

SOIL SCIENTIAN VALUE VAL

Agri-Waste Technology, Inc.
501 N Salem Street, Suite 203, Apex, NC 27502
agriwaste.com | 919.859.0669

Soil Suitability for Domestic Sewage Treatment and Disposal Systems Birchwood Trails – Lot 71 Olive Branch Rd. Fuquay Varina, NC 27526 (Harnett County)

June 1, 2023

Soil suitability for domestic sewage treatment and disposal systems was evaluated on April 24, 2023, for the property located at Olive Branch Rd. in Fuquay Varina, NC (Harnett County). Jeff Vaughan, Heath Clapp, and Trent Bostic of Agri-Waste Technology, Inc. (AWT) conducted the soil evaluation. This evaluation was done to facilitate permitting for a septic system for a 4-bedroom home. This report and attached documents were prepared to this application is to be used to issue an Improvement Permit in accordance with G.S. 130A-335(a2) and (a3). The LSS evaluation is being submitted pursuant to and meets the requirements of G.S. 130A-335(a2).

A drawing of the site plan, septic layout, septic system design, and soil pit/boring locations is included in Attachment 1. Profile descriptions for each soil pit/boring are included in Attachment 2.

The total property area is approximately .56 acres. The house and septic area are an open field. The proposed septic system for the property is a pressure manifold fed, accepted status system for initial and a pressure manifold fed, PPBPS (T&J Panel System) for repair.

Soil Suitability for Domestic Sewage Treatment and Disposal Systems

The drawing in Attachment 1 details the property boundaries, soil pit locations, and layout of drain field trenches. Multiple soil pits and borings were advanced within the proposed septic system area on the property. Soil pits/borings were examined to determine soil suitability for on-site sewage disposal systems in accordance with 15A 18A .1900 Rules for Sewage Treatment and Disposal Systems. All soil pits/borings were provisionally suitable for a conventional style trench. Soil pits/borings are within the proposed drainfield area.

The layout shown in Attachment 1 indicates there is available space for a four-bedroom accepted system. The initial system can be installed with the use of an accepted status drainfield based on the layout in the field.

The proposed LTAR (Long Term Acceptance Rate) by AWT is 0.4GPD/ft². The soils on this property are group III soils within the distribution and treatment zone as used to define the LTAR. With an LTAR of 0.4GPD/ft², 300 linear feet of trench is necessary to support a 4-bedroom home for the initial system with the use of an accepted trench product, and 200 linear feet is necessary to support the a 4-bedroom home for the repair system utilizing a PPBPS (T&J Panel System) trench product. The maximum slope corrected trench depth is 22 inches. The attached drawings substantiate that the necessary linear footage of trench can be installed on the property for the initial and repair system.

Any logging, disturbances, or grading done in the usable area or within the proposed setbacks will change the potential of using the area designated for a drainfield. Prior to moving forward with the development on the property, the Harnett County Health Department should be contacted to complete the necessary Construction Oversight and to issue an OP (Operations Permit) for the property once the septic system has been installed.

Conclusions

An IP (Improvement Permit) and CA (Construction Authorization) for this property can be issued with the site plan that is in Attachment 1. A CA permit will be required to secure a building permit for the property. The county issues an Operation Permit after the system has been installed to meet the specifications of the Authorization to Construct. Additional septic layouts have been or will be performed as needed. It will be critical to not disturb any of the proposed septic area or there is a risk that the IP and CA will be revoked. The LSS/AOWE Evaluation and attached documents were prepared to this application is to be used to issue an Improvement Permit in accordance with G.S. 130A-335(a2) and (a3). The LSS/AOWE evaluation is being submitted pursuant to and meets the requirements of G.S. 130A-335(a2) and (a5).

We appreciate the opportunity to assist you in this matter. Please contact us with any questions, concerns, or comments.

Sincerely,

Jeff Vaughan, NC LSS

fell M/

County: Harnett
IMPROVEMENT PERMIT FOR G.S. 130A-335(a2)/SL2022-11
PIN/Lot Identifier: LOT 71
Issued To: Ballentine Associates
Property Location: Olive Branch Rd. Fuquay Varina, NC 27526
Subdivision: Birchwood Trails Lot #: 71 Block: Section:
LSS Report Provided: Yes ☑ No □
If yes, name and license number of LSS: Jeff Vaughan (1227)
New ☑ Repair □ Expansion □ System Relocation □
Proposed Structure: SFR (4-bedroom)
Proposed Wastewater System Type: Accepted (Initial) PPBPS (T&J Panel System) (Repair)
Fill System: 🗆 Yes 🔻 No If yes, specify: 🗀 New 🗀 Existing (when adding more than 6 inches of fill to system area please provide a fill plan)
Proposed Design Daily Flow: 480 GPD Proposed LTAR (Initial): 0.4 Proposed LTAR (Repair): 0.4
Design Wastewater Strength: ☑ domestic ☐ high strength ☐ industrial process
Number of bedrooms: 4 Number of Occupants: 8 Other:
Pump Required: 🗆 Yes 🗆 No 🗹 May be required based upon final location and elevations of facilities
Artificial Drainage Required: 🗆 Yes 🗵 No If yes, please specify details:
Type of Water Supply: ☐ Private well ☐ Public well ☑ Municipal Supply ☐ Spring ☐ Other:
Drainfield location meets requirements of Rule .1945: Yes ☑ No □
Drainfield location meets requirements of Rule .1950: Yes $oxdot$ No $oxdot$
Permit valid for: 🗹 Five years [site plan submitted pursuant to GS 130A-334(13a)] 🔲 No expiration [plat submitted pursuant to GS 130A-334(7a)
Permit conditions:

The LSS evaluation is being submitted pursuant to and meets the requirements of G.S. 130A-335(a2).

See attached site sketch

Date: 06/01/2023

Licensed Soil Scientist Print Name: <u>Jeff Vaughan</u>

Licensed Soil Scientist Signature: _

		County:	Harnett
This Section for Local H	lealth Denartment II	se Only	
•	•	-	
Initial submittal received:	Date	y Initials	
Permit Number:			
G.S. 130A-335(a4) states the following: 'If a local health departr. submitted pursuant to subsection (a3) of the section within 10 be department shall issue the improvement permit.'			
In accordance with G.S. 130A-335(a3) the improvement permit a	application is:		
$\ \square$ Incomplete (If box is checked, information in this section is a	required.)		
The following items are missing:			
Copies of this were sent to the LSS and the Owner on			
State Authorized Agent:		Date	e:
☐ Denied (See attached report.)			
Copies of this were sent to the LSS and the Owner on			
State Authorized Agent:		Date	2:
□ Complete			
State Authorized Agent:		Date of Issua	nce:
State Authorized Agenta			
This Improvement Permit is issued pursuant to G.S. 130A-335 (attached here. The issuance of this permit by the Health Department holder is responsible for checking with appropriate gover evocation if the site plan, plat, or the intended use changes, or inaccurate or misleading. The Improvement Permit shall not be subject to compliance with the provisions of the Laws and Rule permit. The location and identification of all property lines, ear responsibility of the owner.	rtment in no way gua erning bodies in mee r if information subm e affected by a chang es for Sewage Treatm	arantees the issueting their requir nitted in the app ge in ownership nent and Disposa	ance of other permits. The ements. This site is subject to lication was falsified, of the site. This permit is Il and to conditions of this

any liabilities, duties, and responsibilities imposed by statute or in common law from any claim arising out of or attributed to evaluations, submittals, or actions from a licensed soil scientist or licensed geologist pursuant to GS 130A-335(a2).

The Department, the Department's authorized agents, and the local health departments shall be discharged and released from

Improvement Permit Expiration Date: _____

See attached site sketch

County:	Harnett
country.	<u> </u>

CONSTRUCTION AUTHORIZATION FOR G.S. 130A-335(a2)/SL2022-11

PIN/Lot Identifier: Lot 71
Issued To: Ballentine Associates
Property Location: Olive Branch Rd. Fuquay Varina, NC 27526
AOWE/PE Plans/Evaluations Provided: Yes 🗵 No 🗆 If yes, name and license number of AOWE/PE: <u>Jeff Vaughan</u> , 10003E
Facility Type: SFR (4-bedroom)
✓ New □ Expansion □ Repair System Relocation □
Basement? ☐ Yes ☐ No Basement Fixtures? ☐ Yes ☐ No
Type of Wastewater System** Accepted (Initial) PPBPS (T&J Panel System) (Repair)
Design Daily Flow: 480 GPD Wastewater Strength: \square domestic \square high strength \square industrial process
Session Law 2014-120 Section 53, Engineering Design Utilizing Low-flow Fixtures and Low-flow Technologies? ☐ Yes ☐ No
Installation Requirements/Conditions
Septic Tank Size: 1200 gallons Total Trench/Bed Length: 300 feet Trench/Bed Spacing: 9 feet on center
Drainfield square footage: 900 Trench/Bed Width: 36 inches LTAR: 0.4 gpd/ft ²
Soil Cover: inches Slope Adjusted Maximum Trench/Bed Depth: 22 inches
Aggregate Depth:inches above pipeinches below pipeinches total
Pump Tank Size (if applicable): $\underline{1,250}$ gallons Requires more than 1 pump? \square Yes \square No
Pump Requirements: ft. TDH vs GPM Grease Trap Size (if applicable): gallons
Distribution Method: ☐ Serial ☐ D-Box or Parallel ☐ Pressure Manifold(s) ☐ LPP ☐ Other:
Artificial Drainage Required: Yes No If yes, please specify details:
<u>Legal Agreements</u> (If the answer is "Yes" to any type of legal agreements, please attach a copy of the agreement.)
Multi-party Agreement Required [.1937(h)]: Yes □ No ☑
Easement, Right-of-Way, or Encroachment Agreement Required [.1938(j)]: ☐ Yes ✓ No
Declaration of Restrictive Covenants: ☐ Yes ☐ No
**If applicable:
I understand the system type specified is different from the type specified on the application. I accept the specifications of this permit.
Owner/Legal Representative Print Name:
Owner/Legal Representative Signature: Date:
Pre-Construction Conference Required: Yes □ No □
Conditions:
The construction and installation requirements of Rules .1950, .1952, .1954, .1955, .1956, .1957, .1958, and .1959 are incorporated by reference.
into this permit and shall be met. Systems shall be installed in accordance with the attached system layout.
AOWE/PE Print Name: Jeff Vaughan
AOWE/PE Signature: Date: 06/01/2023 Number
This AOWE/PE submittal is pursuant to and meets the requirements of G.S. 130A-335(a2) and (a5).
See attached site sketch
ALUATOMINI
MINIMINION .

Initial submittal received:		by
	Date	Initials
Permit Number:		
G.S. 130A-335(a6) states the following: 'If a local health depart submitted pursuant to subsection (a5) of the section within 10 by department shall issue the construction authorization.'	-	
In accordance with G.S. 130A-335(a5) the construction authorize	zation applicat	tion is:
$\hfill \square$ Incomplete (If box is checked, information in this section is	required.)	
The following items are missing:		
Copies of this were sent to the AOWE/PE and the Owner on		
	Date	
State Authorized Agent:		Date:
☐ Denied (See attached report.)		
Copies of this were sent to the AOWE/PE and the Owner on		
	Date	
State Authorized Agent:		Date:
□ Complete		
State Authorized Agent:		Date of Issuance:
This Construction Authorization is issued pursuant to G.S. 130		
evaluations attached here. This Construction Authorization is changes, or if information submitted in the application was fa shall not be affected by a change in ownership of the site. This provisions of the Laws and Rules for Sewage Treatment and Didentification of all property lines, easements, water lines, and Final landscaping shall be constructed to divert water and estated	subject to revolsified, inacculis Construction bisposal and to dother approp	vocation if the site plan, plat, or the intended use trate or misleading. The Construction Authorization in Authorization is subject to compliance with the or the conditions of this permit. The location and opriate utilities shall be the responsibility of the own
The Department, the Department's authorized agents, and the any liabilities, duties, and responsibilities imposed by statute plans, evaluations, preconstruction conference findings, submithe General Statutes as a licensed engineer or a person certific Authorized On-Site Wastewater Evaluator in GS 130A-335(a2) agents, and the local health departments shall be responsible obligations under State law or rule, including the issuance of the state o	or in common nittals, or actio ed pursuant to , (a5), and (a7) and bear liabi	n law from any claim arising out of or attributed to ons from a person licensed pursuant to Chapter 89C o Article 5 of Chapter 90A of the General Statutes as 7). The Department, the Department's authorized illity for their actions and evaluations and other
Construction Authorization Expiration Date:		_
See attac	ched site sketo	ch

This Section for Local Health Department Use Only

County:

Sheet _1__ of _2_
PROPERTY ID #: Lot 71
COUNTY: _Harnett

SOIL/SITE EVALUATION for ON-SITE WASTEWATER SYSTEM

(Complete all fields in full)

ADD PROI LOC	CLIENT: Ballentine Associates APPLICATION DATE_5/26/2023 ADDRESS: 221 Providence Rd. Chapel Hill, NC 27514 DATES EVALUATED: 04/24/2023 PROPOSED FACILITY: Single Family Residence PROPOSED DESIGN FLOW (.1949): 480GPD PROPERTY SIZE: 0.56 ac LOCATION OF SITE: Olive Branch Rd. Fuquay Varina, NC 27526 PROPERTY RECORDED: No WATER SUPPLY: Private Public Well Spring Other										
	TER SUPPLY:		☐ Public	□ Wo	*	-					Process Mixed
	LUATION MET	HOD: LAU	ger Boring		t 🗆 Cut	1)	PE OF WAST	EWATER:		e 🗆 industriai	Process Mixed
P R O F I L	.1940 LANDSCAPE	HORIZON	SOII	SOIL MORPHOLOGY (.1941)		Y	Y OTHER PROFILE FACTORS				
#	POSITION/ SLOPE %	DEPTH (IN.)	.1941 STRUCT TEXTU	URE/	.194 CONSISTI MINERA	ENCE/	.1942 SOIL WETNESS/ COLOR	.1943 SOIL DEPTH	.1956 SAPRO CLASS	.1944 RESTR HORIZ	PROFILE CLASS & LTAR
1,		A 0-6"	LS; Gr		NS; NP; VFr		10YR 3/3				Provisionally Suitable
2		E 6-20"	LS; Gr		NS; NP; VFr		10YR 7/6	36"			Sumare
3		Bt1 12-36"	SCL; SBK		SS; SP; Fi-Fr		2.5YR 5/8				0.4GPD/ft2
4											
	DESCRIPTION	INITIAI	SYSTEM	DEDA	AIR SYSTEM	OTHE	D E A CECODO /	10.16)			
					sionally		R FACTORS (. CLASSIFICAT		: <u>Provisio</u>	nally Suitable	
Ava	ilable Space (.1945) Provision Suitable		Suita		EVAL	UATED BY:	Jeff Vaugh	an, Trent Bo	ostic, Heath Cl	app
Syst	tem Type(s)	Accepte	ed		PS (T&J el System)						
Site	LTAR	0.4GPE	D/Ft ²		PD/Ft ²						

LEGEND

COMMENTS

use the following standard abbreviations

	use the jouowing standard door evations							
LANDSCAPE POSITION	GROUP	SOIL <u>TEXTURE</u>	CONVENTIONAL .1955 LTAR*	LPP <u>.1957 LTAR*</u>	MINERALOGY/ CONSISTENCE	STRUCTURE		
CC (Concave Slope) CV (Convex Slope) D (Drainage Way)	I	S (Sand) LS (Loamy Sand)	1.2 - 0.8	0.6 - 0.4	SEXP (Slightly Expansive) EXP (Expansive)	G (Single Grain) M (Massive) CR (Crumb)		

DS (Debris Slump) FP (Flood Plain) FS (Foot Slope)	П	SL (Sandy Loam) L (Loam)	0.8 - 0.6	0.4 - 0.3		GR (Granular) SBK (Subangular Blocky) ABK (Angular Blocky)
H (Head Slope)	III	Si (Silt)	0.6 - 0.3	0.3 - 0.15		PL (Platy)
L (Linear Slope)		SiCL (Silty Clay Loam)				PR (Prismatic)
N (Nose Slope)		CL (Clay Loam)				
R (Ridge)		SCL (Sandy Clay Loam)			MOIST	WET
S (Shoulder Slope)		SiL (Silt Loam)				
T (Terrace)					VFR (Very Friable)	NS (Non-sticky)
	IV	SC (Sandy Clay)	0.4 - 0.1	0.2 - 0.05	FR (Friable)	SS (Slightly Sticky)
		SiC (Silty Clay)			FI (Firm)	S (Sticky)
		C (Clay)			VFI (Very Firm v. Very Sticky)	VS (Very Sticky)
		O (Organic)	None	None	EFI (Extremely Firm)	NP (Non-plastic) SP (Slightly Plastic)
	**** *********					

*Adjust LTAR due to depth, consistence, structure, soil wetness, landscape, position, wastewater flow and quality.

P (Plastic) VP (Very Plastic)

 HORIZON DEPTH
 In inches below natural soil surface

 DEPTH OF FILL
 In inches from land surface

 RESTRICTIVE HORIZON
 Thickness and depth from land surface

 SAPROLITE
 S(suitable) or U(unsuitable)

SOIL WETNESS Inches from land surface to free water or inches from land surface to soil colors with chroma 2 or less - record Munsell color chip designation

S (Suitable), PS (Provisionally Suitable), or U (Unsuitable)

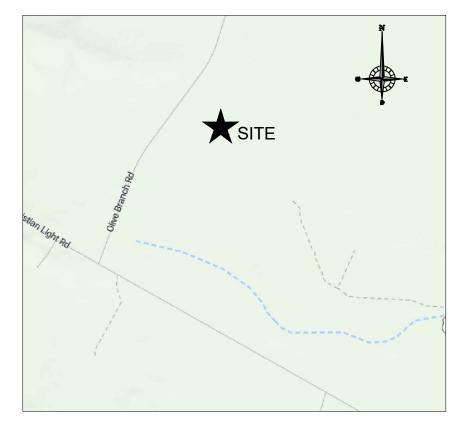
CLASSIFICATION S (S Evaluation of saprolite shall be by pits.

NOTES

Long-term Acceptance Rate (LTAR): gal/day/ft²

Show profile locations and other site features (dimensions, reference or benchmark, and North).

BIRCHWOOD TI	RAILS - LOT 71
Project Location	Olive Branch Rd
	Fuquay Varina, NC 27526
	Harnett County
	PIN:
Project Owner	Ballentine Associates, PA
	221 Providence Rd
	Chapel Hill, NC 27514
	919-929-0481
	dillons@ballentineassociates
Project Consultant	Jeff Vaughan, L.S.S
	(919) 367-6313
	Trent Bostic
	(919) 367-6322
	Agri-Waste Technology, Inc.
	501 N. Salem Street, Suite 203
	Apex, NC 27502
	(919) 859-0669
	(919) 233-1970 Fax
System Overview	Single Family Residence
	Four (4) Bedroom, 480 gpd
	Pressure Manifold Distribution
	Accepted/Innovative Trench Product



VICINITY MAP

Sheet Index

Sheet 1	Cover Sheet
Sheet 2	Property Layout
Sheet 3	Primary Drain Field
Sheet 4	Repair Drain Field
Sheet 5	Detail Sheet 1
Sheet 6	Detail Sheet 2
Sheet 7	Excavation Safety



Ballentine Associates, PA Birchwood Trails - Lot 71

Project Location:
Olive Branch Rd
Fuquay Varina, NC 27526
Harnett County
PIN: ----

Project Owner:
Ballentine Associates, PA
221 Providence Rd
Chapel Hill, NC 27514
919-929-0481 dillons@ballentinea

NC ONSITE WASTEWATER EVALUATOR SEAL



REY. ISSUED DATE DESCRIPTION

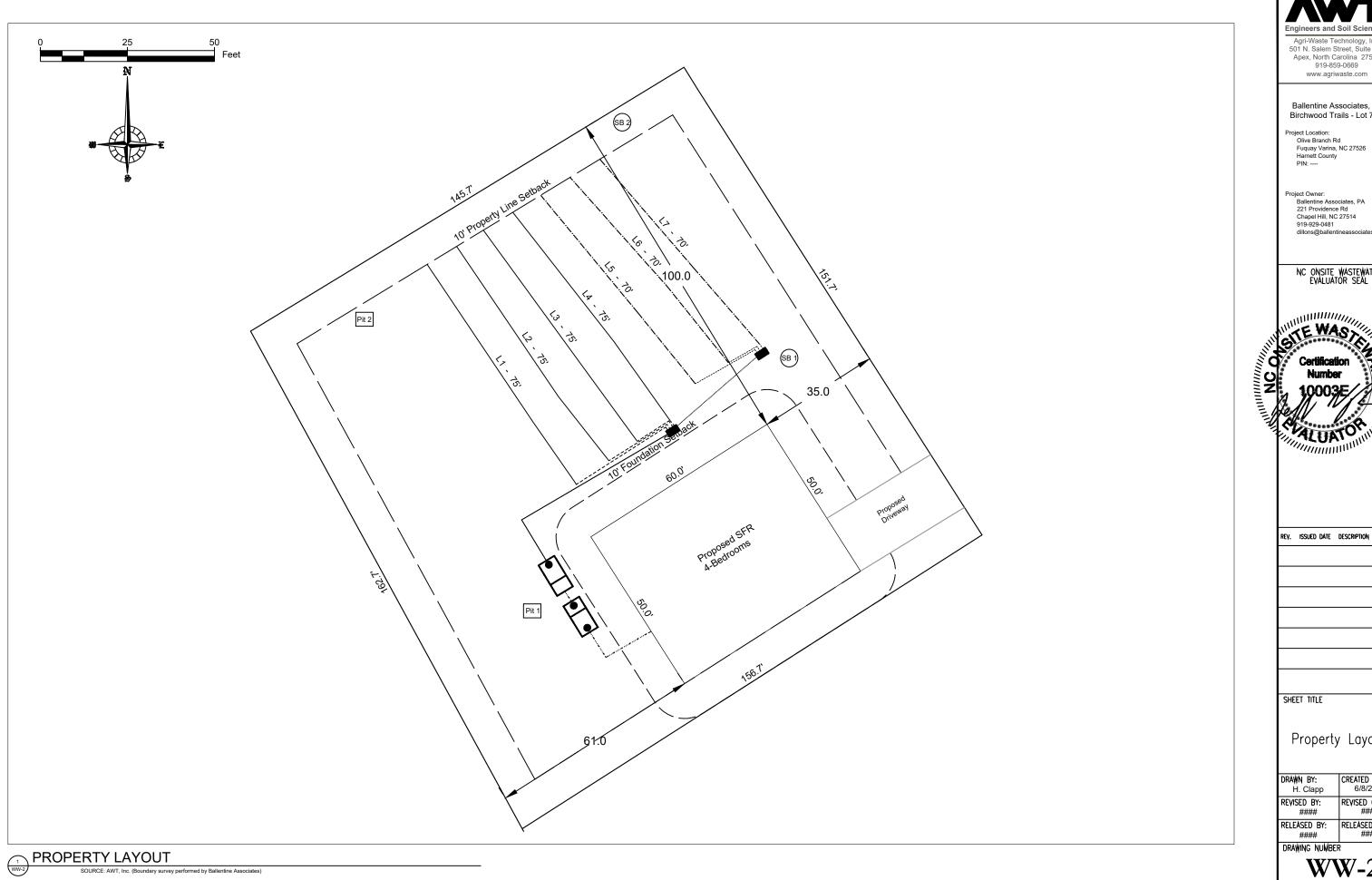
SHEET TITLE

Cover Sheet

CREATED ON: 6/8/2023 DRAWN BY: H. Clapp REVISED BY: REVISED ON: RELEASED BY: RELEASED ON: ####

DRAWING NUMBER





SOURCE: AWT, Inc. (Boundary survey performed by Ballentine Associates)

Agri-Waste Technology, Inc. 501 N. Salem Street, Suite 203 Apex, North Carolina 27502 919-859-0669 www.agriwaste.com

Ballentine Associates, PA Birchwood Trails - Lot 71

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Harnett County
PIN: ----

Project Owner:
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221 Providence Rd
Chapel Hill, NC 27514
919-929-0481 dillons@ballentineasse

NC ONSITE WASTEWATER EVALUATOR SEAL



SHEET TITLE

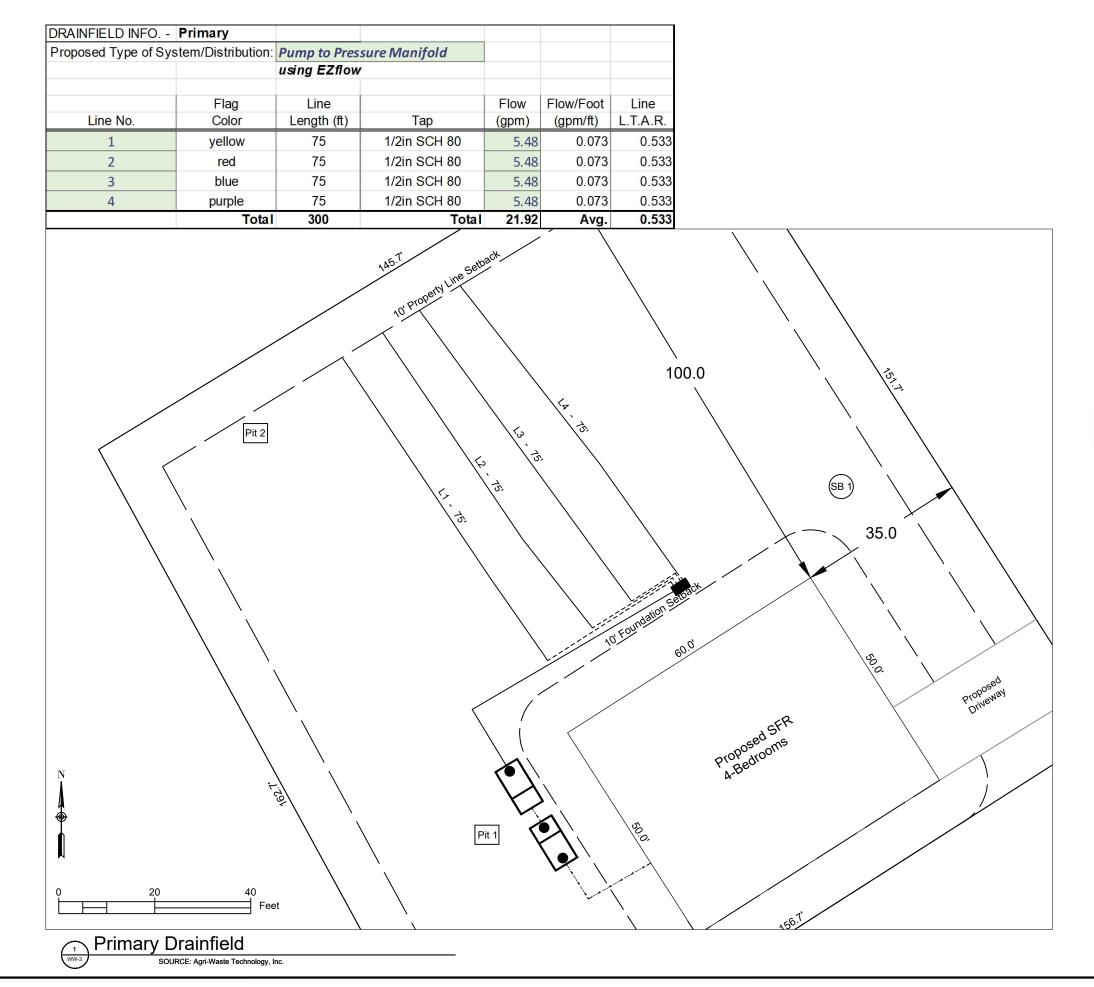
Property Layout

CREATED ON: 6/8/2023 DRAWN BY: H. Clapp REVISED BY: REVISED ON: RELEASED ON: RELEASED BY:

DRAWING NUMBER

General Drainfield Notes:

- Clear all trees less than 8" in diameter (measured at a height 3' from soil surface) from the drainfield.
- 2. Vegetation that will re-grow from a cut stump shall be stumped or pulled from the ground. Stumps shall not be pushed over.
- 3. Drainfield area shall be cleared of all leaves, pine straw, debris, etc. The accumulated material shall be removed from the drainfield.
- 4. In clayey soils, sides of trenches shall be raked and limed per manufacturer's instructions.
- 5. Supply lines shall be installed with a minimum of 18" cover.
- 6. The trenches shall be backfilled appropriately so that no low areas are present.
- 7. Apply lime over the drainfield area as needed. Seed fine fescue over the drainfield at the rate recommended by the seed manufacturer. Hand rake the seed into the soil surface. Straw the seeded area at the rate of 1.5-2 bales per 1000 sq. ft.





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Ballentine Associates, PA Birchwood Trails - Lot 71

www.agriwaste.com

Project Location:
Olive Branch Rd
Fuquay Varina, NC 27526
Harnett County
PIN: ----

Project Owner:
Ballentine Associates, PA
221 Providence Rd
Chapel Hill, NC 27514
919-929-0481
dillons@ballentineassociate



rey, issued date description

SHEET TITLE

Primary Drainfield

DRAWN BY:
H. Clapp
CREATED ON:
6/8/2023
REVISED BY:
####
RELEASED BY: RELEASED ON:

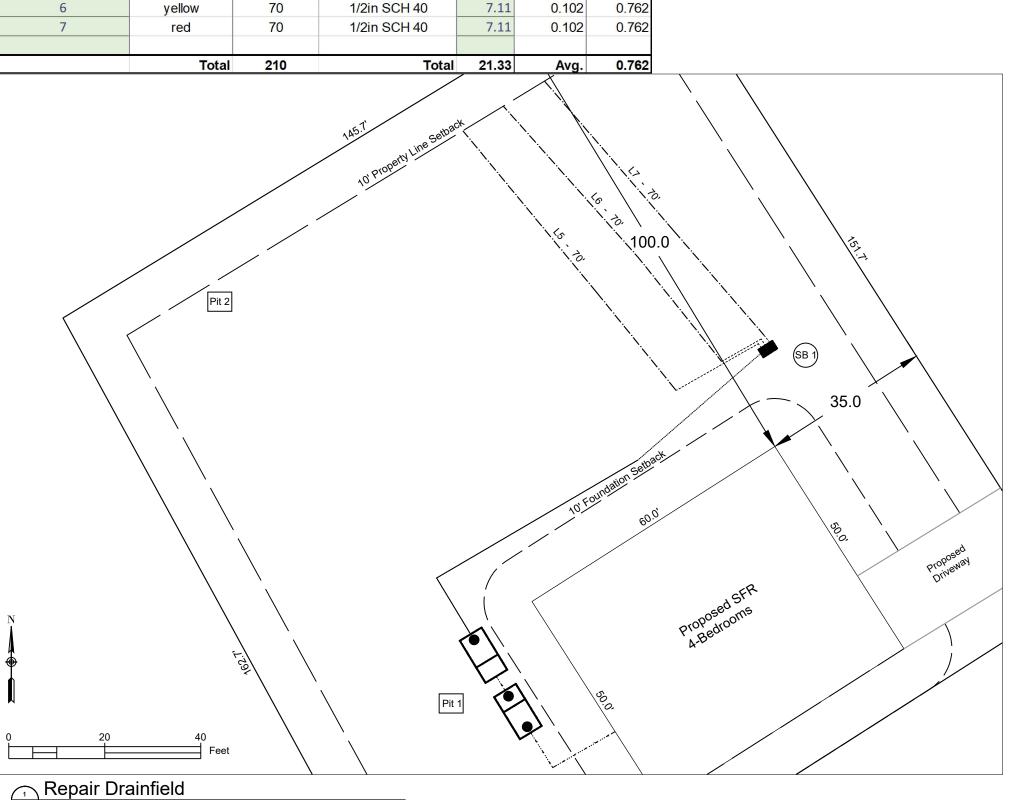
DRAWING NUMBER

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DRAINFIELD INFO	Repair					
Proposed Type of Sy	stem/Distribution:	Pump to Press	sure Manifold			
		using PPBPS, Horizontal				
	Flag	Line		Flow	Flow/Foot	Line
Line No.	Color	Length (ft.)		(gpm)	(gpm/ft)	L.T.A.R.
5	white	70	1/2in SCH 40	7.11	0.102	0.762
6	yellow	70	1/2in SCH 40	7.11	0.102	0.762
7	red	70	1/2in SCH 40	7.11	0.102	0.762
		пососос				
					_	

SOURCE: Agri-Waste Technology, Inc.





Agri-Waste Technology, Inc. 501 N. Salem Street, Suite 203 Apex, North Carolina 27502 919-859-0669 www.agriwaste.com

Ballentine Associates, PA Birchwood Trails - Lot 71

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919-929-0481
dillons@ballentineassociate



rey, issued date description

SHEET TITLE

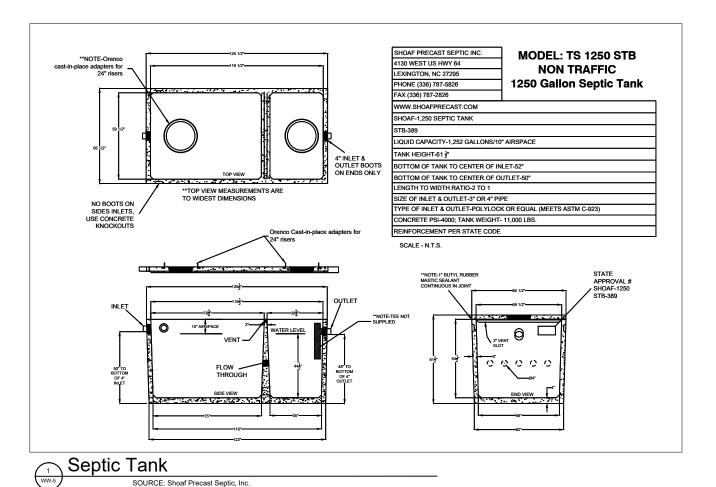
Repair Drainfield

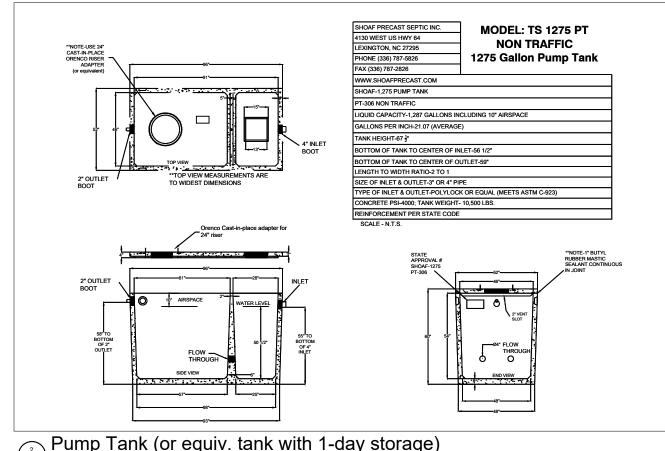
DRAWN BY: CREATED ON: 6/8/2023

REVISED BY: REVISED ON: ####

RELEASED BY: RELEASED ON: #####

DRAWING NUMBER





SOURCE: Shoaf Precast Septic, Inc.

NOTES

1. Installation to follow all NC DHHS and Harnett County applicable rules and regulations.

2. Harnett County to perform construction inspections and final system certification.

3. Septic Tank to have approved effluent filter.

4. Contractor to abide by all safety regulations during system installation.

5. Contractor shall backfill around all access areas such that storm water is shed away from potential entry points.

6. Invert elevations of all components to be verified in field by contractor to insure proper operation.

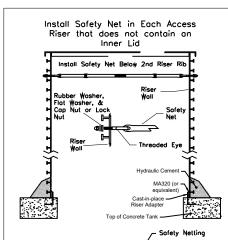
7. All system piping to be SCH40 PVC (except where noted).

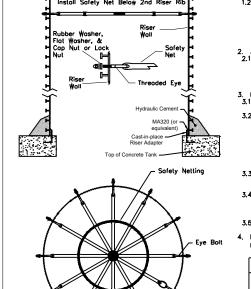
8. All gravity elbows to be long radius or long sweeping type elbows.

9. Actual installation and placement of treatment system to be overseen by Contractor.

10. Tanks to be set on 6" minimum gravel base. Use #5 or #57 stone for base.

- 11. Contractor to seed and/or mulch disturbed areas to coincide with existing landscape. Area shall not be left with uncovered
- 12. Mount Control Panel a minimum of 24" above grade.
- 13. Power to panel to be installed by licensed electrician per code. One 15-amp circuit and one 20-amp circuit with individual neutrals to be run from house to control panel.
- 14. All risers to have cast-in-place tank adapters and be single-piece riser. Risers to extend 6" above soil surface and be designed to prevent surface water inflow.
- 15. Backfill around tank(s) shall be gravel or tank hole shall be over-excavated a minimum of 2' in all directions to allow for mechanical tamping of backfill.
- 16. All penetrations to be sealed.
- 17. Spigot to be located on outside of building within 50' of
- 18. All pressure lines to maintain 18" min. cover.
- 19. Contractor to adjust tank placement to meet site constraints.





- sandpaper.
 Use a clean cloth and acetone or olcohol to clean the banding surfaces of the adopter and riser. The banding surfaces must be clean and dry for a good fit and watertight joint. Let the acetone or alcohol dry completely. by Adhesive

- odopter. One /-oz packet of MA320 adhesive is typical for one 24" riser.

 3. Instoll Riser

 3.1. If the riser has penetrations, align the riser correctly.

 3.2. Firmly press the riser onto the adopter until the bottom of the riser is resting on the concrete (cost-in-adopters) or the adopter flange (botted-down adopters). Twist the riser back and forth slightly to fully seat it an to create a good band.

 3.3. Apply a bead of methacrylate adhesive to the inside of the access riser-adopter joint.

 3.4. Use a tangue depressor, putty knife, or clean cloth to make a continuous fillet on the inside of the access riser-adopter joint.

 3.5. Apply hydroulic cement to bond
- 3.5. Apply hydraulic cement to bond outer riser wall and top of tank.

 4. Ensure safety net and inner lid are

FOR RISER WALL PI	ENETRATIONS
Grommet Size, Inches (Nominal IPS Pipe Size)	Hole Saw Size, Inches
1/2	1
3/4	1 1/4
1	1 9/16
1 1/4	1 3/4
1 1/2	2 1/8
2	2 3/4
3	3 7/8
4	5

Riser Safety Nets

NOTE: Install 4 Ring Clamps i 8" (or larger) Filter is Used

SOURCE: SIM-TECH. Inc.



Ballentine Associates, PA Birchwood Trails - Lot 71

919-859-0669

www.agriwaste.com

Olive Branch Rd Fuquay Varina, NC 27526 Harnett County

Project Owner Ballentine Associates, PA 221 Providence Rd Chapel Hill, NC 27514 919-929-0481 dillons@ballenti



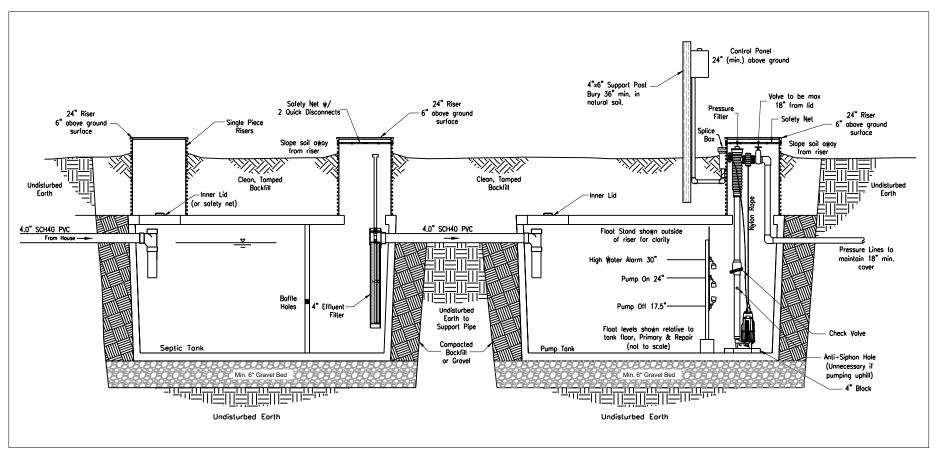
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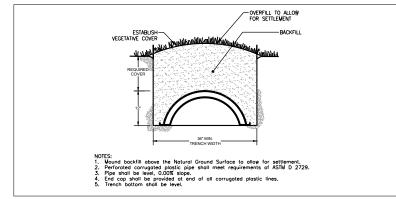
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Detail Sheet '

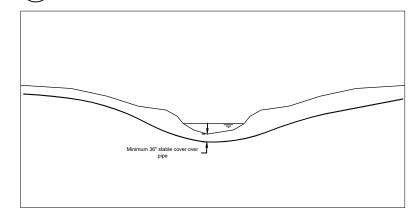
CREATED ON: DRAWN BY: H. Clapp REVISED BY: REVISED ON: RELEASED BY: RELEASED ON:

DRAWING NUMBER

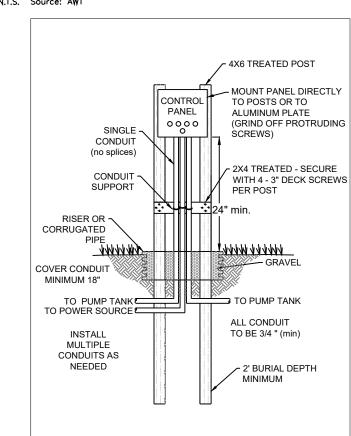




TRENCH X-SECTION (Typical) WW-6 N.T.S. Source: AWT

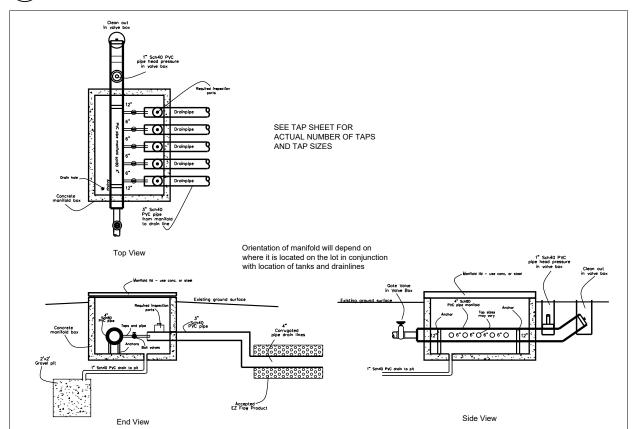


STREAM CROSSING MIN. DEPTH WW-6 N.T.S. Source: AWT

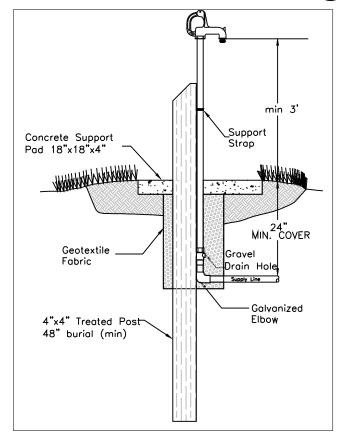


6 CONTROL PANEL SUPPORT

SYSTEM PROFILE VIEW N.T.S.



PRESSURE MANIFOLD INSTALLATION (Typical) - For Illustration Only WW-6 N.T.S. SOURCE: AWT



YARD HYDRANT (if required)

N.T.S. SOURCE: AWT

WW-6 N.T.S. SOURCE: AWT

501 N. Salem Street, Suite 203 Apex, North Carolina 27502 919-859-0669 www.agriwaste.com

Ballentine Associates, PA Birchwood Trails - Lot 71

Proiect Location: Olive Branch Rd Fuquay Varina, NC 27526 Harnett County PIN: ----

Project Owner Ballentine Associates, PA 221 Providence Rd Chapel Hill, NC 27514 919-929-0481 dillons@ballentine



REV. ISSUED DATE DESCRIPTION

SHEET TITLE

Detail Sheet 2

CREATED ON: DRAWN BY: H. Clapp 6/8/2023 REVISED ON: REVISED BY: #### RELEASED BY: RELEASED ON:

DRAWING NUMBER

Trenching and Excavation Safety

The employer must comply with the trenching and excavation requirements of 29 CFR 1926.651 and 1926.652 or comparable OSHA-approved state plan requirements

Inspection of Excavations

OSHA standards require that a competent person inspect trenches daily and as conditions change before worker entry to ensure elimination of excavation hazards. A competent person is an individual who is capable of identifying existing and predictable hazards or working conditions that are hazardous, unsanitary, or dangerous to workers, soil types and protective systems required, and who is authorized to take prompt corrective measures to eliminate these hazards and conditions.

OSHA standards require safe access and egress to all excavations, including ladders, steps, ramps, or other safe means of exit for employees working in trench excavations 4 feet (1.22 meters) or deeper. These devices must be located within 25 feet (7.6 meters) of all workers.

Heavy equipment and trucks should stay as far as possible from the edge of any trench. Always use pads under stabilizers to minimize ground pressures that could lead to failures

"Cemented soil" means a soil in which the particles are held together by a chemical agent, such as calcium carbonate, such that a hand-size sample cannot be crushed into powder or individual soil particles by finger

"Cohesive soil" means clay (fine grained soil), or soil with a high clay content, which has cohesive strength. Cohesive soil does not crumble, can be excavated with vertical sideslopes, and is plastic when moist. Cohesive soil is hard to break up when dry, and exhibits significant cohesion when submerged. Cohesive soils include clayey silt, sandy clay, silty clay, clay and organic clay.

"Dry soil" means soil that does not exhibit visible signs of moisture content.

"Fissured" means a soil material that has a tendency to break along definite planes of fracture with little

resistance, or a material that exhibits open cracks, such as tension cracks, in an exposed surface.

"Granular soil" means gravel, sand, or silt (coarse grained soil) with little or no clay content. Granular soil has no cohesive strength. Some moist granular soils exhibit apparent cohesion. Granular soil cannot be molded

when moist and crumbles easily when dry. "Lavered system" means two or more distinctly different soil or rock types arranged in lavers. Micaceous

seams or weakened planes in rock or shale are considered layered.

"Moist soil" means a condition in which a soil looks and feels damp. Moist cohesive soil can easily be shaped into a ball and rolled into small diameter threads before crumbling. Moist granular soil that contains some e material will exhibit signs of cohesion between particles.

"Plastic" means a property of a soil which allows the soil to be deformed or molded without cracking, or

EXCAVATIONS IN TYPE A SOILS

appreciable volume change.

"Saturated soil" means a soil in which the voids are filled with water. Saturation does not require flow. Saturation, or near saturation, is necessary for the proper use of instruments such as a pocket penetrometer or

All simple slope excavations 20 feet or less in depth shall have a maximum allowable slope of 3/2:1.

"Soil classification system" means, for the purpose of this subpart, a method of categorizing soil and rock deposits in a hierarchy of Stable Rock, Type A, Type B, and Type C, in decreasing order of stability. The categories are determined based on an analysis of the properties and performance characteristics of the deposits and the characteristics of the deposits and the characteristics of the deposits and the environmental conditions of exposure.

"Stable rock" means natural solid mineral matter that can be excavated with vertical sides and remain intact

"Submerged soil" means soil which is underwater or is free seeping.

"Type A" means cohesive soils with an unconfined, compressive strength of 1.5 ton per square foot (tsf) (144 kPa) or greater. Examples of cohesive soils are: clay, silty clay, sandy clay, clay loam and, in some cases silty clay loam and sandy clay loam. Cemented soils such as caliche and hardpan are also considered Type

However, no soil is Type A if: The soil is fissured; or

- The soil is subject to vibration from heavy traffic, pile driving, or similar effects; or
- (iii) The soil has been previously disturbed; or
- (iv) The soil is part of a sloped, layered system where the layers dip into the excavation on a slope of four
- horizontal to one vertical (4H:1V) or greater; or

 (v) The material is subject to other factors that would require it to be classified as a less stable material

- (i) Cohesive soil with an unconfined compressive strength greater than 0.5 tsf (48 kPa) but less than 1.5
- tsf (144 kPa); or

 (ii) Granular cohesionless soils including: angular gravel (similar to crushed rock), silt, silt loam, sandy
- loam and, in some cases, silty clay loam and sandy clay loam.

) Previously disturbed soils except those which would otherwise be classed as Type C soil
- (iv) Soil that meets the unconfined compressive strength or cementation requirements for Type A, but is fissured or subject to vibration; or
- (vi) Material that is part of a sloped, layered system where the layers dip into the excavation on a slope less steep than four horizontal to one vertical (4H:1V), but only if the material would otherwise be classified as Type B.

"Type C" means

- Cohesive soil with an unconfined compressive strength of 0.5 tsf (48 kPa) or less; or
- (ii) Granular soils including gravel, sand, and loamy sand; or (iii) Submerged soil or soil from which water is freely seeping; or
- (iv) Submerged rock that is not stable or

horizontal to one vertical (4H:1V) or steeper

(v) Material in a sloped, layered system where the layers dip into the excavation or a slope of four

"Unconfined compressive strength" means the load per unit area at which a soil will fail in compression. It can be determined by laboratory testing, or estimated in the field using a pocket penetrometer, by thumb penetration tests, and other methods.

"Wet soil" means soil that contains significantly more moisture than moist soil, but in such a range of values that cohesive material will slump or begin to flow when vibrated. Granular material that would exhibit cohe properties when moist will lose those cohesive properties when wet.

(c) Requirements

- (1) Classification of soil and rock deposits. Each soil and rock deposit shall be classified by a competent person as Stable Rock, Type A, Type B, or Type C in accordance with the definitions set forth in paragraph (b) of this appendix.
- (2) Basis of classification. The classification of the deposits shall be made based on the results of at least one visual and at least one manual analysis. Such analyses shall be conducted by a competent person using tests described in paragraph (d) below, or in other recognized methods of soil classification and testing such as those adopted by the American Society for Testing Materials, or the U.S. Department of Agriculture textural classification system.
- (3) Visual and manual analyses. The visual and manual analyses, such as those noted as being acceptable in paragraph (d) of this appendix, shall be designed and conducted to provide sufficient quantitative and qualitative information as may be necessary to identify properly the properties, factors, and conditions affecting the classification of the deposits.
- (4) Layered systems. In a layered system, the system shall be classified in accordance with its weakest layer. However, each laver may be classified individually where a more stable laver lies under a less stable laver
- (5) Reclassification. If, after classifying a deposit, the properties, factors, or conditions affecting its classification change in any way, the changes shall be evaluated by a competent person. The deposit shall be sified as necessary to reflect the changed circumstances

(d) Acceptable visual and manual tests

- (1) Visual tests. Visual analysis is conducted to determine qualitative information regarding the excavation site
- of particle sizes and the relative amounts of the particle sizes. Soil that is primarily composed of fine-grained material material is cohesive material. Soil composed primarily of coarse-grained sand or
- breaks up easily and does not stay in clumps is granular.
- side, the soil could be fissured. Small spalls are evidence of moving ground and are indications of potentially hazardous situations.
- (iv) Observe the area adjacent to the excavation and the excavation itself for evidence of existing utility and
- surface water, water seeping from the sides of the excavation, or the location of the level of the water
- that may affect the stability of the excavation face.

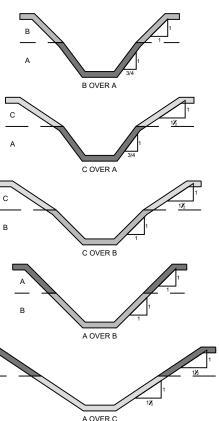
EXCAVATIONS IN TYPE C SOILS

- (2) Manual tests. Manual analysis of soil samples is conducted to determine quantitative as well as qualitative properties of soil and to provide more information in order to classify soil properly
- (i) Plasticity. Mold a moist or wet sample of soil into a ball and attempt to roll it into threads as thin as 1/8-inch in diameter. Cohesive material can be successfully rolled into threads without crumbling. For example, if at least a two inch (50 mm) length of 1/8-inch thread can be held on one end without
- (ii) Dry strength. If the soil is dry and crumbles on its own or with moderate pressure into individual grains or fine powder, it is granular (any combination of gravel, sand, or sitt). If the soil is dry and falls into clumps which break up into smaller clumps, but the smaller clumps can only be broken up with difficulty, it may be clay in any combination with gravel, sand or silt. If the dry soil breaks into clumps which do not break up into small clumps and which can only be broken with difficulty, and there is no visual indication the soil is fissured, the soil may be considered unfissured.
- (iii) Thumb penetration. The thumb penetration test can be used to estimate the unconfined compressive strength of cohesive soils. (This test is based on the thumb penetration test described in American Society for Testing and Materials (ASTM) Standard designation D2488 - "Standard Recommended Practice for Description of Soils (Visual - Manual Procedure).") Type A soils with an unconfined compressive strength of 1.5 tsf can be readily indented by the thumb; however, they can be penetrate by the thumb only with very great effort. Type C soils with an unconfined compressive strength of 0.5 tsf can be easily penetrated several inches by the thumb, and can be molded by light finger pressure. This test should be conducted on an undisturbed soil sample, such as a large clump of spoil, as soon as practicable after excavation to keep to a minimum the effects of exposure to drying influences. If the excavation is later exposed to wetting influences (rain, flooding), the classification of the soil must be changed accordingly
- (iv) Other strength tests. Estimates of unconfined compressive strength of soils can also be obtained by use of a pocket penetrometer or by using a hand-operated shearvane.
- (v) Drying test. The basic purpose of the drying test is to differentiate between cohesive material with fissures, unfissured cohesive material, and granular material. The procedure for the drying test involved. drying a sample of soil that is approximately one inch thick (2.54 cm) and six inches (15.24 cm) in eter until it is thoroughly dry:
- (A) If the sample develops cracks as it dries, significant fissures are indicated.
- (B) Samples that dry without cracking are to be broken by hand. If considerable force is necessary to break a sample, the soil has significant cohesive material content. The soil can be classified as an unfissured cohesive material and the unconfined compressive strength should be determined.

 (C) If a sample breaks easily by hand, it is either a fissured cohesive material or a granular material.
- To distinguish between the two, pulverize the dried clumps of the sample by hand or by stepping on them. If the clumps do not pulverize easily, the material is cohesive with fissures. If they pulverize easily into very small fragments, the material is granular.

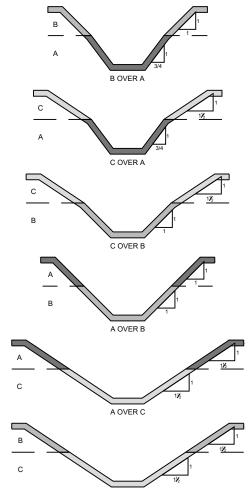
EXCAVATIONS IN LAYERED SOILS

All excavations 20 feet or less in depth made in layered soils shall have a maximum allowable



All other sloped excavations shall be in accordance with the other options permitted in § 1926.652(b).

slope for each layer as set forth below



20' Max

20' Max

- in general, the soil adjacent to the excavation, the soil forming the sides of the open excavation, and the soil taken as samples from excavated material.
- (i) Observe samples of soil that are excavated and soil in the sides of the excavation. Estimate the range gravel is granular material.

 Observe soil as it is excavated. Soil that remains in clumps when excavated is cohesive. Soil that
- (iii) Observe the side of the opened excavation and the surface area adjacent to the excavation. Crack-like openings such as tension cracks could indicate fissured material. If chunks of soil spall off a vertical
- other underground structures, and to identify previously disturbed soil.

 (v) Observe the opened side of the excavation to identify layered systems. Examine layered systems to
- identify if the layers slope toward the excavation. Estimate the degree of slope of the layers.

 (vi) Observe the area adjacent to the excavation and the sides of the opened excavation for evidence of
- (vii) Observe the area adjacent to the excavation and the area within the excavation for sources of vibration

All simple slope excavations 20 feet or less in depth shall have a maximum allowable slope of 1½:1.

SIMPLE SLOPE

VERTICALLY SIDED LOWER PORTION

All other sloped excavations shall be in accordance with the other options permitted in § 1926.652(b).

All excavations 20 feet or less in depth which have vertically sided lower portions that are supported or shielded shall have a maximum allowable slope of 1½:1. The support or shield m must extend at least 18 inches above the top of the vertical sid

501 N. Salem Street, Suite 203 Apex, North Carolina 27502

> www.agriwaste.com Ballentine Associates, PA

919-859-0669

Birchwood Trails - Lot 71

Olive Branch Rd Fuquay Varina, NC 27526 Harnett County

oiect Owner:

Ballentine Associates, PA 221 Providence Rd Chapel Hill, NC 27514 dillons@ballentineas

NC ONSITE WASTEWATER EVALUATOR SEAL



rev. Issued date description

SHEET TITLE

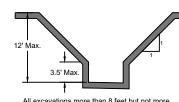
Excavation Safety

DRAWN BY: CREATED ON: 6/8/2023 H. Clapp REVISED BY: REVISED ON: #### RELEASED ON: RELEASED BY: ####

drawing number



All excavations 8 feet or less in depth which have unsupported vertically sided lower portions shall have a maximum vertical side of 31/2 feet



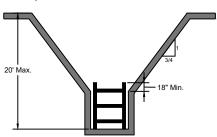
Exception: Simple slope excavations which are open 24 hours or less (short term) and which are 12 feet or less in depth shall have a

maximum allowable slope of 1/2:1

All excavations more than 8 feet but not more than 12 feet in depth with unsupported vertically sided lower portions shall have a maximum allowable slope of 1:1 and a maximum vertical side of 31/2 fee

All excavations 20 feet or less in depth which have vertically sided lower portions that are supported or shielded shall have a maximum allowable slope of 3/4:1. The support or shield system must extend at least 18 inches above the top of the vertical side

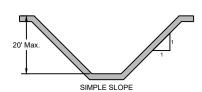
All benched excavations 20 feet or less in depth shall have a maximum allowable slope of 3/2:1



All other simple slope, compound slope, and vertically sided lower portion excavations shall be in accordance with the other options permitted under § 1926.652(b).

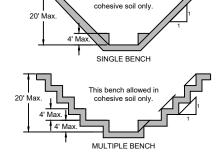
EXCAVATIONS IN TYPE B SOILS

All simple slope excavations 20 feet or less in depth shall have a maximum allowable slope of 1:1.

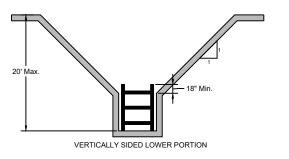


All benched excavations 20 feet or less in depth shall have a maximum allowable slope of 1:1 and maxium bench dimensions as follows

This bench allowed in



All excavations 20 feet or less in depth which have vertically sided lower portions that are supported or shielded shall have a maximum allowable slope of 1:1. The support or shield system must extend at least 18 inches above the top of the vertical side.



All other sloped excavations shall be in accordance with the other options permitted in § 1926.652(b).

Septic System Design - Summary Page

Engineers and Soil Scientists

Agri-Waste Technology, Inc. 501 N Salem Street, Suite 203, Apex. NC 27502 agriwaste.com | 919.859.0669

Project Manager:

Jeff Vaughan, PhD, LSS jvaughan@agriwaste.com 919-859-0669

Engineer:

Heath Clapp

hclapp@agriwaste.com

Project: Birchwood Trails, Lot 71

Property: Olive Branch Road,

Fuquay Varina, NC 27526

Subdiv.:

Lot #: 71

Owner: Ballentine Associates, PA

Address: 221 Providence Road,

Chapel Hill, NC 27514

Phone: 919-929-0481

Email: dillons@ballentineassociates.com

EHS:

Soil Parameters

Soil Evaluation By:

LTAR:

gpd/ft² 0.400

Special Conditions/Notes:

Design Parameters

Type of Establishment: Residence, 5 or fewer bedrooms

Unit: Bedroom # of Units:

Septic Tank Specifications

Min. Tank Capacity: 1,000 gal **Actual Tank Volume:** 1,250 gal **Tank Manufacturer:** Shoaf

Tank Model: TS 1250 STB

Exterior Interior

Length: 125.5 119.5 in. Width: 65.5 59.5 in. Depth: 62.0 54.5 in.

Primary Drainfield Specifications

Type of Distribution: Parallel Pressure Manifold

Trench Media: EZflow **Trench Width:** 3 ft

Trench Depth: 22 in. (or as specified on permit) **Trench Bottom Area: Minimum Drain Line:**

Actual Drain Line:

1200 300

ft 300 ft

 ft^2

Date:

County:

Permit #:

PIN:

Type of System:

6/8/2023

Harnett

III b

0

Number of Lines: 4

9

Minimum Line Spacing: ft O.C.

Wastewater Treatment System Design Calculations

Project: Birchwood Trails, Lot 71 **Location:** Olive Branch Road,

Fuquay Varina, NC 27526

County: Harnett

Septic Tank Sizing

Daily Flow Estimate:

Unit	# of Units	Flow/Unit	Flow/Day
Bedroom	4	120	480
			0
			0
		Q=	480

Septic Tank Minimum Capacity:

Per NCAC T15A:18A .1952(b)(1):

For individual residences with 4 bedrooms,

Minimum Liquid Capacity (V)= 1,000 gal

Septic Tank Specs:

Manufacturer: Shoaf

Model: TS 1250 STB

Volume: 1,250 gal Weight: 11,000 lbs

 Exterior
 Interior

 Length:
 125.5
 119.5
 in.

 Width:
 65.5
 59.5
 in.

 Depth:
 62.0
 54.5
 in.

Shape of Risers: Circular

Diameter: 2.00 ft

Pump Tank Storage & Float Settings

Project: Birchwood Trails, Lot 71 **Location:** Olive Branch Road,

Fuguay Varina, NC 27526

County: Harnett

Tank Manufacturer Shoaf

Tank Model TS 1275 PT

Interior Height (in.) 60.5 in. Avg. Storage 21.07 gal/in.

Primary System

Elevations, measured from bottom towards top (0 = Interior Bottom of Tank):

Top of pump (including 4" block) 15.7 in. (Pump height = 11 11/16")

Pump Off 17.5 in.

Pump On 24.0 in. (set for dose volume)
Alarm On 30.0 in. (6 in. above On Float)

Emergency Storage Available

Pump Tank 643 gal
Days of Storage 1.34 days
(determined from "interior top of tank" - "High Water Alarm")

Repair System

Elevations, measured from bottom towards top (0 = Interior Bottom of Tank):

Top of pump (including 4" block) 15.7 in. (Pump height = 11 11/16")

Pump Off 17.5 in.

Pump On 26.5 in. (set for dose volume)
Alarm On 32.5 in. (6 in. above On Float)

Emergency Storage Available

Pump Tank 590 gal
Days of Storage 1.23 days
(determined from "interior top of tank" - "High Water Alarm")

ELEVATIONS

Project: Birchwood Trails, Lot 71 Location: Olive Branch Road, Fuquay Varina, NC 27526

County:	Harnett	5
Benchmark	0	
BM Elev	0 ft	
Septic Tank	1,250 gal	
Ground Surface		292.00 ft
Depth of Soil Cover	16 in.	1.33 ft
Overall Ht of Tank	61.5 in.	5.13 ft
Elev, Base of Tank		285.54 ft
Ht to 4" Inlet Invert	50 in.	4.17 ft
Elev, 4" Inlet Invert		289.71 ft
Ht to 4" Outlet Invert	48 in.	4.00 ft
Elev, 4" Outlet Invert		289.54 ft
Gravel Base	6 in.	0.50 ft
Elev, Bot of Excavation		285.04 ft
Pump Tank	1275 gal	
Ground Surface		292.00 ft
Depth of Soil Cover	18 in.	1.50 ft
Overall Ht of Tank	67.5 in.	5.63 ft
Elev, Base of Tank		284.88 ft
Ht to 4" Inlet Invert	54.5 in.	4.54 ft
Elev, 4" Inlet Invert		289.42 ft
Ht to 2" Outlet Invert	58 in.	4.83 ft
Elev, 2" Outlet Invert		289.71 ft
Gravel Base	6 in.	0.50 ft
Elev, Bot of Excavation		284.38 ft
ST Inlet Pipe	_	
Grade @ Stub-out		292 ft
Depth of Stub-out, top		
Dopin or otal oat, top		1.5 ft
Elev, Stub-out Invert		1.5 ft 290.15 ft
	L	
Elev, Stub-out Invert		290.15 ft 289.71 ft 10 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert		290.15 ft 289.71 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length		290.15 ft 289.71 ft 10 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope	4 in.	290.15 ft 289.71 ft 10 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT	4 in. 4.5 in.	290.15 ft 289.71 ft 10 ft 4.4 %
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT		290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD		290.15 ft 289.71 ft 10 ft 4.4 %
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert		290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope		290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 %
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length		290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope		290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 %
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe		290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 %
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe	4.5 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 1.94 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe Pump Reqmt. Floor Thickness Elev, Pump Tank Floor Pump Block Ht.	4.5 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 1.94 ft 0.33 ft 0.33 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe Pump Reqmt. Floor Thickness Elev, Pump Tank Floor	4.5 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 % 1.94 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe Pump Reqmt. Floor Thickness Elev, Pump Tank Floor Pump Block Ht.	4.5 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 1.94 ft 0.33 ft 0.33 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe Pump Reqmt. Floor Thickness Elev, Pump Tank Floor Pump Block Ht. Elev, Pump Intake	4.5 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 % 1.94 ft 0.33 ft 0.33 ft 285.21 ft 0.33 ft 285.54 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe Pump Reqmt. Floor Thickness Elev, Pump Tank Floor Pump Block Ht. Elev, Pump Intake Grade @ Primary Manifold	4 in. 4 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 % 1.94 ft 0.33 ft 285.21 ft 0.33 ft 285.54 ft
Elev, Stub-out Invert Elev @ ST Inlet Invert Length Slope Pipe, ST to PT ID OD Elev, ST Outlet Invert Elev, PT Inlet Invert Length Slope Cover over inlet pipe Pump Reqmt. Floor Thickness Elev, Pump Tank Floor Pump Block Ht. Elev, Pump Intake Grade @ Primary Manifold Grade @ Repair Manifold	4 in. 4 in.	290.15 ft 289.71 ft 10 ft 4.4 % 0.33 ft 0.38 ft 289.54 ft 289.42 ft 4 ft 3.1 % 1.94 ft 0.33 ft 285.21 ft 0.33 ft 285.54 ft 294.00 ft 295.00 ft

Elev Diff, Repair

7.96 ft

Drainfield Design

Project Birchwood Trails, Lot 71 Location Olive Branch Road,

Fuquay Varina, NC 27526

County Harnett

Drainfield Sizing			
Primary			
LTAR	0.4 gpd/ft ²		
Daily Design Flow	480 gpd	Type of Drainfield Media	EZflow
Req. Drainfield Area	1,200 ft ²	Required Drainline	
Trench Width, Eff.	3 ft	After 25% Reduction	300 ft
Required Drainline	400 ft	Minimum Line Spacing	9 ft (O.C.)
Repair			
LTAR	0.4 gpd/ft ²		
Daily Design Flow	480 gpd	Type of Drainfield Media	PPBPS, Horizontal
Req. Drainfield Area	1,200 ft ²	Required Drainline	
Trench Width, Eff.	3 ft	After 50% Reduction	200 ft
Required Drainline	400 ft	Minimum Line Spacing	8 ft (O.C.)

Drainfield Layout

Line	Use	Flag Color	Elevation (ft)	Line Length (ft)	Used as Primary (ft)	Used as Repair (ft)
1	Layout Line	yellow		75	75.0	
2	Layout Line	red		75	75.0	
3	Layout Line	blue		75	75.0	
4	Layout Line	purple		75	75.0	
5	Layout Line	white		70		70.0
6	Layout Line	yellow		70		70.0
7	Layout Line	red		70		70.0
8	Layout Line					
9	Layout Line					
10	Layout Line					
11	Layout Line					
12	Layout Line					
13	Layout Line					
14	Layout Line					
	•		Total	510	300	210
			Count	7	4	3

Note: Line length totals are shown to the nearest foot.

PRESSURE MANIFOLD DESIGN (Primary)

Site Information

Birchwood Trails, Lot 71 Project: Location: Oliva Branch Road,

Fuquay Varina, NC 27526

County: Harnett

Design Information

Estimated Daily Flow 480 gal/day 0.4 gal/day/ft² L.T.A.R. 0.420 gal/day/ft² L.T.A.R. + 5% Trench Width 3 ft. 400 ft. Line Length Required Length after 25% Reduction 300 ft

0.533 gal/day/ft² L.T.A.R. Reduced 0.560 gal/day/ft² L.T.A.R. Reduced + 5%

DRAINFIELD INFO. - Primary

Proposed Type of System/Distribution: Pump to Pressure Manifold

using EZflow

	Flag	Line	_	Flow	Flow/Foot	Line
Line No.	Color	Length (ft)	Тар	(gpm)	(gpm/ft)	L.T.A.R.
1	yellow	75	1/2in SCH 80	5.48	0.073	0.533
2	red	75	1/2in SCH 80	5.48	0.073	0.533
3	blue	75	1/2in SCH 80	5.48	0.073	0.533
4	purple	75	1/2in SCH 80	5.48	0.073	0.533
	Total	300	Total	21.92	Avg.	0.533

Note: Line lengths are calculated in 4'4" increments to reflect use of PPBPS product.

Total Run Time	21.90 min.
Drainfield Capacity	195.9 gal
% of Drainfield Cap	69.9%

(Req. Range 66-75%)

Dose Volume 136.9 gal/dose

6.2 minutes Run Time/Dose 21.07 gal/in. Volume/depth Estimated Drawdown 6.50 in.

Range 5-7 minutes unless uphill, checked (Per tank manufacturer's specifications)

Manifold Box

Number of Taps	4	
Manifold Length	3.5	ft.

0 Split(s) with (approximate)

PRESSURE MANIFOLD SYSTEM DESIGN (Repair)

Site Information

Project: Birchwood Trails, Lot 71 Location: Olive Branch Road,

Fuquay Varina, NC 27526

County: Harnett

Design Information

 $\begin{array}{ccc} \text{L.T.A.R. Reduced} & 0.800 \text{ gal/day/ft}^2 \\ \text{L.T.A.R. Reduced} + 5\% & 0.840 \text{ gal/day/ft}^2 \\ \end{array}$

DRAINFIELD INFO. - Repair

Proposed Type of System/Distribution: Pump to Pressure Manifold

using PPBPS, Horizontal

Line No.	Flag Color	Line Length (ft.)		Flow (gpm)	Flow/Foot (gpm/ft)	Line L.T.A.R.
5	white	70	1/2in SCH 40	7.11	0.102	0.762
6	yellow	70	1/2in SCH 40	7.11	0.102	0.762
7	red	70	1/2in SCH 40	7.11	0.102	0.762
	Total	210	Total	21.33	Avg.	0.762

Note: Line lengths are calculated in 4'4" increments to reflect use of PPBPS product.

Total Run Time 22.50 min.
Drainfield Capacity 345.6 gal
% of Drainfield Cap 54.9%

(Max. 105.0% to not exceed 7.2 gal/panel)

Dose Volume
Run Time/Dose
Volume/depth
nated Drawdown
Volume/depth
page 21.07 gal/in.
9.00 in.

Time to deliver max. 7.2 gal/panel (Per tank manufacturer's specifications)

Estimated Drawdown
Manifold Box

Number of Taps 3 with 0 Split(s)
Manifold Length 3.0 ft. (approximate)

PUMP DESIGN

System (initial/repair): Primary

Project: Birchwood Trails, Lot 71 Location: Olive Branch Road,

Fuquay Varina, NC 27526

County: Harnett

Friction Losses

Suction Head	0 ft	(submersible 0)
Elev. Difference (highest point from pump)	6.96 ft	
Design Pressure At Outlet	2 ft	
Supply Line - 1.5" Schedule 40 PVC		
Pipe Diameter, Nominal 1.5 in.		
Pipe Diameter (ID) 1.59 in.	Flow	21.92 gpm
Pipe Length 62 ft	Velocity	3.54 ft/sec
Pipe Length for Fittings 6.2 ft	Me	eets requirement that 2 ft/s < v < 5 ft/s.
Equivalent Length 68.2 ft		
Estimated Friction Loss in Supply Line	2.13 ft	
Friction Loss - Taps/Special Fittings	3.5 ft	
TOTAL	14.59 ft.	

Flow for Anti-Siphon Hole

3/16 in. Hole Diameter Hole Flowrate 1.58 gpm

Pump Efficiency 0.7 (assumed, typical) 0.9 (assumed for electric pumps) Motor Efficiency

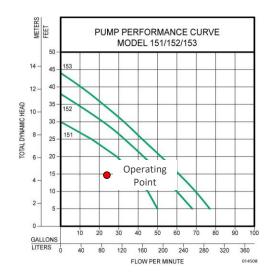
Flow 23.50 gpm

0.14 hp Required Horsepower TDH

14.59 ft

Pump Selection

Manufacture	Zoeller
Mode	N151
Horsepowe	0.33



PUMP DESIGN

System (initial/repair): Repair

Project: Birchwood Trails, Lot 71
Location: Olive Branch Road,
Fuquay Varina, NC 27526

County: Harnett

Friction Losses

Suction Head	0 ft	(submersible 0)
Elev. Difference (highest point from pump)	7.96 ft	
Design Pressure At Outlet	2 ft	
Supply Line - 1.5" Schedule 40 PVC		
Pipe Diameter, Nominal 1.5 in.		
Pipe Diameter (ID) 1.59 in.	Flow	21.33 gpm
Pipe Length 96 ft	Velocity	3.45 ft/s
Pipe Length for Fittings 9.6 ft	Me	eets requirement that 2 ft/s < v < 5 ft/s.
Equivalent Length 105.6 ft		
Estimated Friction Loss in Supply Line	3.14 ft	
Friction Loss - Taps/Special Fittings	3.5 ft	
TOTAL	16.59 ft.	

Flow for Anti-Siphon Hole

Hole Diameter 3/16 in.
Hole Flowrate 1.69 gpm

Pump Efficiency 0.7 (assumed, typical)

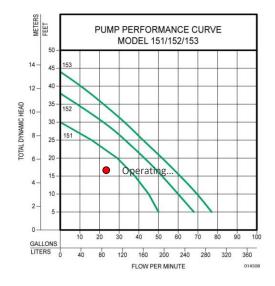
Motor Efficiency 0.9 (assumed for electric pumps)

Flow 23.02 gpm

Required Horsepower 0.15 hp TDH 16.59 ft.

Pump Selection

Manufacturer:	Zoeller
Model:	N151
Horsepower:	0.33



Septic Tank Buoyancy Calculation

Project: Birchwood Trails, Lot 71
Location: Olive Branch Road,

Fuquay Varina, NC 27526

County: Harnett

Tank Size (nominal) 1250 gal

Properties/Assumptions:

Min. liquid level to be maintained in tank at all times after initial installation.						
Min. depth to water table Effluent Density Concrete Density	12.0 in. 62.4 lb/ft ³ 142.6 lb/ft ³	from ground surface (Specific Weight of Water)				
Soil App. Sp. Grav.	1.3	(typical value)				
Soil Cover Over Tank Additional Cover	12 in. 4 in.	(minimum) for pipe grade				
Unsubmerged wt of soil Submerged wt of soil	81.1 lb/ft ³ 49.9 lb/ft ³	50% Porosity Assumed				

Tank Dimensions (from supplier):

Tank Dimensions	(пот варрі	101).				
		<u>Exterior</u>		<u>Inter</u>	<u>ior</u>	
		Тор	Bottom	Тор	Bottom	
Tank	Length	125.5	122.0	119.5	116.0 in.	
	Width	65.5	62.0	59.5	56.0 in.	
	Height	58.5	(w/o lid)	54.5	in.	
Lid	Length	125.5 i	n.			
	Width	65.5 i	n.			
	Height	3.0 i	n.			
Aı	rea of Riser	Openings	6.28 ft ²			
Permanent Liquid Depth in Tank 0.0 in.					0.00 ft	
Tank Weight 11,000 lb					per manufacturer)	

Buoyancy Force Calculation:

Buoyancy Force Specific Weight of Water x Displaced Volume

Displaced Volume 283.5 ft³ *

Buoyancy Force 17,689 lb.

Weight Calculation:

Tank Weight	11000 lb		
Water Weight in Tank	0 lb	Volume	0.0 ft ³ *
Soil Weight Over Tank	4966 lb		
Soil Friction Force	4037 lb		
Total Weight	20,004 lb		

Factor of Safety = 1.13

Note: Total weight must be greater than buoyancy force so that tank will not float during high water table conditions.

^{*} Volume calculated by the prismoidal formula.

Pump Tank Buoyancy Calculation

Project: Birchwood Trails, Lot 71
Location: Olive Branch Road,

Fuquay Varina, NC 27526

County: Harnett

Tank Size (nominal) 1275 gal

Properties/Assumptions:

Min. liquid level to be maintained in tank at all times after initial installation.						
Min. depth to water table Effluent Density	12 in. 62.4 lb/ft ³	from ground surface (Specific Weight of Water)				
Concrete Density Soil App. Sp. Grav.	142.6 lb/ft ³ 1.3	(typical value)				
Soil Cover Over Tank	12 in.	(minimum)				
Additional Cover	6 in.	for pipe grade				
Unsubmerged wt of soil	81.1 lb/ft ³					
Submerged wt of soil	49.9 lb/ft ³	50% porosity assumed				

Tank Dimensions (from supplier):

Tank Dimensions	(Irom supp	iler):			
·		<u>Exterior</u>		<u>Inte</u>	<u>rior</u>
		Тор	Bottom	Тор	Bottom
Tank	Length	108.0	104.0	102.0	98.0 in.
	Width	58.0	54.0	52.0	48.0 in.
	Height	64.5	(w/o lid)	60.5	in.
Lid	Length	108.0	in.		
	Width	58.0	in.		
	Height	3.0	in.		
A	rea of Riser	Openings	3.14	ft ²	
Permanent	Liquid Dep	th in Tank	0.0	in.	0.00 ft
	Tar	nk Weight	10500	lb	(per manufacturer)

Buoyancy Force Calculation:

Buoyancy Force	14,606 lb					
Displaced Volume	234.1 ft ³ *					
Buoyancy Force Specific Weight of Water x Displaced Volume						
buoyancy i orce calculation.						

Weight Calculation:

Tank Weight	10500 lb		
Water Weight in Tank	0 lb	Volume	0.0 ft ³ *
Soil Weight Over Tank	4281 lb		
Soil Friction Force	4227 lb		
Total Weight	19,008 lb		

Factor of Safety = 1.30

Note: Total weight must be greater than buoyancy force so that tank will not float during high water table conditions.

^{*} Volume calculated by the prismoidal formula.



GKROHL



ACORD'

1/23/2023

THIS CERTIFICATE IS ISSUED AS A MATTER OF INFORMATION ONLY AND CONFERS NO RIGHTS UPON THE CERTIFICATE HOLDER. THIS CERTIFICATE DOES NOT AFFIRMATIVELY OR NEGATIVELY AMEND, EXTEND OR ALTER THE COVERAGE AFFORDED BY THE POLICIES BELOW. THIS CERTIFICATE OF INSURANCE DOES NOT CONSTITUTE A CONTRACT BETWEEN THE ISSUING INSURER(S), AUTHORIZED REPRESENTATIVE OR PRODUCER. AND THE CERTIFICATE HOLDER.

IMPORTANT: If the certificate holder is an ADDITIONAL INSURED, the policy(ies) must have ADDITIONAL INSURED provisions or be endorsed. If SUBROGATION IS WAIVED, subject to the terms and conditions of the policy, certain policies may require an endorsement. A statement on this certificate does not confer rights to the certificate holder in lieu of such endorsement(s).

PRODUCER	CONTACT Connie Garkains					
Hartsfield & Nash Agency, Inc. 10405 Ligon Mill Rd., Ste H	PHONE (A/C, No, Ext): (919) 556-3698 FAX (A/C, No): (919)	556-8758				
Wake Forest, NC 27587	E-MAIL ADDRESS: Connie@hartsfield-nash.com					
	INSURER(S) AFFORDING COVERAGE	NAIC #				
	INSURER A: Selective Insurance Company of the Southeast	39926				
INSURED	INSURER B: ACCIDENT FUND INSURANCE COMPANY OF AMERICA	10166				
Agri-Waste Technology Inc	INSURER C : Evanston Insurance Company					
501 N. Salem St Ste 203	INSURER D:					
Apex, NC 27502	INSURER E:					
	INSURER F:					

COVERAGES CERTIFICATE NUMBER: REVISION NUMBER:

THIS IS TO CERTIFY THAT THE POLICIES OF INSURANCE LISTED BELOW HAVE BEEN ISSUED TO THE INSURED NAMED ABOVE FOR THE POLICY PERIOD INDICATED. NOTWITHSTANDING ANY REQUIREMENT, TERM OR CONDITION OF ANY CONTRACT OR OTHER DOCUMENT WITH RESPECT TO WHICH THIS CERTIFICATE MAY BE ISSUED OR MAY PERTAIN, THE INSURANCE AFFORDED BY THE POLICIES DESCRIBED HEREIN IS SUBJECT TO ALL THE TERMS, EXCLUSIONS AND CONDITIONS OF SUCH POLICIES. LIMITS SHOWN MAY HAVE BEEN REDUCED BY PAID CLAIMS.

INSR LTR	TYPE OF INSURANCE	ADDL S	UBR	POLICY EFF (MM/DD/YYYY)	POLICY EXP	LIMIT	s
Α	X COMMERCIAL GENERAL LIABILITY			(······	EACH OCCURRENCE	\$ 2,000,000
	CLAIMS-MADE X OCCUR		S 2253659	1/18/2023	1/18/2024	DAMAGE TO RENTED PREMISES (Ea occurrence)	\$ 300,000
						MED EXP (Any one person)	\$ 10,000
						PERSONAL & ADV INJURY	\$ 2,000,000
	GEN'L AGGREGATE LIMIT APPLIES PER:					GENERAL AGGREGATE	\$ 4,000,000
	POLICY X PRO- JECT LOC					PRODUCTS - COMP/OP AGG	\$ 4,000,000
	OTHER:						\$
Α	AUTOMOBILE LIABILITY					COMBINED SINGLE LIMIT (Ea accident)	\$ 1,000,000
	X ANY AUTO		S 2253659	1/18/2023	1/18/2024	BODILY INJURY (Per person)	\$
	OWNED SCHEDULED AUTOS					BODILY INJURY (Per accident)	\$
	HIRED NON-OWNED AUTOS ONLY					PROPERTY DAMAGE (Per accident)	\$
							\$
Α	X UMBRELLA LIAB X OCCUR					EACH OCCURRENCE	\$ 2,000,000
	EXCESS LIAB CLAIMS-MADE		S 2253659	1/18/2023	1/18/2024	AGGREGATE	\$ 2,000,000
	DED RETENTION \$						\$
В	WORKERS COMPENSATION AND EMPLOYERS' LIABILITY					X PER OTH- STATUTE ER	
	ANY PROPRIETOR/PARTNER/EXECUTIVE Y/N	N/A	100003072	1/18/2023	1/18/2024	E.L. EACH ACCIDENT	\$ 1,000,000
	(Mandatory in NH)	N/A				E.L. DISEASE - EA EMPLOYEE	
	If yes, describe under DESCRIPTION OF OPERATIONS below					E.L. DISEASE - POLICY LIMIT	\$ 1,000,000
С	Prof & Pollution		MKLV3ENV103400	8/22/2022	8/22/2023	Each Claim	5,000,000
Α	Leased / Rented		S 2253659	1/18/2023	1/18/2024	Equipment	25,000

DESCRIPTION OF OPERATIONS / LOCATIONS / VEHICLES (ACORD 101, Additional Remarks Schedule, may be attached if more space is required)

CERTIFICATE HOLDER CANCELLATION

***This is ONLY For Informational Purposes Contact Agency for Specific Holder info to be added SHOULD ANY OF THE ABOVE DESCRIBED POLICIES BE CANCELLED BEFORE THE EXPIRATION DATE THEREOF, NOTICE WILL BE DELIVERED IN ACCORDANCE WITH THE POLICY PROVISIONS.

AUTHORIZED REPRESENTATIVE

Bina Krohl