

PIEDMONT

CEDAR POINTE
LOT 21



QUALITY | INTEGRITY | VALUE

PLAN ID: 040123

110 VILLAGE TRAIL SUITE 215
WOODSTOCK, GA. 30188

DRAWING INDEX

A0.0	COVER SHEET
A1.1	FRONT ELEVATIONS
A2.1	SIDE & REAR ELEVATIONS
A3.1	SLAB FOUNDATIONS
A5.1	FIRST FLOOR PLANS & DETAILS
A6.1	ROOF PLANS
A7.2	ELECTRICAL PLANS

AREA TABULATION

FIRST FLOOR	1501
TOTAL	1501
GARAGE	401
FRONT PORCH (COVERED)	70
REAR PATIO	200

PLAN REVISIONS

DATE	BY	REVISION	PAGE #
10/23/2019	AW	Prototype walk changes - see revision sheet	A2.1-A2.3, A3.1-A5.1.2, A7.2-A8.1
4/1/2023	AW	PCR #5158 relocated PDS to Garage	A5.1, A6.1-A6.1.2, A7.2
11/1/2023	AW	PCR#5604 Shifted rear door/window/patio/covered patio 12" towards Owner's Suite to avoid roof conflict w/covered patio	ALL

GOVERNMENTAL CODES & STANDARDS

HOME TO BE BUILT TO CONFORM TO ALL APPLICABLE LOCAL CODES, PRACTICES AND STANDARDS

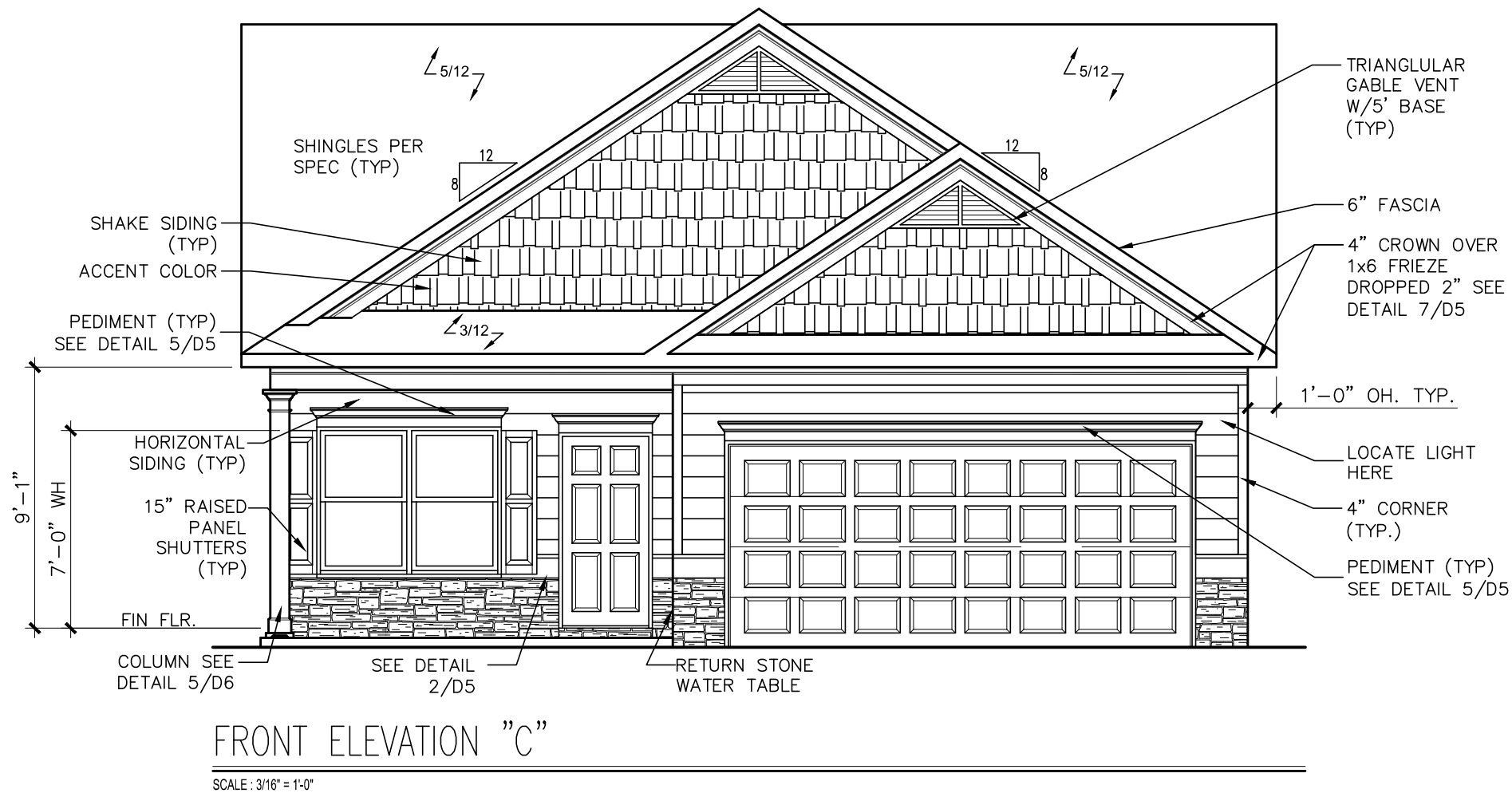
BUILDING CODE ANALYSIS / DESIGN CRITERIA

HOME TO BE BUILT TO MEET OR EXCEED ALL LOCAL CODES AND DESIGN CRITERIA

CEDAR POINTE
LOT 21

ALL NON-MASONRY RETURNS TO
BE HORIZONTAL SIDING

SEE SHEET D3 OF SDH TYPICAL
DETAILS FOR SOFFIT DETAILS PER
SOFFIT MATERIAL



DATE	BY	REVISION
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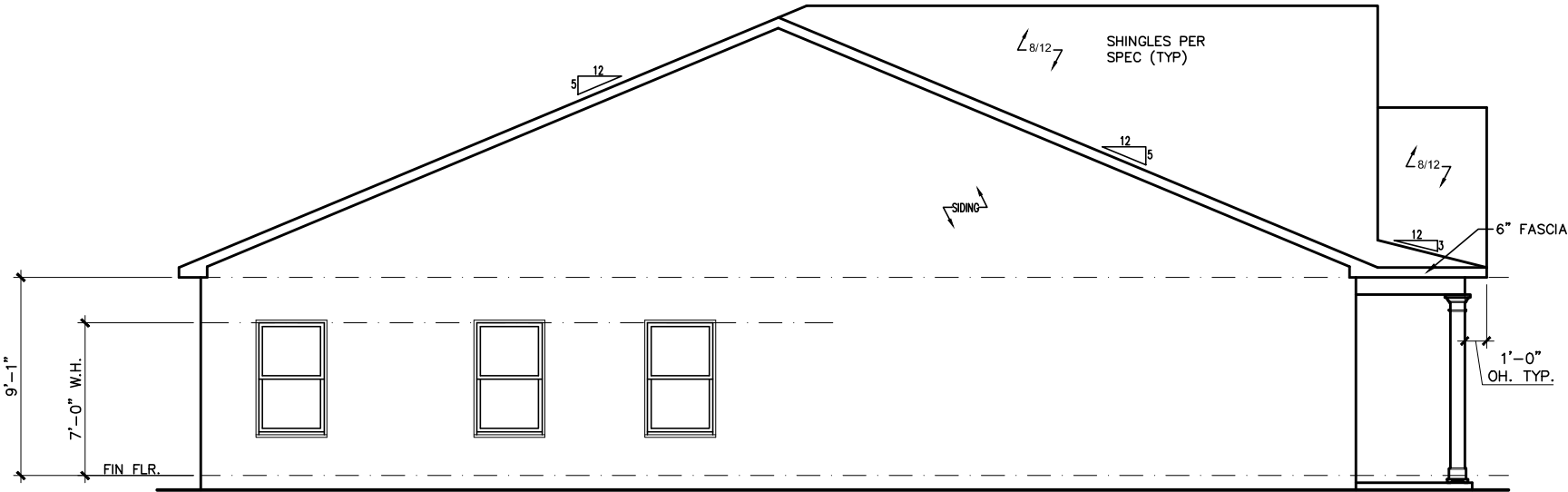
ELEVATIONS
FRONT ELEVATION
PIEDMONT

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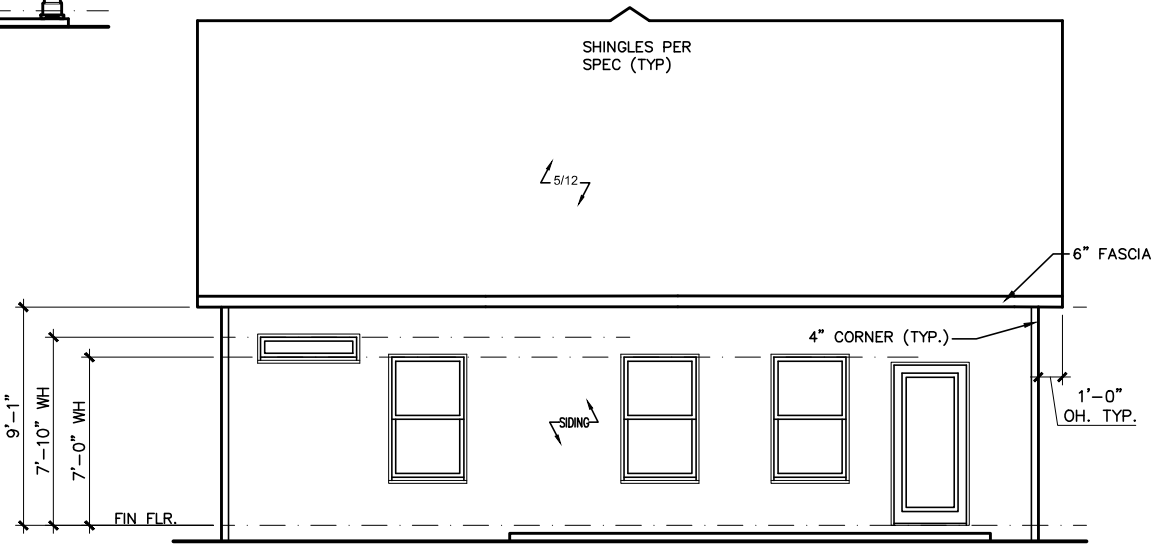
BY: AAP	CH: AW
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PAGE NO: A1.1	

CEDAR POINTE
LOT 21



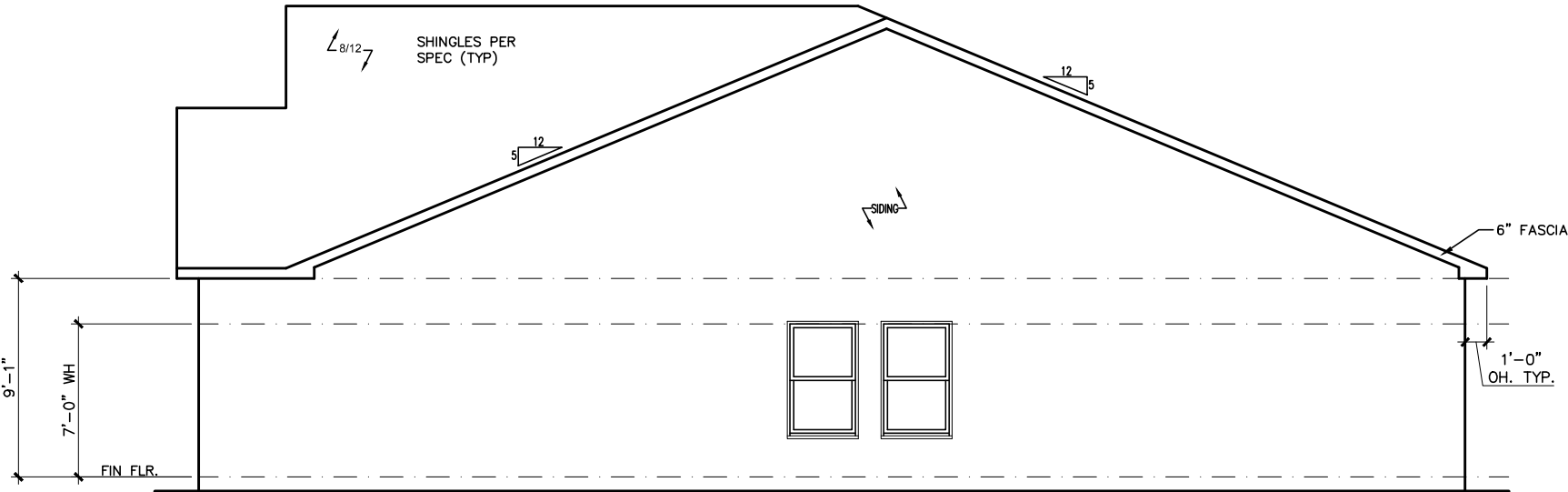
LEFT ELEVATION "C"

SCALE: 1/8" = 1'-0"



REAR ELEVATION "C"

SCALE: 1/8" = 1'-0"



RIGHT ELEVATION "C"

SCALE: 1/8" = 1'-0"

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REVISION						
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ELEVATIONS

SIDES AND REAR

PIEDMONT

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110 VILLAGE TRAIL

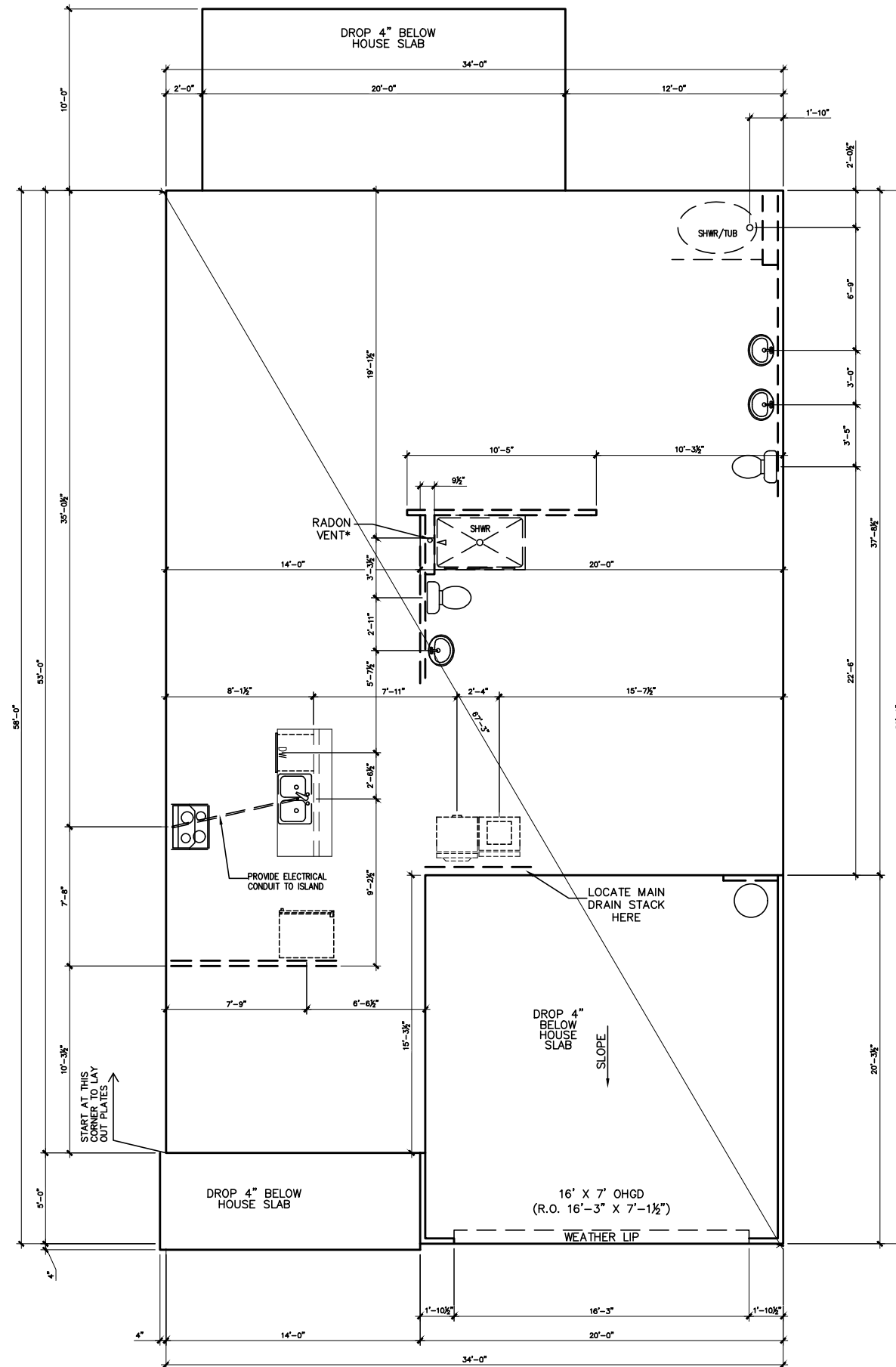
SUITE 115

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SLAB PLAN

SCALE : 1/8" = 1'-0"

CEDAR POINTE LOT 21

*RADON VENT PROVIDED
PER LOCAL CODE

REFER TO DETAIL 3/D1
FOR BRICK LEDGE
DETAIL WHEN BRICK
VENEER IS CHOSEN

DATE	BY	REVISION
#	#	#
#	#	#
#	#	#
#	#	#
#	#	#

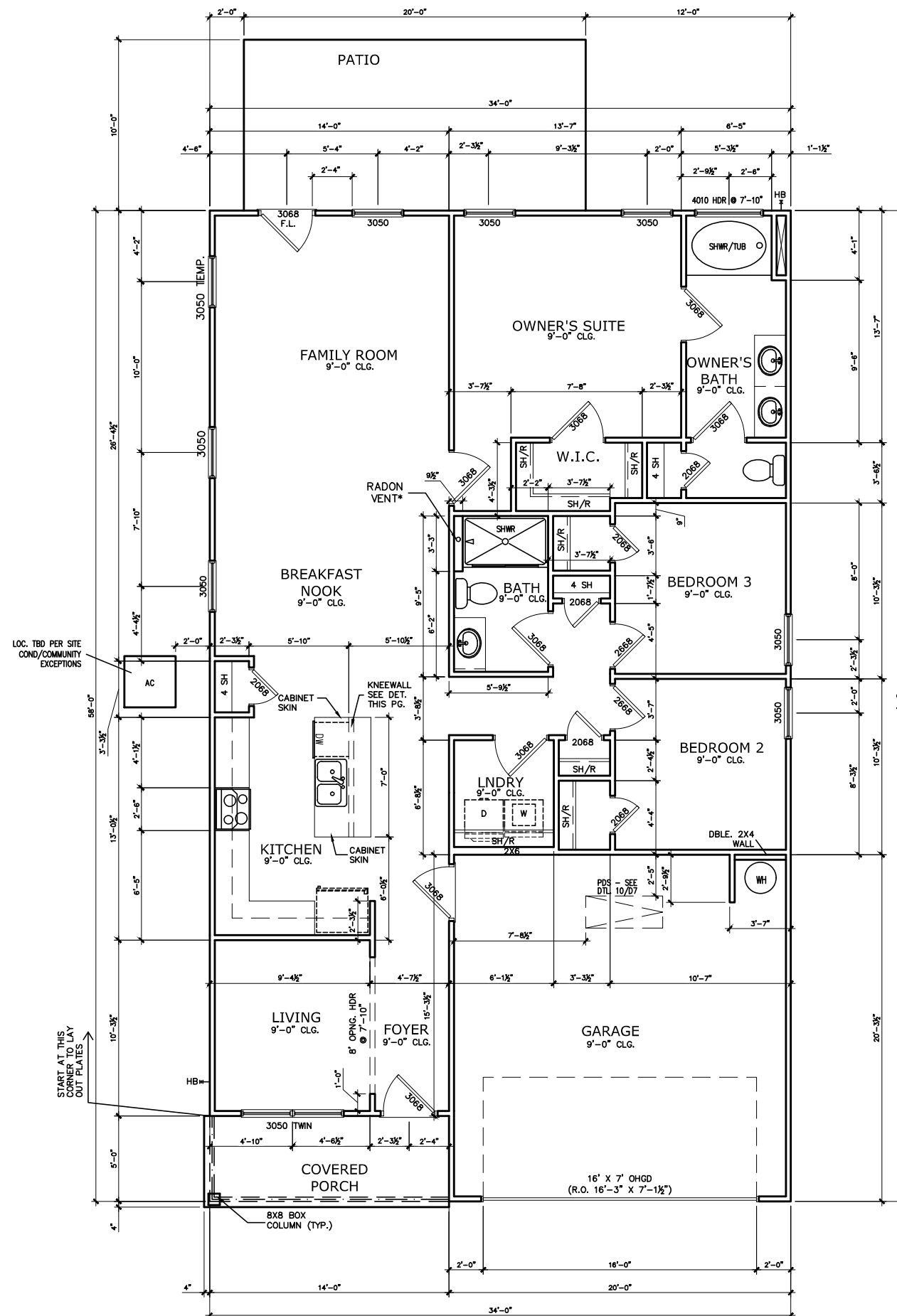
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FOUNDATION PLAN
SLAB PLAN
PIEDMONT

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REFER TO MANUFACTURER'S SPECS.
FOR DRAIN LOCATIONS ON DETAIL
SHEETS D12, D12.1, & D12.2

*RADON VENT PROVIDED
PER LOCAL CODE

SCALE : 1/8" = 1'-0"

[illegible]

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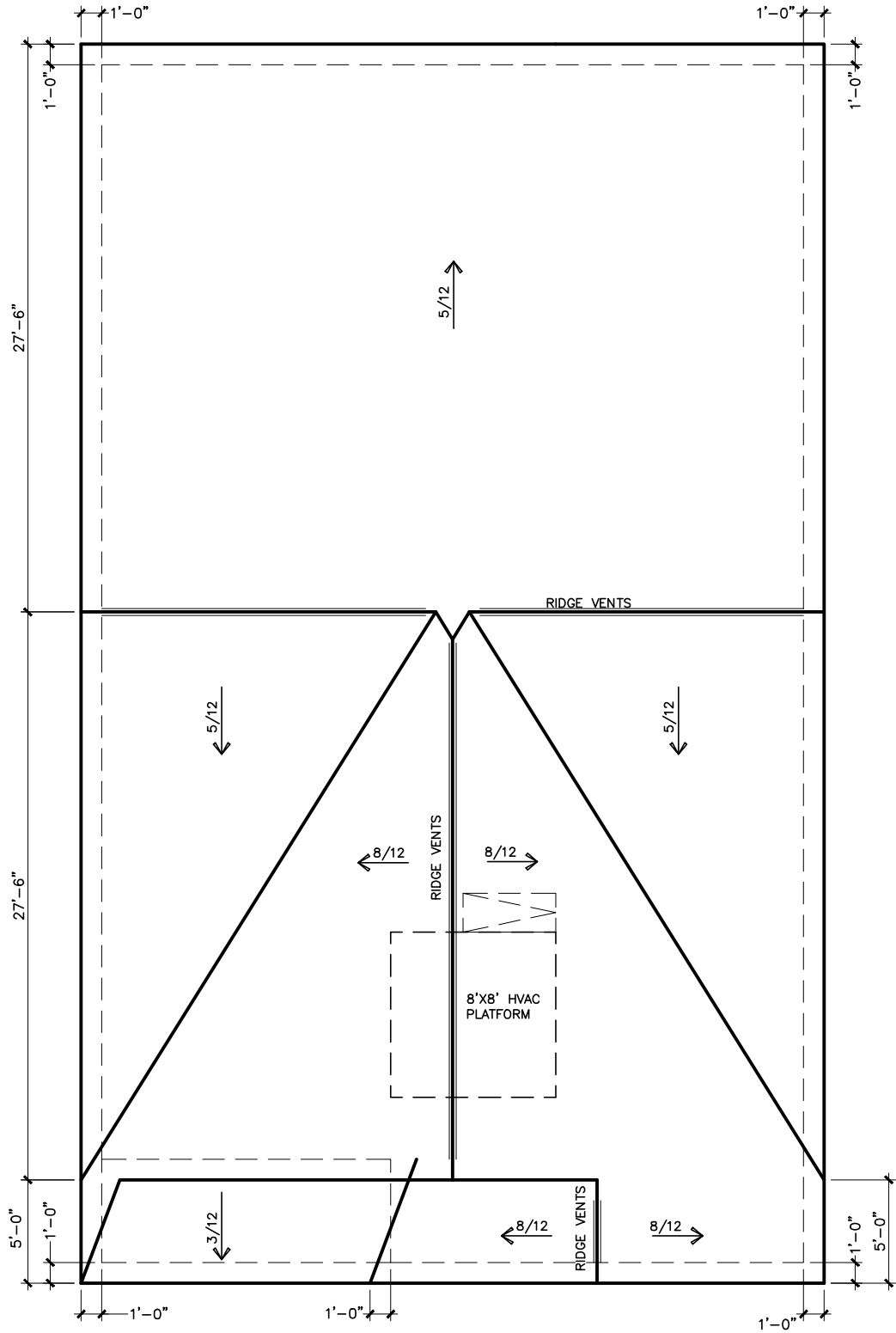
FLOOR PLAN
FIRST FLOOR
PIEDMONT

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CEDAR POINTE
LOT 21



ROOF LAYOUT "C"
SCALE : 1/8" = 1'-0"

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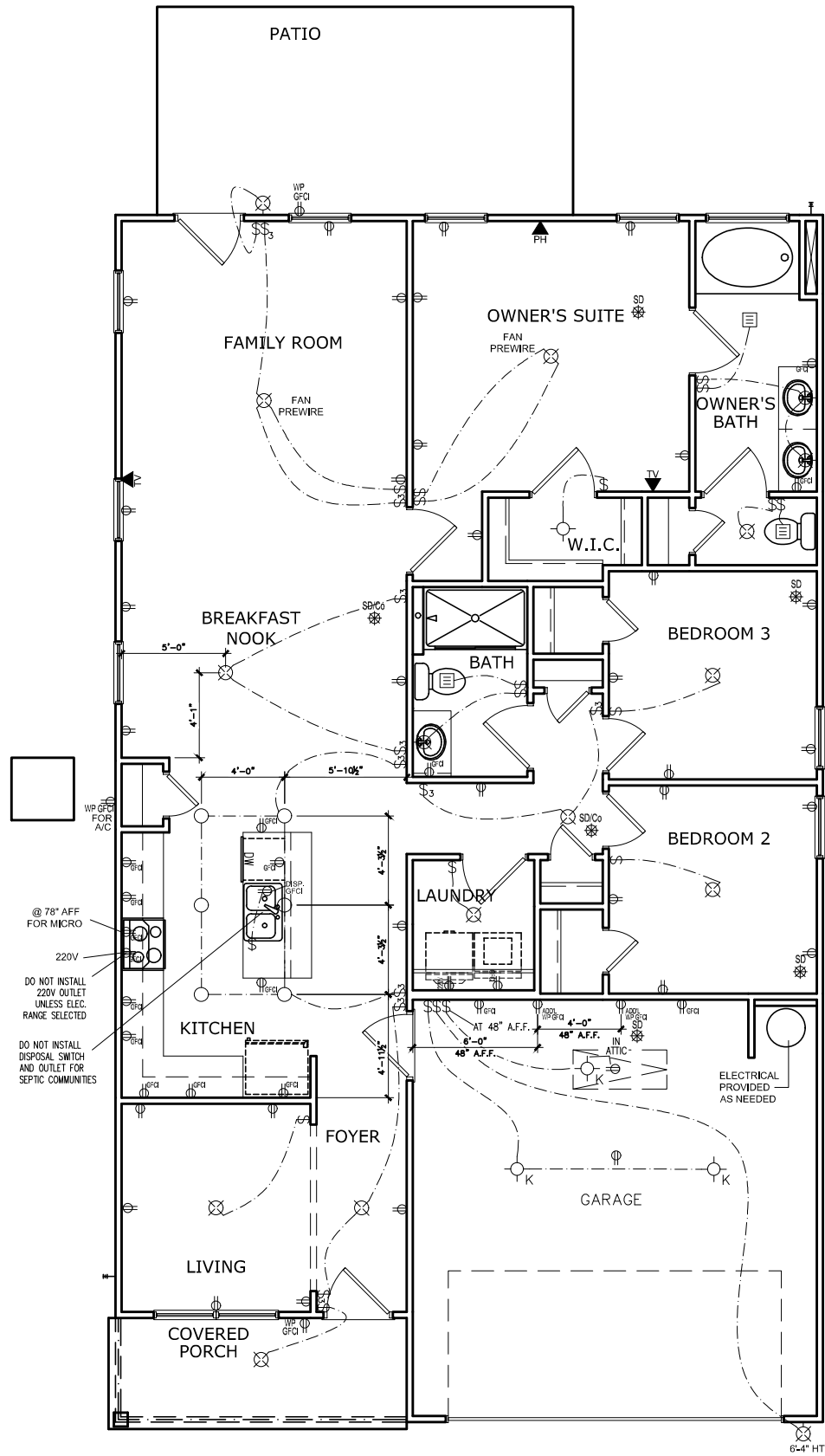
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ROOF PLAN
ROOF PLAN
PIEDMONT

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FIRST FLOOR ELECTRICAL PLAN

SCALE: 1/8" = 1'-0"

CEDAR POINTE

LOT 21

ELECTRICAL LEGEND

\$	SWITCH	TV	TV
\$3	3 WAY SWITCH	120V	120V RECEPTACLE
\$4	4 WAY SWITCH	120V	120V SWITCHED RECEPTACLE
⊗	CEILING FIXTURE	220V	220V RECEPTACLE
⊕ _k	KEYLESS	GFCI	GFCI OUTLET
⊕ _w	WALL MOUNT FIXTURE	ARCH	ARCH FAULT CIRCUIT INTERRUPTER
○	CEILING FIXTURE	GL	GAS LINE
●	FLEX CONDUIT	WL	WATER LINE
CH	CHIMES	↓	HOSE BIBB
PH	TELEPHONE	⊕	FLOOD LIGHT
SD/Co	SMOKE DETECTOR & CARBON MONOXIDE	1x4	1x4 LUMINOUS FIXTURE
SO	SECURITY OUTLET	⊗	CEILING FAN
□	GARAGE DOOR OPENER		ELECTRICAL WIRING
⊖	EXHAUST FAN	⊕	CEILING FIXTURE
⊖	FAN/LIGHT		

ELECTRICAL PLANS TO FOLLOW ALL LOCAL CODES

APPROX. FIXTURE HGTS (MEASURED FROM BOTTOM OF FIXTURE)

BREAKFAST/DINING ROOM	63" ABOVE FINISHED FLOOR
KITCHEN PENDANT LIGHTS	33" ABOVE COUNTER TOP
TWO STORY FOYER FIXTURE	96" ABOVE FINISHED FLOOR
CEILING FAN	96" ABOVE FINISHED FLOOR

NOTE: FINAL PLACEMENT OF PHONE/CABLE T.B.D. ON SITE BY THE BUILDER

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ELECTRICAL PLAN

FIRST FLOOR

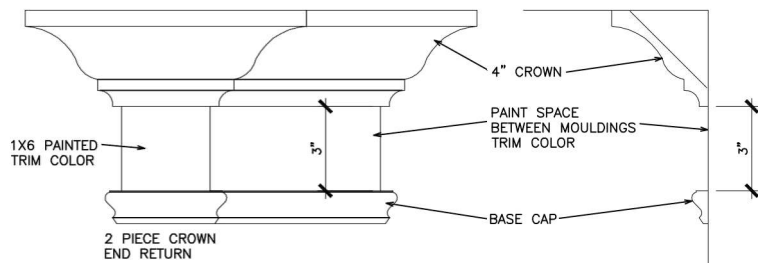
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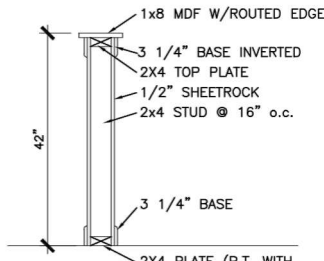
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DATE:	7/18/25		
FACADE OPT:	C		
PLAN ID:			
PND:	ALL	RELEV:	C
PAGE NO:	A7.2		

REFER TO LOT SPECIFIC PLAN TO
DETERMINE WHICH DETAILS APPLY



TYPICAL TWO PIECE CROWN

N.T.S.



TYP. KNEEWALL SECTION

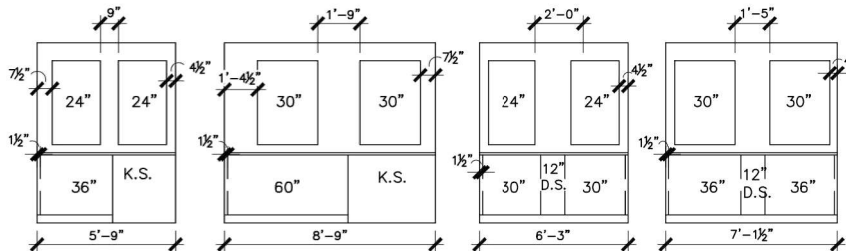
N.T.S.



TYP. 2ND FLOOR KNEE WALL STABILITY

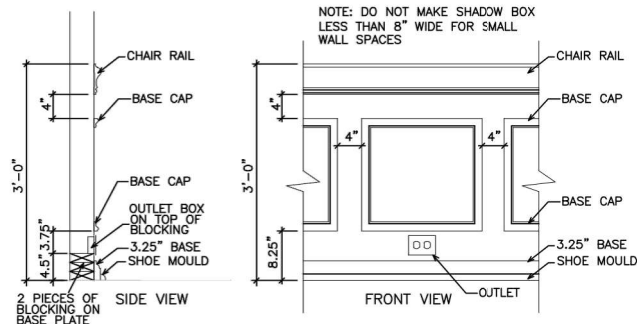
N.T.S.

1. MIRRORS ARE TO BE CENTERED ON THE CABINET OR KNEESPACE BELOW.
2. SPACE BETWEEN MIRROR AND WALL/CABINET END, MAY NOT MATCH ON EACH SIDE
3. MIRRORS ARE LIMITED TO 2 SIZES: 24" & 30"
 - a. VANITIES 30" & SMALLER RECEIVE THE 24" WIDE MIRROR.
 - b. VANITIES 33" & LARGER RECEIVE THE 30" WIDE MIRROR.
 - c. HEIGHTS DO NOT CHANGE.
 - d. SEE P.O. FOR EXACT WIDTH.
4. SEE THE BELOW EXAMPLE DRAWINGS. DIMENSIONS ARE APPROXIMATE.



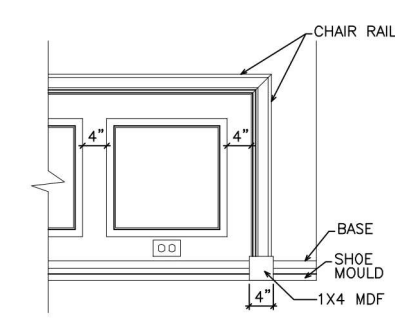
TYPICAL SPLIT MIRROR SCENARIOS

N.T.S.



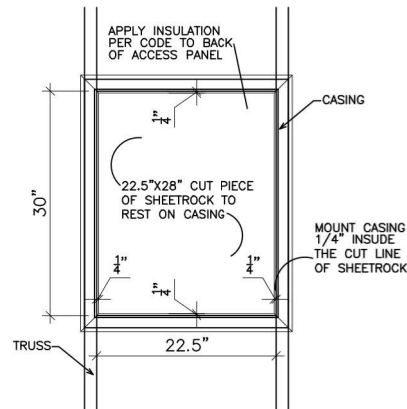
TYPICAL CHAIR RAIL & SHADOW BOX DETAIL

N.T.S.



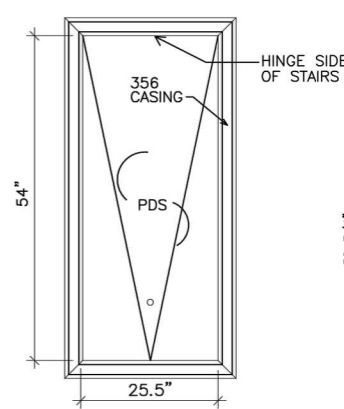
CHAIR RAIL END TRIM DETAIL

N.T.S.



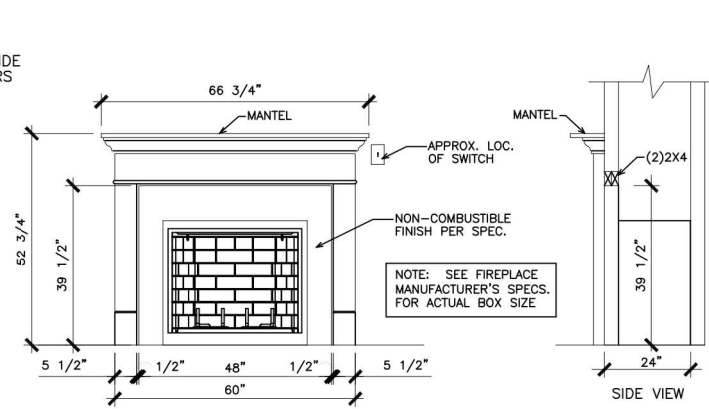
SCUTTLE HOLE DETAIL

N.T.S.



PDS TRIM DETAIL

N.T.S.



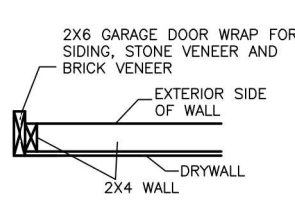
GAS/ELECTRIC FIREPLACE DETAIL
WITH WESCOTT WOOD MANTEL

N.T.S.

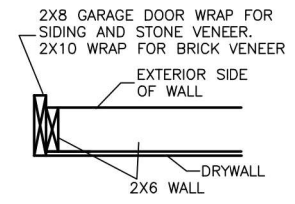
NOTE: SEE FIREPLACE
MANUFACTURER'S SPECS.
FOR ACTUAL BOX SIZE.

ELECTRIC FRAMING
DIMENSIONS:
W.=37"
D.=24"
H.=31 1/4"

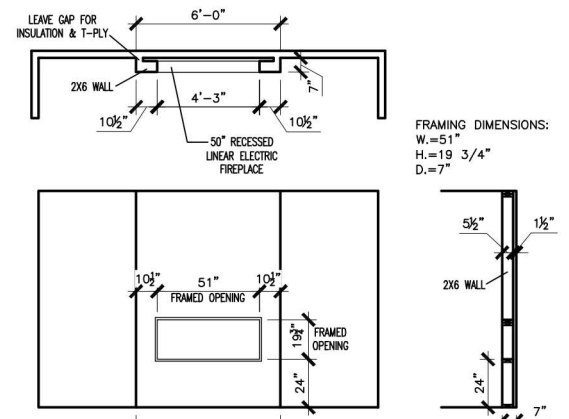
GAS FRAMING
DIMENSIONS:
W.=37"
D.=24"
H.=34 3/4"



SECTION VIEW
2X4 PORTAL WALL

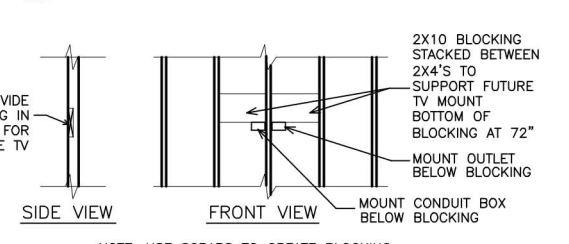


SECTION VIEWS
2X6 PORTAL WALL



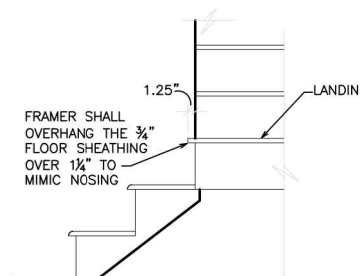
LINEAR ELECTRIC FIREPLACE DETAIL

N.T.S.



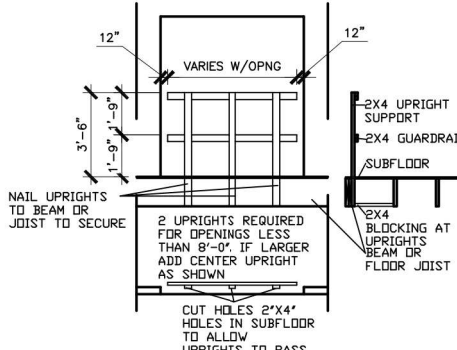
TYP. TV WALL PREP

N.T.S.



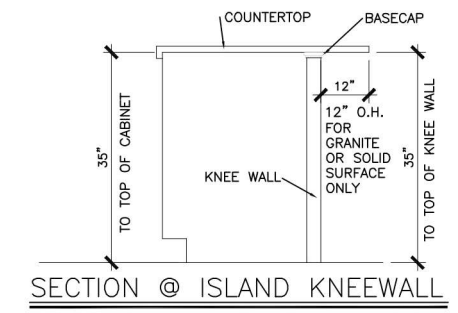
BOX STEP OVERHANG

N.T.S.



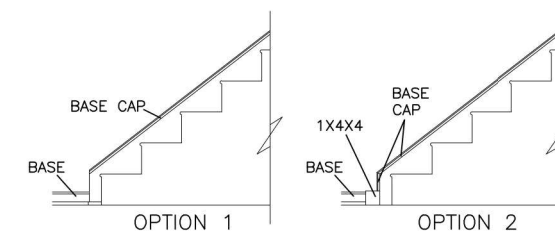
GUARD RAIL DTL. AS REQ'D

N.T.S.



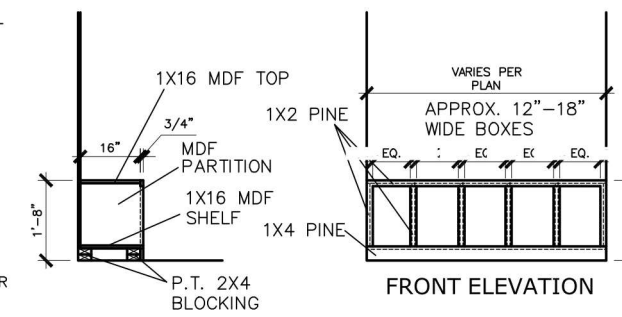
SECTION @ ISLAND KNEEWALL

N.T.S.



STAIR TRIM DETAILS

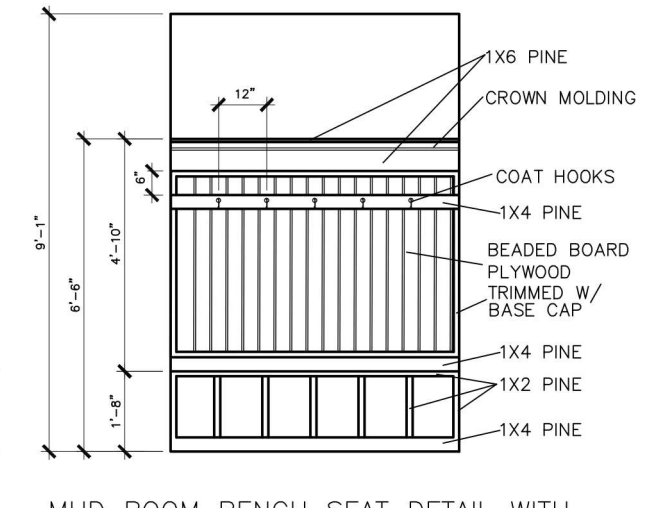
N.T.S.



SIDE ELEVATION

MUD ROOM BENCH SEAT DETAIL

N.T.S.



MUD ROOM BENCH SEAT DETAIL
WITH BEADED BOARD, HOOKS, & CROWN

N.T.S.

(IF TRIM CHOSEN WITHOUT
BENCH CONTINUE TO FLOOR)

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DATE					

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INTERIOR TRIM
DETAILS

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FND:	ELEV:
PAGE NO:	D1.1

CS1

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of UES Professional Solutions, Inc. (UES) or the SER. For the purposes of these construction documents the SER and UES shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to UES for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or UES.
- Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to UES before construction begins.
- The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of local building codes.
- All structural assemblies are to meet or exceed to requirements of the current local building code.

FOUNDATIONS:

- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer.
- The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

STRUCTURAL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- All steel shall have a minimum yield stress (F_y) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D11. Electrodes for shop and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.

CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'_c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
 - Footings: 5%
 - Exterior Slabs: 5%
- No admixtures shall be added to any structural concrete without written permission of the SER.
- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished.
- Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- All welded wire fabric (WWF) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (15 pounds per cubic yard)
- Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
 - E = 1,300,000 psi
 - Fb = 2600 psi
 - Fv = 285 psi
 - Fc = 100 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/A308 standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached with (3) 10d nails @ 24" O.C.
- Four and five ply beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 24" O.C. unless noted otherwise.

WOOD TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

- Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- All structurally required fiberboard sheathing shall bear the mark of the AFA.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

Cedar Pointe
LOT 21

05.02.2024



STRUCTURAL MEMBERS ONLY



UES PROFESSIONAL SOLUTIONS 29, INC.
FORMERLY SUMMIT ENGINEERING,
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10121 Pineville Distribution St
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Office: 704.504.1717
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PROJECT:
Piedmont - RH
Coversheet

CLIENT:
Smith Douglas Homes - Raleigh
2520 Reliance Ave.
Apex, NC 27539

DRAWING

DATE: 05/02/2024

SCALE: 1/8"=1'-0"

PROJECT #: A20117.00403.000

DRAWN BY: EMB

CHECKED BY: GUS

ORIGINAL INFORMATION

PROJECT #
3832379

DATE
10/19/20

REFER TO COVER SHEET FOR A
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SHEET

CS2

1. FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE W/ CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE w/ ALL LOCAL AMENDMENTS.
2. STRUCTURAL CONCRETE TO BE $F_c = 3000$ PSI, PREPARED AND PLACED IN CONFORMANCE WITH SECTION 804.01 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
3. FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.
4. FOUNDATIONS BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
5. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. MINIMUM 12" MINIMUM CLEARANCE FROM THE FACE OF MASONRY WALLS.
6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION 804.01 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
7. PLASTERERS TO BE NOTIFIED TO PERFORMER FOUNDATION WALL.
8. PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
9. PROVIDE PERFORMER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
10. CORREL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEERS.
11. ORAIL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
12. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE. ANCHORS SHALL BE 1/4" DIA. BOLT SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE MINIMUM 2" ON CENTER BOLT'S PER PLATE SECTION AND (1) LOCATED NOT MORE THAN 12" FROM THE CORNER. ANCHOR BOLT'S SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
13. ABBREVIATIONS:

14. ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x16" MASONRY, TYPICAL. (WFO)
15. WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
16. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS QUALIFIED REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION USES PREPARED SOIL LOGS TO RECORD THE OBSERVATIONS. THE CONTRACTOR TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
17. ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 35% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

NOTE: FOUNDATION ANCHORAGE HAS BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PAIR IN ACCORDANCE WITH METHOD 3 OF SECTION R602 OF THE 2018 NIRC.

SCALE: 1/8"=1'



Cedar Point
LOT 21



S1.0m

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWINGS FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:
MICROALLOY (LVL): $F_y = 26000 \text{ PSI}$, $F_u = 285 \text{ PSI}$, $E = 1.9 \times 10^6 \text{ PSI}$
TIMBERSTRAND (LSL): $F_y = 2325 \text{ PSI}$, $F_u = 330 \text{ PSI}$, $E = 1.95 \times 10^6 \text{ PSI}$
PARALLAM (PSL): $F_y = 29000 \text{ PSI}$, $F_u = 290 \text{ PSI}$, $E = 1.25 \times 10^6 \text{ PSI}$
- ALL WOOD MEMBERS SHALL BE $\frac{1}{2}$ SYP UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE $\frac{1}{2}$ SYP (UNO).
- ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 $\frac{1}{2}$ SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".
- FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R403.1.6. MINIMUM 1/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION AND (1) LOCATED NOT MORE THAN 12" FROM THE CORNER. ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- FLITCH BEAMS, 4-PLY LVL'S AND 3-PLY SIDE LOADED LVL'S SHALL BE SECURED TOGETHER WITH (2) ROWS OF 1/2" DIA. LAG SCREWS SPACED AT 24" O.C. (MAX, STAGGERED) OR EQUIVALENT CONNECTIONS PER DETAIL 2/DT. MIN. EDGE DISTANCE SHALL BE 2" AND (2) SCREWS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP $\frac{1}{2}$ DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIFLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP $\frac{1}{2}$ DROPPED. (UNLESS NOTED OTHERWISE)
- ABBREVIATIONS:

DJ = DOUBLE JOIST
GT = GIRDER TRUSS
SC = STUD COLUMN
EE = EACH END
TJ = TRIPLE JOIST
CL = CENTER LINE

SJ = SINGLE JOIST
FT = FLOOR TRUSS
DR = DOUBLE RAFTER
TR = TRIPLE RAFTER
OC = ON CENTER
PL = POINT LOAD

WALL STUD SCHEDULE (10 FT HEIGHT)				
STUD SIZE	STUD SPACING (O.C.)			
	ROOF ONLY	ROOF & 1 FLOOR	ROOF & 2 FLOORS	NON-LOAD BEARING
2x4	24"	16"	12"	24"
2x6	24"	24"	16"	24"

NOTES:

- BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C.
- STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 16" O.C.
- TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZONTAL BLOCKING @ 6'-0" O.C. VERTICALLY.

LINTEL SCHEDULE		
TAG	SIZE	OPENING SIZE
①	L3x3x1/4"	LESS THAN 6'-0"
②	L5x3x1/4"	6'-0" TO 10'-0"
③	L5x3-1/2x5/16"	GREATER THAN 10'-0"
④	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS

NOTES:

- SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED AT 16" O.C. (TYP FOR OPENINGS GREATER THAN 10'-0").
- ALL HEADERS WHERE BRICK IS PRESENT, TO BE ① (UNO.)

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: FLOOR JOISTS SHALL BE DESIGNED TO SUPPORT ADDITIONAL LOAD UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE:
— = DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED/REVISED ON 04/02/23. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

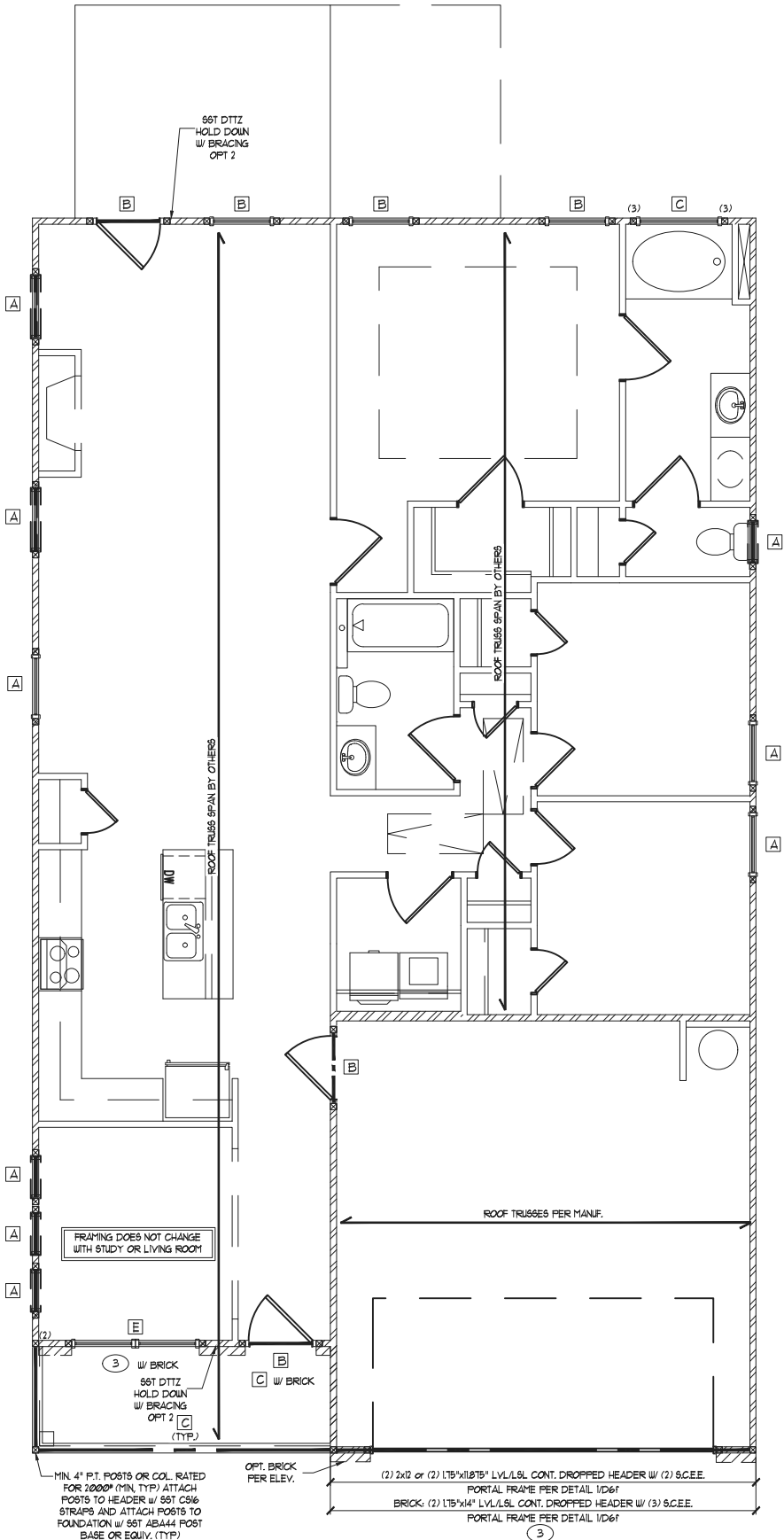
STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

SCALE: 1/8"=1'



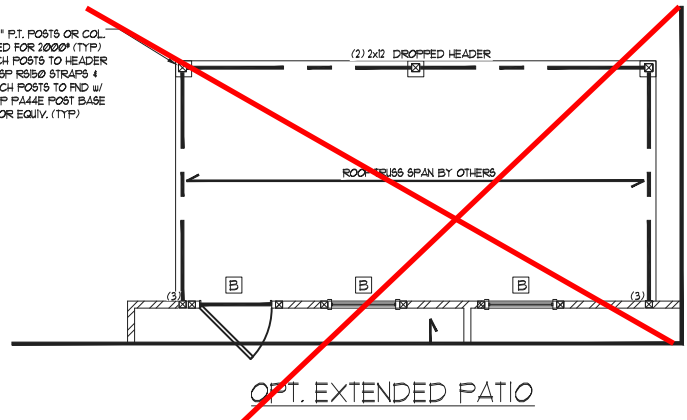
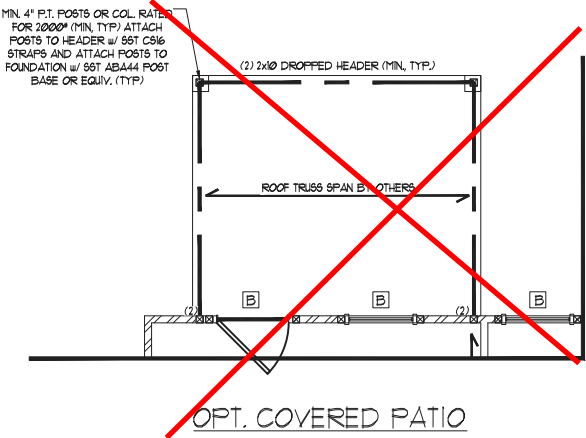
ELEVATIONS C.F.I.

HEADER/BEAM SCHEDULE			
HEADER TAG	BEAM TAG	SIZE	JACKS (EACH END)
-	B1	(1) 14" FLOOR JOIST	(2)
-	B2	(2) 14" FLOOR JOISTS	(2)
A	B3	(2) 2x6	(1)
B	B4	(2) 2x8	(2)
C	B5	(2) 2x10	(2)
D	B6	(2) 2x12	(2)
E	B7	(2) 9-1/4" LSL/LVL	(3)
F	B8	(2) 11-7/8" LSL/LVL	(3)
G	B9	(2) 14" LSL/LVL	(3)
H	B10	(2) 16" LVL	(3)
I	B11	(2) 18" LVL	(3)
J	B12	(2) 24" LVL	(4)
K	B13	(3) 9-1/4" LSL/LVL	(3)
L	B14	(3) 11-7/8" LSL/LVL	(3)
M	B16	(3) 14" LSL/LVL	(3)
N	B17	(3) 16" LVL	(3)
O	B18	(3) 18" LVL	(3)
P	B19	(3) 24" LVL	(4)

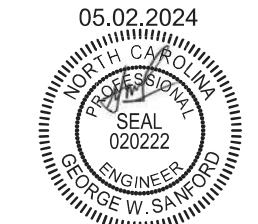
NOTES:
1. SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.
2. ALL HEADERS TO BE DROPPED (UNO.).
3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (UNO.).

KING STUD SCHEDULE	
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
3'-0"	(1)
4'-0"	(2)
8'-0"	(3)
12'-0"	(5)
16'-0"	(6)

KING STUD REQUIREMENT LISTED ABOVE DO NOT APPLY TO OPENING WHERE PORTAL FRAME IS SPECIFIED



Cedar Pointe
LOT 21



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UES PROFESSIONAL SOLUTIONS 29, INC.
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Office: 704.504.1717
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www.teamues.com



PROJECT: Piedmont - RH
First Floor Framing

CLIENT: Smith Douglas Homes - Raleigh
2520 Reliance Ave.
Apex, NC 27539

DRAWING

DATE: 05/02/2024

SCALE: 1/8"=1'-0"

PROJECT #: A20117.000403.000

DRAWN BY: EMB

CHECKED BY: GWS

ORIGINAL INFORMATION

PROJECT #
3832379

DATE
10/19/20

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S3.2

TRUSS UPLIFT CONNECTOR SCHEDULE			
MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO END
600 LBS	H2.5A	PER WALL SHEATHING & FASTENERS	
1200 LBS	(2) H2.5A	C516 (END = 11')	DTT2Z
1450 LBS	HT520	C516 (END = 11')	DTT2Z
2000 LBS	(2) MT520	(2) C516 (END = 11')	DTT2Z
2900 LBS	(2) HT520	(2) C516 (END = 11')	HTT4
3685 LBS	LGT3-SD625	M5TC52	HTT4
1. ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE, EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS. 2. UPLIFT VALUES LISTED ARE FOR SYP #2 GRADE MEMBERS AND INCLUDE ADDITIONAL WITHDRAW STRENGTH FROM REQUIRED TRUSS TO TOP PLATE TOE WALLING PER CHAPTER 6 OF THE NCRC. 3. REFER TO TRUSS LAYOUT PER MANUFACTURER FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS, CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE. 4. TRUSS MANUFACTURER IS RESPONSIBLE FOR VERIFYING CONNECTORS SATISFIES ALL TRUSS BEARING REQUIREMENTS. 5. CONTACT UES FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.			

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION R602.11.11. WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC. REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED/REVISED ON 04/02/23. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UES PROFESSIONAL SOLUTIONS, INC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. UES CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

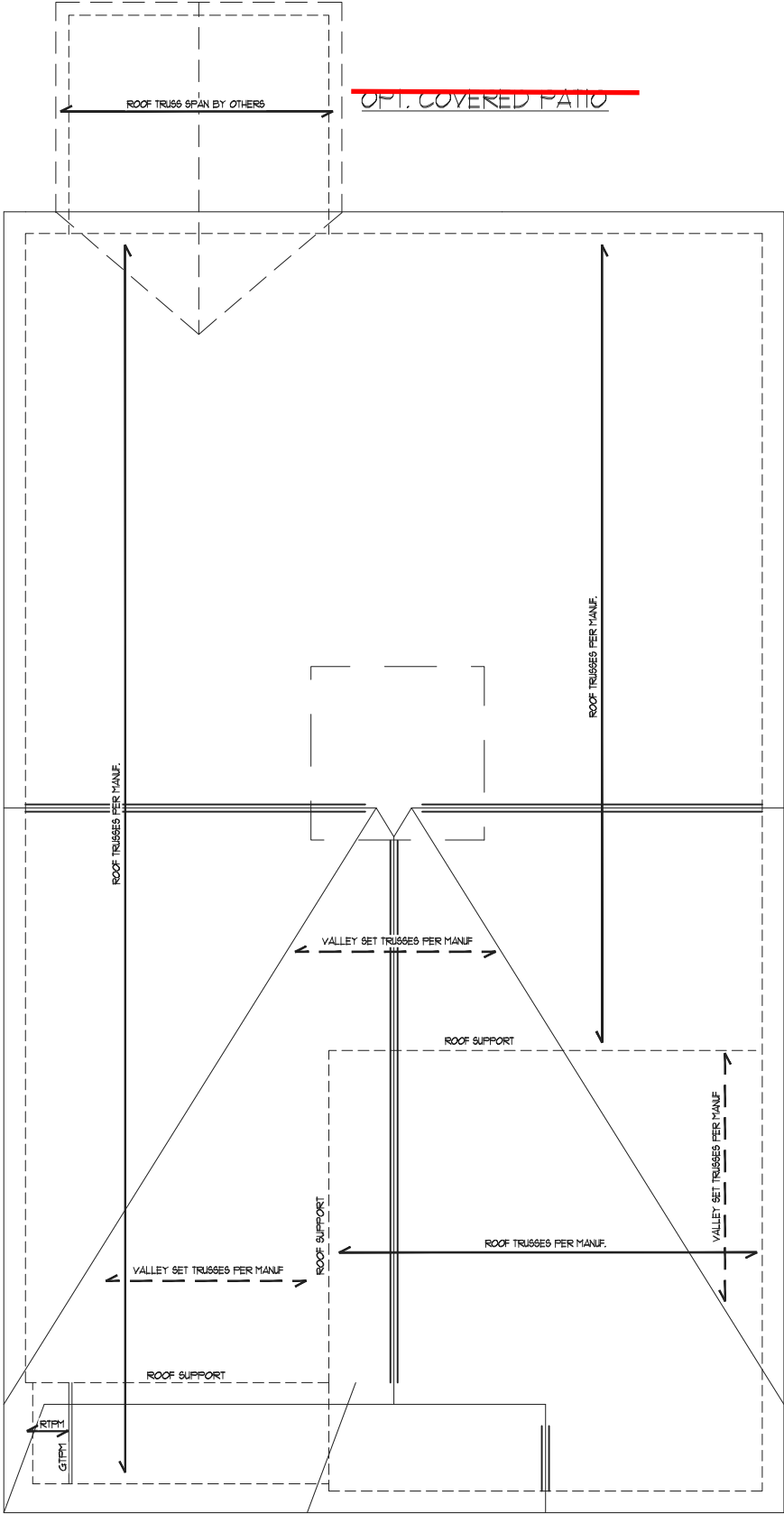
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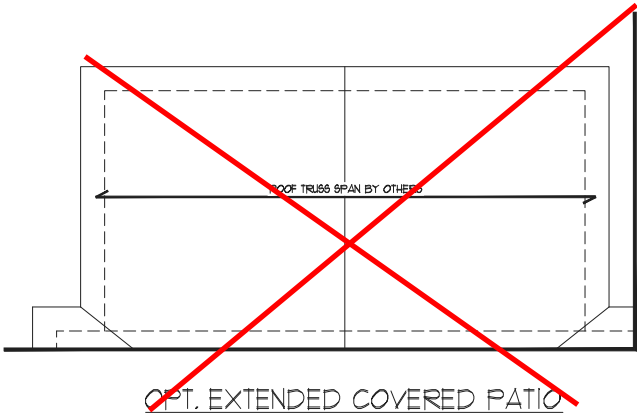
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN

SCALE: 1/8"=1'



ELEVATIONS C.F.I.



Cedar Pointe
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Pineville, NC 28134
Office: 704.504.1717
Fax: 704.504.1125
www.teamues.com



PROJECT: Piedmont - RH
Roof Framing Plan
CLIENT: Smith Douglas Homes - Raleigh
2520 Reliance Ave.
Apex, NC 27539

DRAWING
DATE: 05/02/2024
SCALE: 1/8"=1'-0"
PROJECT #: A20111.00403.000
DRAWN BY: EMB
CHECKED BY: GWS

ORIGINAL INFORMATION
PROJECT # 3832379
DATE 10/19/20

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S5.2

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, Inc. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones above 130mph)
- Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- Crawl spaced to be graded level and clear of all debris
- Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
3.1. Footings: 5%
3.2. Exterior Slabs: 5%
- No admixtures shall be added to any structural concrete without written permission of the SER
- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of W.W.F.

CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
2.1. E = 2,000,000 psi
2.2. Fb = 2600 psi
2.3. Fv = 285 psi
2.4. Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWP standard C-15. All other moisture exposed wood shall be treated in accordance with AWP standard C-2
- Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- Exterior and load bearing stud walls are to be 2x4 SPF#2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one 10d nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached wth (3)10d nails @ 24" O.C.
- Fitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

WOOD TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

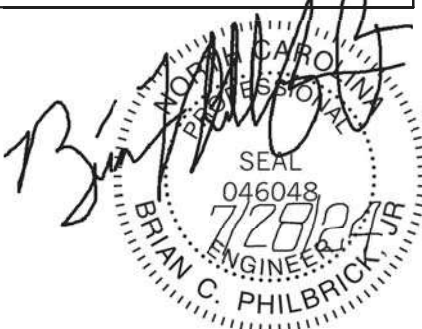
- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

EXTERIOR WOOD FRAMED DECKS:

- Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

STRUCTURAL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shopt and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.



STRUCTURAL MEMBERS ONLY



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10121 Pineville Distribution St
Pineville, NC 28134
Office: 704.504.1717
Fax: 704.504.1125
www.teamues.com



PROJECT

Standard Details

Notes and Specifications

CLIENT

Smith Douglas Homes

110 Village Trail, Suite 215

Woodstock, GA 30188

CURRENT DRAWING

DATE: 07/23/2024

SCALE: NTS

PROJECT #: A24117.01650.000

DRAWN BY: MGC

CHECKED BY: GWS

ORIGINAL DRAWING

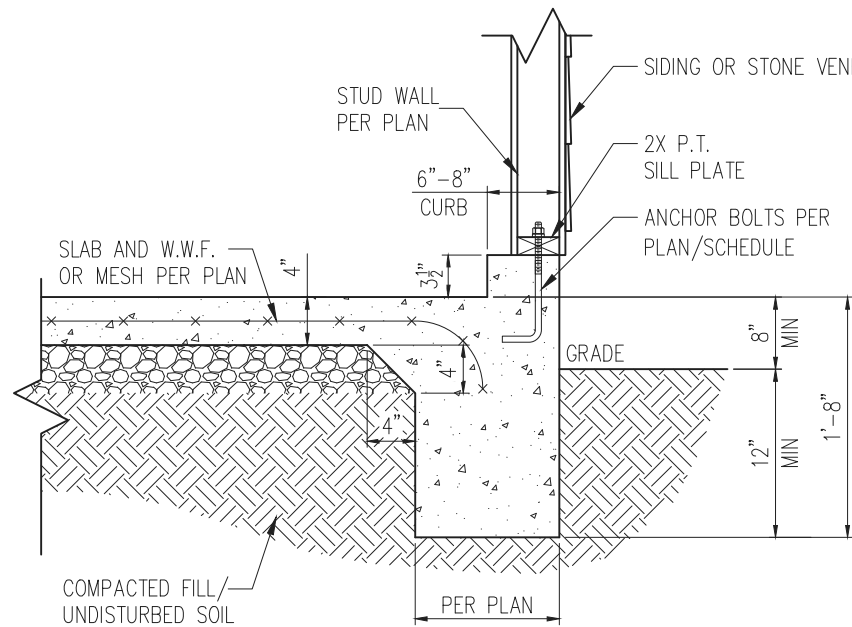
DATE PROJECT #

7/23/2024 A24117.01650.000

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

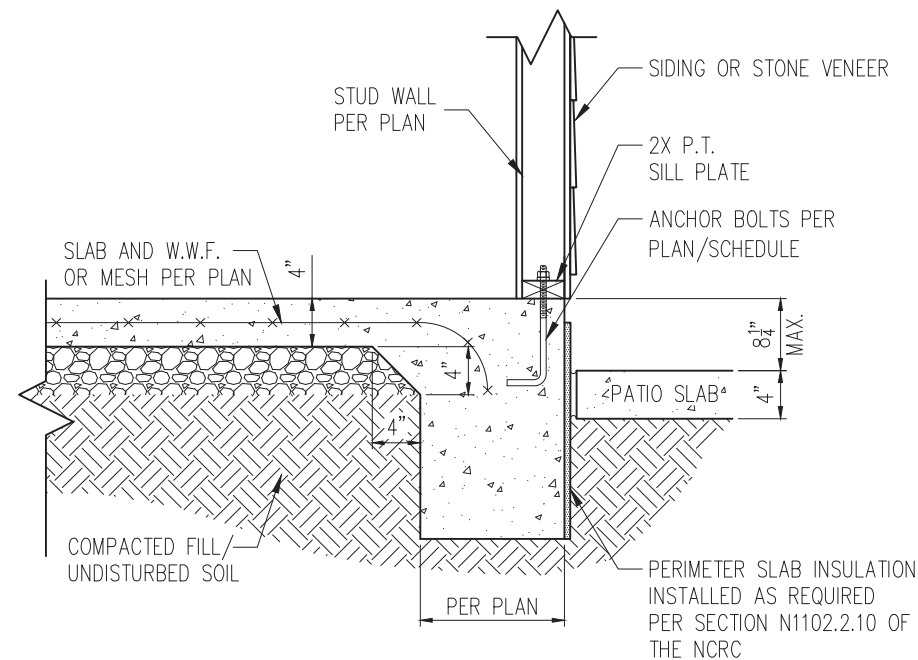
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CS1



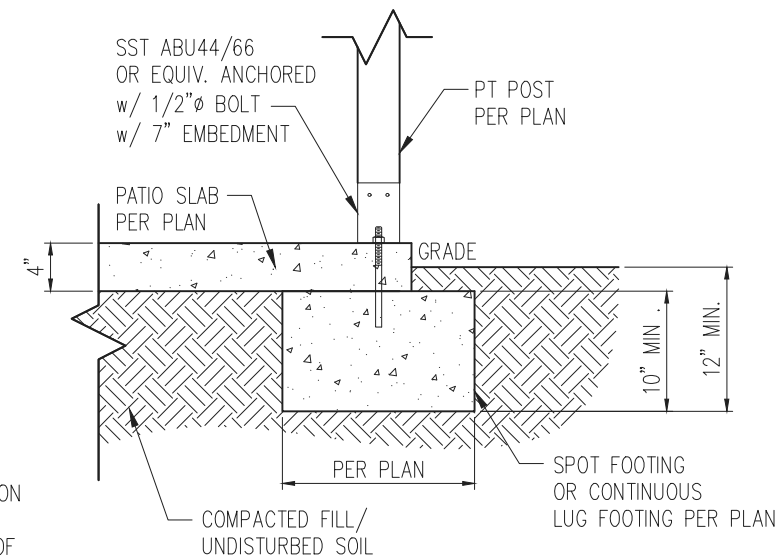
STANDARD – SIDING/STONE

1 TYP. GARAGE CURB DETAIL
D1m 3/4" = 1'-0"

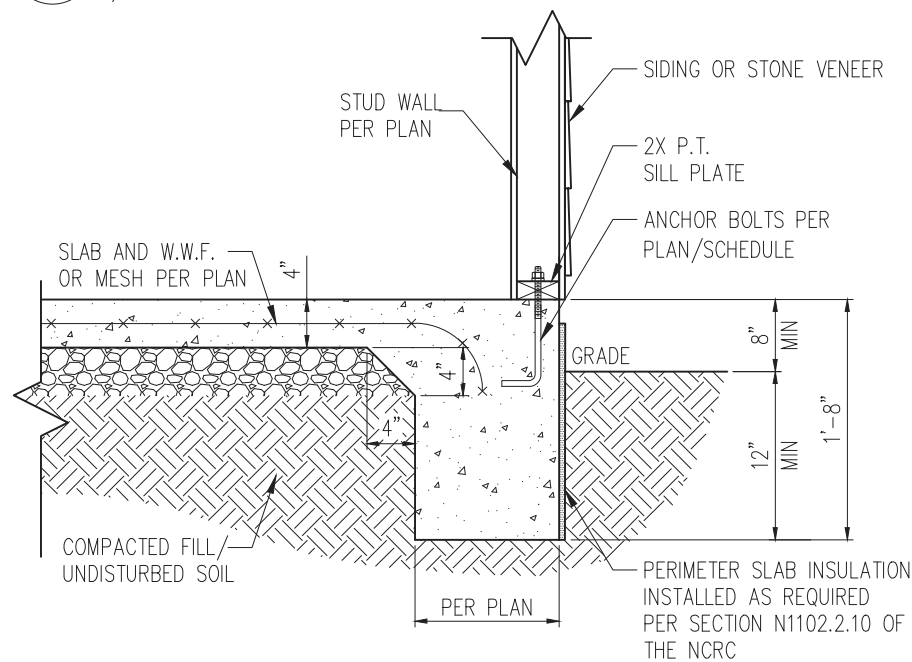


STANDARD – SIDING/STONE

2 PATIO SLAB DETAIL
D1m 3/4" = 1'-0"

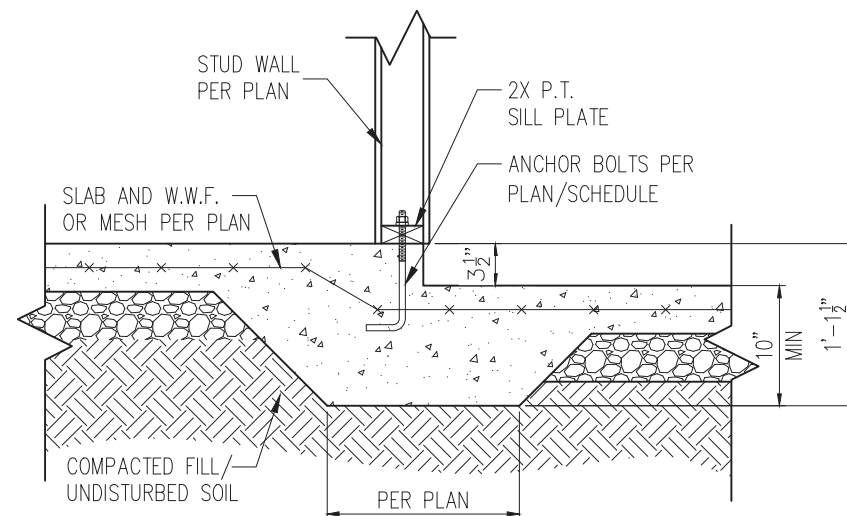


2A COVERED PATIO DETAIL
D1m 3/4" = 1'-0"



STANDARD – SIDING/STONE

3 TYP. SLAB DETAIL
D1m 3/4" = 1'-0"



4 STEP IN GARAGE
D1m 3/4" = 1'-0"

MONOLITHIC FOOTING WIDTH

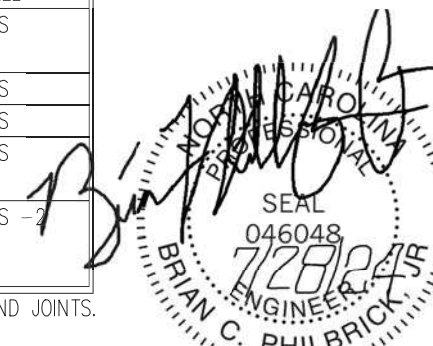
# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY		
	1500 PSF	2000 PSF	2500 PSF
1 STORY – STD.	16"	16"	16"
1 STORY – BRICK VENEER	21"	21"	21"
2 STORY – STD.	20"	16"	16"
2 STORY – BRICK VENEER	25"	21"	21"

*5" BRICK LEDGE HAS BEEN ADDED TO THE MONOLITHIC FOOTING WIDTH FOR BRICK SUPPORT

WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC. EMBEDMENT	SPACING	INTERIOR WALL	EXTERIOR WALL
1/2"Ø A307 BOLTS w/ STD. 90° BEND	7"	6'-0"	YES	YES
SST – MAS	4"	5'-0"	NO	YES
SIMPSON TITEN HD 1/2"Ø – 8"	6-1/2"	6'-0"	YES	YES
1/2"Ø HILTI THREADED ROD w/ HIT HY150 ADHESIVE	7"	6'-0"	YES	YES
1/2"Ø HILTI KWIK BOLT, SST WEDGE-ALL, OR EQUIVALENT WEDGE ANCHORE	7"	6'-0"	YES	YES – 2

NOTE: 1. INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.
2. EXPANSION ANCHORS MAY BE INSTALLED ONLY AS ALLOWED PER MANUFACTURER SPECIFICATIONS.



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PROJECT

Standard Details
Monolithic Slab Details
CLIENT
Smith Douglas Homes
110 Village Trail, Suite 215
Woodstock, GA 30188

CURRENT DRAWING

DATE: 07/23/2024

SCALE: NTS

PROJECT #: A24117.01650.000

DRAWN BY: MGC

CHECKED BY: GWS

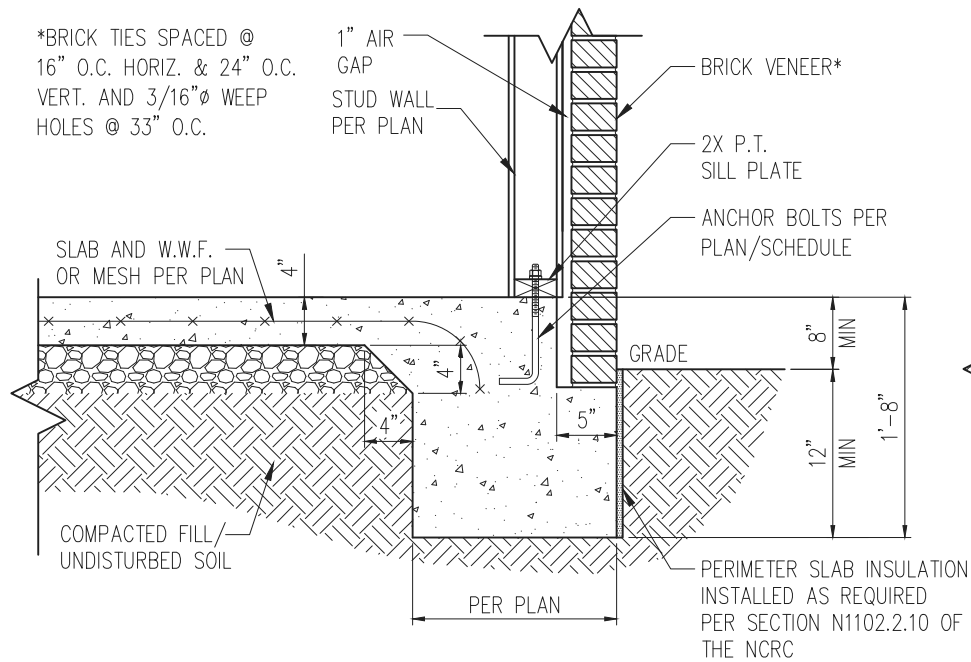
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DATE PROJECT #
7/23/2024 A24117.01650.000

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

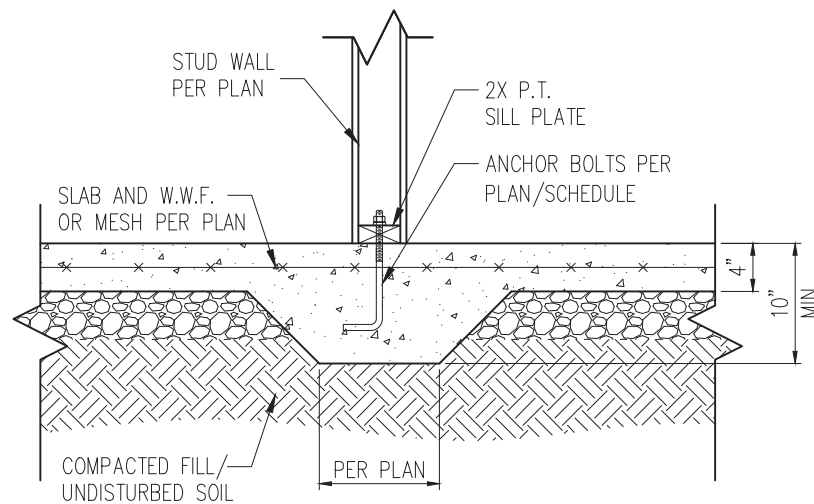
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D1m

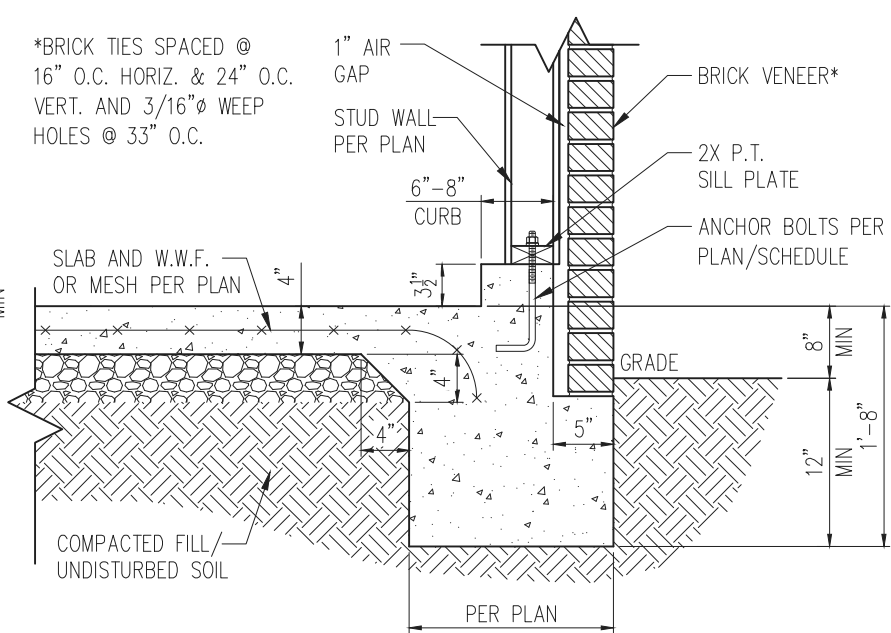


STANDARD – BRICK

1 TYP. SLAB DETAIL W/ BRICK VENEER
D2m 3/4" = 1'-0"

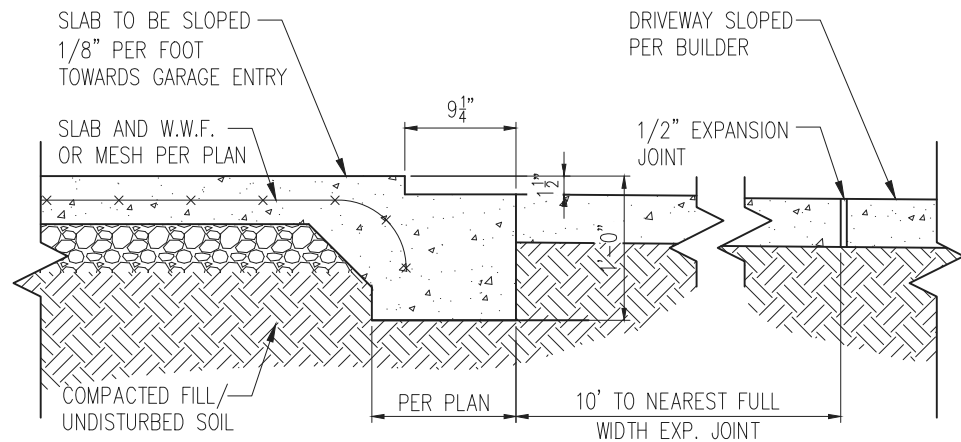


3 TYP. THICKENED SLAB DETAIL
D2m 3/4" = 1'-0"

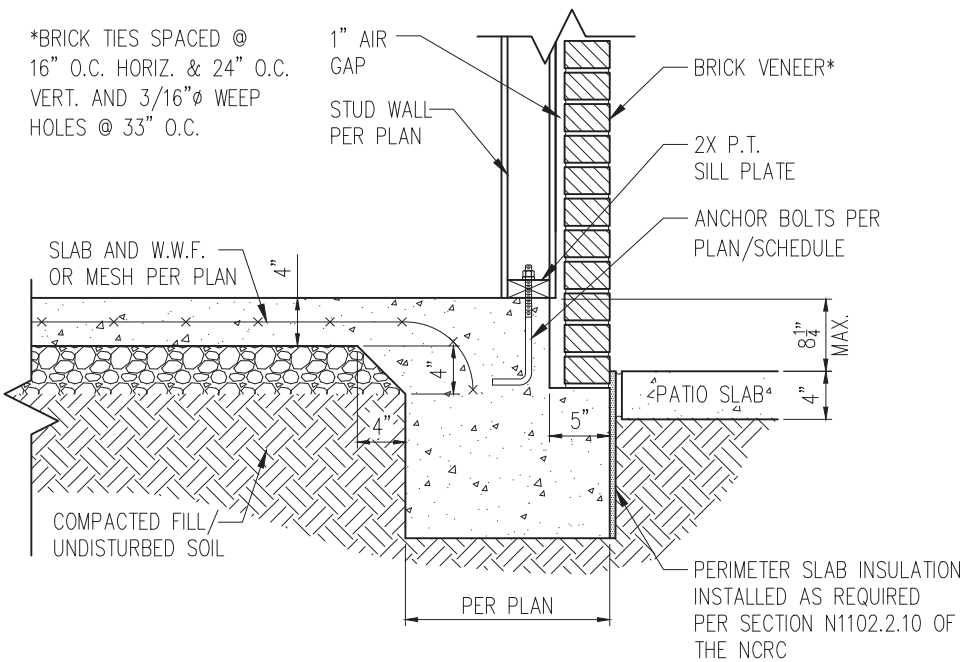


STANDARD – BRICK

5 TYP. GARAGE CURB DETAIL W/ BRICK VENEER
D2m 3/4" = 1'-0"



2 SLAB AT GARAGE DOOR
D2m 3/4" = 1'-0"



STANDARD – BRICK

4 PATIO SLAB DETAIL W/ BRICK VENEER
D2m 3/4" = 1'-0"

NOTES:

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



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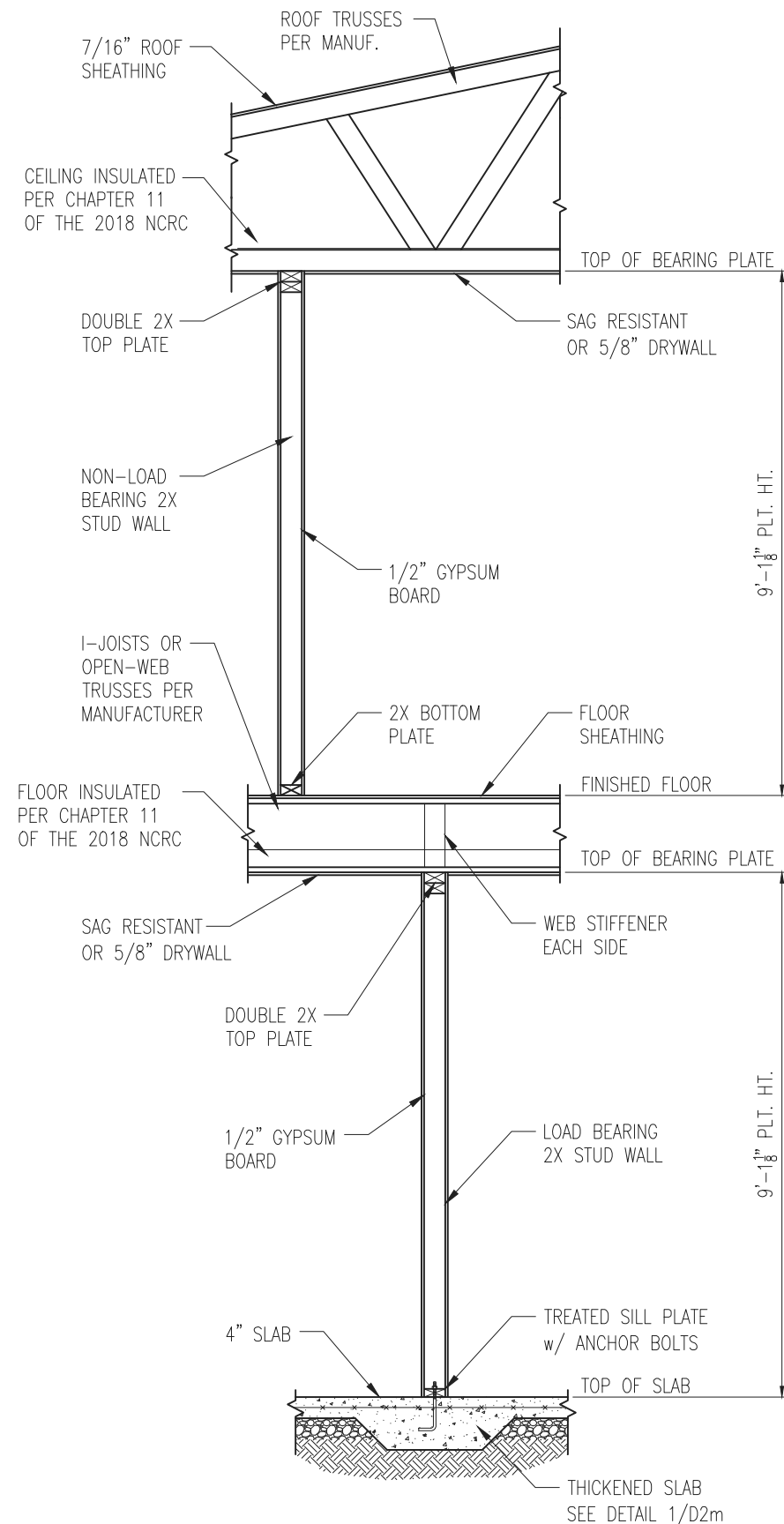
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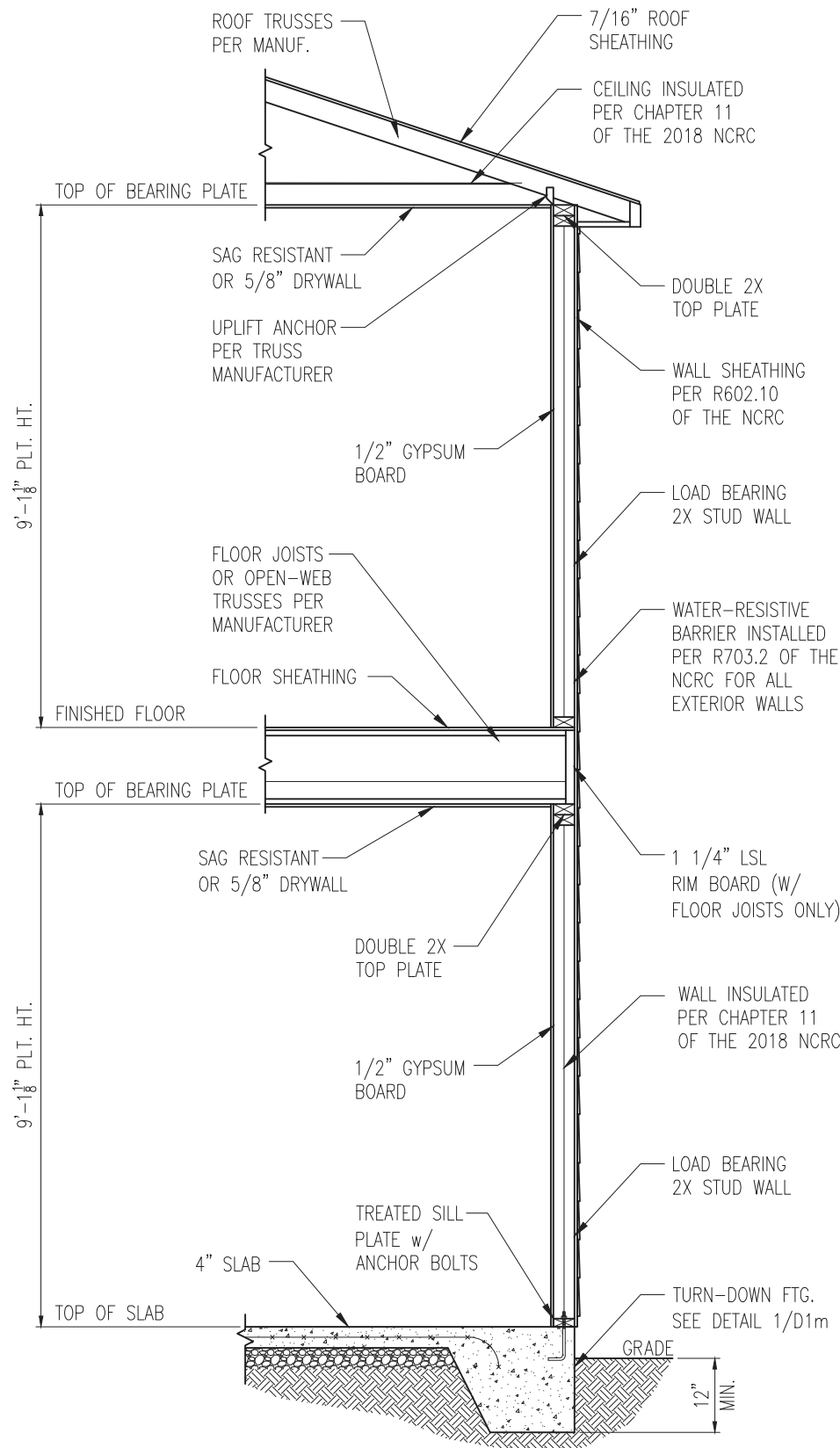
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

D2m

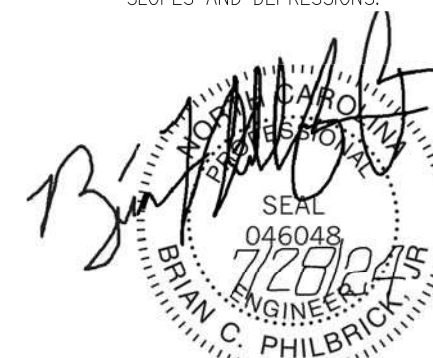


1 TYP. INTERIOR LOAD BEARING WALL SECTION
D3m 3/4" = 1'-0"



2 TYP. EXTERIOR LOAD BEARING WALL SECTION
D3m 3/4" = 1'-0"
-SIMILAR w/ BRICK AND STONE
-BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT.
-MIN. 3/16"Ø WEEP HOLES @ 33" O.C.

- NOTES:
1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



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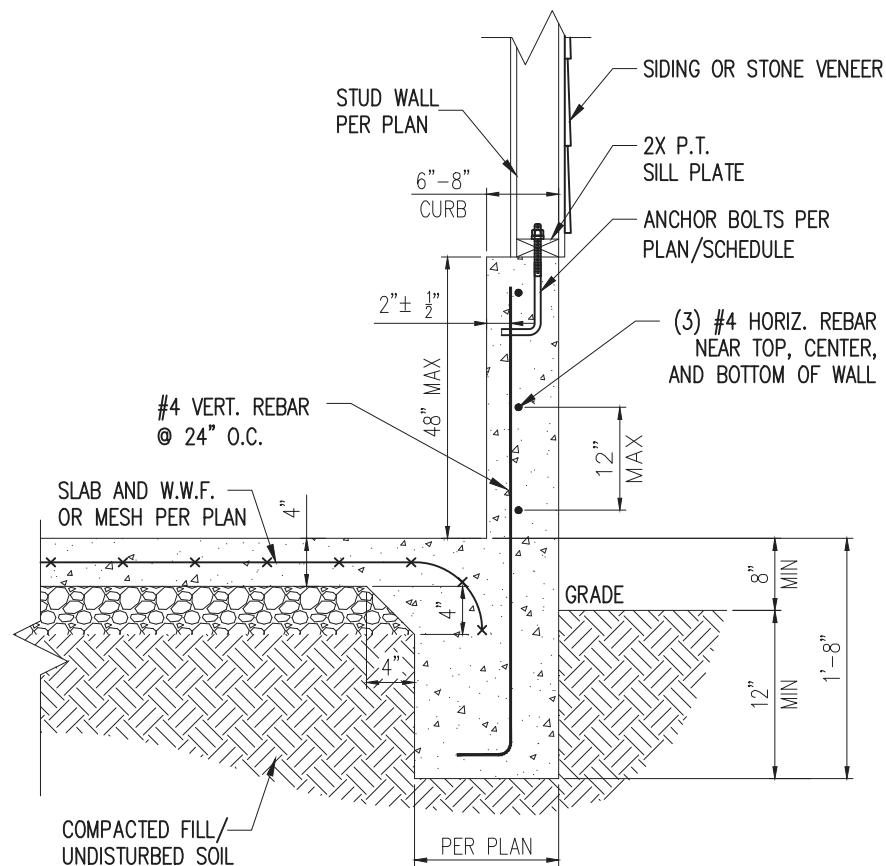
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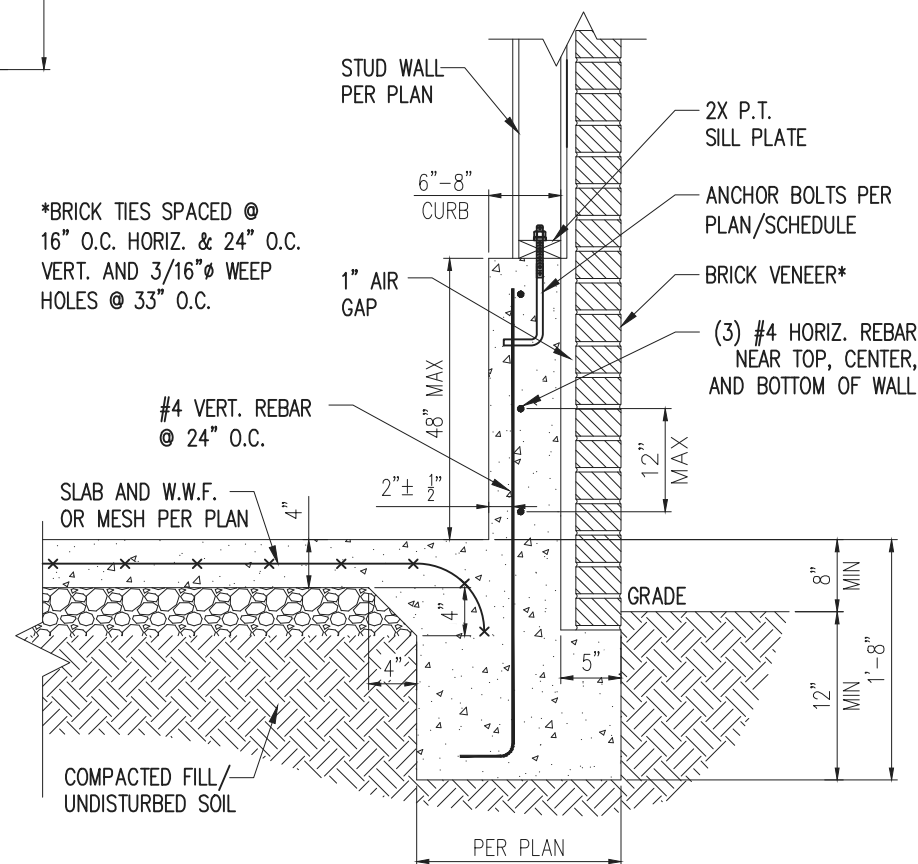
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D3m



STANDARD - SIDING/STONE

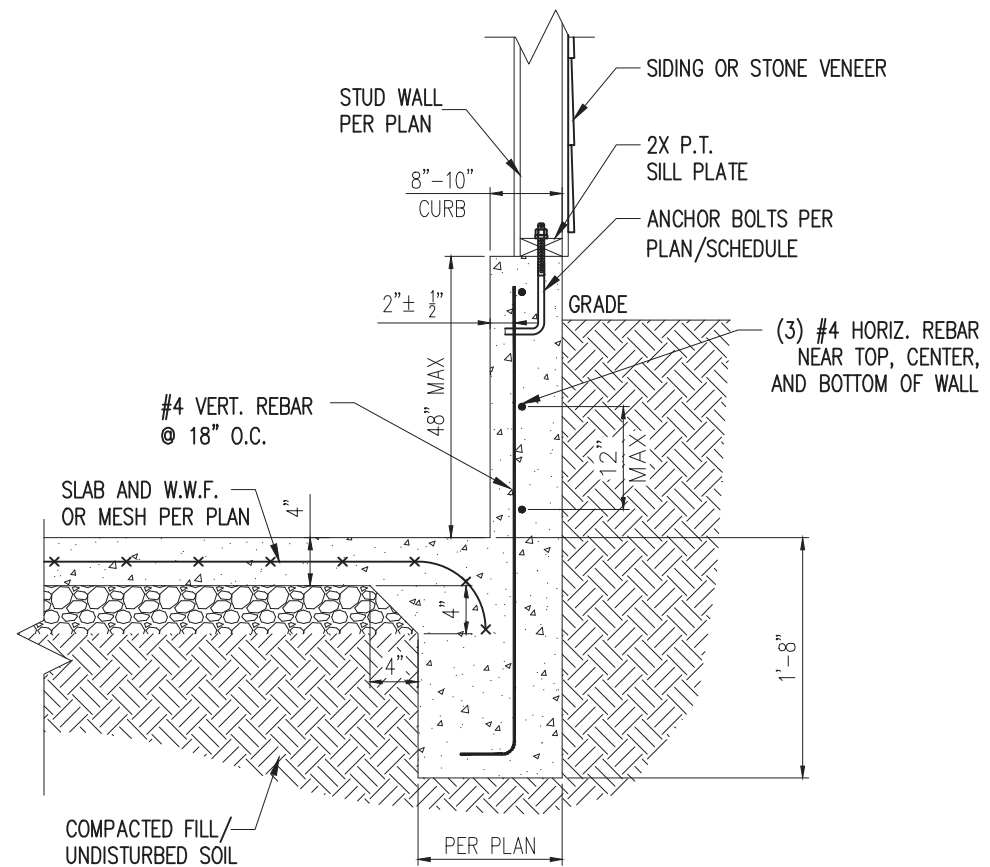
1 EXTENDED GARAGE CURB DETAIL
D4m NTS

*BRICK TIES SPACED @
16" O.C. HORIZ. & 24" O.C.
VERT. AND 3/16" Ø WEEP
HOLES @ 33" O.C.



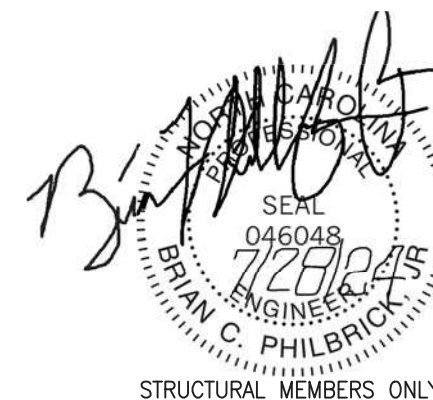
STANDARD - BRICK

3 EXTENDED GARAGE CURB DETAIL
D4m NTS
W/ BRICK VENEER



STANDARD - SIDING/STONE

2 EXTENDED GARAGE CURB DETAIL
D4m NTS
W/ UNBALANCED FILL

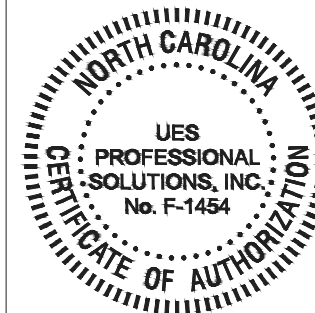


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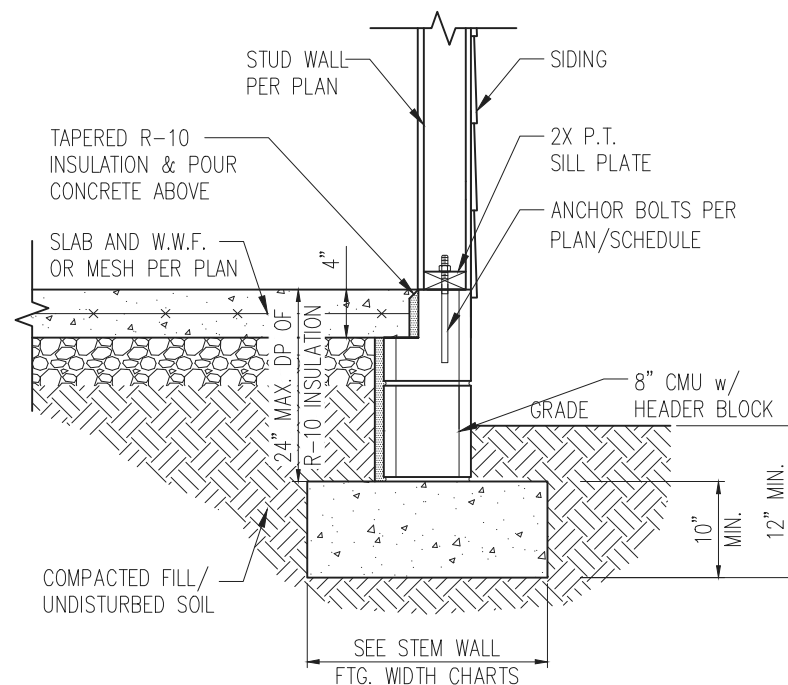
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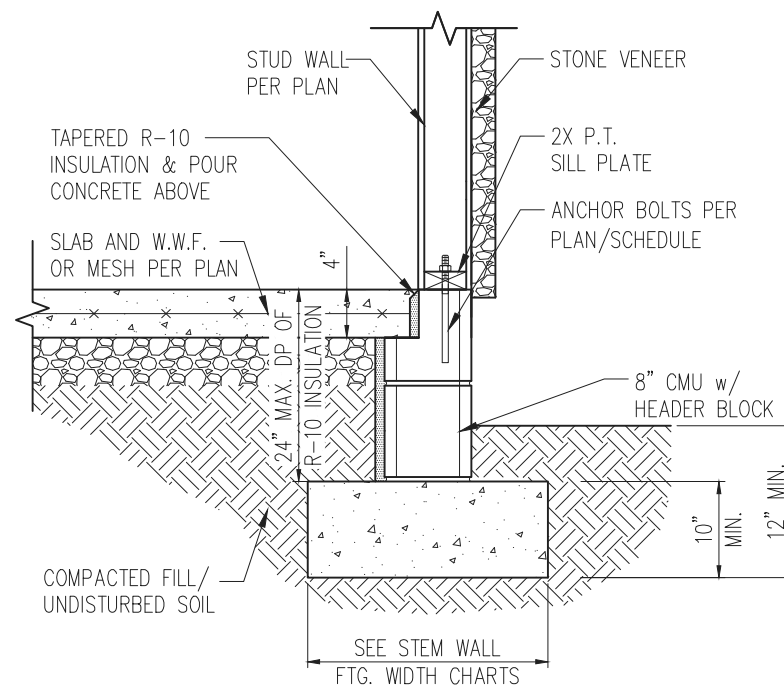
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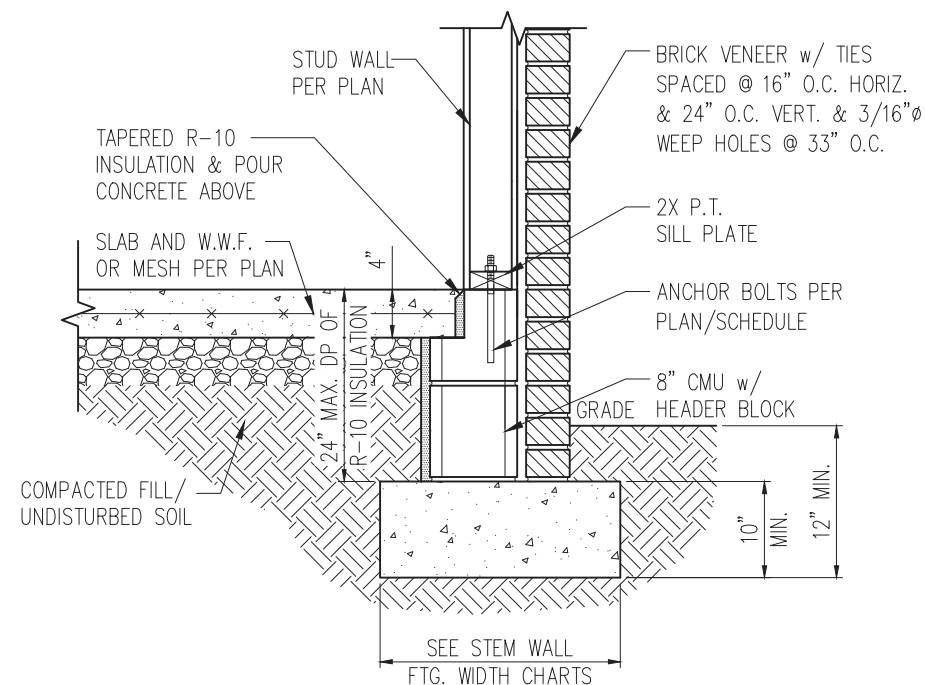
D4m



STANDARD – SIDING

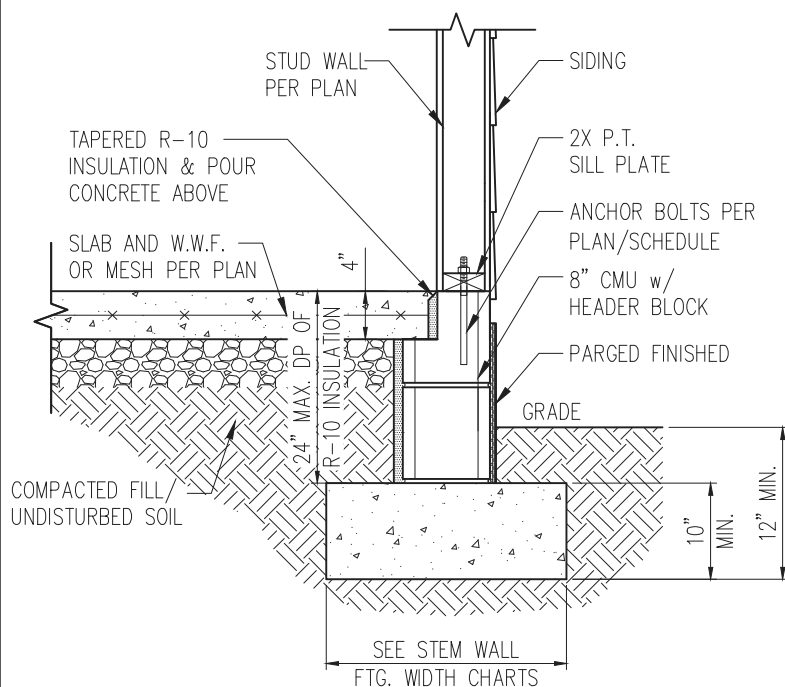


STANDARD – STONE

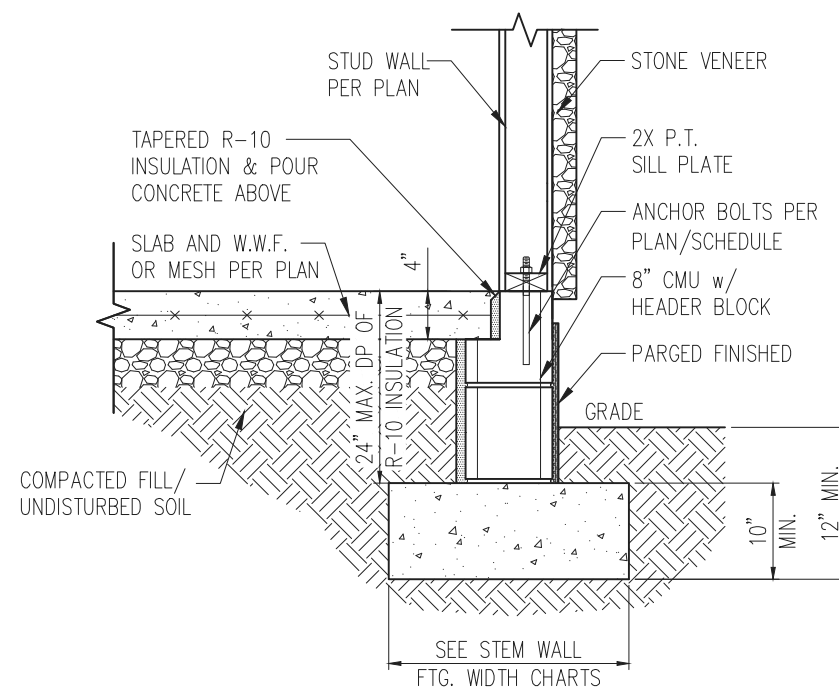


STANDARD – BRICK

1 TYP. STEM WALL DETAIL
D1s 3/4" = 1'-0"



STANDARD – SIDING



STANDARD – STONE

1a STEM WALL DETAIL w/ PARGED FINISH
D1s 3/4" = 1'-0"

STEM WALL FOOTING WIDTH

# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY		
	1500 PSF	2000 PSF	2500 PSF
1 STORY – STD.	16"	16"	16"
1 STORY – BRICK VENEER	21"*	21"*	21"*
2 STORY – STD.	20"	16"	16"
2 STORY – BRICK VENEER	25"*	21"*	21"*

*5" BRICK LEDGE HAS BEEN ADDED TO THE STEM WALL FOOTING WIDTH FOR BRICK SUPPORT

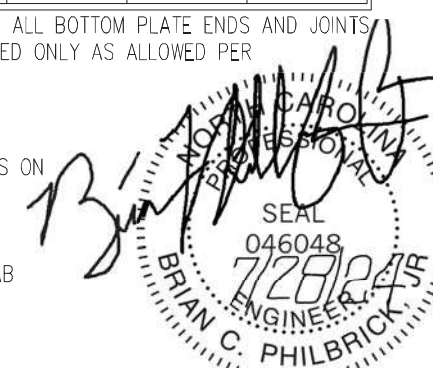
WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC. EMBEDMENT	SPACING	INTERIOR WALL	EXTERIOR WALL
1/2" Ø A307 BOLTS w/ STD. 90° BEND	7"	6'-0"	YES	YES
SST – MAS	4"	5'-0"	NO	YES
SIMPSON TITEN HD 1/2" Ø – 8"	6-1/2"	6'-0"	YES	YES
1/2" Ø HILTI THREADED ROD w/ HIT HY150 ADHESIVE	7"	6'-0"	YES	YES
1/2" Ø HILTI KWIK BOLT, SST WEDGE-ALL, OR EQUIVALENT WEDGE ANCHORE	7"	6'-0"	YES	YES – 2

NOTE: 1. INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.
2. EXPANSION ANCHORS MAY BE INSTALLED ONLY AS ALLOWED PER MANUFACTURER SPECIFICATIONS.

NOTES:

- REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPS AND DEPRESSIONS.



STRUCTURAL MEMBERS ONLY



PROJECT
Standard Details
Stemwall Details
CLIENT
Smith Douglas Homes
110 Village Trail, Suite 215
Woodstock, GA 30188

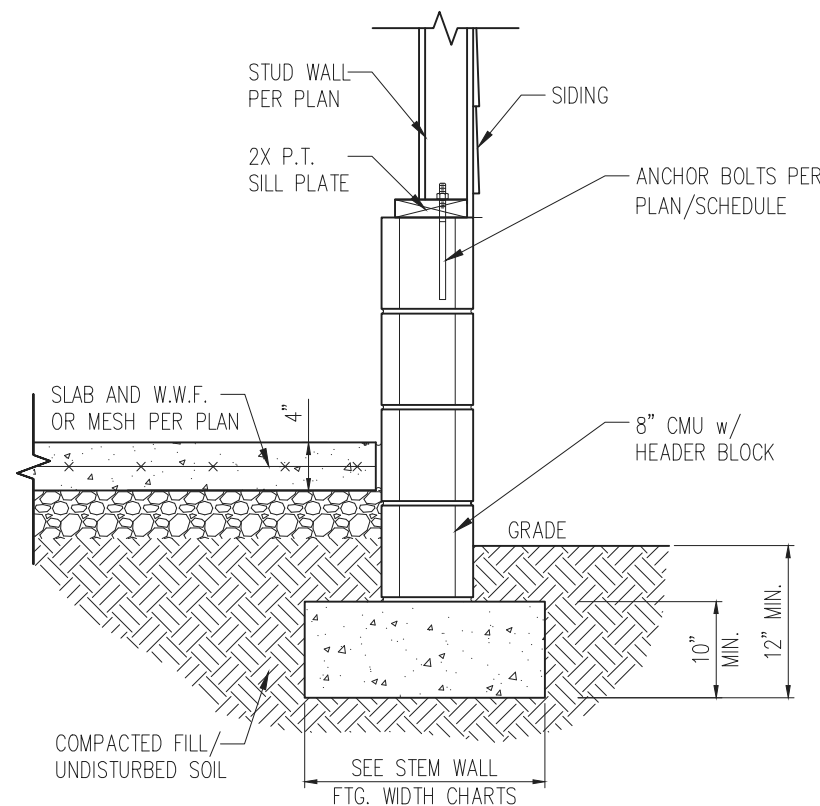
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SCALE: NTS
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DRAWN BY: MGC
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ORIGINAL DRAWING
DATE PROJECT #
7/23/2024 A24117.01650.000

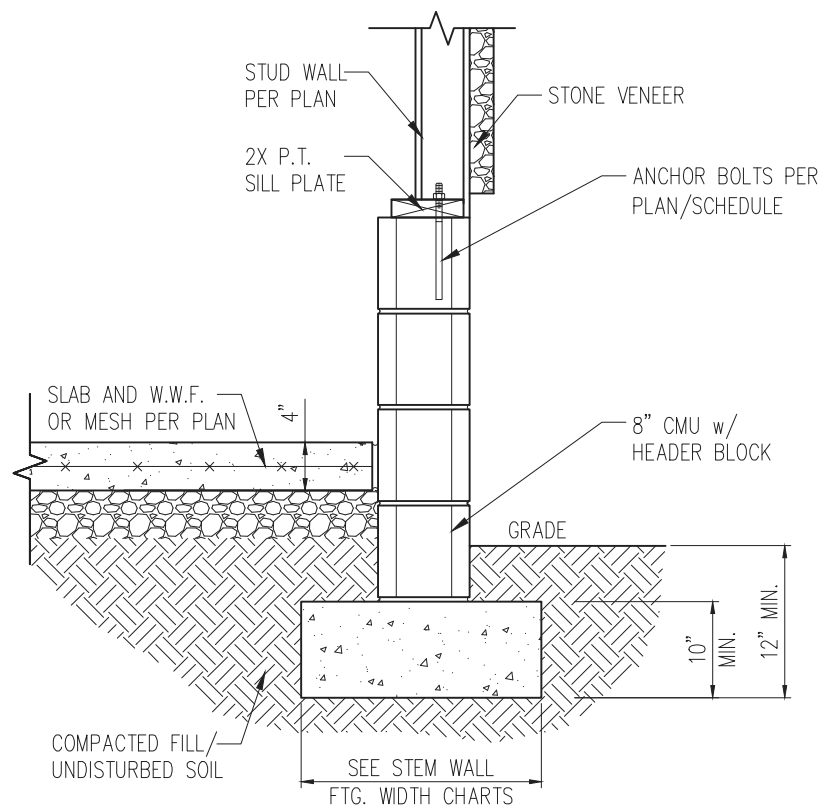
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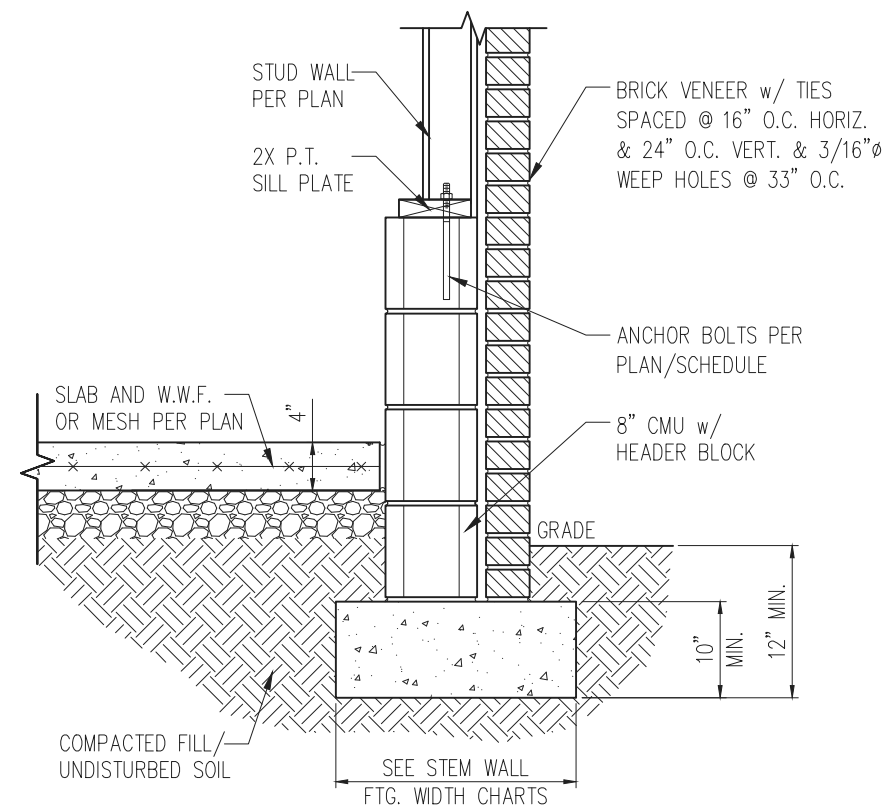
D1s



STANDARD - SIDING

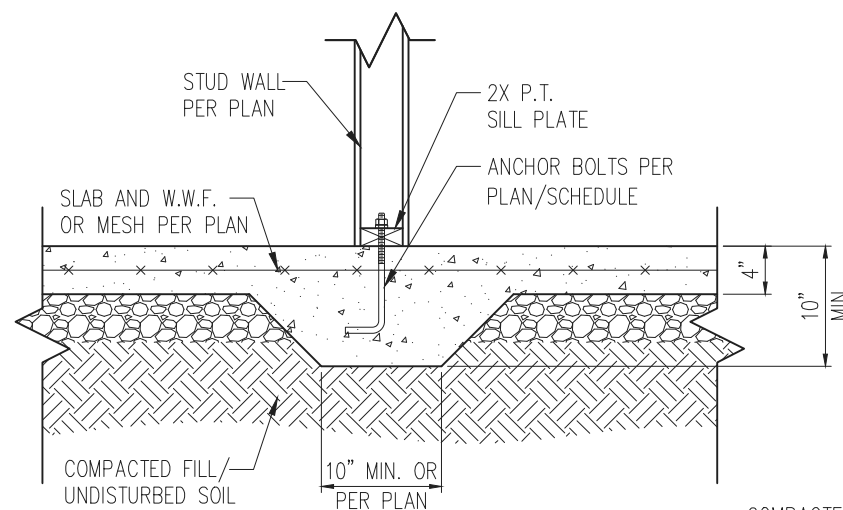


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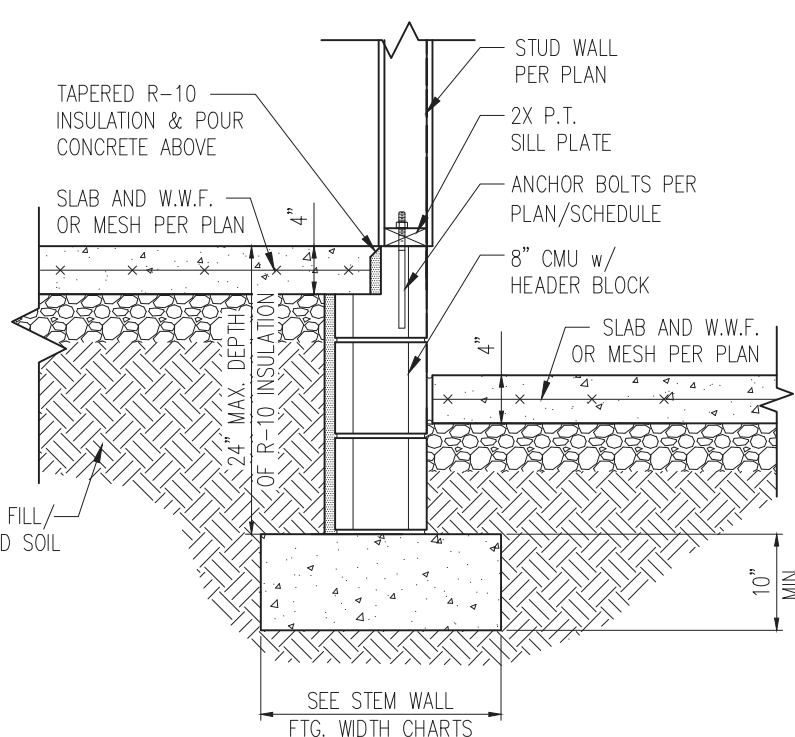


STANDARD - BRICK

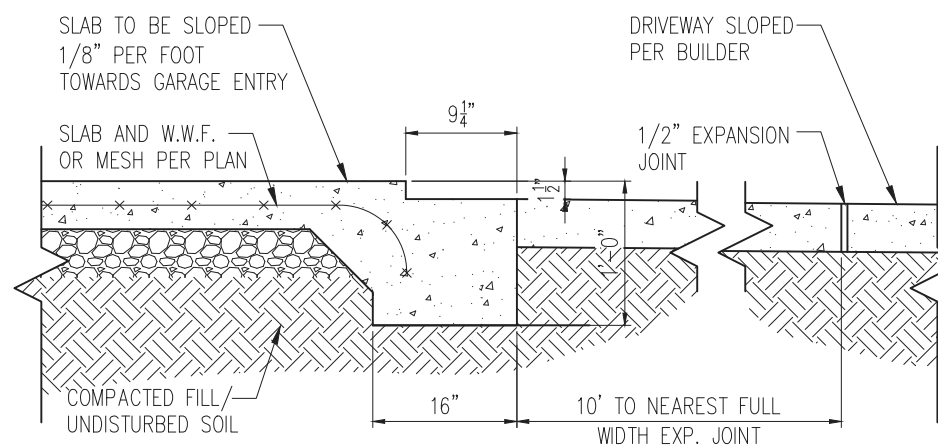
1 TYP. GARAGE CURB DETAIL
D2s 3/4" = 1'-0"



2 TYP. THICKENED SLAB DETAIL
D2s 3/4" = 1'-0"



3 HOUSE/GARAGE WALL DETAIL
D2s 3/4" = 1'-0"



4 SLAB AT GARAGE DOOR
D2s 3/4" = 1'-0"

- NOTES:
- REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
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STRUCTURAL MEMBERS ONLY



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Stemwall Details
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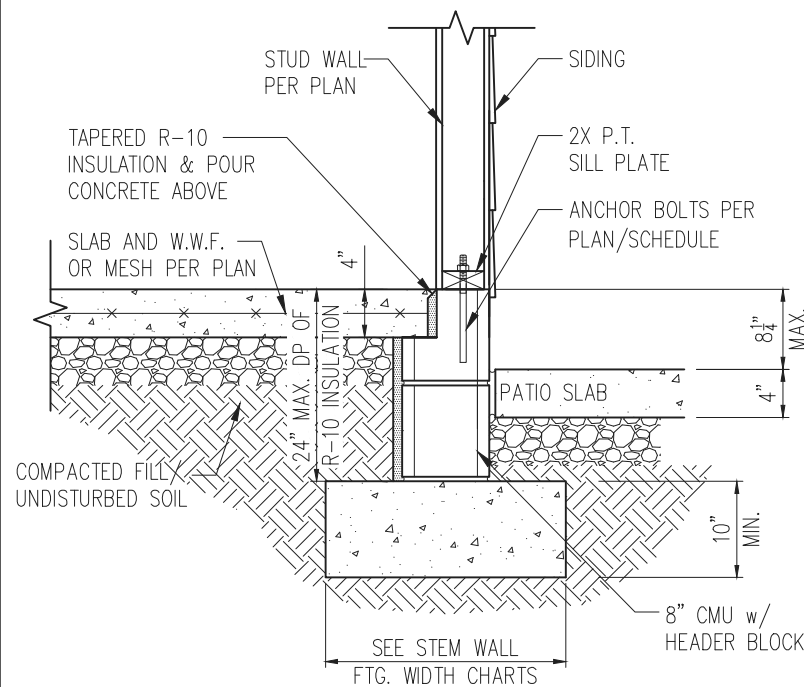
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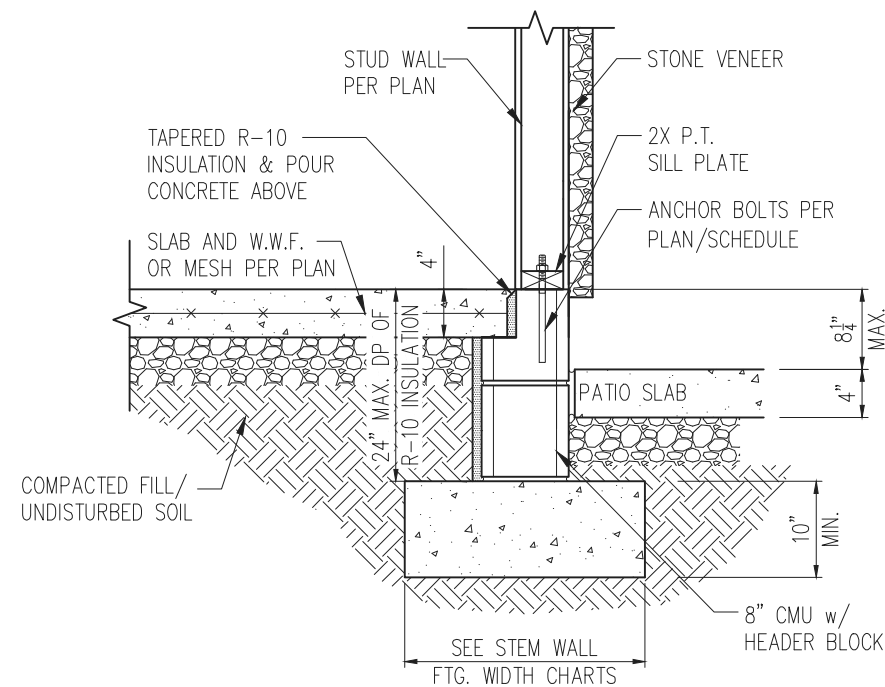
REFER TO COVER SHEET FOR A
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SHEET

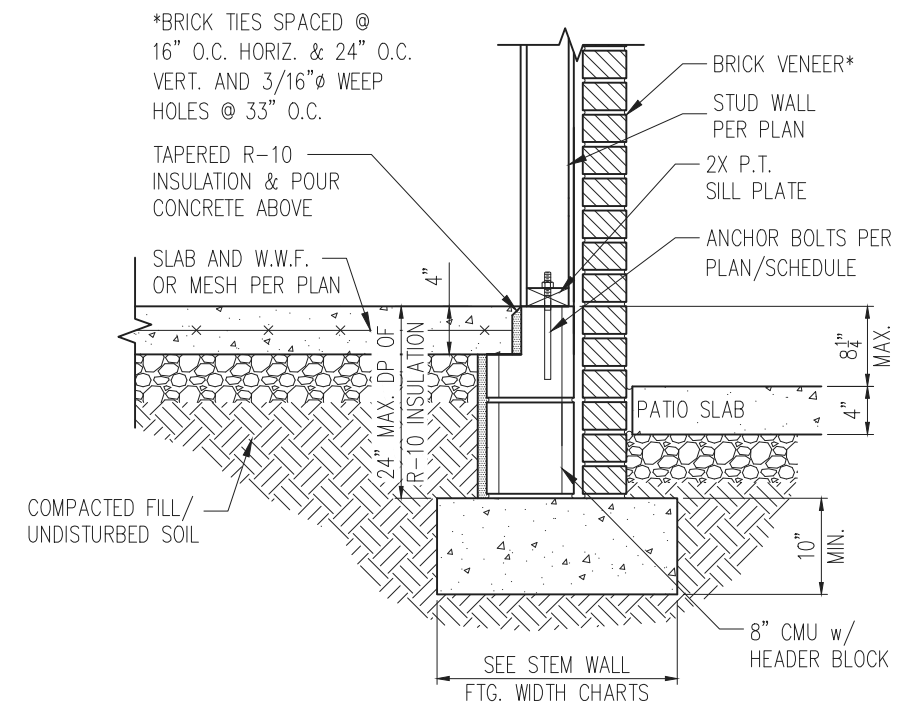
D2s



STANDARD – SIDING

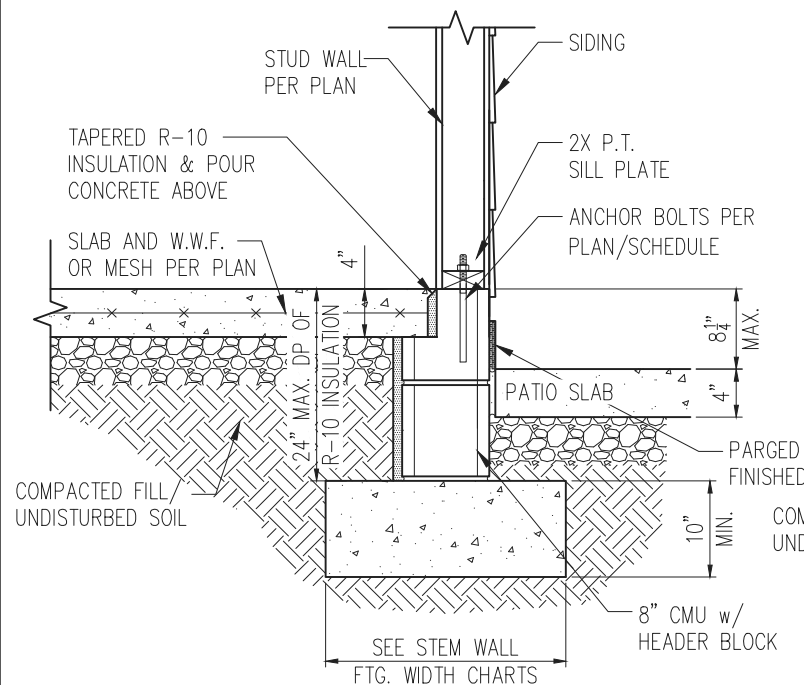


STANDARD – STONE

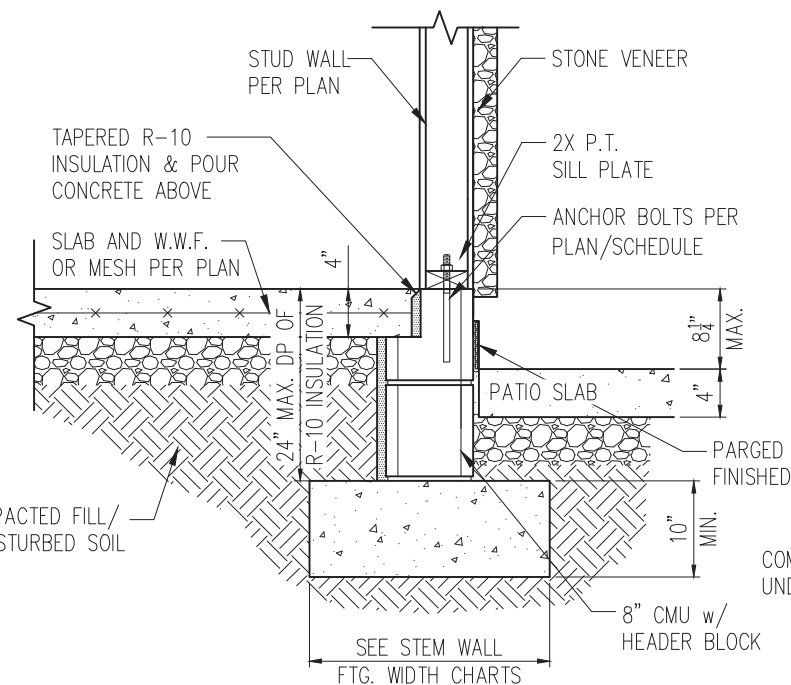


STANDARD – BRICK

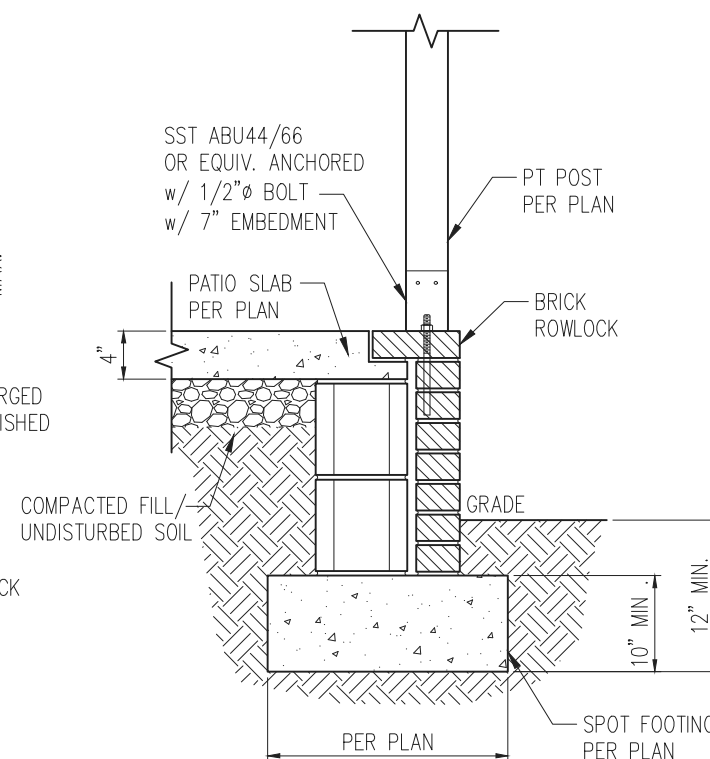
1 PORCH SLAB DETAIL
D3s 3/4" = 1'-0"



STANDARD – SIDING



STANDARD – STONE



2 COVERED PORCH DETAIL
D3s 3/4" = 1'-0"

- NOTES:
1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



STRUCTURAL MEMBERS ONLY



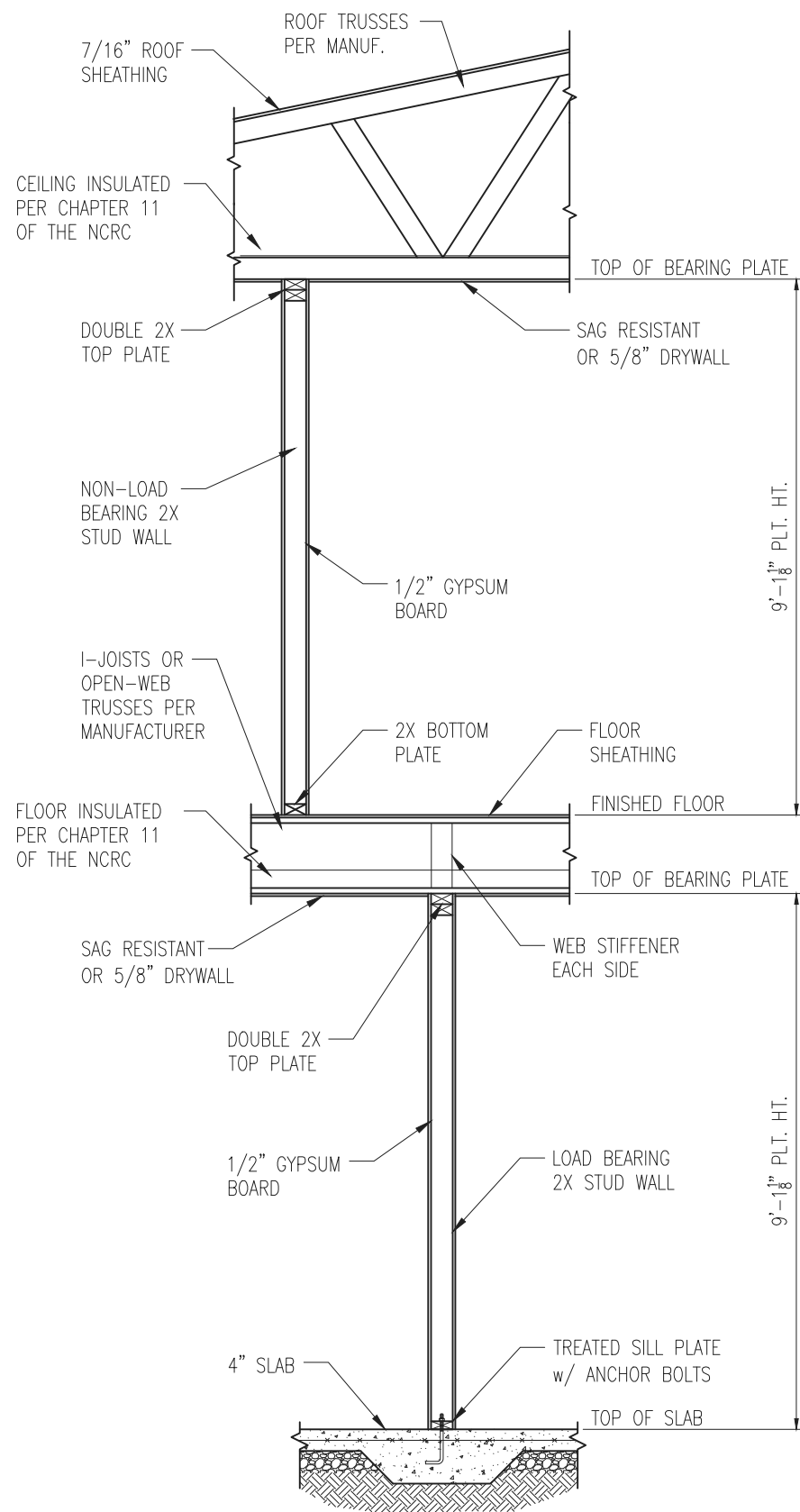
PROJECT
Standard Details
Stemwall Details
CLIENT
Smith Douglas Homes
110 Village Trail, Suite 215
Woodstock, GA 30188

CURRENT DRAWING
DATE: 07/23/2024
SCALE: NTS
PROJECT #: A24117.01650.000
DRAWN BY: MGC
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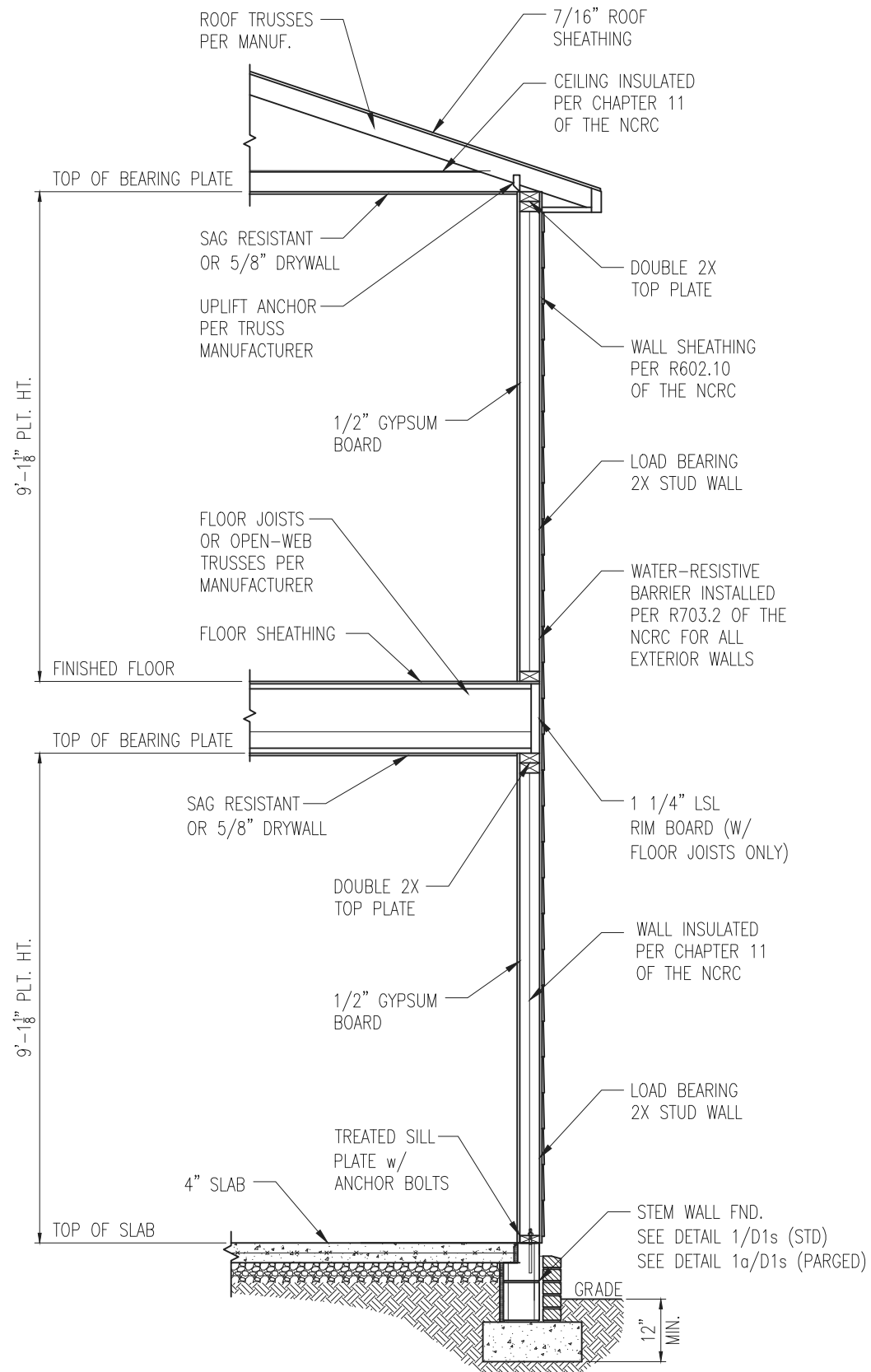
ORIGINAL DRAWING
DATE PROJECT #
7/23/2024 A24117.01650.000

REFER TO COVER SHEET FOR A
COMPLETE LIST OF REVISIONS

SHEET
D3s



1 TYP. INTERIOR LOAD BEARING WALL SECTION
D4s 3/4" = 1'-0"



2 TYP. EXTERIOR LOAD BEARING WALL SECTION
D4s 3/4" = 1'-0"

-SIMILAR w/ BRICK AND STONE
-BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT.
-MIN. 3/16"Ø WEEP HOLES @ 33" O.C.

- NOTES:
1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



STRUCTURAL MEMBERS ONLY



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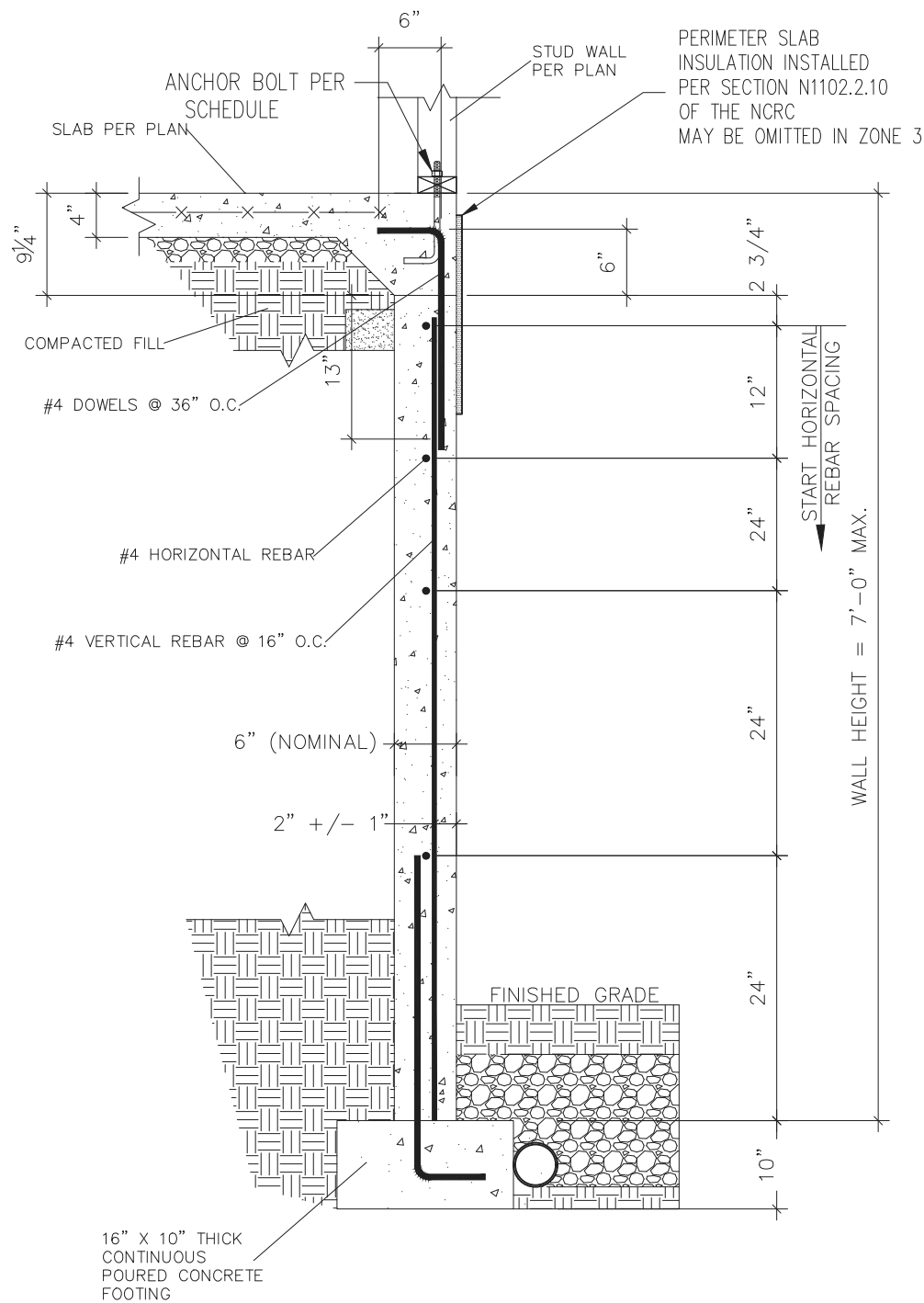
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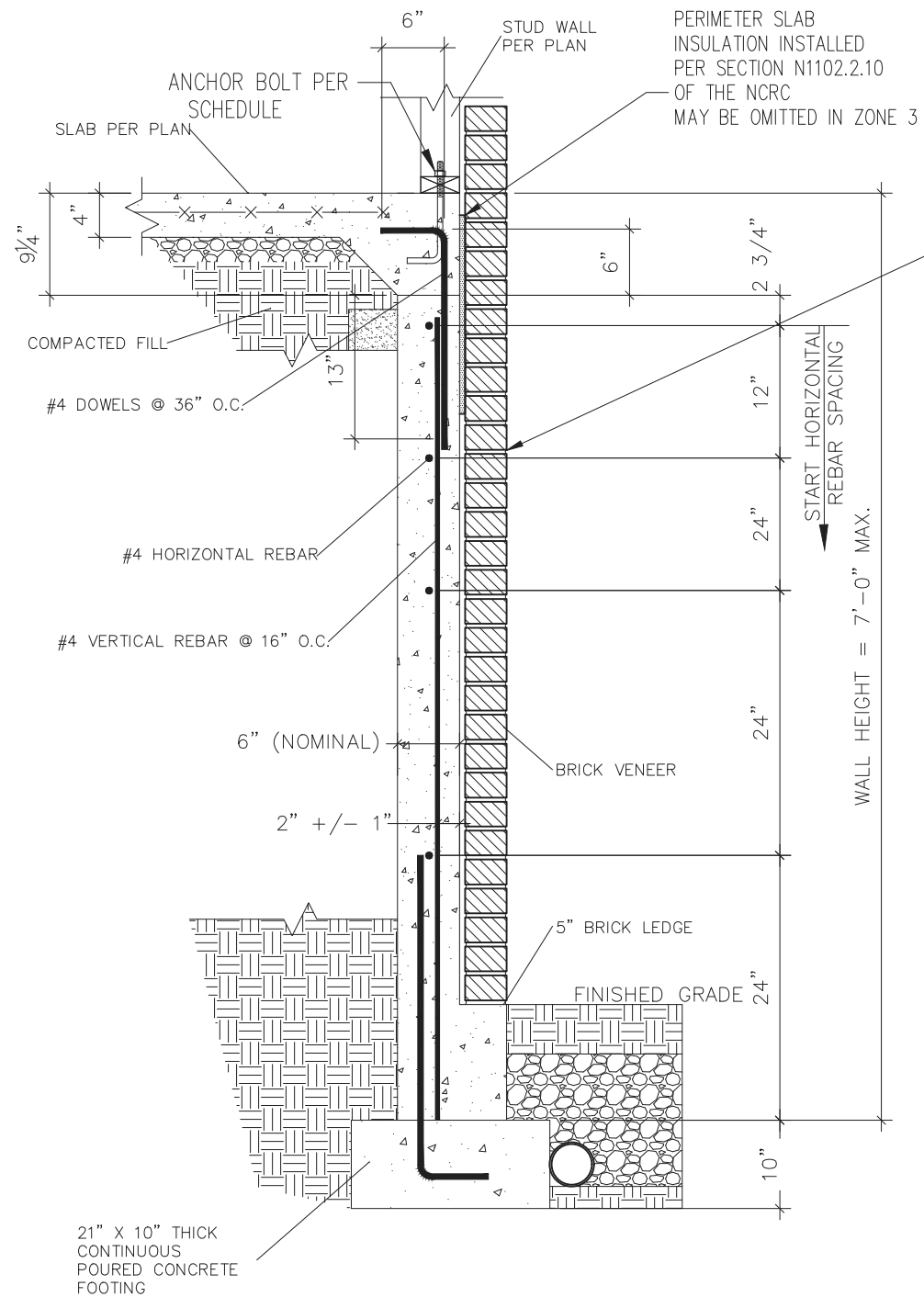
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SHEET
D4s

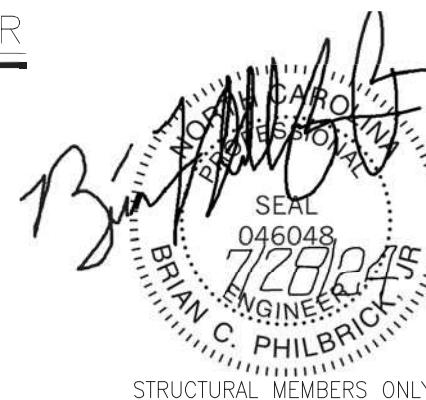


1 SUBWALL FOUNDATION
D5s 3/4" = 1'-0"



2 SUBWALL FOUNDATION W/ BRICK VENEER
D5s 3/4" = 1'-0"

PROVIDE LADDER WIRE
OR METAL TIES,
INSTALLED PER R608.1.2
OF THE NCRC, AND
FULLY GROUT BETWEEN
BRICK AND CONCRETE.



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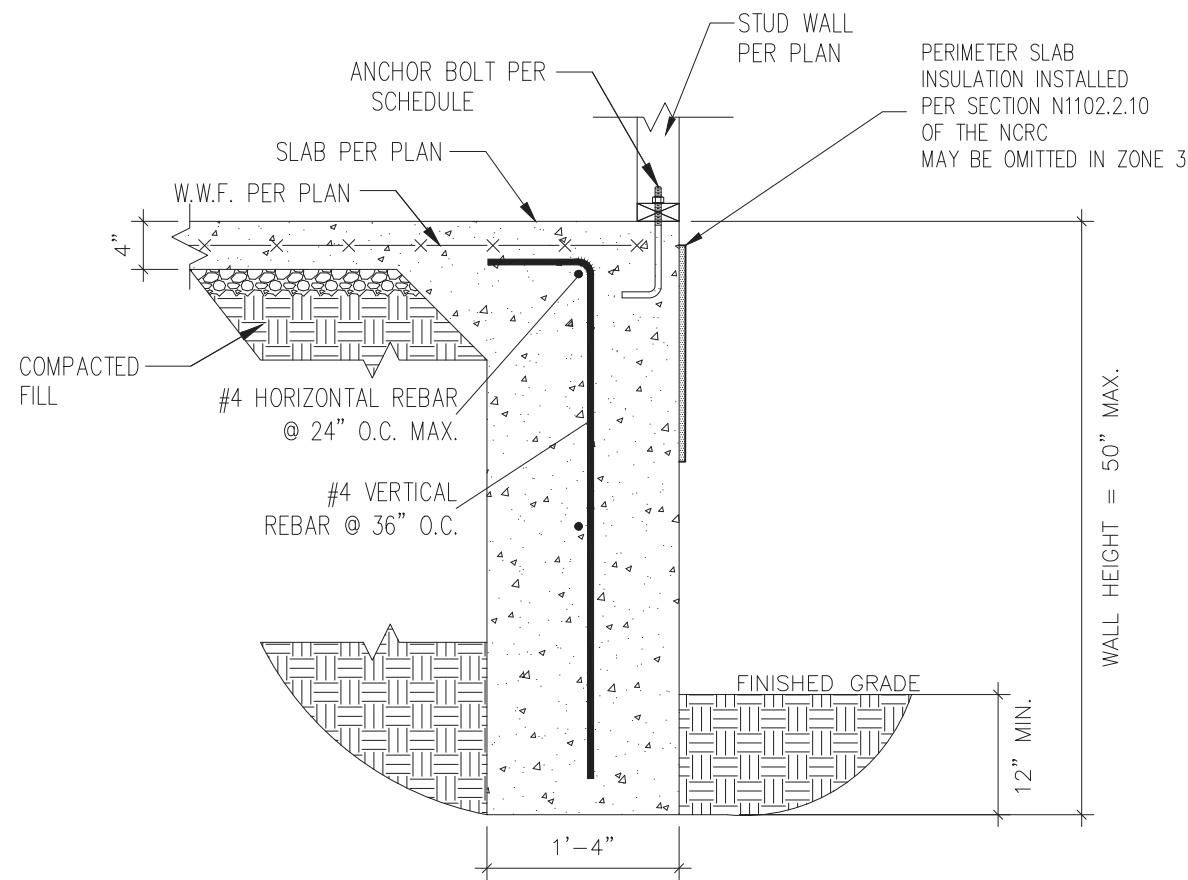
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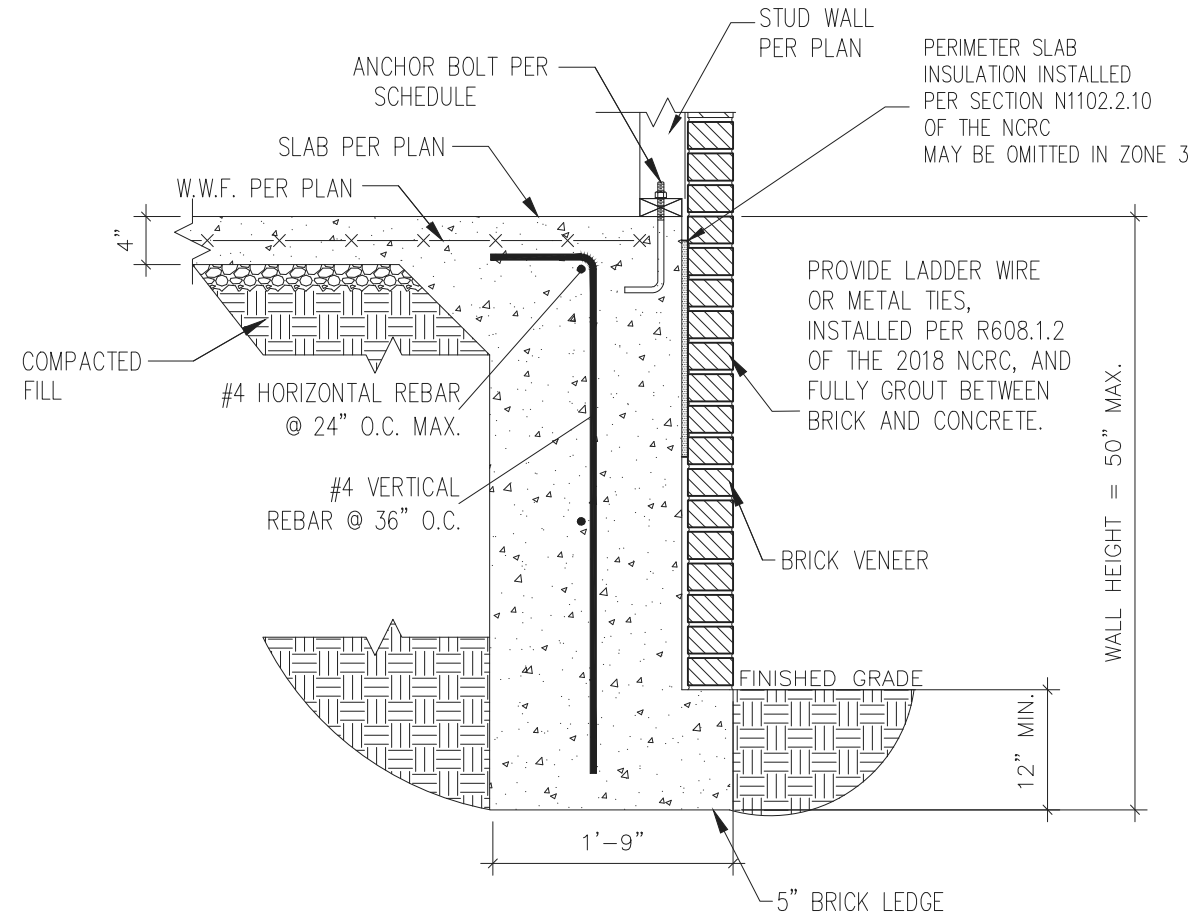
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COMPLETE LIST OF REVISIONS

SHEET

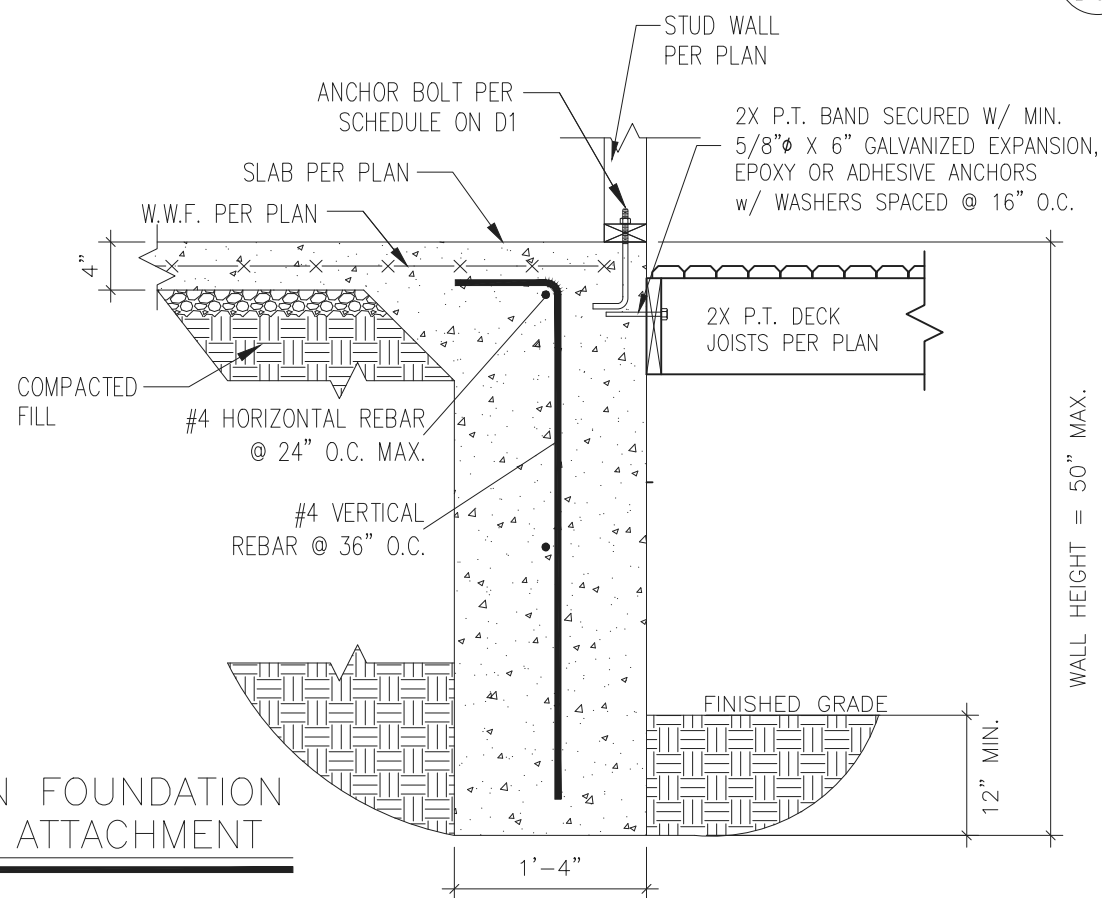
D5s



1 TURNDOWN FOUNDATION
D6s 3/4" = 1'-0"



2 TURNDOWN FOUNDATION W/ BRICK VENEER
D6s 3/4" = 1'-0"



3 TURNDOWN FOUNDATION W/ DECK ATTACHMENT
D6s 3/4" = 1'-0"



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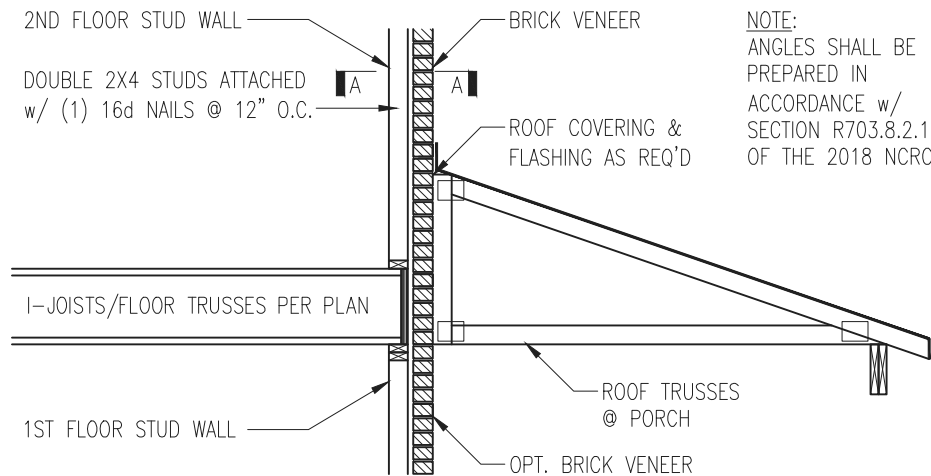
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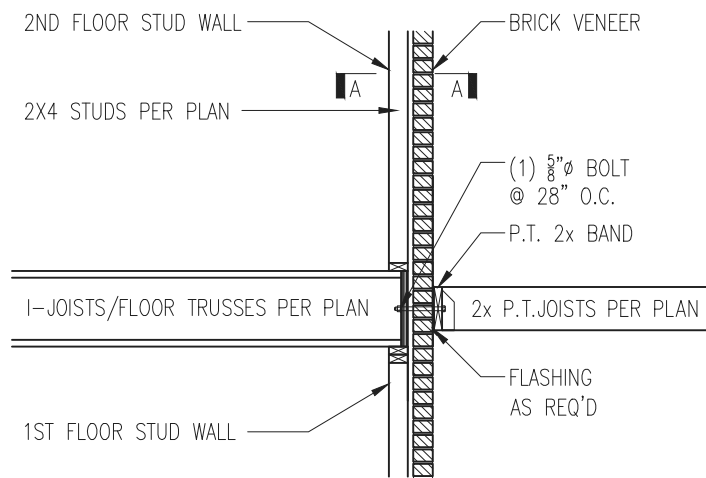
D6s



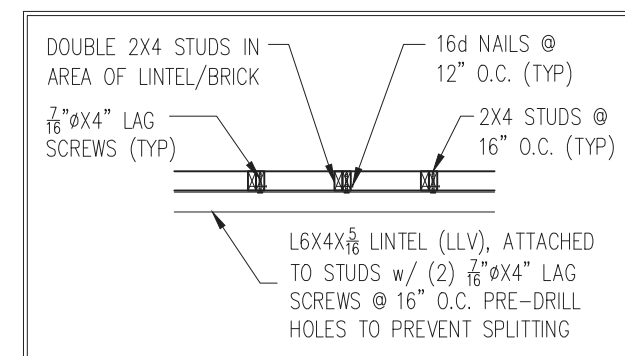
NOTE:
ANGLES SHALL BE PREPARED IN ACCORDANCE w/ SECTION R703.8.2.1 OF THE 2018 NCR

TRUSSES PERPENDICULAR TO STUD WALL

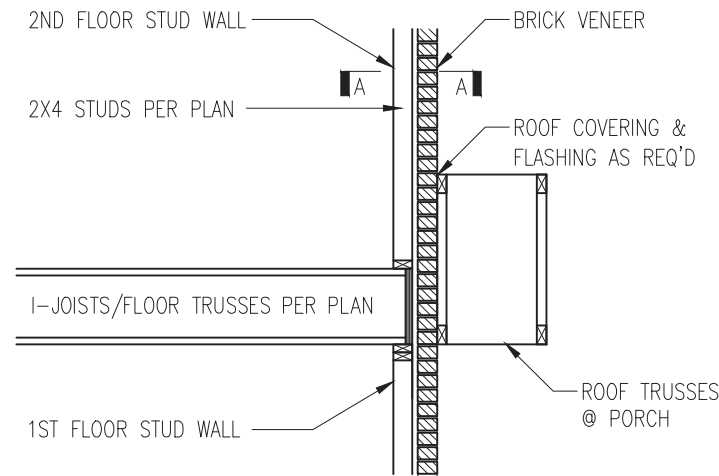
1 BRICK SUPPORT ABOVE STORAGE/PORCH ROOF DETAIL
D5f NTS



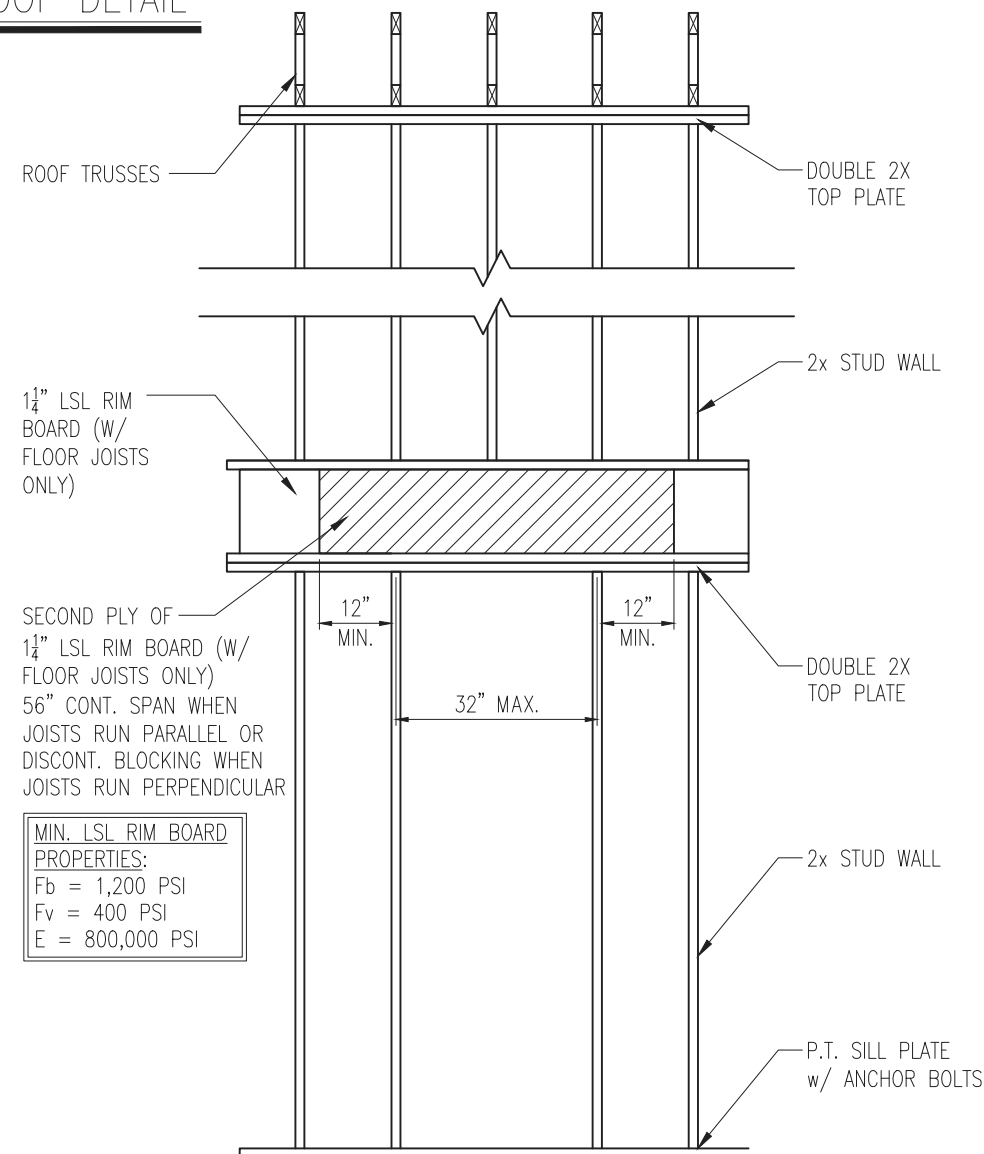
3 BALCONY JOIST ATTACHMENT
D5f NTS



SECTION A-A
NTS



TRUSSES PARALLEL TO STUD WALL
w/ CONTINUOUS BRICK VENEER



MIN. LSL RIM BOARD PROPERTIES:
Fb = 1,200 PSI
Fv = 400 PSI
E = 800,000 PSI

4 TYP. RANGE VENT FRAMING
D5f VENTED TO EXTERIOR WALL



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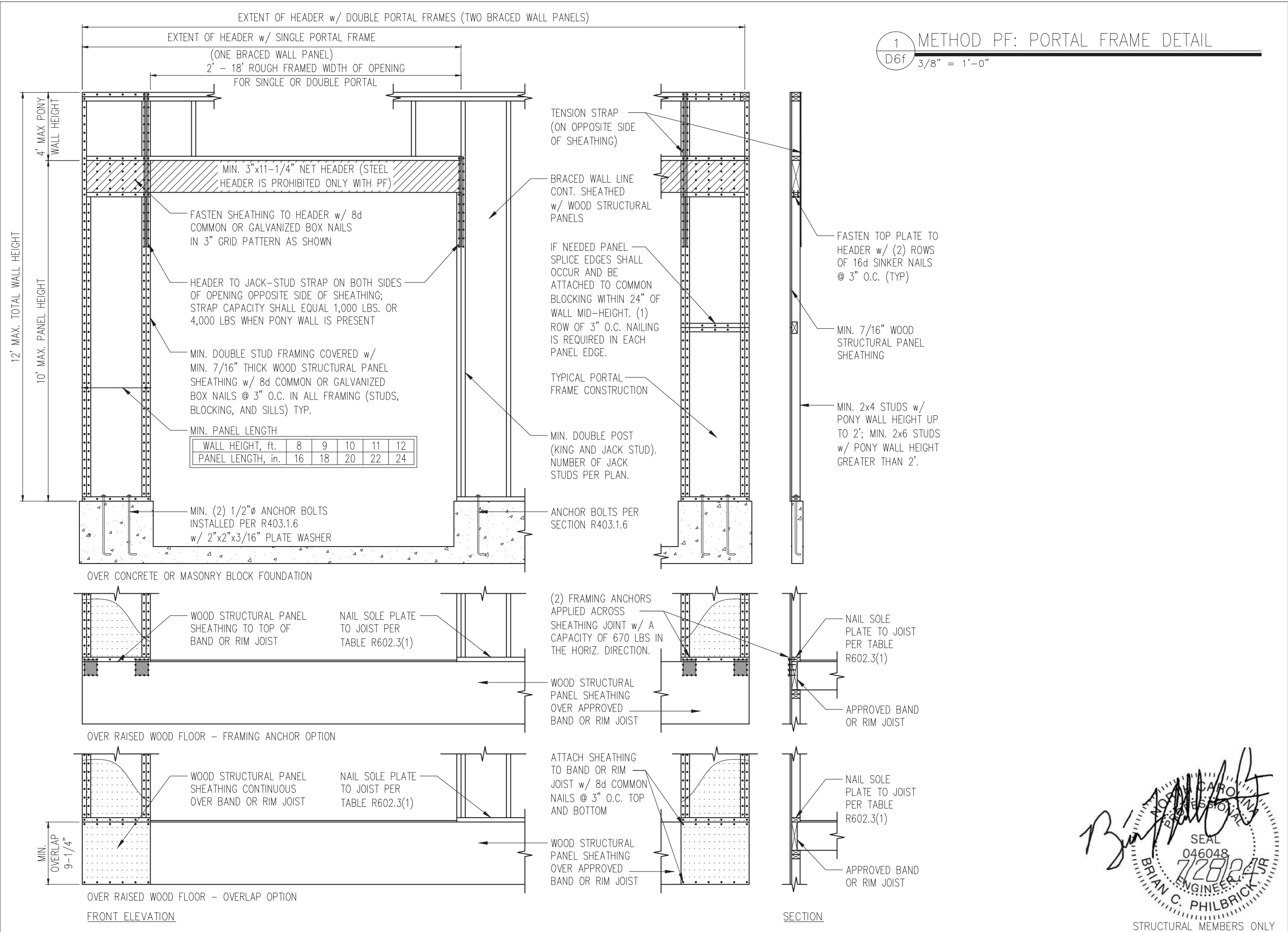
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SHEET
D5f



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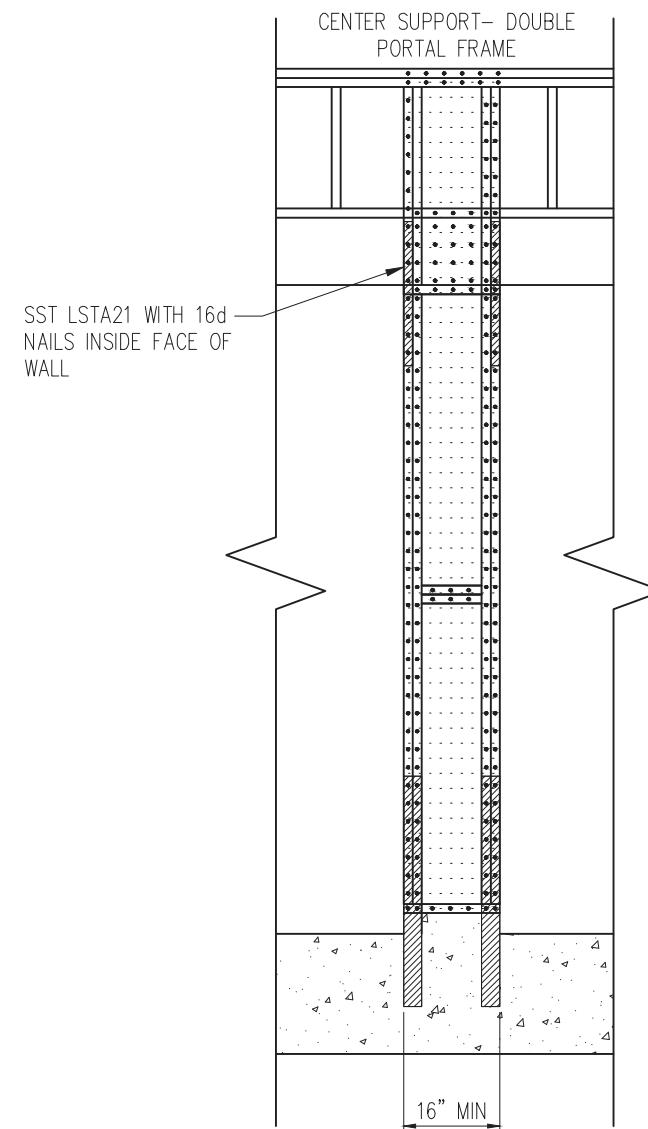
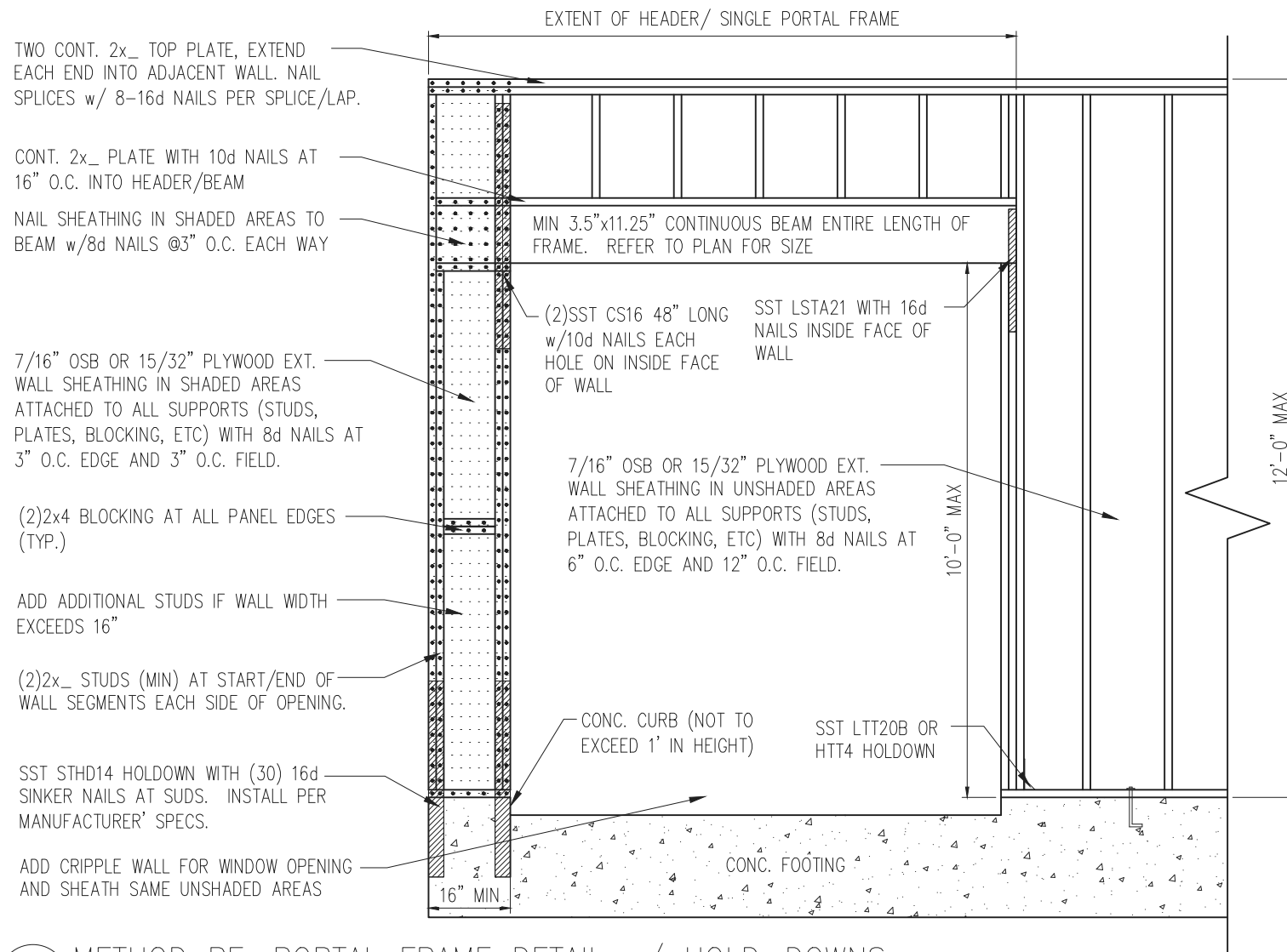
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SHEET

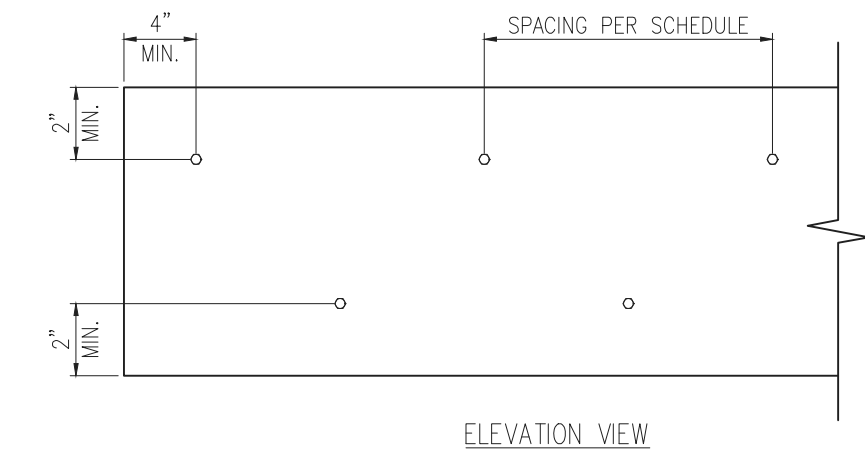
D6f



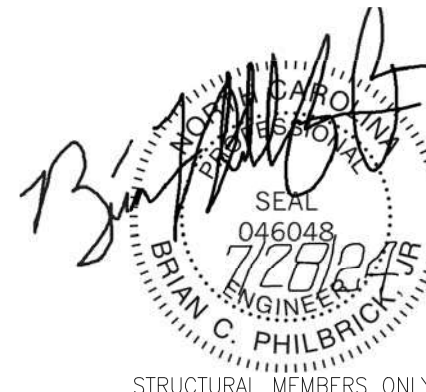
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1 METHOD PF: PORTAL FRAME DETAIL w/ HOLD-DOWNS



2 MULTI-PLY BEAM CONNECTION DETAIL



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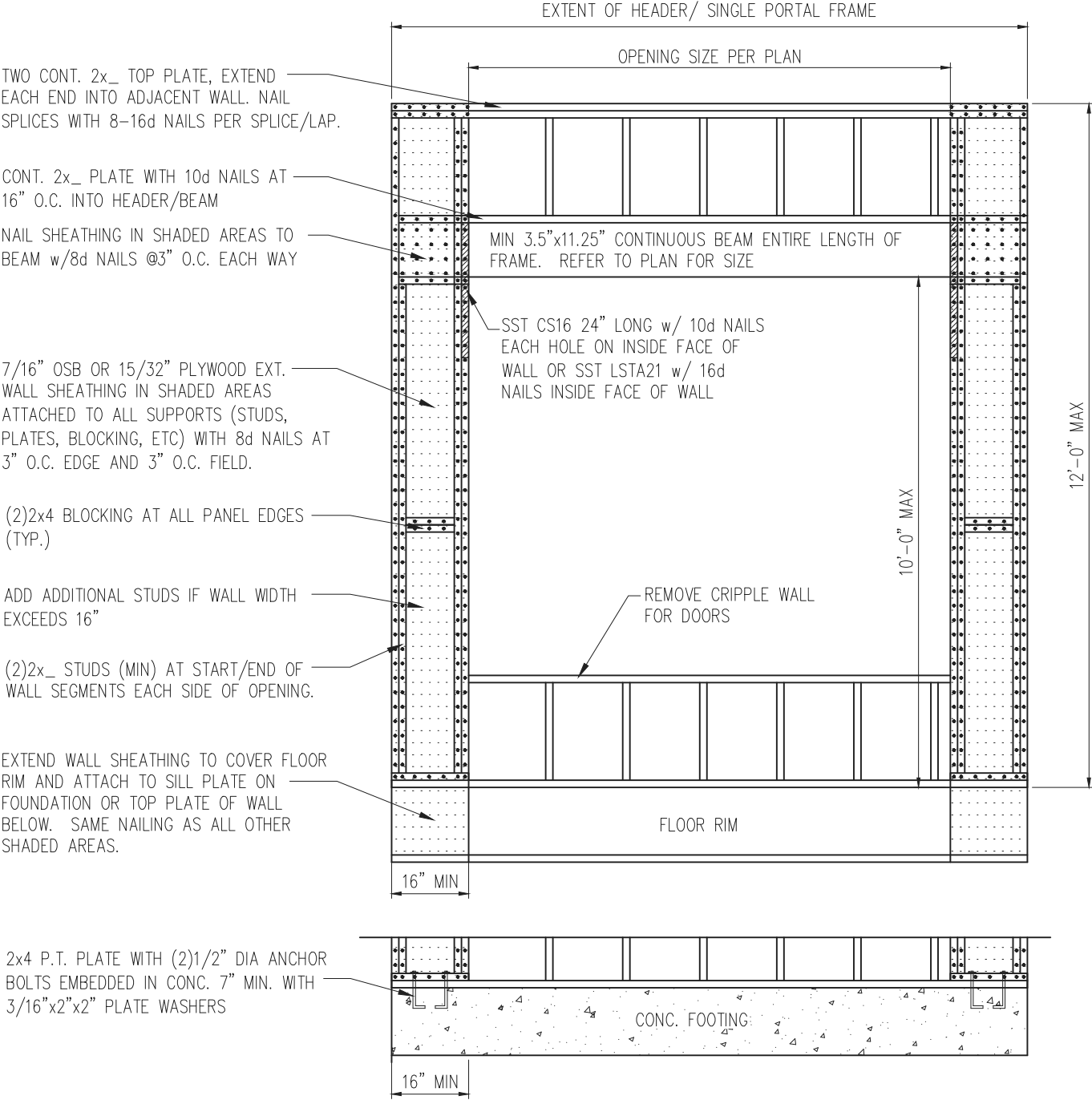
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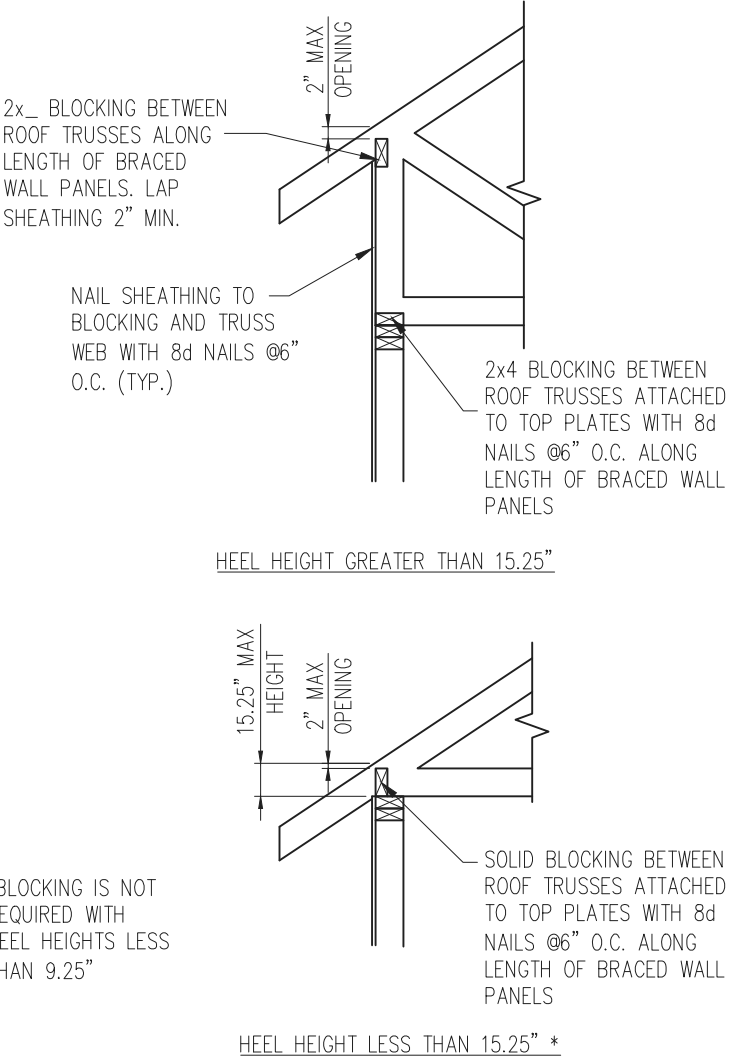
REFER TO COVER SHEET FOR A
COMPLETE LIST OF REVISIONS

SHEET

D7f



1 METHOD PF: PORTAL FRAME DETAIL
D8f 3/4" = 1'-0" OPENINGS UNDER 8'-0"



2 TYP. WALL PANEL TO ROOF TRUSS CONNECTION
D8f 1" = 1'-0"



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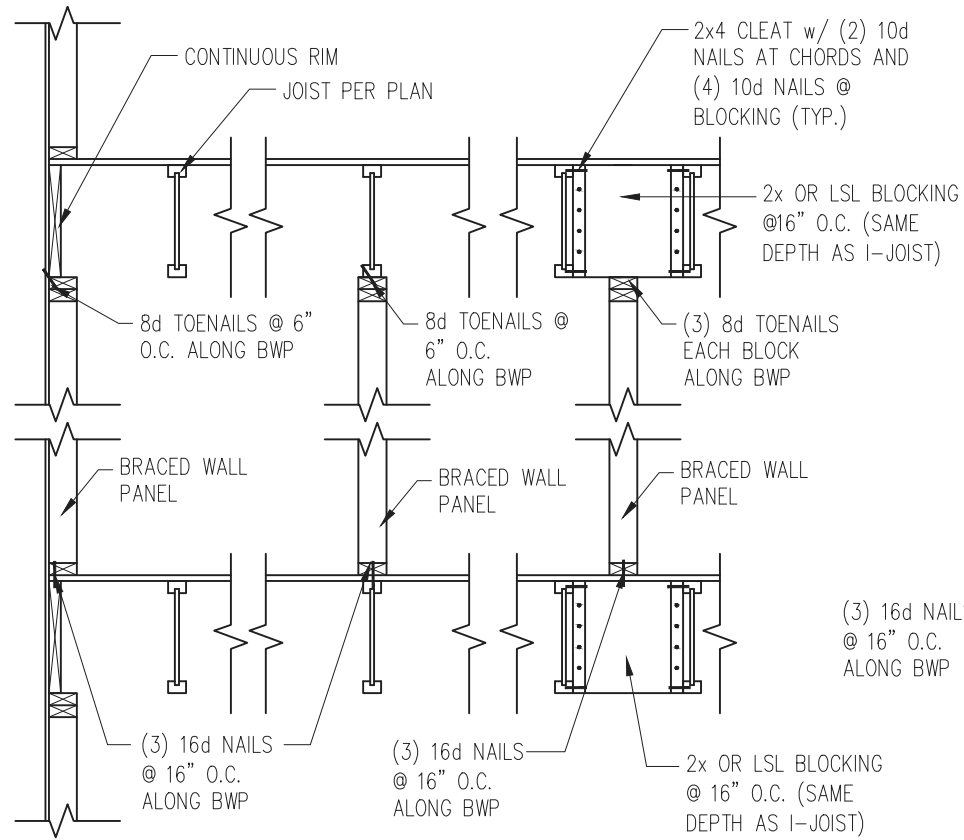
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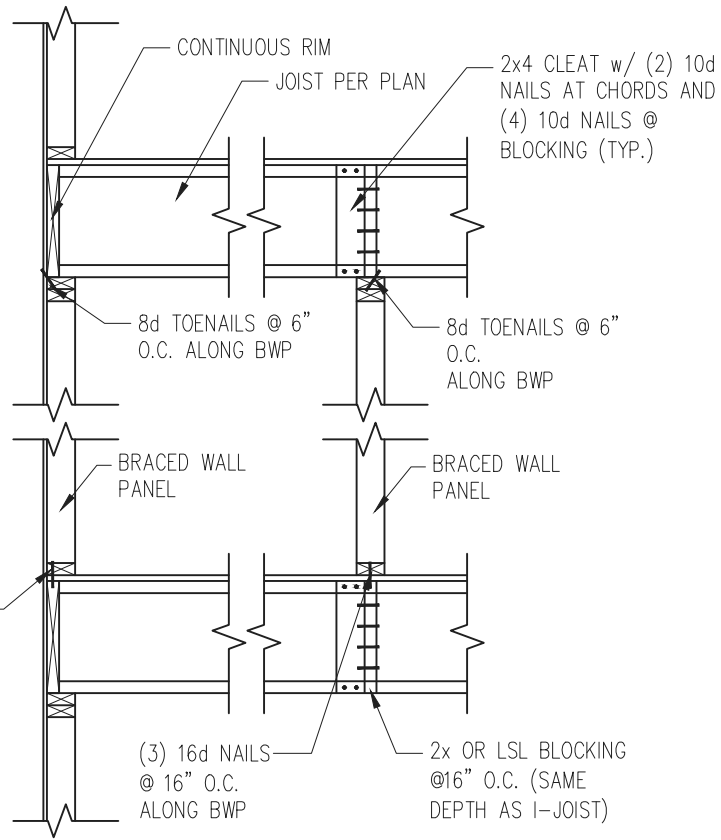
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SHEET
D8f

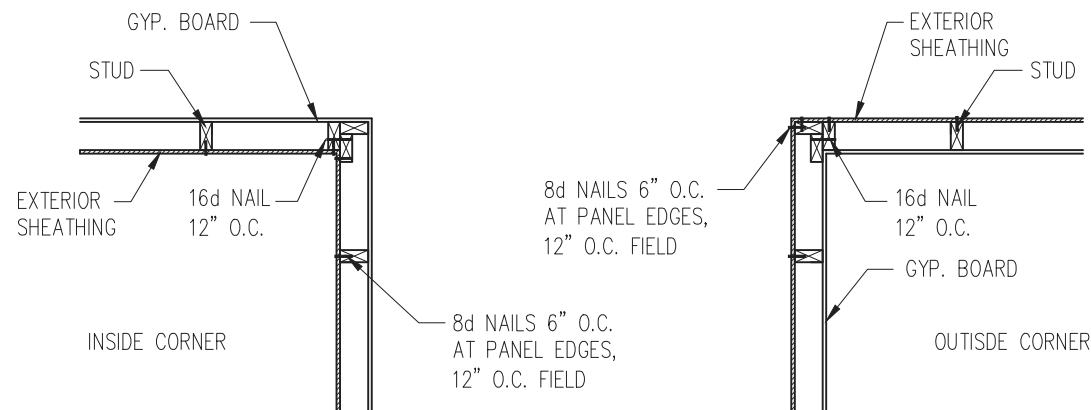


JOISTS PARALLEL TO BRACED WALLS



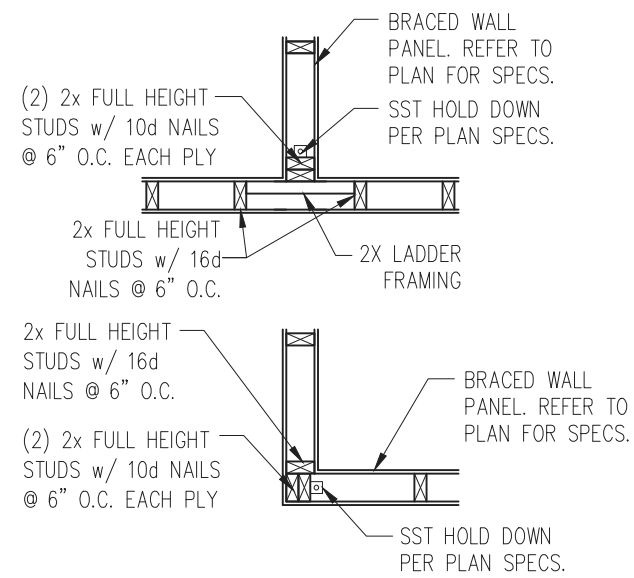
JOISTS PERPENDICULAR TO BRACED WALLS

1 TYP. WALL PANEL TO FLOOR/CEILING CONNECTION
D9f 1" = 1'-0"

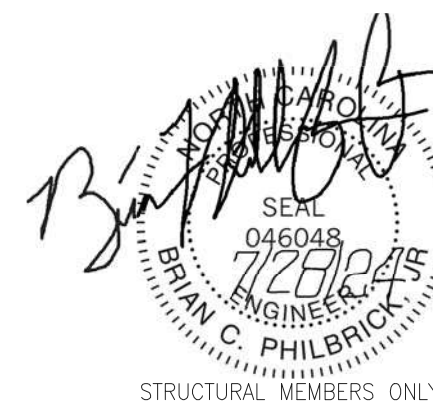


2 TYP. EXTERIOR CORNER FRAMING
D9f 1" = 1'-0"

3 INTERIOR 3-STUD WALL INTERSECTION
D9f 1" = 1'-0"



4 TYP. HOLD DOWN DETAIL
D9f 1" = 1'-0"



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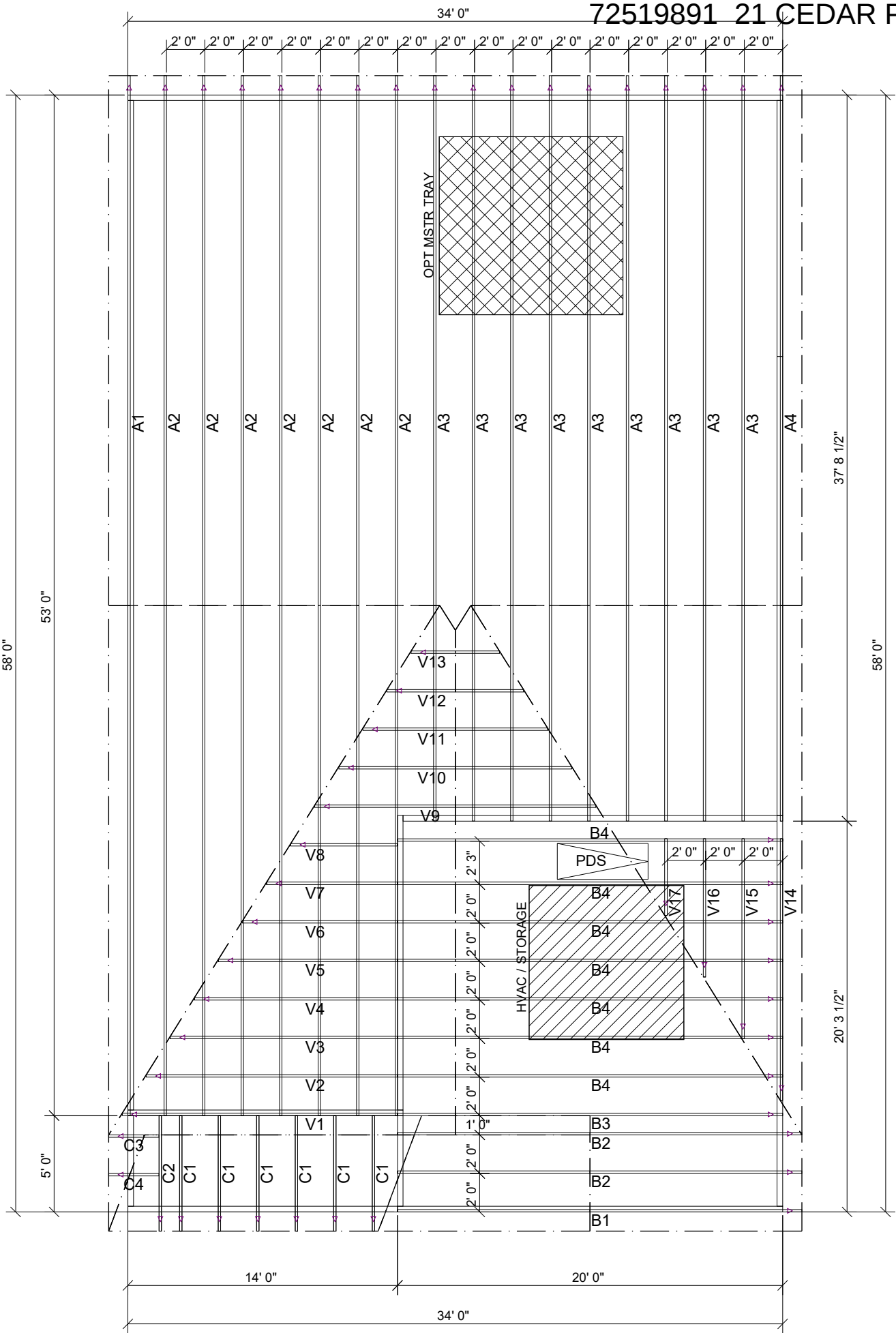
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SHEET
D9f

TRUSS TO WALL CONNECTIONS, IF SHOWN, ARE FOR UPLIFT ONLY AND DO NOT CONSIDER LATERAL LOADS. ALL CONNECTORS ON THIS PROJECT ARE TO BE INSTALLED PER THE CONNECTOR MANUFACTURER'S SPECIFICATIONS. ALL CONNECTORS SHOWN THAT ARE NOT "TRUSS TO TRUSS" ARE SUGGESTIONS ONLY AND ARE TO BE VERIFIED BY THE BUILDING DESIGNER OR ENGINEER OF RECORD FOR SUITABILITY TO THIS PARTICULAR PROJECT. UFP MID-ATLANTIC, LLC ACCEPTS NO RESPONSIBILITY FOR THE SPECIFIC APPLICATION OR SUITABILITY OF ANY CONNECTOR THAT IS NOT "TRUSS TO TRUSS" AS THEY APPLY TO THIS SPECIFIC STRUCTURE.



PIEDMONT CFI

ROOF AREA: 2421.29_RIDGE LINE: 65.58 ft _ VALLEY LINES: 74.27 _ HIP LINES:8.74 _ Indicates Left End of Truss

Customer
SMITH DOUGLAS

Job Name
PIEDMONT-BC CFI

Date:10-12-20
Scale:NTS
Revision Date1:
Revision Date2:

Quality Products for Quality Builders

Drawn By: AS
Checked By: ***
Drawing Number
MASTER

UFP MID-ATLANTIC, LLC
A UFP INDUSTRIES COMPANY

BURLINGTON, NC	PHONE (800) 476-9356
CHESAPEAKE, VA	PHONE (800) 476-3190
CLINTON, NC	PHONE (910) 590-3220
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1. TEMPORARY BRACING TO BE INSTALLED w/T.P.I. STANDARD BCSI-B1.

2. SEE ENGINEERED DRAWING FOR PERMANENT BRACING MINIMUM REQUIREMENTS.

3. FRAMER TO VERIFY ALL DIMENSIONS, DROP, & RISE LOCATIONS PRIOR TO TRUSS PLACEMENT.

4. BLDR/FRAMER RESPONSIBLE FOR ADJUSTMENT OF TRUSS SPACING TO MISS PLUMBING DROPS, UNLESS NOTED OTHERWISE.

5. THIS LAYOUT IS NOT AN ENGINEERED DRAWING. THIS DRAWING WAS CREATED TO ESTABLISH TRUSS PLACEMENT ONLY. IT IS THE RESPONSIBILITY OF THE BUILDER TO PROVIDE ADEQUATE SUPPORT FOR ALL THE ELEMENTS SHOWN IN THIS DRAWING.