name:	Level: 0	2.0 RigidLam DF LVL 1-3/4	Design
City:	Label: GDH - i47	x 11-7/8	Passed
Customer Ph...	Type: Beam		

DESIGN NOTES

- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 0.99

PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

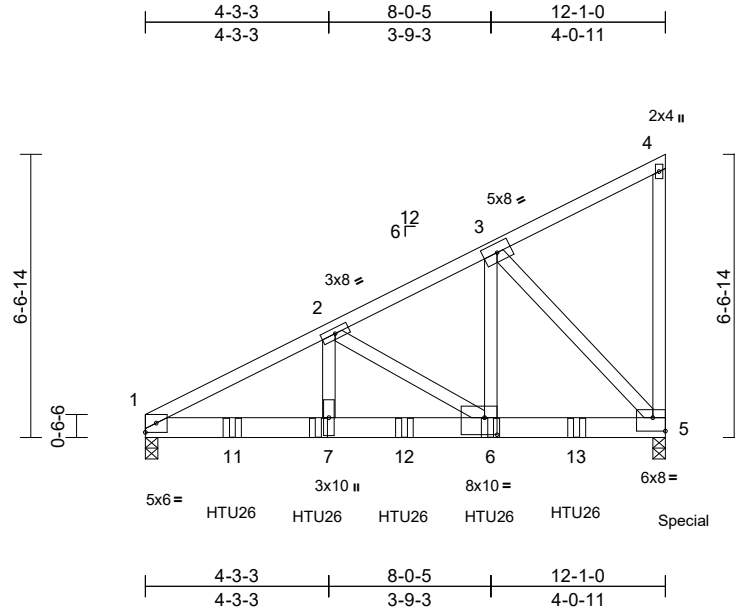
Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T1GR	Monopitch Girder	1	2	176482293
					Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 9.06 E 8.73 Dec 5 2024 Print: 8.730 E Dec 5 2024 MiTek Industries, Inc. Fri Sep 19 11:11:22

Page: 1

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Scale = 1:53.5

Plate Offsets (X, Y): [1:Edge,0-2-9], [5:Edge,0-3-12], [6:0-3-8,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	0.06	6-7	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.45	Vert(TL)	-0.14	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.81	Horiz(TL)	0.03	5	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 159 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 3-6:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=4366/0-3-8, 5=4492/0-3-8
Max Horiz 1=323 (LC 10)
Max Uplift 1=-1224 (LC 11), 5=-1343 (LC 11)
Max Grav 1=4382 (LC 17), 5=4573 (LC 17)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-7235/2016, 2-3=-4011/1128
BOT CHORD 1-11=-1889/6431, 7-11=-1889/6431, 7-12=-1889/6431, 6-12=-1889/6431, 6-13=-1036/3563, 5-13=-1036/3563
WEBS 2-7=-795/2975, 2-6=-3362/1041, 3-6=-1461/5212, 3-5=-5170/1564

NOTES

- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1343 lb uplift at joint 5.
- LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2012 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-4 from the left end to 10-0-4 to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.
- LGT2 Hurricane ties must have two studs in line below the truss.
- Special hanger(s) or other connection device(s) shall be provided at 11-11-4 from the left end sufficient to connect truss(es) to back face of bottom chord. The design/selection of such special connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-62, 5-8=-20

Concentrated Loads (lb)

Vert: 7=-1577 (B), 6=-1577 (B), 11=-1577 (B), 12=-1577 (B), 13=-1577 (B)



September 19,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

ENGINEERING BY
TRENCO
A MITEK COMPANY

818 Soundside Road
Edenton, NC 27932

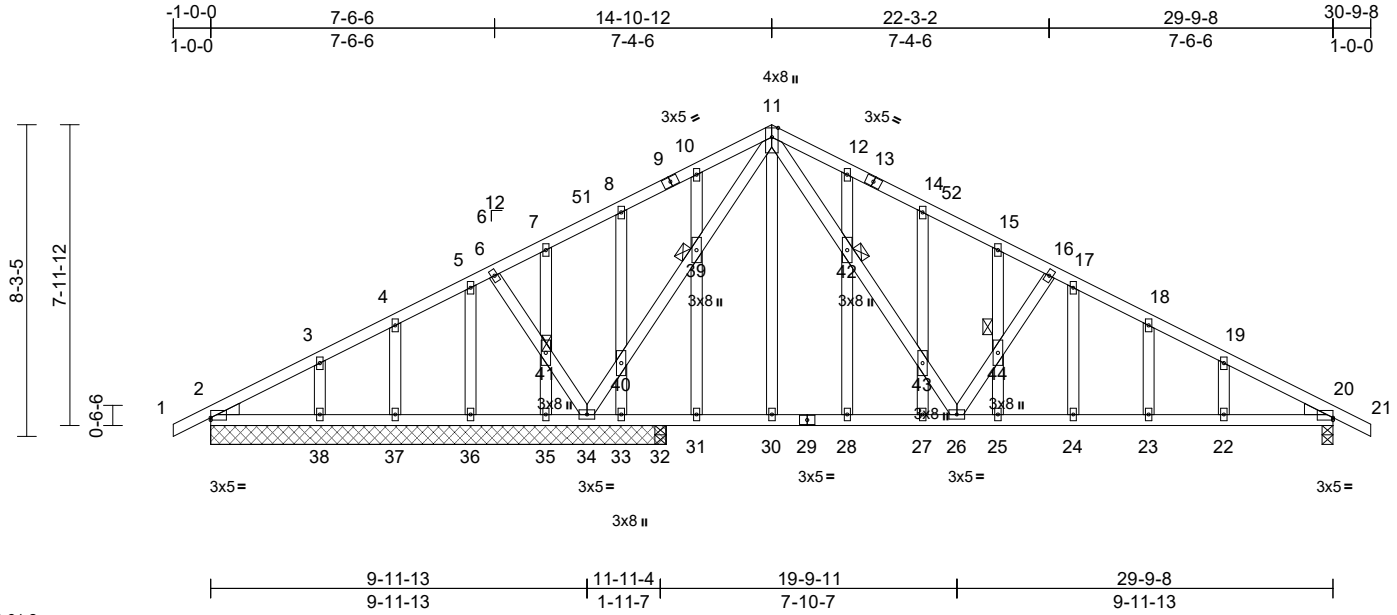
Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T1SG	Common Structural Gable	1	1	176482294
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:21

Page: 1

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Scale = 1:61.2

Plate Offsets (X, Y): [2:Edge,0-0-12], [20:Edge,0-0-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	0.16	22-23	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.63	Vert(TL)	-0.24	22-23	>903	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.55	Horiz(TL)	0.01	20	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 219 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3 Right: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-11-2 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 2-38,37-38,36-37,35-36,34-35.
JOINTS	1 Brace at Jt(s): 39, 41, 42, 44

REACTIONS

(size)	2=12-1-0, 20=0-3-8, 32=0-3-8, 33=12-1-0, 34=12-1-0, 35=12-1-0, 36=12-1-0, 37=12-1-0, 38=12-1-0
Max Horiz	2=178 (LC 19)
Max Uplift	2=-34 (LC 11), 20=-279 (LC 16), 32=-62 (LC 15), 33=-83 (LC 15), 34=-161 (LC 16), 35=-45 (LC 15), 36=-17 (LC 15), 37=-72 (LC 15), 38=-134 (LC 15)
Max Grav	2=157 (LC 33), 20=855 (LC 23), 32=291 (LC 22), 33=57 (LC 22), 34=884 (LC 23), 35=105 (LC 33), 36=105 (LC 33), 37=149 (LC 1), 38=231 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/46, 2-3=-148/131, 3-4=-69/124,
4-5=-16/139, 5-6=0/138, 6-7=0/178,
7-8=0/202, 8-10=-10/206, 10-11=-22/256,
11-12=-839/592, 12-14=-888/551,
14-15=-886/491, 15-16=-913/462,
16-17=-969/484, 17-18=-1112/517,
18-19=-1128/464, 19-20=-1187/415,
20-21=0/46

BOT CHORD

2-38=-120/180, 37-38=-83/180,
36-37=-83/180, 35-36=-83/180,
34-35=-83/180, 33-34=0/329, 32-33=0/329,
31-32=0/329, 30-31=0/329, 28-30=0/332,
27-28=0/332, 26-27=0/332, 25-26=-279/995,
24-25=-279/995, 23-24=-279/995,
22-23=-279/995, 20-22=-279/995
11-42=-400/774, 42-43=-399/761,
26-43=-406/782, 26-44=-458/292,
16-44=-475/303, 34-40=-888/185,
39-40=-832/177, 11-39=-889/186,
6-41=-98/102, 34-41=-108/112,
11-30=-55/186, 10-39=-155/96,
31-39=-181/106, 8-40=-208/138,
33-40=-188/131, 7-41=-85/82, 35-41=-78/72,
5-36=-61/40, 4-37=-120/120, 3-38=-163/156,
12-42=-75/66, 28-42=-59/66, 14-43=-162/111,
27-43=-188/119, 15-44=-28/41,
25-44=-55/53, 17-24=-80/199, 18-23=-77/91,
19-22=-49/85

WEBS

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.



September 19,2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T1SG	Common Structural Gable	1	1	I76482294
					Job Reference (optional)

- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 .
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 2 and 34 lb uplift at joint 2.
- 12) N/A

13) N/A

14) N/A

LOAD CASE(S) Standard

 **WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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818 Soundside Road
Edenton, NC 27932

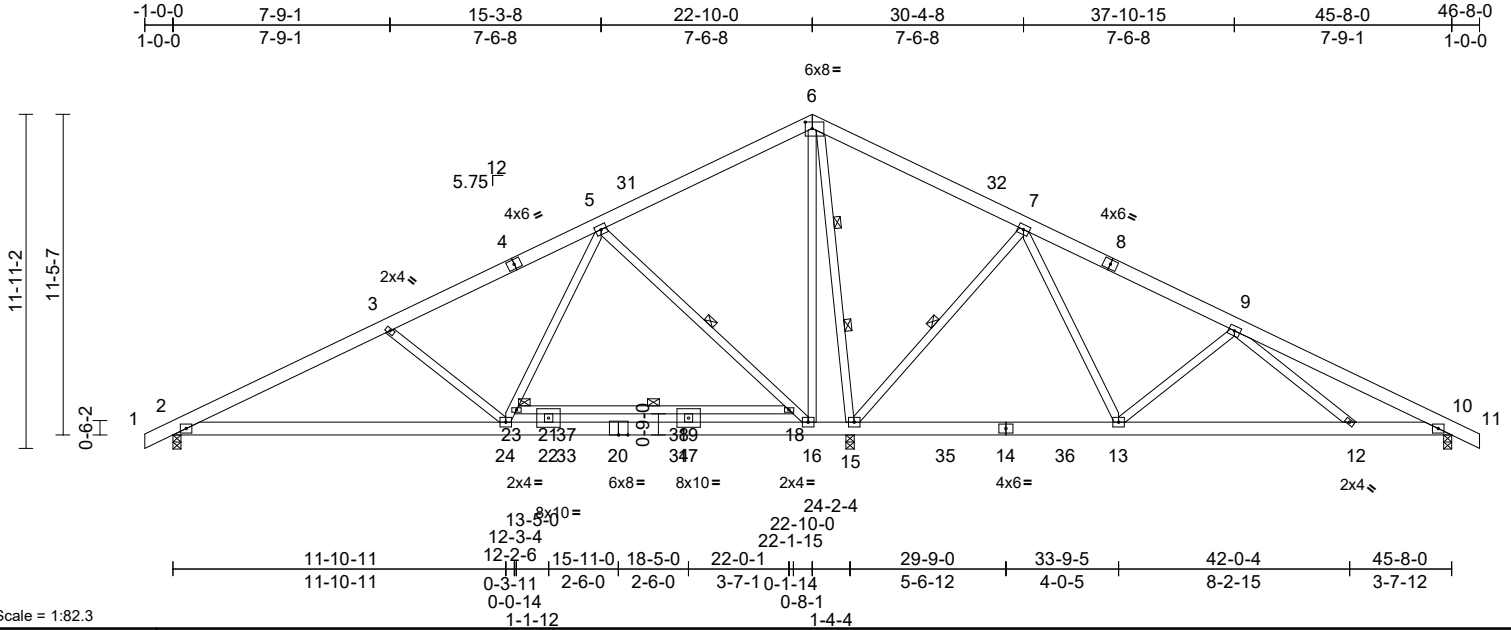
Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2	Common	10	1	176482295
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:21

Page: 1

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Scale = 1:82.3

Plate Offsets (X, Y): [6:0-3-0,0-2-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.54	Vert(LL)	-0.21	19-21	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.80	Vert(TL)	-0.61	19-21	>473	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.81	Horiz(TL)	0.03	10	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 353 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2 *Except* 23-18:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 16-6,16-5,15-6:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-16, 7-15
WEBS 2 Rows at 1/3 pts 6-15

REACTIONS

(size) 2=0-3-8, 10=0-3-8, 15=0-3-8
Max Horiz 2=257 (LC 15)
Max Uplift 2=-177 (LC 15), 10=-276 (LC 16), 15=-288 (LC 15)
Max Grav 2=988 (LC 22), 10=688 (LC 23), 15=2746 (LC 4)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1482/397, 3-5=-1146/96, 5-6=-32/557, 6-7=-43/825, 7-9=-381/287, 9-10=-1032/376, 10-11=0/45
BOT CHORD 2-24=-560/1573, 22-24=0/627, 17-22=0/627, 16-17=0/627, 15-16=-424/572, 13-15=-247/237, 12-13=-205/629, 10-12=-430/920, 21-23=-48/0, 19-21=-48/0, 18-19=-48/0
WEBS 23-24=0/991, 5-23=0/999, 6-16=0/1229, 3-24=-460/439, 5-18=-1197/428, 16-18=-1257/381, 7-15=-947/552, 7-13=-172/578, 9-13=-521/420, 9-12=0/302, 6-15=-2022/156, 21-22=-189/0, 17-19=-122/0

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 6) 200.0lb AC unit load placed on the bottom chord, 15-11-0 from left end, supported at two points, 5-0-0 apart.
- 7) All plates are 4x5 MT20 unless otherwise indicated.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 10, 177 lb uplift at joint 2 and 288 lb uplift at joint 15.

LOAD CASE(S) Standard



September 19,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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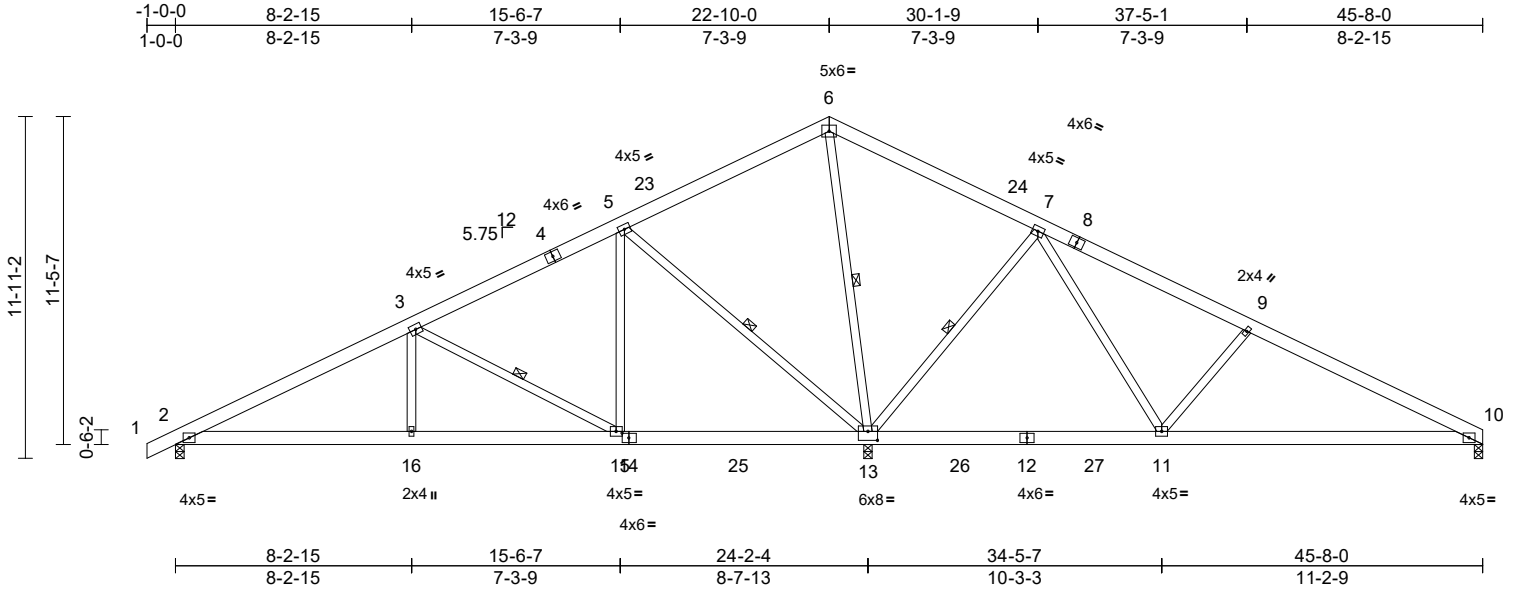
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2A	Common	1	1	176482296
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:21
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Page: 1



Scale = 1:80.5

Plate Offsets (X, Y): [13:0-4-0,0-3-12], [14:0-2-13,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.51	Vert(LL)	-0.10	11-13	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.38	Vert(TL)	-0.17	11-22	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.62	Horiz(TL)	0.02	13	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 321 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except* 13-5,13-6:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS 1 Row at midpt 3-15, 5-13, 6-13, 7-13

REACTIONS

(size) 2=0-3-8, 10=0-3-8, 13=0-3-8
Max Horiz 2=268 (LC 15)
Max Uplift 2=-256 (LC 15), 10=-226 (LC 16), 13=-561 (LC 15)
Max Grav 2=849 (LC 22), 10=642 (LC 23), 13=2466 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1341/527, 3-5=-472/200, 5-6=-68/706, 6-7=-112/845, 7-9=-486/287, 9-10=-1239/401
BOT CHORD 2-16=-718/1465, 15-16=-386/951, 13-15=-74/351, 11-13=-283/274, 10-11=-499/1404
WEBS 3-16=0/175, 3-15=-719/420, 5-15=-101/531, 5-13=-1061/543, 6-13=-1009/361, 7-13=-948/559, 7-11=-237/702, 9-11=-458/413

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 .
- 8) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 13, and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



September 19,2025

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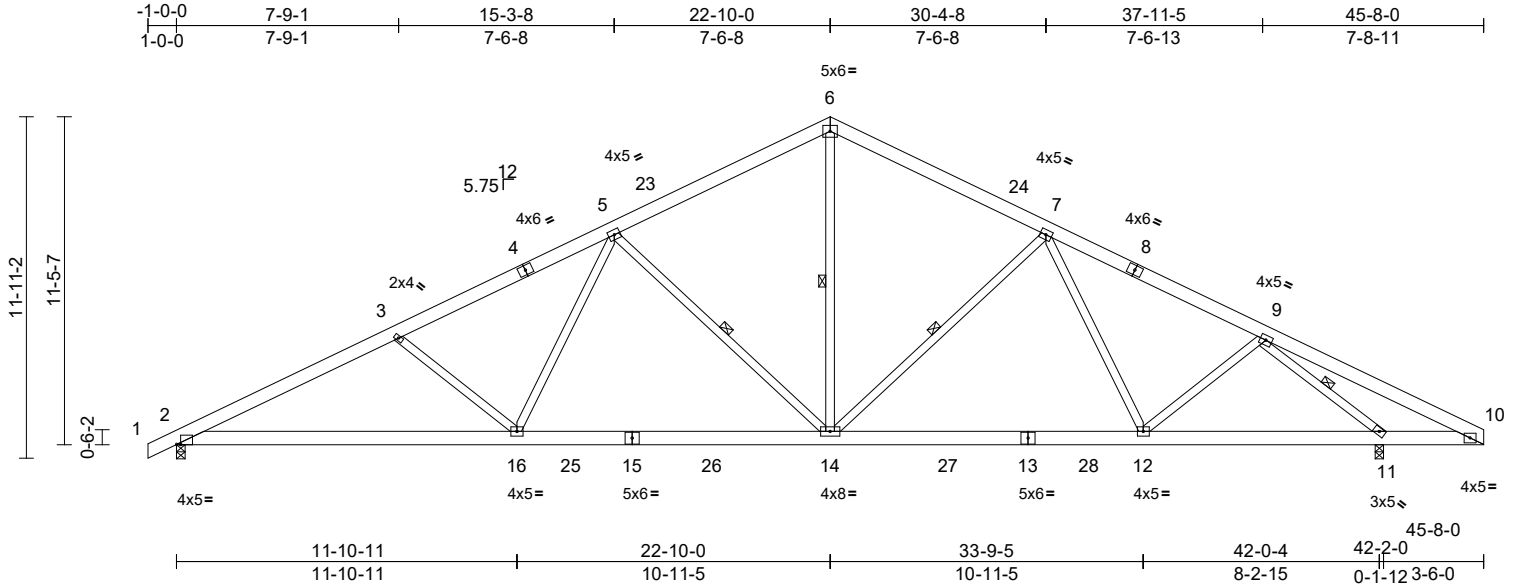
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2B	Common	6	1	176482297
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.20	14-16	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.76	Vert(TL)	-0.43	14-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.62	Horiz(TL)	0.12	11	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 321 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except* 14-5,14-6,14-7:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 9-11, 5-14, 6-14, 7-14

REACTIONS

(size) 2=0-3-8, 11=0-3-8
Max Horiz 2=268 (LC 15)
Max Uplift 2=-496 (LC 15), 11=-524 (LC 16)
Max Grav 2=1783 (LC 1), 11=2004 (LC 1)

FORCES

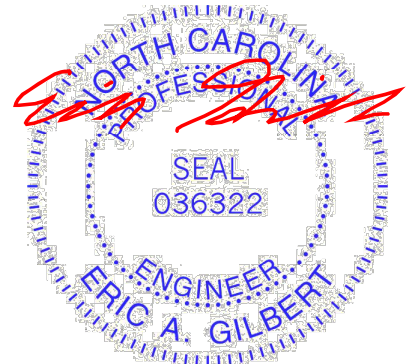
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-3217/1380, 3-5=-2858/1250, 5-6=-1927/985, 6-7=-1926/984, 7-9=-2248/976, 9-10=-321/362
BOT CHORD 2-16=-1115/2830, 14-16=-738/2269, 12-14=-601/1958, 11-12=-585/1692, 10-11=-247/386
WEBS 9-11=-2573/1289, 5-16=-144/598, 3-16=-446/414, 5-14=-971/561, 6-14=-525/1221, 7-14=-571/387, 7-12=-67/126, 9-12=0/413

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



September 19,2025

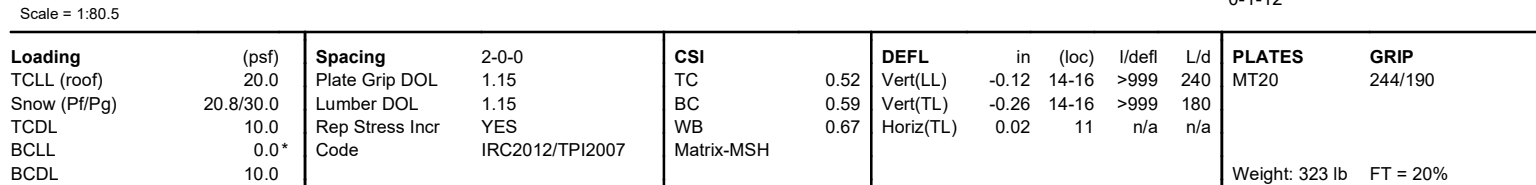
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:21 Page: 1
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- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 4x5 MT20 unless otherwise indicated.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 .
- 9) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jlt(s) 17 and 11. This connection is for uplift only and does not consider lateral forces.

September 19, 2025

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Edenton, NC 27932

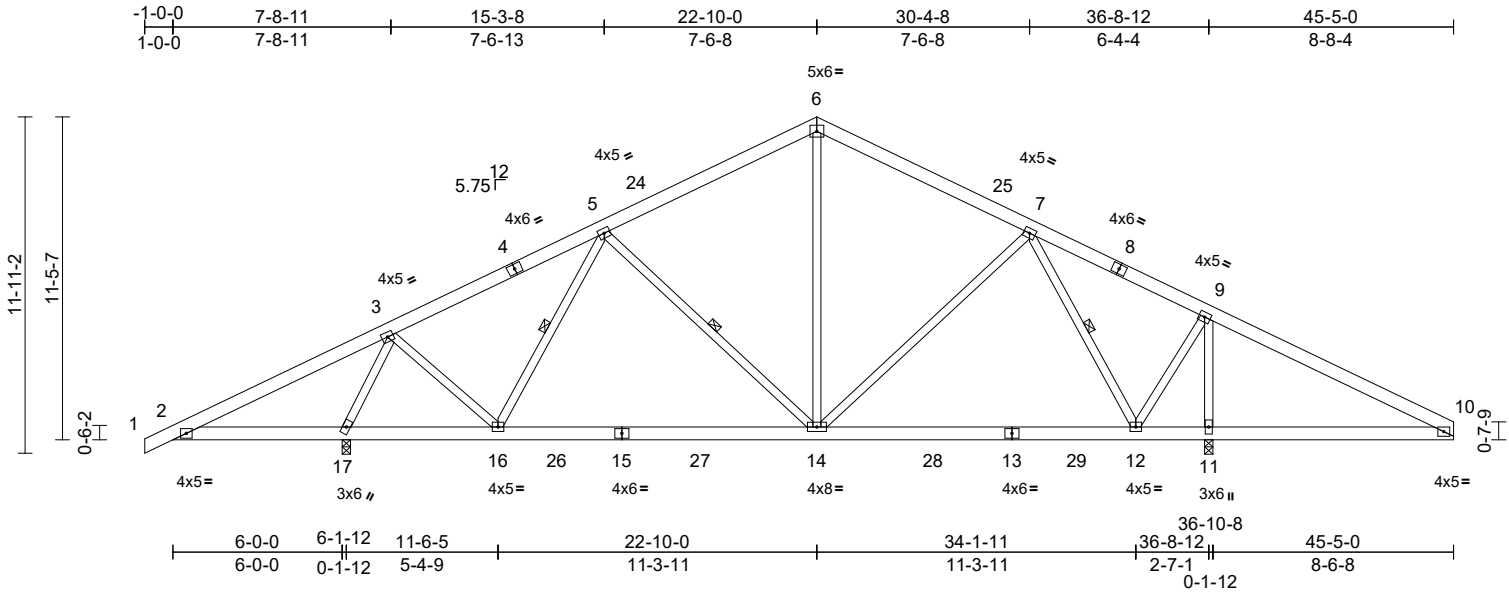
Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2D	Common	1	1	176482299
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:21

Page: 1

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Scale = 1:81.7												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	-0.12	14-16	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.59	Vert(TL)	-0.26	14-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.65	Horiz(TL)	0.02	11	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 322 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except* 14-6,14-5,7-14:2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-14, 5-16, 7-12

REACTIONS (size) 11=0-3-8, 17=0-3-8
Max Horiz 17=271 (LC 15)
Max Uplift 11=558 (LC 16), 17=538 (LC 15)
Max Grav 11=2049 (LC 23), 17=1841 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-710/800, 3-5=-933/259, 5-6=-968/413, 6-7=-926/409, 7-9=-310/162, 9-10=-600/743
BOT CHORD 2-17=-629/775, 16-17=-156/344, 14-16=-257/935, 12-14=-57/588, 11-12=-558/635, 10-11=-558/635
WEBS 3-17=-1815/958, 6-14=-82/355, 3-16=-226/867, 5-14=-278/300, 5-16=-477/345, 7-14=-27/330, 7-12=-1088/624, 9-11=-1789/844, 9-12=-312/1247

NOTES
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 4x5 MT20 unless otherwise indicated.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2.
- 9) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17 and 11. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



September 19,2025

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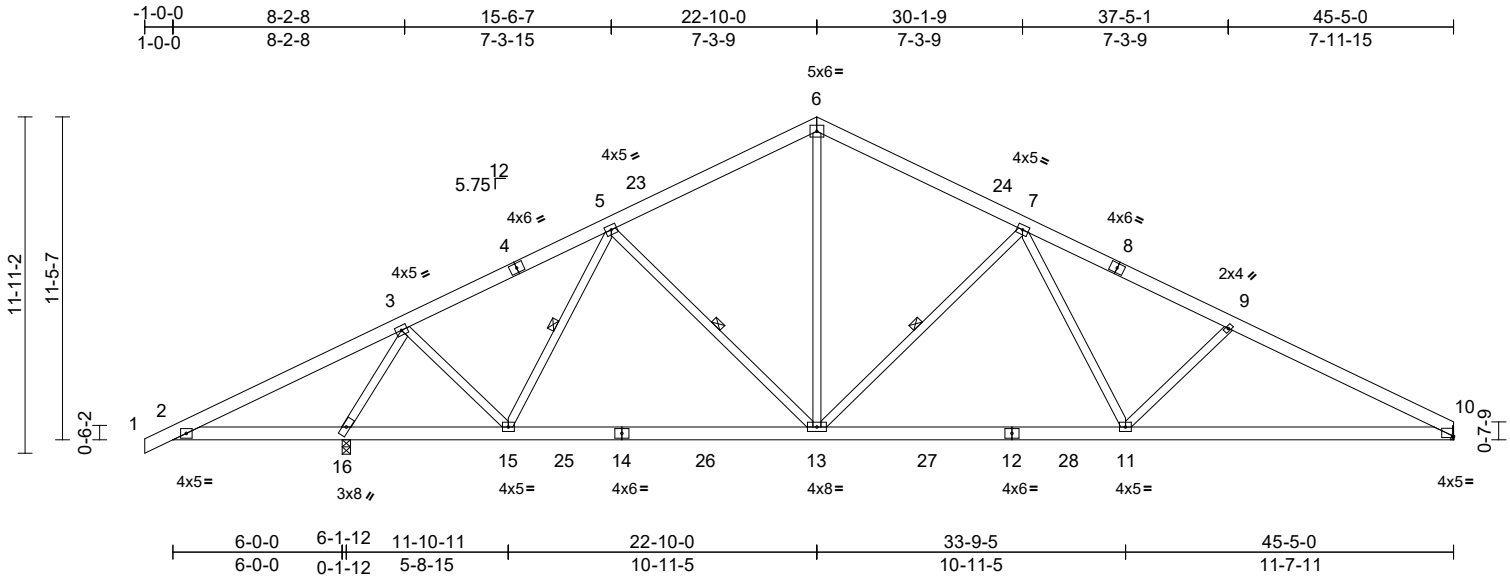
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2E	Common	5	1	176482300
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:22
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Page: 1



Scale = 1:81.7

Plate Offsets (X, Y): [10:0-0-3,0-0-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.46	Vert(LL)	-0.18	11-13	>999	240	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.73	Vert(TL)	-0.36	11-13	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.86	Horiz(TL)	0.08	10	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH								
BCDL	10.0											
											Weight: 317 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except* 13-6:2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-15, 5-13, 7-13

REACTIONS

(size) 10= Mechanical, 16=0-3-8
Max Horiz 16=271 (LC 15)
Max Uplift 10=-434 (LC 16), 16=-596 (LC 15)
Max Grav 10=1597 (LC 23), 16=2210 (LC 1)

FORCES

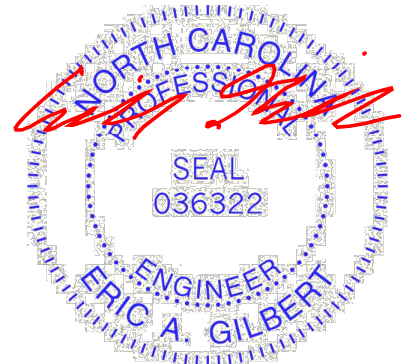
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-739/835, 3-5=-1379/498, 5-6=-1603/807, 6-7=-1660/808, 7-9=-2631/1091, 9-10=-2957/1185
BOT CHORD 2-16=-659/806, 15-16=-218/600, 13-15=-323/1390, 11-13=-570/2035, 10-11=-938/2588
WEBS 3-16=-2294/1255, 5-15=-608/436, 3-15=-336/998, 5-13=-165/273, 6-13=-379/964, 7-13=-972/556, 7-11=-182/638, 9-11=-448/412

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 16 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 10.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



September 19, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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ENGINEERING BY
TRENCO
A MITEK COMPANY

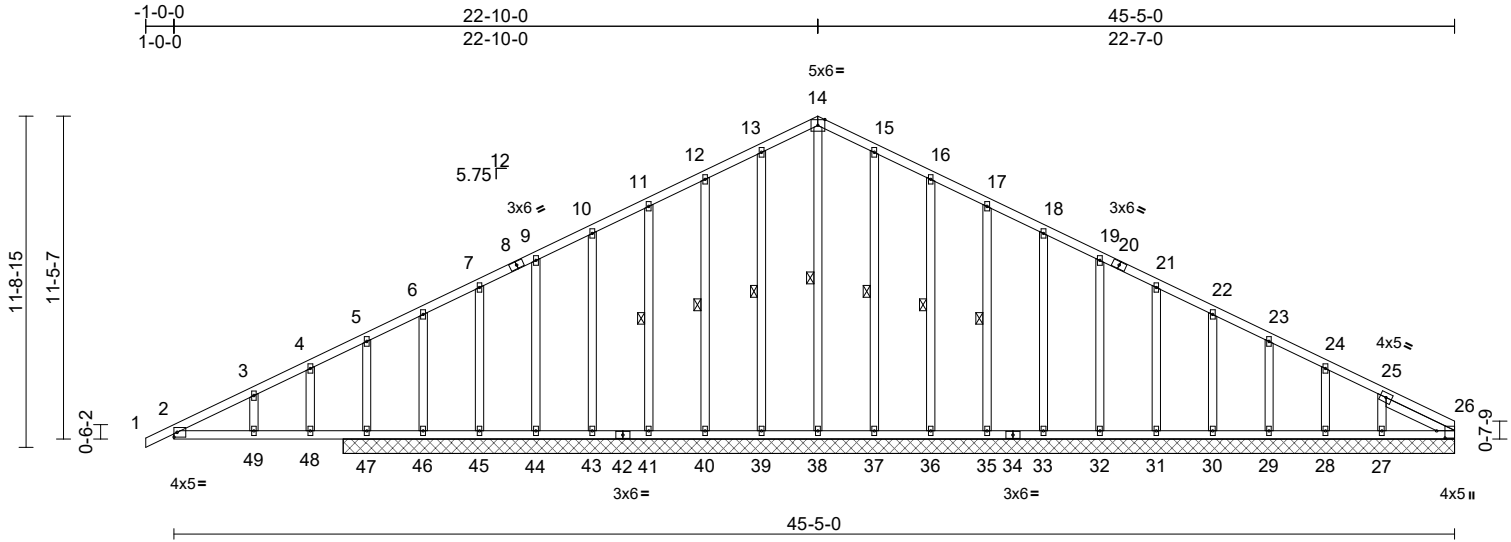
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2EG	Common Supported Gable	1	1	176482301
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:22
ID:TqWLBpX9ITXQrP0YxW4Y1ozD?kw=RfC?PsB70Hq3NSgPqnL8w3uITxbGKWRCdoi7J4zJC7f

Page: 1



Scale = 1:81.7

Plate Offsets (X, Y): [26:0-3-0,0-3-9]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.62	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.34	Horiz(TL)	-0.04	26	n/a	n/a	
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH							
BCDL	10.0										
Weight: 332 lb FT = 20%											

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3 *Except* 38-14:2x4 SP No.2
SLIDER Right 2x4 SP No.3 -- 2-8-8

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-7-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS 1 Row at midpt 14-38, 13-39, 12-40, 11-41, 15-37, 16-36, 17-35

REACTIONS (size)
26=39-5-0, 27=39-5-0, 28=39-5-0, 29=39-5-0, 30=39-5-0, 31=39-5-0, 32=39-5-0, 33=39-5-0, 35=39-5-0, 36=39-5-0, 37=39-5-0, 38=39-5-0, 39=39-5-0, 40=39-5-0, 41=39-5-0, 43=39-5-0, 44=39-5-0, 45=39-5-0, 46=39-5-0, 47=39-5-0
Max Horiz 47=271 (LC 15)
Max Uplift 26=-274 (LC 33), 27=-152 (LC 16), 28=-69 (LC 16), 29=-83 (LC 16), 30=-80 (LC 16), 31=-80 (LC 16), 32=-80 (LC 16), 33=-80 (LC 16), 35=-79 (LC 16), 36=-85 (LC 16), 37=-74 (LC 16), 39=-71 (LC 15), 40=-86 (LC 15), 41=-79 (LC 15), 43=-82 (LC 15), 44=-71 (LC 15), 45=-117 (LC 15), 46=-215 (LC 22), 47=-335 (LC 15)

Max Grav 26=172 (LC 11), 27=289 (LC 1), 28=147 (LC 23), 29=168 (LC 1), 30=162 (LC 23), 31=163 (LC 1), 32=164 (LC 23), 33=163 (LC 1), 35=205 (LC 23), 36=239 (LC 23), 37=272 (LC 23), 38=634 (LC 22), 39=280 (LC 22), 40=232 (LC 22), 41=203 (LC 22), 43=168 (LC 1), 44=141 (LC 22), 45=256 (LC 1), 46=123 (LC 11), 47=750 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-643/601, 3-4=-617/634, 4-5=-592/671, 5-6=-403/555, 6-7=-420/628, 7-9=-342/612, 9-10=-285/615, 10-11=-223/616, 11-12=-162/632, 12-13=-101/636, 13-14=-37/610, 14-15=-37/599, 15-16=-101/619, 16-17=-162/614, 17-18=-223/615, 18-19=-284/615, 19-21=-345/615, 21-22=-406/615, 22-23=-467/614, 23-24=-529/615, 24-25=-587/611, 25-26=-216/195
BOT CHORD 2-49=-530/677, 48-49=-530/677, 47-48=-530/677, 46-47=-530/626, 45-46=-530/626, 44-45=-530/626, 43-44=-530/626, 41-43=-530/626, 40-41=-530/626, 39-40=-530/626, 38-39=-530/626, 37-38=-530/626, 36-37=-530/626, 35-36=-530/626, 33-35=-530/626, 32-33=-530/626, 31-32=-530/626, 30-31=-530/626, 29-30=-530/626, 28-29=-530/626, 27-28=-530/626, 26-27=-530/626

WEBS 14-38=-593/111, 13-39=-240/123, 12-40=-192/119, 11-41=-164/117, 10-43=-124/119, 9-44=-117/110, 7-45=-160/158, 6-46=-65/51, 5-47=-391/414, 4-48=-51/66, 3-49=-53/76, 15-37=-232/123, 16-36=-199/119, 17-35=-165/118, 18-33=-123/118, 19-32=-124/118, 21-31=-123/118, 22-30=-123/117, 23-29=-125/119, 24-28=-116/111, 25-27=-218/212

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33



September 19, 2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2EG	Common Supported Gable	1	1	I76482301
Job Reference (optional)					

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 .
- 11) N/A

12) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard

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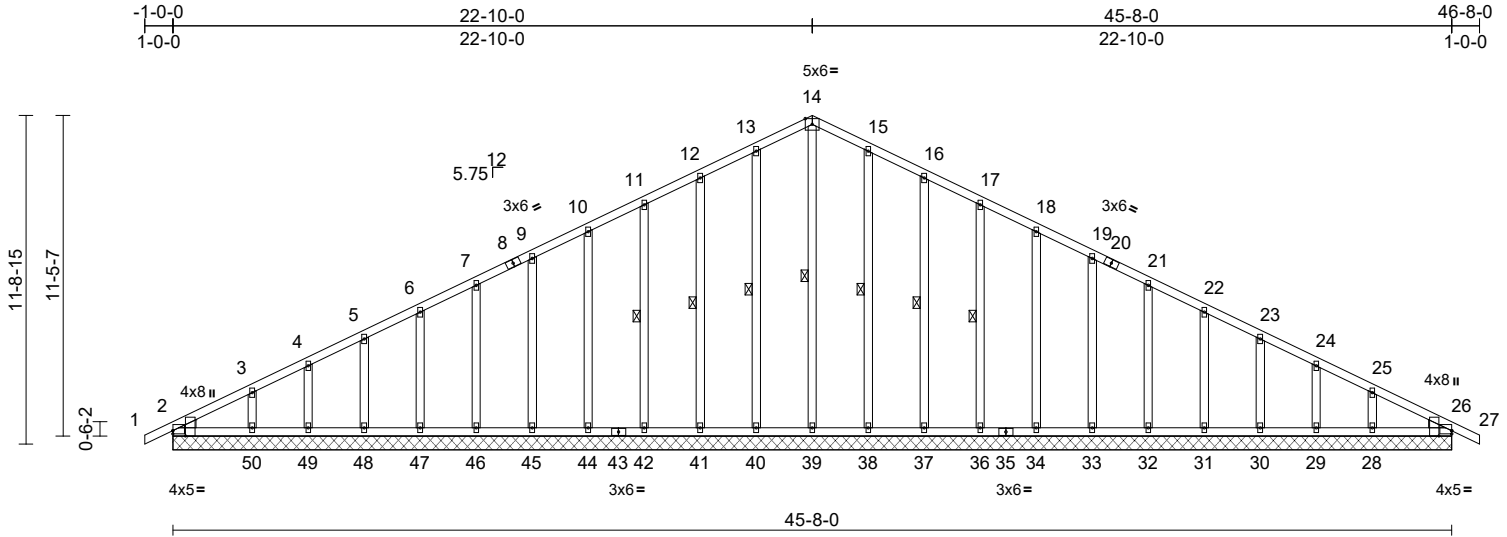
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2G	Common Supported Gable	1	1	176482302
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:22
ID:eVSDPn1VPnqTusEcOLqOkfzD?kG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKwRCDoi7J4zJC?f

Page: 1



Scale = 1:82.3

Plate Offsets (X, Y): [2:Edge,0-1-6], [2:0-2-4,Edge], [26:Edge,0-1-6], [26:0-2-4,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horiz(TL)	0.02	26	n/a	n/a	
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH							
BCDL	10.0										
Weight: 332 lb FT = 20%											

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3 *Except* 39-14:2x4 SP No.2
WEDGE Left: 2x4 SP No.2
Right: 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 14-39, 13-40, 12-41, 11-42, 15-38, 16-37, 17-36

REACTIONS (size)
2=45-8-0, 26=45-8-0, 28=45-8-0, 29=45-8-0, 30=45-8-0, 31=45-8-0, 32=45-8-0, 33=45-8-0, 34=45-8-0, 36=45-8-0, 37=45-8-0, 38=45-8-0, 39=45-8-0, 40=45-8-0, 41=45-8-0, 42=45-8-0, 44=45-8-0, 45=45-8-0, 46=45-8-0, 47=45-8-0, 48=45-8-0, 49=45-8-0, 50=45-8-0
Max Horiz 2=257 (LC 15)
Max Uplift 2=45 (LC 16), 26=-5 (LC 12), 28=-132 (LC 16), 29=-63 (LC 16), 30=-85 (LC 16), 31=-79 (LC 16), 32=-80 (LC 16), 33=-80 (LC 16), 34=-80 (LC 16), 36=-79 (LC 16), 37=-87 (LC 16), 38=-68 (LC 16), 40=-73 (LC 15), 41=-85 (LC 15), 42=-79 (LC 15), 44=-80 (LC 15), 45=-80 (LC 15), 46=-81 (LC 15), 47=-79 (LC 15), 48=-86 (LC 15), 49=-59 (LC 15), 50=-145 (LC 15)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-311/117, 3-4=-243/126, 4-5=-192/144, 5-6=-147/163, 6-7=-107/195, 7-9=-76/226, 9-10=-88/258, 10-11=-110/289, 11-12=-131/340, 12-13=-153/405, 13-14=-173/458, 14-15=-173/458, 15-16=-153/405, 16-17=-131/340, 17-18=-110/279, 18-19=-88/218, 19-21=-67/157, 21-22=-46/96, 22-23=-54/54, 23-24=-83/35, 24-25=-135/37, 25-26=-200/71, 26-27=0/45
BOT CHORD 2-50=-67/237, 49-50=-67/237, 48-49=-67/237, 47-48=-67/237, 46-47=-67/237, 45-46=-67/237, 44-45=-67/237, 42-44=-67/237, 41-42=-67/237, 40-41=-67/237, 39-40=-67/237, 38-39=-67/237, 37-38=-67/237, 36-37=-67/237, 34-36=-67/237, 33-34=-67/237, 32-33=-67/237, 31-32=-67/237, 30-31=-67/237, 29-30=-67/237, 28-29=-67/237, 26-28=-67/237

WEBS
14-39=-284/59, 13-40=-214/100, 12-41=-206/126, 11-42=-163/116, 10-44=-123/118, 9-45=-124/118, 7-46=-123/118, 6-47=-123/117, 5-48=-125/119, 4-49=-115/111, 3-50=-149/143, 15-38=-214/100, 16-37=-206/126, 17-36=-163/116, 18-34=-123/118, 19-33=-124/118, 21-32=-123/118, 22-31=-123/117, 23-30=-125/119, 24-29=-115/111, 25-28=-149/144

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33



September 19,2025

Continued on page 2

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	T2G	Common Supported Gable	1	1	I76482302
					Job Reference (optional)

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 .
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 2, 5 lb uplift at joint 26, 45 lb uplift at joint 2 and 5 lb uplift at joint 26.
- 13) N/A

LOAD CASE(S) Standard

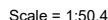
 **WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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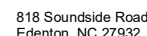
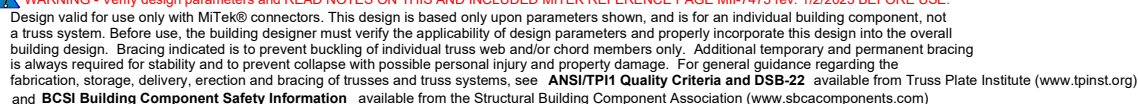


818 Soundside Road
Edenton, NC 27932

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:22 Page: 1
ID: ATE8qoHsmDockEmpI7NwCBzD?y9-RfC?PsB70Hg3NSoPqnL8w3uITXBgKWKrCD0i7J4zJC?f

Weight: 115 lb FT = 20%

LOAD CASE(S) Standard

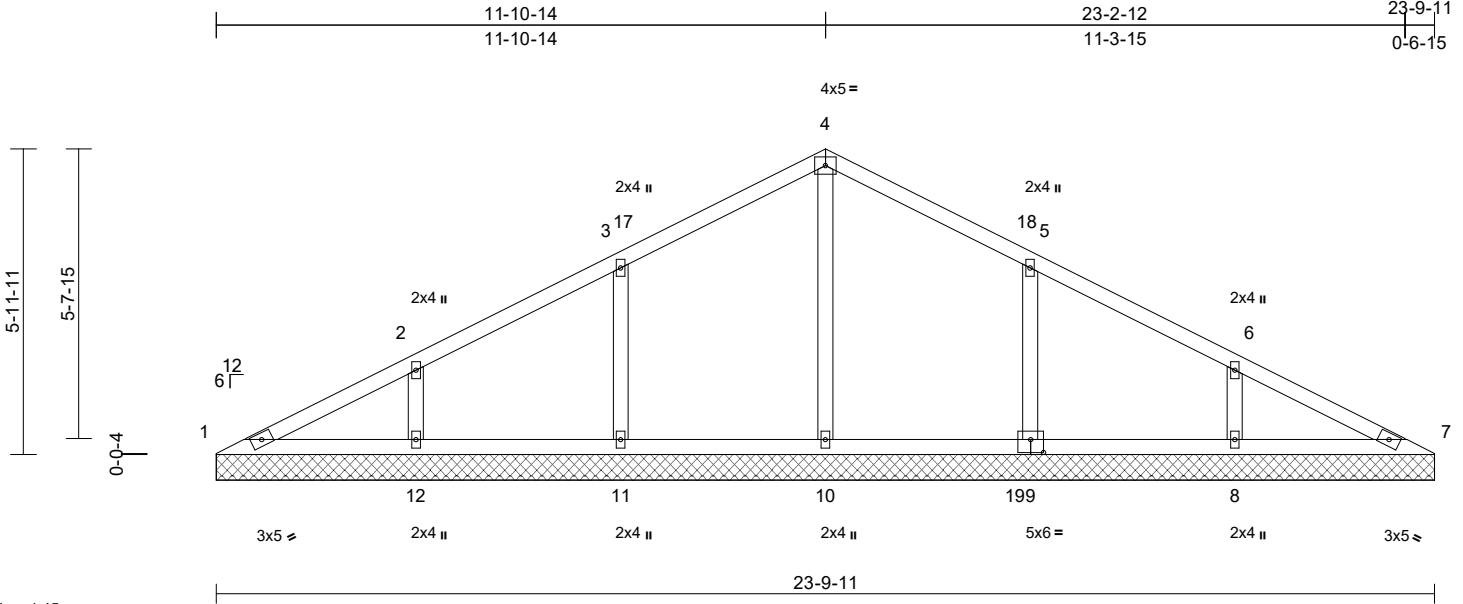


Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	V2	Valley	1	1	176482304
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:22
ID:77uKpIvH3DvW9jPQdDNTBzD?xs-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:45
Plate Offsets (X, Y): [9:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.19	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.17	Horiz(TL)	0.00	7	n/a	n/a	
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH							
BCDL	10.0										
Weight: 95 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=23-9-11, 7=23-9-11, 8=23-9-11, 9=23-9-11, 10=23-9-11, 11=23-9-11, 12=23-9-11
Max Horiz 1=-131 (LC 20)
Max Uplift 1=-26 (LC 16), 7=-9 (LC 16), 8=-165 (LC 16), 9=-180 (LC 16), 11=-180 (LC 15), 12=-167 (LC 15)
Max Grav 1=115 (LC 21), 7=114 (LC 22), 8=354 (LC 1), 9=389 (LC 22), 10=450 (LC 24), 11=389 (LC 5), 12=354 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-166/130, 2-3=-49/149, 3-4=-48/184, 4-5=-48/184, 5-6=-14/106, 6-7=-166/105
BOT CHORD 1-12=-73/163, 11-12=-73/126, 10-11=-73/126, 8-10=-74/127, 7-8=-74/142
WEBS 4-10=-265/15, 3-11=-316/257, 2-12=-246/222, 5-9=-316/257, 6-8=-246/222

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 4-0-0 oc.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) All bearings are assumed to be SP No.2.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 1 and 9 lb uplift at joint 7.
- 12) N/A

LOAD CASE(S) Standard



September 19,2025

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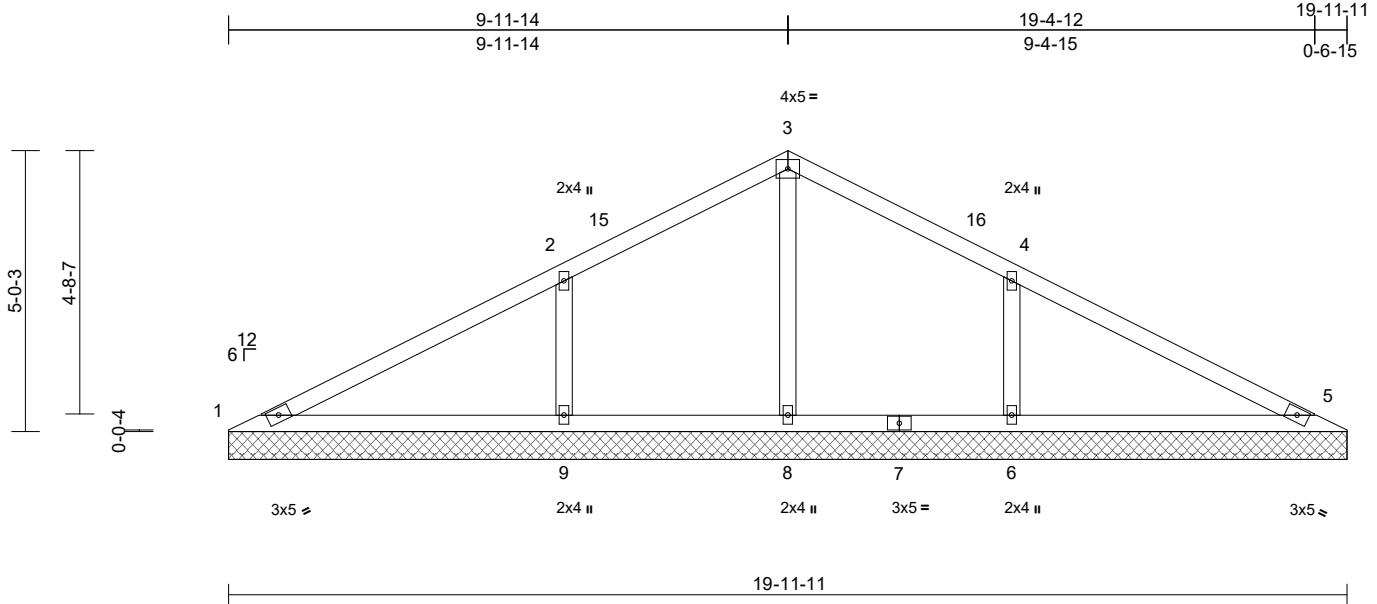
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	V3	Valley	1	1	176482305
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:22
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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.48	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.31	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.24	Horiz(TL)	-0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH							Weight: 75 lb	FT = 20%
BCDL	10.0											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 1=19-11-11, 5=19-11-11,
6=19-11-11, 8=19-11-11,
9=19-11-11
Max Horiz 1=114 (LC 15)
Max Uplift 1=-53 (LC 33), 6=-247 (LC 16),
8=-2 (LC 16), 9=-256 (LC 15)
Max Grav 1=116 (LC 32), 5=1 (LC 1), 6=517
(LC 22), 8=563 (LC 1), 9=529 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-159/476, 2-3=0/435, 3-4=0/442,
4-5=-281/476
BOT CHORD 1-9=-353/210, 8-9=-353/210, 6-8=-353/210,
5-6=-353/210
WEBS 3-8=-530/127, 2-9=-377/309, 4-6=-373/308

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 1.
- N/A

LOAD CASE(S) Standard



September 19, 2025

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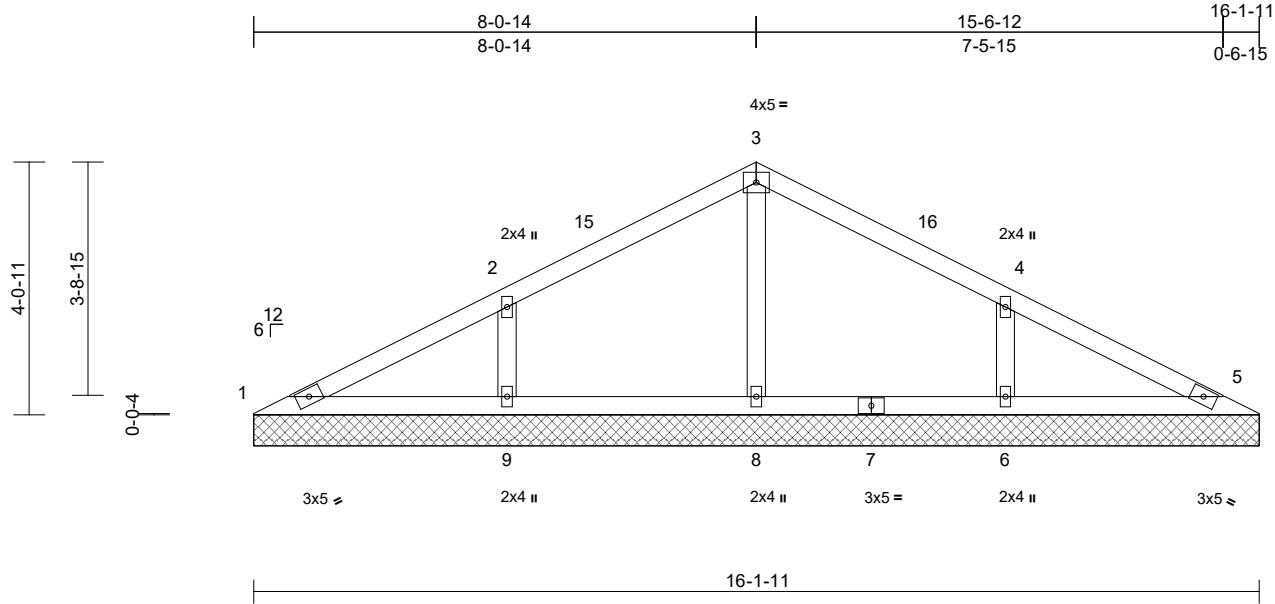
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	V4	Valley	1	1	176482306
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:23
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Page: 1



Scale = 1:37

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.31	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.12	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horiz(TL)	-0.01	5	n/a	n/a	
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MSH							
BCDL	10.0										
Weight: 59 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 1=16-1-11, 5=16-1-11, 6=16-1-11, 8=16-1-11, 9=16-1-11
Max Horiz 1=92 (LC 15)
Max Uplift 1=-14 (LC 33), 6=-191 (LC 16), 8=-12 (LC 15), 9=-195 (LC 15)
Max Grav 1=84 (LC 32), 6=394 (LC 22), 8=481 (LC 1), 9=394 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-101/279, 2-3=0/270, 3-4=0/271, 4-5=-133/279
BOT CHORD 1-9=-198/124, 8-9=-198/124, 6-8=-198/124, 5-6=-198/124
WEBS 3-8=-415/128, 2-9=-292/253, 4-6=-293/253

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1.
- N/A

LOAD CASE(S) Standard



September 19,2025

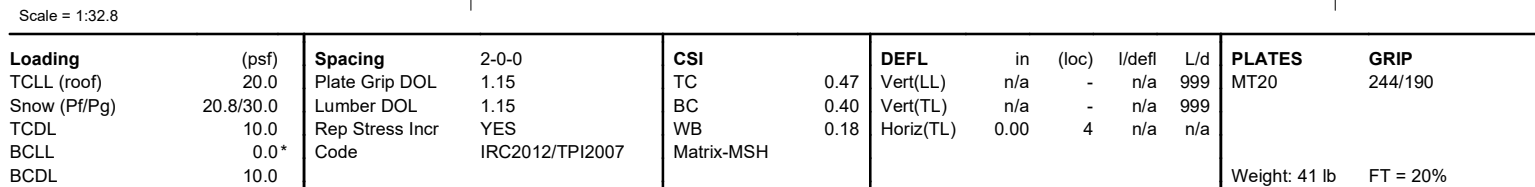
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:23 Page: 1
ID:Evig6YtbDH?f6m0ABwIPhWzD?xO-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f



- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4'-0" oc.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0"-0" tall by 2'-0"-0" wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 .
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 46 lb uplift at joint 3.
- 11) N/A

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1

LOAD CASE(S) Standard



September 19, 2025

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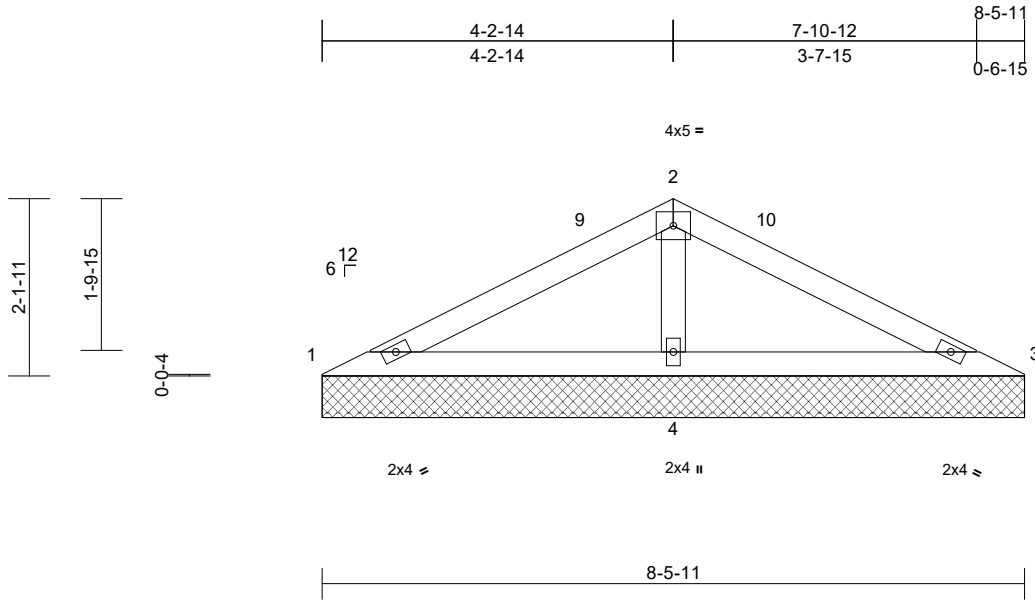
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	77 Blackberry Manor-Roof-Yarborough A RH
25090112-01	V6	Valley	1	1	176482308
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Fri Sep 19 06:55:23
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Page: 1



Scale = 1:27.8

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	n/a	-	999	MT20	244/190
Snow (Pf/Pg)	20.8/30.0	Lumber DOL	1.15	BC	0.22	Vert(TL)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horiz(TL)	0.00	4	n/a		
BCLL	0.0*	Code	IRC2012/TPI2007	Matrix-MP							
BCDL	10.0									Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 8-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 1=8-5-11, 3=8-5-11, 4=8-5-11
Max Horiz 1=45 (LC 15)
Max Uplift 1=-18 (LC 15), 3=-28 (LC 16), 4=-143 (LC 15)
Max Grav 1=80 (LC 32), 3=80 (LC 33), 4=590 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-169/297, 2-3=-169/297
BOT CHORD 1-4=-237/234, 3-4=-237/234
WEBS 2-4=-419/305

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) V (IRC2012)=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=20.8 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 28 lb uplift at joint 3.
- N/A

LOAD CASE(S) Standard



September 19,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

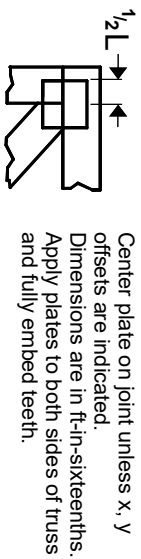
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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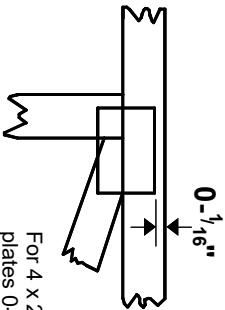
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

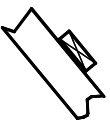
—
—
This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITek software or upon request.

PLATE SIZE

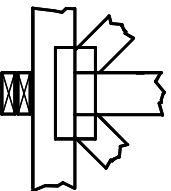
4 X 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

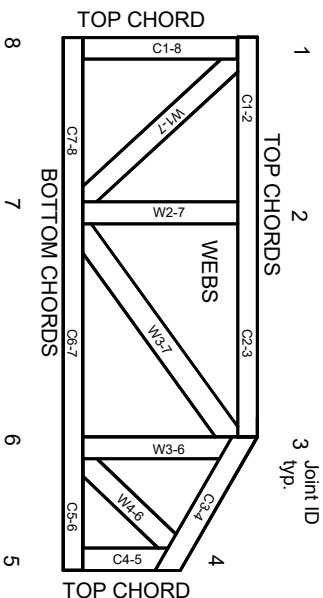


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-EES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3 These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.