STONEFIELD-RALE

RALEIGH - LOT 00.0028 CAMPBELL RIDGE SF (MODEL# 1635)

ELEVATION 7 - GR

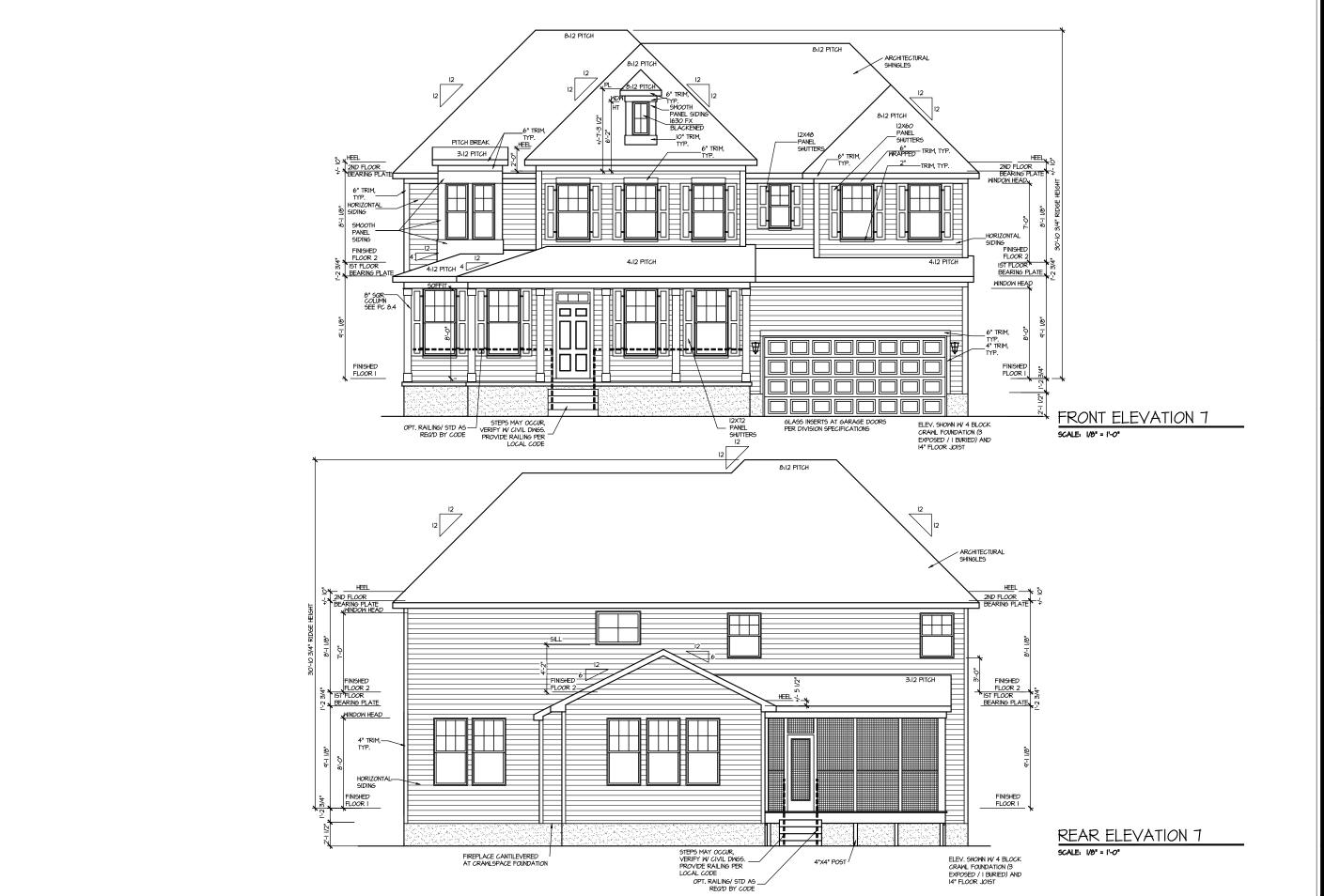
INDEX



ADEA CALCIII ATIONIC			
<u>AREA CALCULATIONS</u>		COVERED /	
ELEVATION 7	HEATED	UNHEATED	UNCOVERED
FIRST FLOOR	1542 SF		
GARAGE		496 SF	
FRONT PORCH - ELEVATION 7		214 SF	
SECOND FLOOR	1622 SF		
OPTIONS			
EXT. BRKFST W/ EXT. OWNER'S SUITE/ COVERED	+186 SF	+160 SF	
BED 5 W/ BATH 3	+55 SF	-55 SF	
FIREPLACE	+10 SF		
TOTAL	3415 SF	815 SF	

21 Alden Way

$I \cap T$	SPECIFIC	
1	LOT 00.0028	CAMPBELL RIDGE SF
		STONEFIELD REV. RALE—3 ELEVATION 7
2	ADDRESS	21 ALDEN WAY ANGIER, NC 27501
	ADDRESS	ZI ALDEN WAT ANGIER, NC 2/301
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 MASTER PLAN INFORMATION
 UPDATED DATE

 S-RALE
 07-31-2024
 04-30-2025

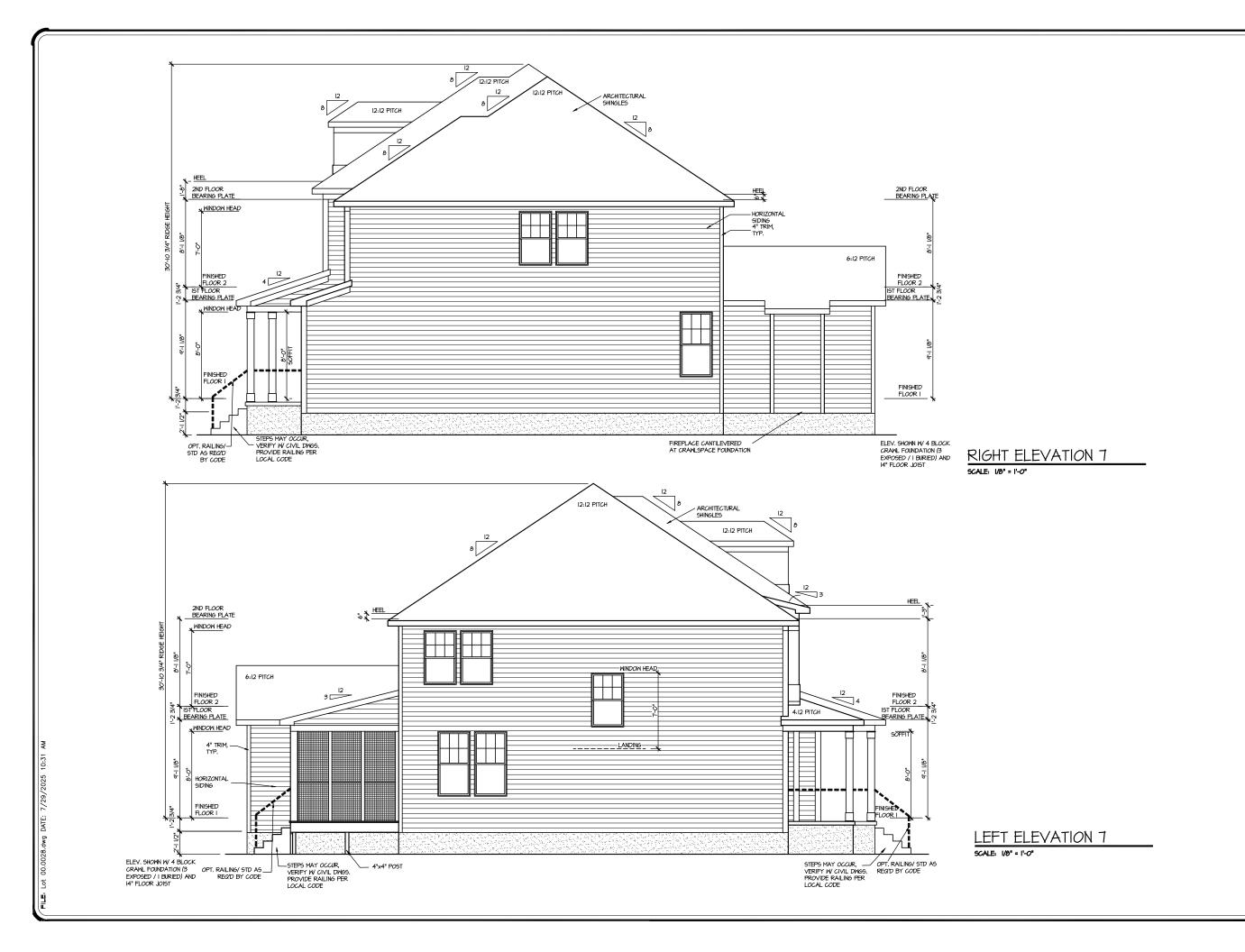
DRAWN BY:
ITS
DATE:
07/29/2025
PLAN NO.
1635



DRAWING TITLE FRONT & REAR ELEVATIONS

HOUSE NAME:
STONEFIELE
DRAWING TITLE

SHEET No.



| MASTER PLAN INFORMATION | WASTER PLAN INFORMATION | REVISION | DATE | 3-RALE | 07-31-2024 | 04-30-2025 |

DRAWN BY: ITS DATE: 07/29/2025 PLAN NO. 1635



HOUSE NAME:
STONEFIELD
DRAWING TITLE
RIGHT & LEFT ELEVATIONS

SHEET No.

ATTIC VENT CALCULATION FOR PLAN 'T'

UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA I = 1784 50. FT.

OVERAL REQUIRED VENTILATION.

1 TO 800 = 11421 50. FT.

1 TO 800 = 55.463 50. FT.

50% IN TOP THIRD = 2.48 50. FT. (1 TO 800)

NET FREE AREA OF NOTED SOFFT = 35 50. IN / LINEAR FT.

NET FREE AREA OF RIDGE VENT = 16 50. IN / LINEAR FT.

LOWER VENTINS. (BOTTON 2/3 RDS)

82 LINEAR FEET OF SOFFIT X 5.1 50. IN. = 3.245 50. FT.

IPPER VENTINS. (10P U/8 RD)

24 LINEAR FEET OF RIDGE X IB 50. IN. = 3 50. FT.

3 50. FT. AT 50%

(1 TO 500 ALLOYED)

UPPER ROOF VENTILATION CALCULATIONS:
ROOF AREA 3 = \$5.50. FT
OVERAL SEGUED VENTILATION:
10 300 = 0.00 50. FT
10 300 = 0.00 50. FT
506 IN TOP THERD = 0.005 50. FT, () TO 300)

NET FREE APEA OF VANIED SOFFIT = 5.1 SO, IN / LINEAR FT.
NET FREE APEA OF RIDGE VENT = 10 SQ, INV LINEAR FT.
LOWER VENTING. (BOTTOM 2/8 EDS)
25 LINEAR FEET OF COFFIT X 5.1 SQ, IN = 0.91 SQ, FT.
12PEX VANIES, LOTO 2/8 EDS
1 SQ, FT. SETWEN SO, IN = 1 SQ, FT.
1 SQ, FT. SETWEN SO, IN = 1 SQ, FT.
1 (TO 2/00 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS:
ROOF AREA 2 = 252 502 FT.
OATEAL SEQUED VENTILATION
1 DO 160 502 FT.
1 TO 302 160 502 FT.
505 IN TOP THRD = 042 502 FT. (TO 300)
150 IN TOP THRD = 042 502 FT. (TO 300)
INTEREST EACH A SEASON CONTILET AND AN ALMEAD ET.

NET FREE AREA OF VANIED SOFFIT = 51 50. IN / LINEAR FT.

NET FREE AREA OF RIDGE VEHT = 16 50. IN / LINEAR FT.

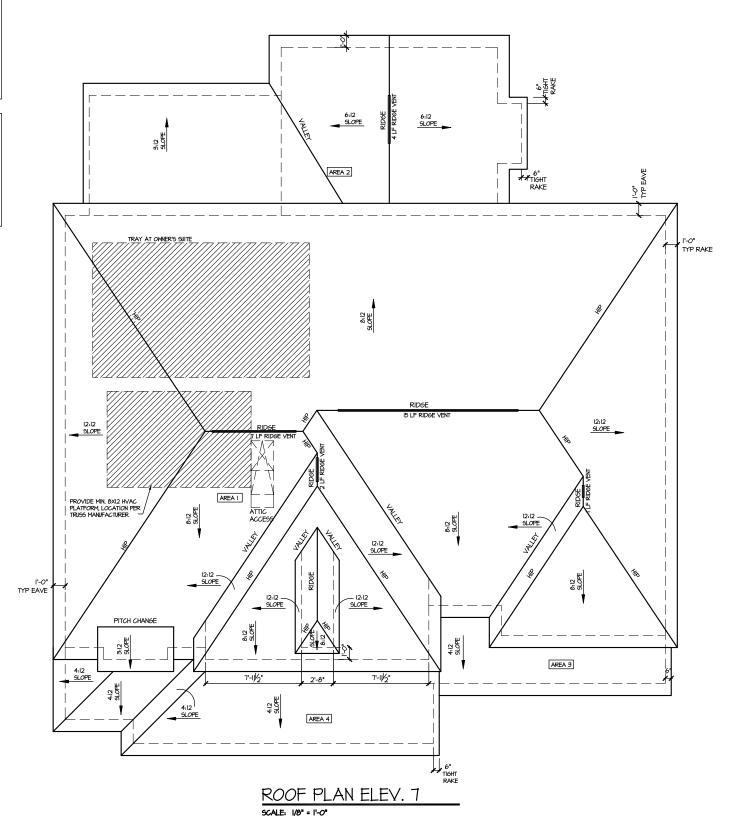
LOWER VEHTING. BOTTOM 2/3 FIDS!

LINEAR FEET OF SOFFIT X 57 50. IN = 0.485 50. FT.

THE SOFFIT OF SOFFIT X 57 50. IN = 0.5 50. FT.

0.150. FT. AT 50%
(17 30 00 ALLOWED)

NOTE: ROOF PLANS SHOWN W MIN. REQ'D RIDGE VENT LOCATIONS. ACTUAL RIDGE VENT LOCATIONS AND QUANTITY PER BUILDER



LE: Lot 00.0028.dwg DATE: 7/29/2025 10:31 AM

HOUSE NAME:
STONEFIELD
SPANING TITLE

DRAWN BY:

DATE: 07/29/2025 PLAN NO. 1635

AI.3

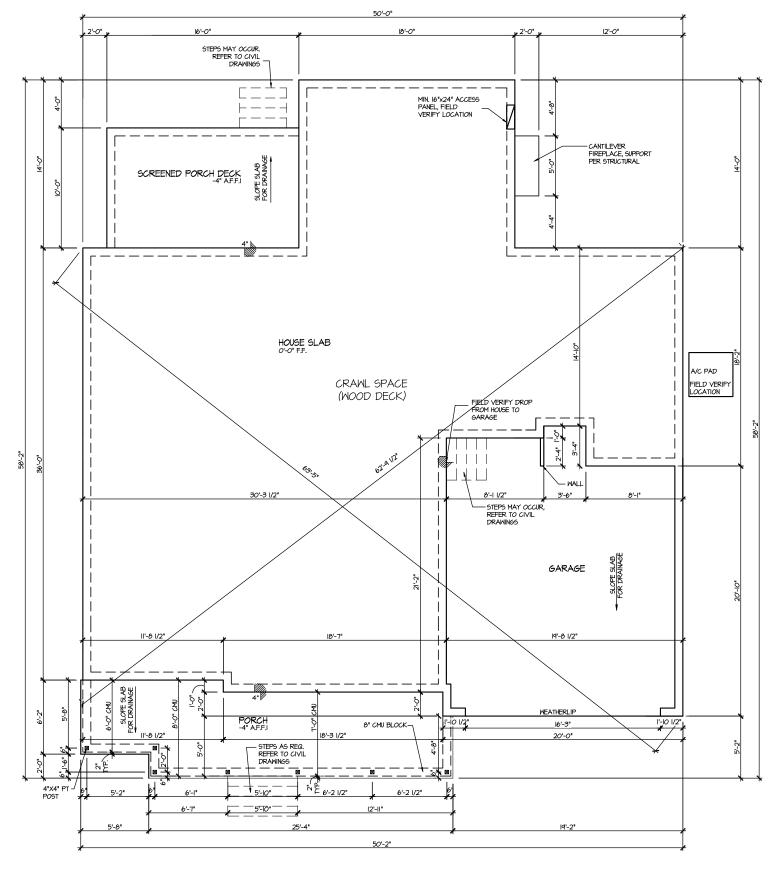
CRAWL SPACE VENT CALCULATIONS: ELEV 7 CRAWL AREA = 1542 SQ. FT. OVERALL REQUIRED VENTILATION:

I SQ. IN. PER I SQ. FT. = 1542 SQ. IN.

NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

<u>VENTING REQUIREMENT:</u> 1542 SQ. IN. / 72 SQ. IN. = 21.4 VENTS = 22 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED. YENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1



ELEVATION 7 CRAWL SLAB PLAN SCALE: 1/8" = 1'-0"

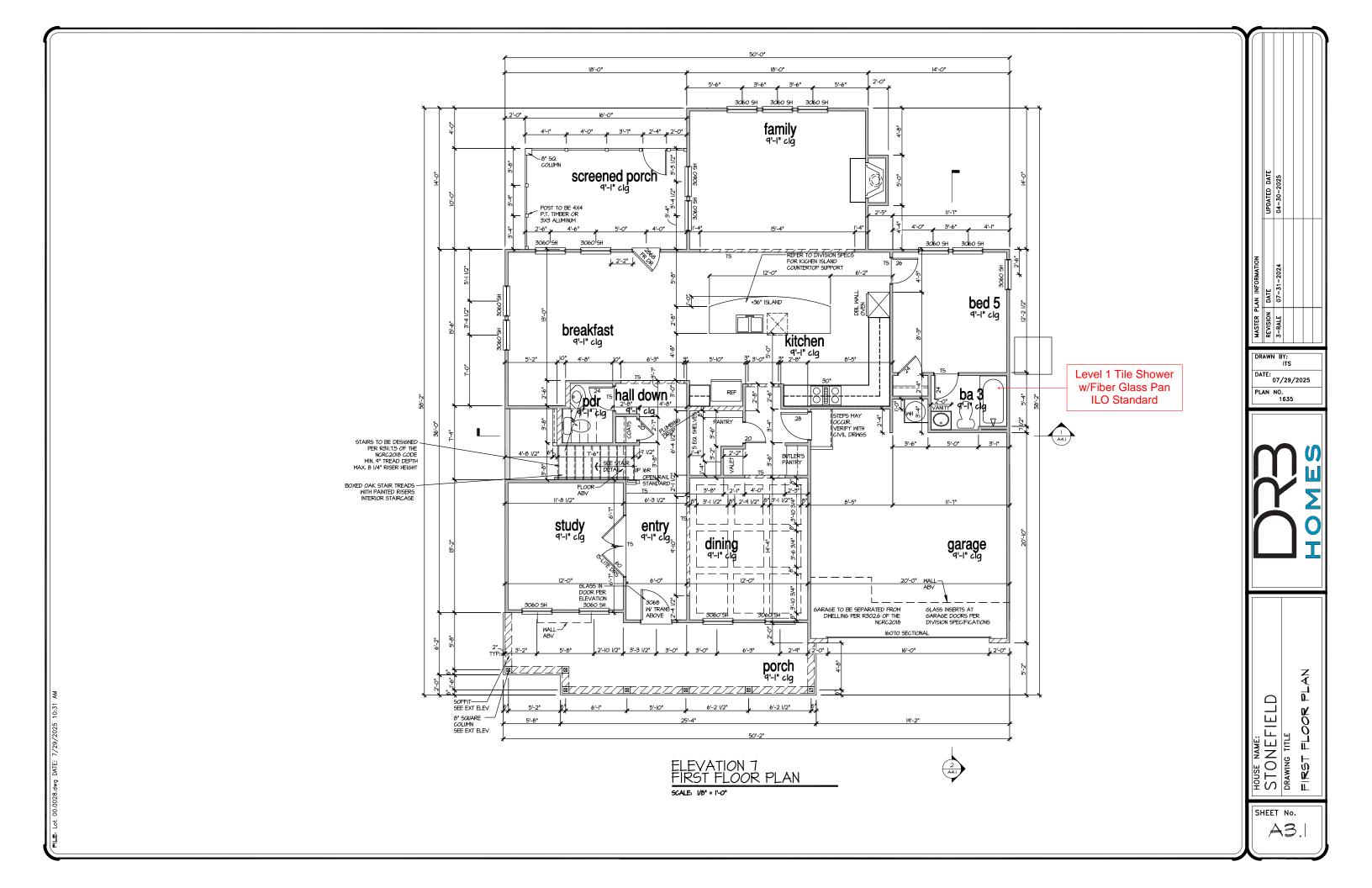
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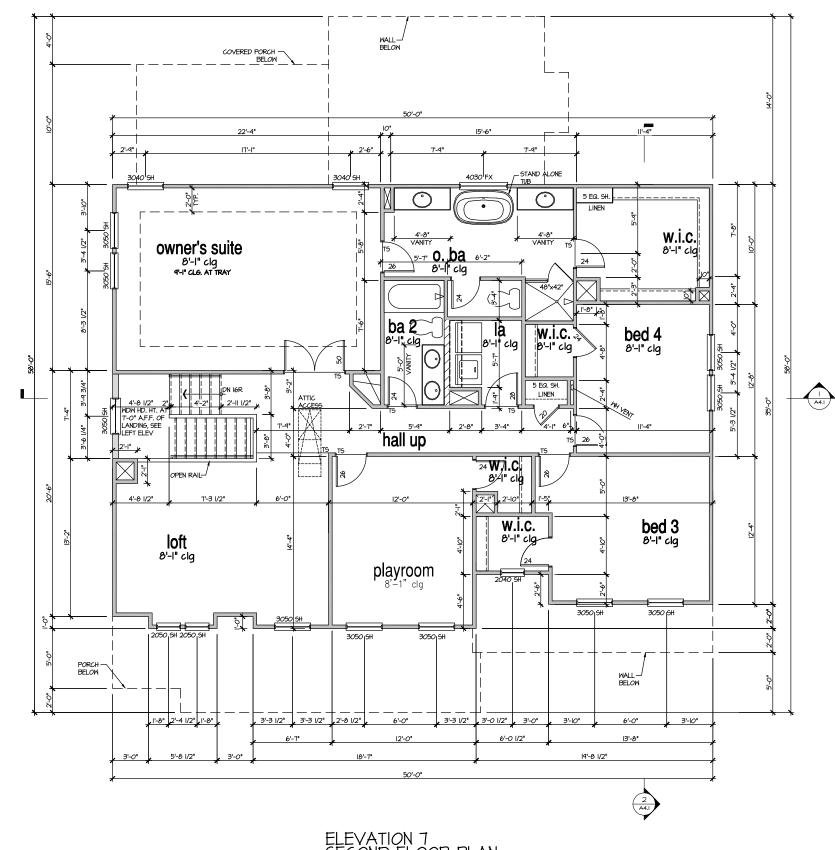


Ш HOUSE NAME:
STONEFIELE
DRAWING TITLE SPACE

SHEET No.

A2.





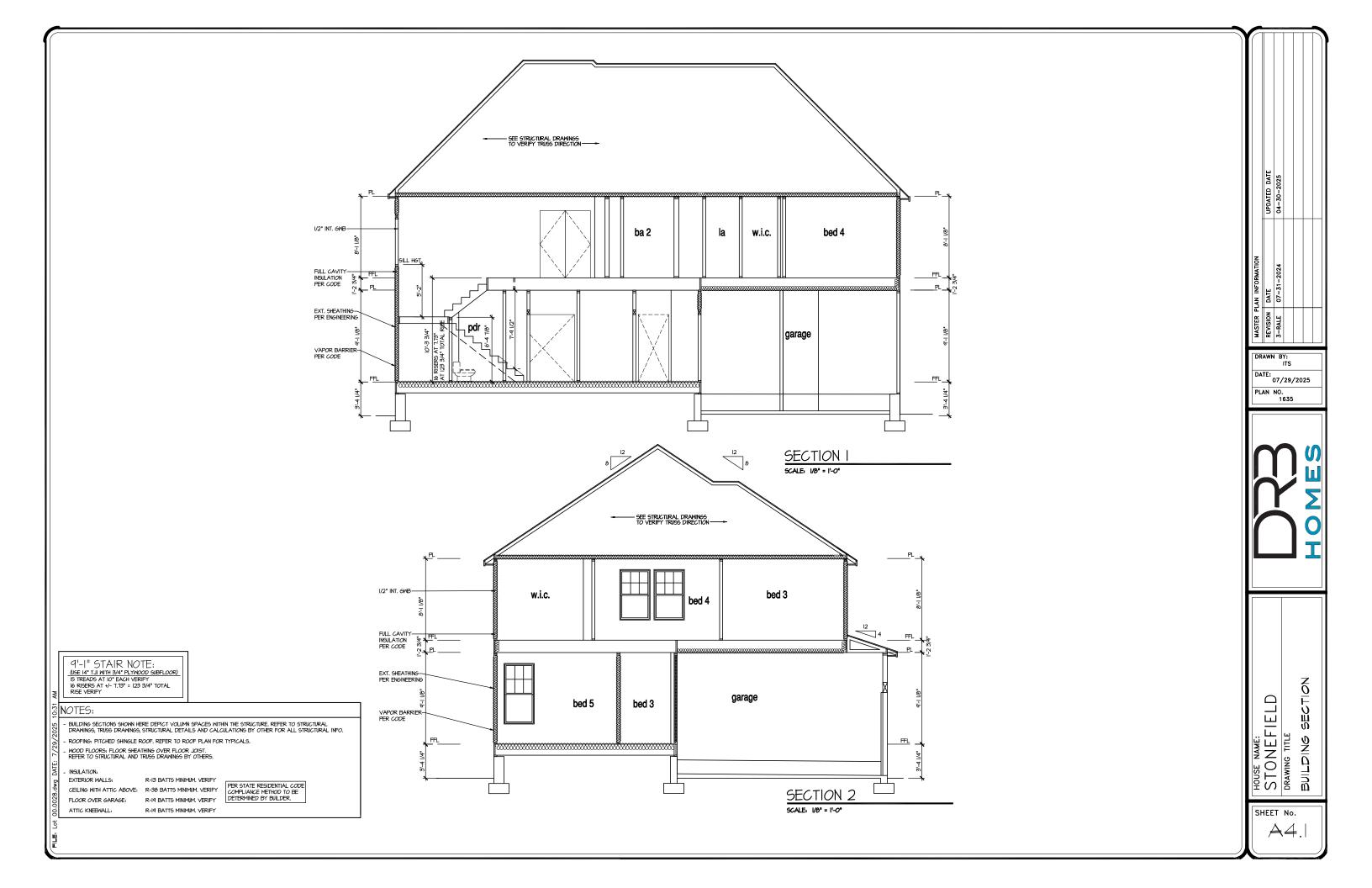
ELEVATION 7 SECOND FLOOR PLAN SCALE, 108" = 11-0" HOUSE NAME:
STONEFIELD
DRAWING TITLE
SECOND FLOOR F

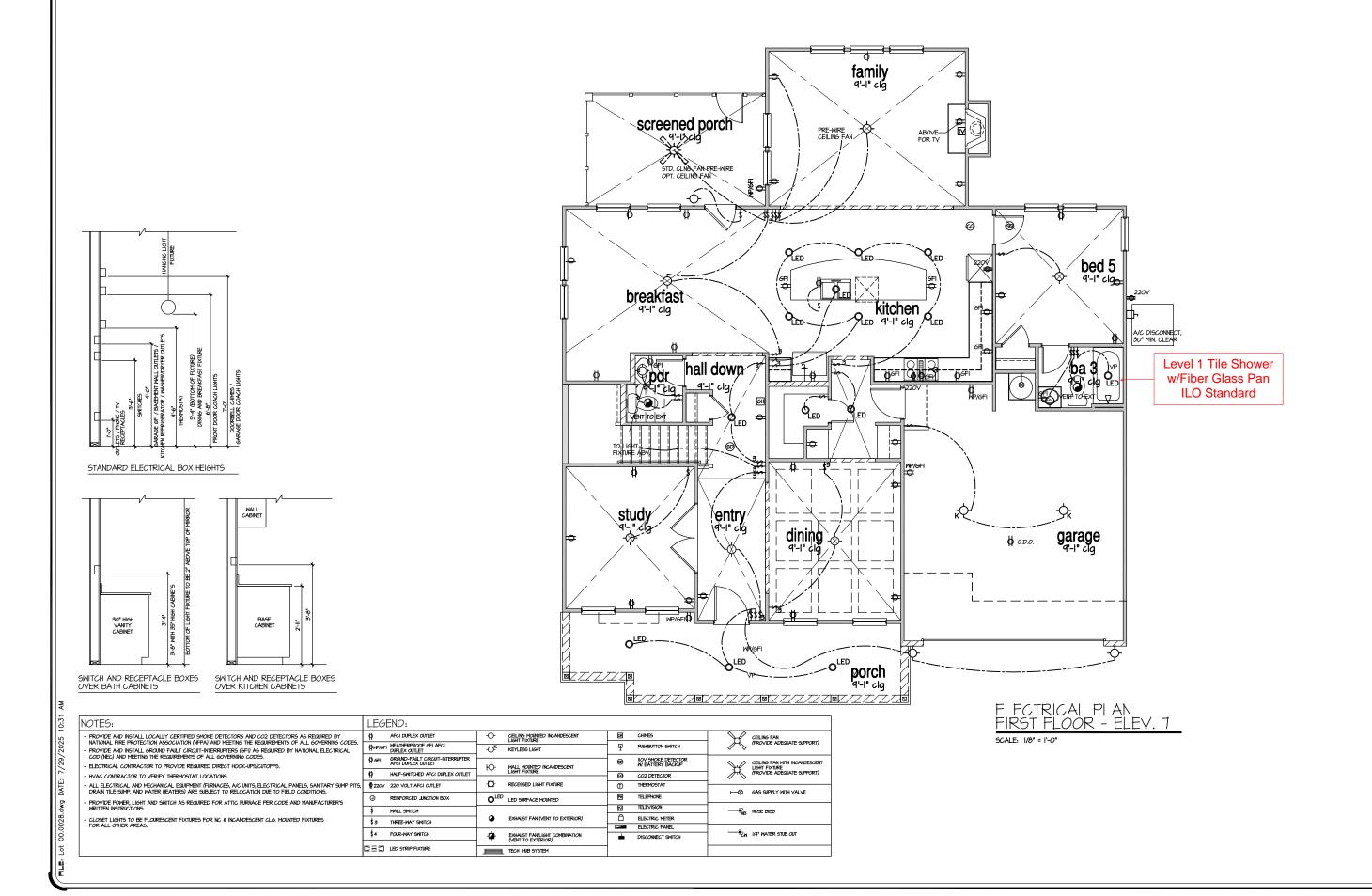
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PLAN NO. 1635

SHEET No.

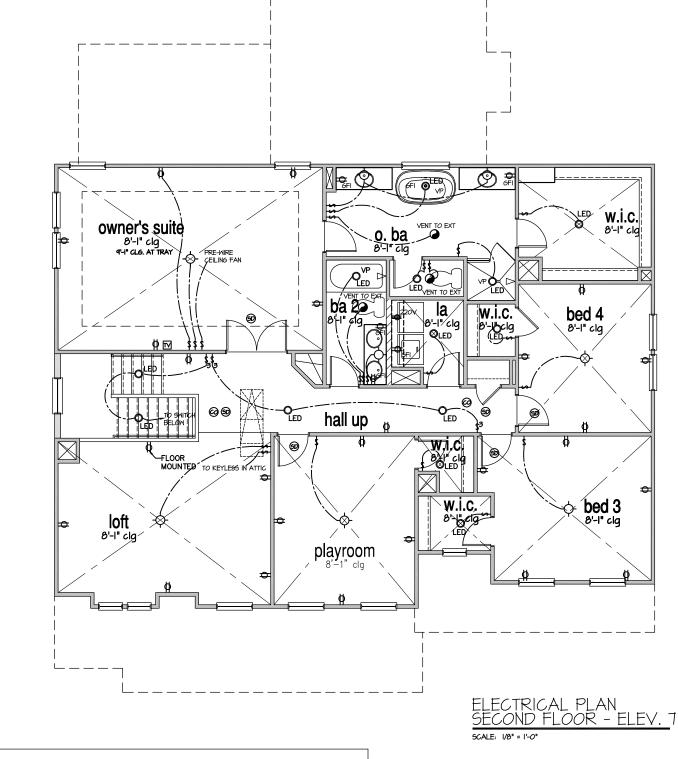
A3.2





DRAWN BY: DATE: 07/29/2025 PLAN NO. 1635 V 山

HOUSE NAME:
STONEFIELE
DRAWING TITLE



NOTES: PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL COD (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. - ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.

WALL CABINET

SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.

SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

STANDARD ELECTRICAL BOX HEIGHTS

- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUM DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.

PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

- CLOSET LIGHTS TO BE FLOURESCENT FIXTURES FOR NC & INCANDESCENT CLG. MOUNTED FIXTURES FOR ALL OTHER AREAS.

LEGEND: AFGI DUPLEX OUTLET - CEILING MOUNTED INCANDESCENT CHIMES CEILING FAN (PROVIDE ADEQUATE SUPPORT) MP/GFI WEATHERPROOF GFI AFCI DUPLEX OUTLET PUSHBUTTON SMITCH GFI GROUND-FAULT CIRCUIT-INTERRUIT
AFCI DUPLEX OUTLET IIOV SMOKE DETECTOR W BATTERY BACKUP CEILING FAN WITH INCANDESCENT LIGHT FIXTURE (PROVIDE ADEQUATE SUPPORT) H- WALL MOUNTED INCANDESCENT LIGHT FIXTURE HALF-SMITCHED AFCI DUPLEX OUTLET \$220V 220 VOLT AFCI OUTLET RECESSED LIGHT FIXTURE ① THERMOSTAT PH TELEPHONE REINFORCED JUNCTION BOX O^{LED} LED SURFACE MOUNTED TELEVISION WALL SMITCH EXHAUST FAN (VENT TO EXTERIOR) ☐ ELECTRIC METER \$ 3 THREE-WAY SMITCH ELECTRIC PANEL CM 1/4" WATER STUB OUT FOUR-WAY SWITCH EXHAUST FAN/LIGHT COMBINATION (VENT TO EXTERIOR) DISCONNECT SMITCH □□□ LED STRIP FIXTURE TECH HUB SYSTEM

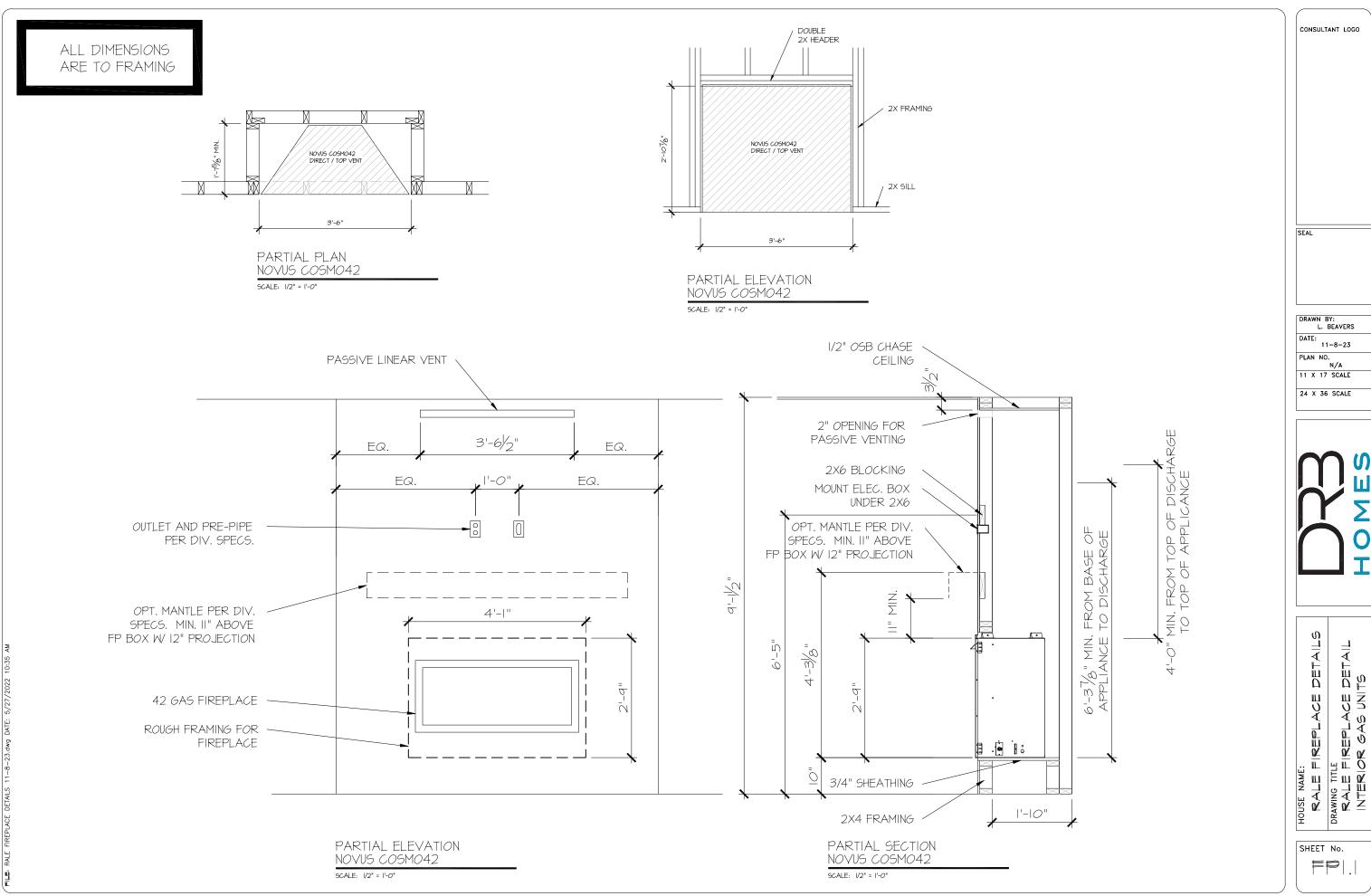
DRAWN BY:

DATE: 07/29/2025 PLAN NO. 1635



ᇳ HOUSE NAME:
STONEFIELE
DRAWING TITLE

SHEET No.



CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 11-8-23 PLAN NO. 11 X 17 SCALE



SHEET No.

DESCRIPTION OF BLDG. ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS 🛭 4" O.C.	(3) NAILS @ 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS • 8" O.C.	TOENAILS @ 6" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS @ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" o.c.	NAILS @ 16" O.C.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP ② CORNERS € INTERSECTING WALLS	(2) NAILS	(2) NAILS

2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS.
 (ONLY ACCEPTABLE WHERE * ARE SHOWN)

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION, THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENT IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO: FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS IOTED OTHERWISE ON PLAN

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD

LOAD. (NOT DIFFERENTIAL DEFLECTION)

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

ROOF DEAD = 7 PSF T.C., 10 PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION, ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP. U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
 I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (I)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15). ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- 'LSL' Fb=2325 psi; Fv=3i0 psi; E=1.55xi0^6 psi
- 'LVL' Fb=2600 psi; Fv=265 psi; E=2.0x10^6 psi 'PSL' - FB=2400 PSI; FV=240 PSI; E=2.0XIO^6 PSI
- M+K SHALL BE FILLY INDEMNIFIED FOR ANY AND ALL ISSUES RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THE OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
- FOR 2 \$ 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x31/2" SIMPSON SDS SCREMS (OR 3½" TRUSSLOK SCREMS) & 16" O/C. USE A MINIMUM OF 3 ROMS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM FDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF $\frac{1}{4}$ "x6" SIMPSON SDS SCREMS (OR 6 $\frac{3}{4}$ " TRUSSLOK SCREMS) • 16" O/C. USE A MINIMUM OF 4 ROMS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE. A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MEK FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I 1/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131" NAILS @ 6"o.c. @ PANEL EDGES & @ 12"o.c. FIELD. - 2 🖣 × 0.120" NAILS • 4" O.C. • PANEL EDGES € • 8" O.C. FIELD.
- 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN, SCREMS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER (U.N.O.) FASTENED TO:
 - RIM BOARD w/ (2) 3"x0.131" NAIL 5 @ 16" O.C. MAX. (1-1015T5) TRUSS VERTICALS w/ (3) 3"x0.131" NAILS • 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS @ 6"O.C. @ PANEL EDGES & @ 12" O.C. FIELD. - w/ 2 3 × 0.120" NAILS • 4"o.c. • PANEL EDGES € • 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"0.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.

VENEER LINTEL SCHEDULE

12.22.12.112.20.12.02.			
SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size	
3'-0 '	20 FT. MAX	L3"x3"x/4"	
6'-0"	3 FT. MAX	L3"x3"x/4"	
	I2 FT. MAX	L4"x3"x/4"	
	20 FT. MAX	L5"x3½"x5%"	
ð'-O"	3 FT. MAX	L4"x4"x¼" "	
	I2 FT. MAX	L5"x3½"x5%"	
	I6 FT. MAX	L6"x3½"x%"	
4'-6"	I2 FT. MAX	L6"x31½"x5%"	
16'-0"	2 FT. MAX	L7"x4"x/2" **	
	3 FT. MAX	L6"x4"x½" **	

SHALL SUPPORT 2 %" - 3 ½" VENEER W/ 40 psf MAXIMUM WEIGHT

- : 16' SHALL HAVE 8" MIN, BEARING : 16' SHALL NOT BE FASTENED BACK TO HEADER.
- 6' SHALL BE FASTENED BACK TO WOOD HEADER IN WALL #48"0. W ½" DIA, x 3½" LONG LAG SCREWS IN 2" LONG VERTICALLY SLOTTED HOLES.
- MAX, VENEER HT, APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
 WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 1/2" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT
- FINISHING.
 SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPAGED BY THE ABOVE PARAMETERS, FOR ANY LINTEL FASTENESS SHALL MAINTAIN A 25' MINIMAN LEAR DISTANCE FROM BOTTOM OF BEAM. FOR QUEEN VENEER USE L4x3x
- FOR 3½" VENEER ONLY. SEE PLAN FOR VENEER SUPPORT IF

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RO

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301211) EXP. B. RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NOSEC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

EXT. WALL SHEATHING SPECIFICATION

- 1/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ・ALT. STAPLE CONNECTION SPEC: 1 名" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD.

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 3/8" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 3/4" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C IN FIELD, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN. IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- <u>PRE-MANUFACTURED PANELIZED WALLS:</u> FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWALL
OR 3" O.C. OSB SHEARWALL.

► INDICATES HOLDOWN BELOW

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED, BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING
- 1/2" DIA. ANCHOR BOLTS 6'-0" O.C, 7" MIN. EMBEDMENT (CONC), 15" MIN, EMBEDMENT (CMU)
- SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS 2'-8" O.C. (CMU)
- (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER. • CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN
- THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: FOUNDATION WALLS 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE 60,000 psi
- BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
- TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (91/3" FOR 10" THICK WALL).
- * BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC, ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED PRIOR TO BACKELLING BY ADEQUATE TEMPORARY BRACING OR INSTALL IST FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.
- FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 w/ (2)2x6 JACK STUDS, U.N.O
- LARGER OPENINGS SHALL BE PER PLAN. • ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS
- THAN 5% OR MORE THAN 7% AIR ENTRAINMENT ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- * PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY
- TO DEVELOP. 15'-0" OC (MAXIMUM)
- JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I.I RATIO), WITH A MAXIMUM OF I.I.5 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL SI ARS
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fim=1500 psi). MORTAR SHALL BE ASTM C270, TYPE 5. CMU DESIGN PER ACI 530 \$ 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x6 (MIN.) x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE FASTENED PER ANCHORAGE SPECIFICATIONS NOTED ABOVE. TOP 2 COURSES (MIN.) OF PIER TO BE GROUTED SOLID (8 COURSE MAX, PIER HEIGHT).
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE. TOP 2 COURSES (MIN.) OF WALL TO BE GROUTED SOLID (8 COURSE MAX. WALL HEIGHT)
- * DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR UBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

HOLD-DOWN SCHEDULE

5YMBOL	SPECIFICATION	
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)	
► HD-2	SIMPSON MSTC666 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC66B3 ALTERNATE	
▶ HD-3	SIMPSON STHDI4/STHDI4RJ	

***** UTILIZE THE SGTB24 ANCHOR BOLT **②** ALL MONOSLAB **\$** INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED.

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 5" (FOR 1/8" DIA.) MIN. EMBEDMENT INTO CONCRETE.

INSTALL PER MANUF, INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D. <u>DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE</u>

AMPBEL

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at 8/1/25

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I&K project numbe

rawn by:

FVISIONS

126-24045

ssue date: 08-01-2

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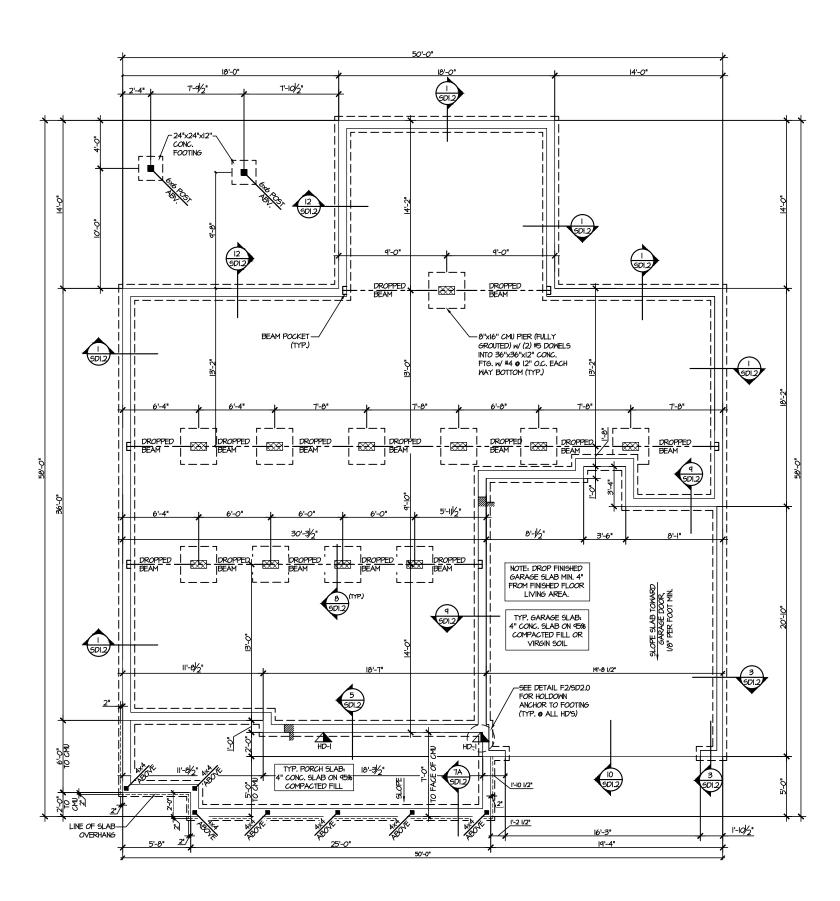
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RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number: 126-24045

JTR drawn by: issue date: 08-01-25

REVISIONS:

CAMPBELL RIDGE LOT 28 - STONEFIELD 7 RALEIGH, NC

OUNDATION

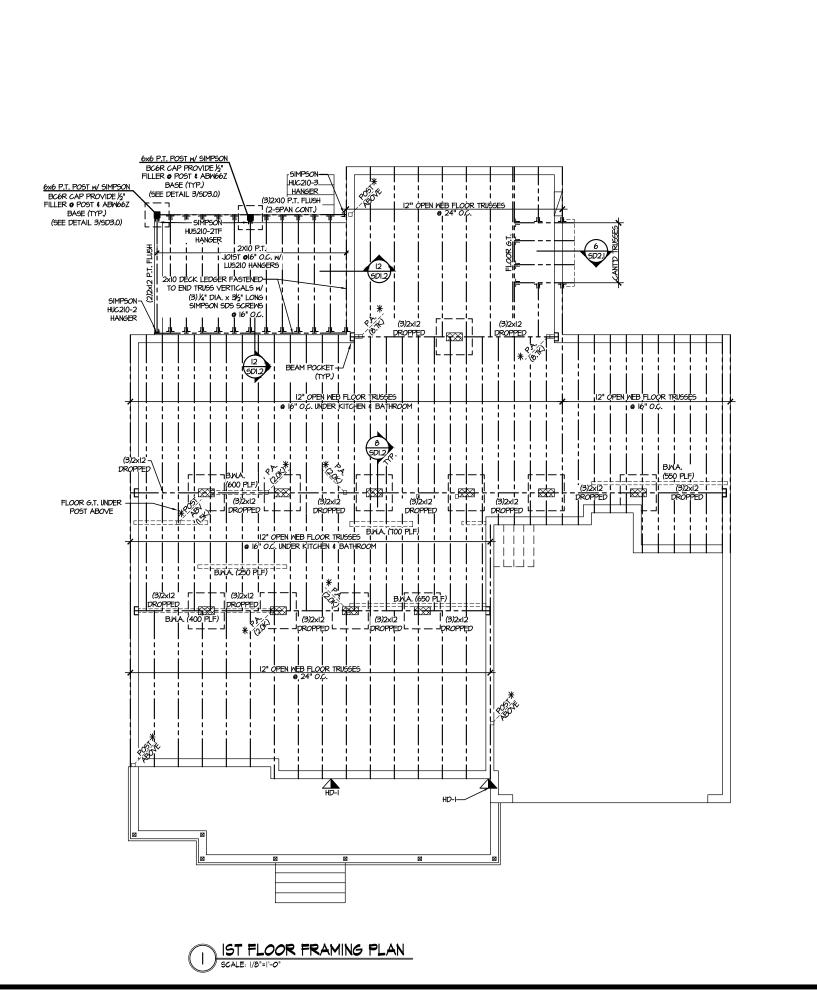
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- IIIIII INTERIOR BEARING WALL • □===□ BEARING WALL ABOVE
- ---- BEAM / HEADER • = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JIL METAL HANGER
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE,

LEGEND

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



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M&K project number: 126-24045

project mgr: JTR drawn by: JAD issue date: 08-01-25

REVISIONS:

SD2.I REFERS TO SD2.IA FOR LYL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LEGEND

- INTERIOR BEARING WALL
- □□□□□□ BEARING WALL ABOVE
- --- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR
TYPICAL STRUCTURAL NOTES

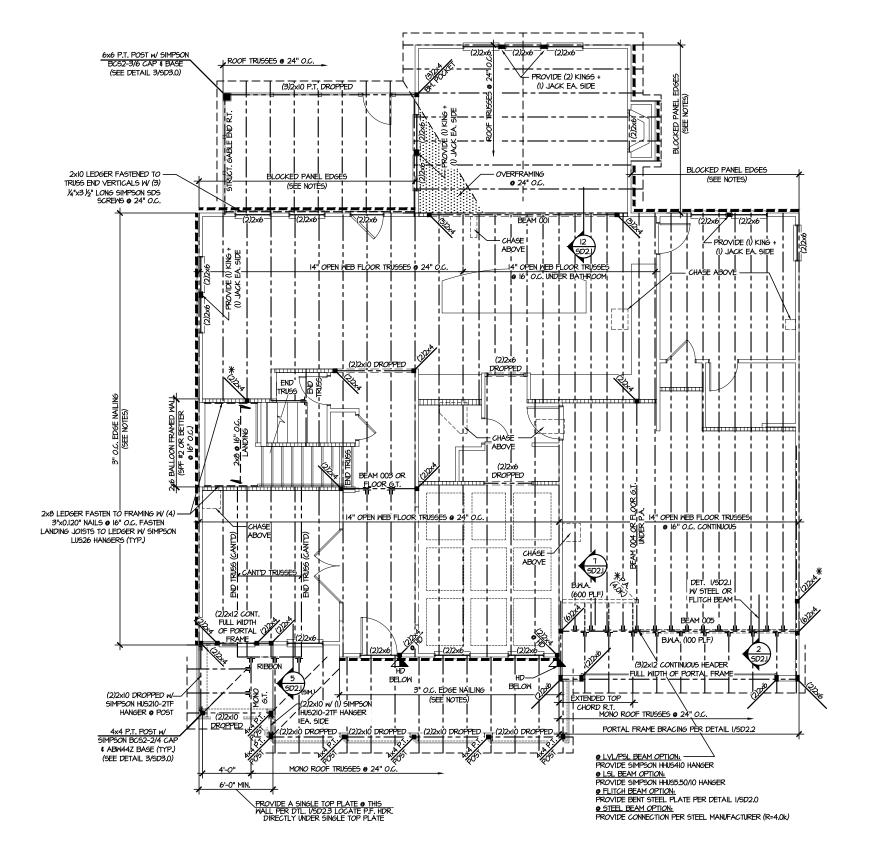
\$ SCHEDULES

sheet: S2.0

OR

CAMPBELL RIDGE LOT 28 - STONEFIELD 7 RALEIGH, NC

ANS



2ND FLOOR FRAMING PLAN

ENGINEERED BEAM MATERIAL SCHEDULE LVL OPTION LSL OPTION FLITCH OPTION 2)2x12 + (1) %"x114" STEE FLITCH PLATES - D 001 (2)13/4"×16" - I 3½"x16" - D WI2xI4 - D (2)2xI2 + (I) ¼"xII¼" STEEI FLITCH PLATES - D 002 (2)13/4"x113/6" - D 3½"xII%" - D (3)13/4"x117/6" - D WI2xI4 - D (2)2xi2 + (i)从"xii以" STEEL FLITCH PLATES - F 003 (2)13/4"x14" - 1 (2)13/4"x14" - F WI2xI4 - F 36"x|4" - F 004 (2)13/4"x14" - F 3½"xl4" - F WI2xI4 - F (3)2xl2 + (2) %"xll4" STEEL FLITCH PLATES -005 (3)134"×18" - FT 5¼"xl8" - FT WI2x26 - F (3)2xl2 + (2) %"xl以" STEEL FLITCH PLATES -006 (3)13/4"x18" - FT 5½"xl8" - FT N/A WI2x26 - F (2)2xl2 + (I) ¼"xll¼" Steel Flitch Plates - F 007 (2)13/4"x14" - F (2)13/4"x14" - F 36"x14" - F WI2xI4 - F (2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - F (2)13/4"x14" - F (2)13/4"×14" - F 3½"xl4" - F WI2xI4 - F (2)2x10 + (1) ¼"x4¼" STEE FLITCH PLATES - D 009 (2)134"×94" - D 3½"x9¼" - D (2)134"x94" - D WI0x12 - D う)2xi2 + (2) ½"xii¼" STEEL FLITCH PLATES - H 010 (2)13/4"x16" - H 3½"x16" - H (3)13/"v16" - H N/A (2)2xl2 + (I)从"xli以" STEEL FLITCH PLATES - D (2)|%"x||%" - D 3½"xII%" - D (3)13/4"x117/4" - D WI2xI4 - D 012 (3)1¾"x16" - D 5¼"x16" - D (4)134"x16" - D W12x19 - D 2)2xl2 + (I) 从"xll以" STEEI FLITCH PLATES - D 013 (2)134"x1136" - D 3½"xII%" - D (3)134"x1176" - D)2xi2 + (2) %"xilk" steel Flitch Plates - D 014 (2)|%"x|6" - D (3)|%"x|6" - D WI2xI4 - 12 3%"x16" - D (3)2x12 + (2) ¼"xI¼" STEEL FLITCH PLATES - D (3)13/4"x113/4" - D (3)13/4"x113/6" - D WI2xI4 - D N/A (2)2xl2 + (1) ¼ "xl¼" STEEL FLITCH PLATES - F (2)134"×1136" - F (3)13/4"x113/6" - F WI2xI4 - F 3½"xII%" - F

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM

- "FB" INDICATES FLUSH BOTTOM BEAM

- "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER

PLATES IN SUCCESSION W/ (2) 3"XO.120" NAILS @ 8" O.C.

PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C.

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- ■ INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP.
 REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN *** - SEE PLAN FOR EXTENT OF 3-PLY BEAM

MULHERN+KUL RESIDENTIAL STRUCTURAL ENSINEERI l&K project number 126-24045 **JTR** rawn by:

ssue date: 08-01-2

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RESIDENTIAL STRUCTURAL ENGINEERI



M&K project number: 126-24045

JTR drawn by: issue date: 08-01-25

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CAMPBELL RIDGE LOT 28 - STONEFIELD 7 RALEIGH, NC

 INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE

• --- BEAM / HEADER

• == INDICATES SHEAR WALL & EXTENT

EXTENT OF OVERFRAMING

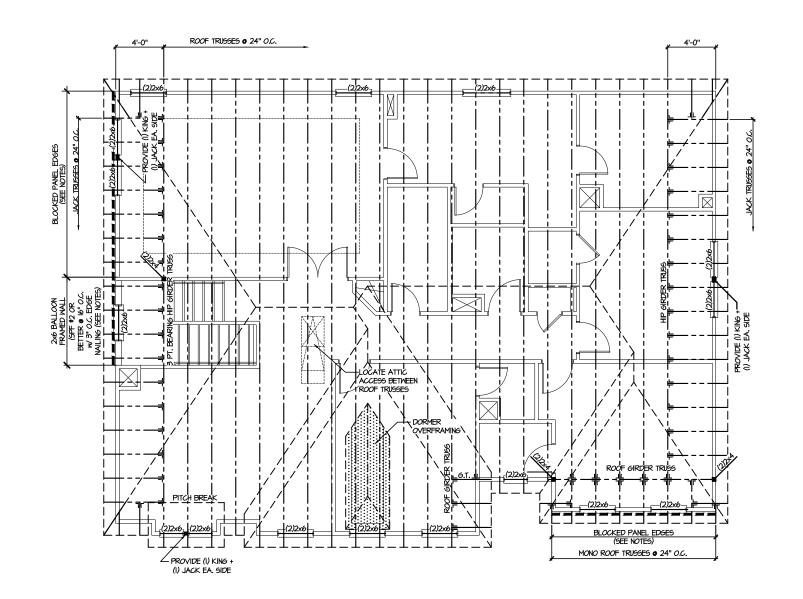
JL METAL HANGER

* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

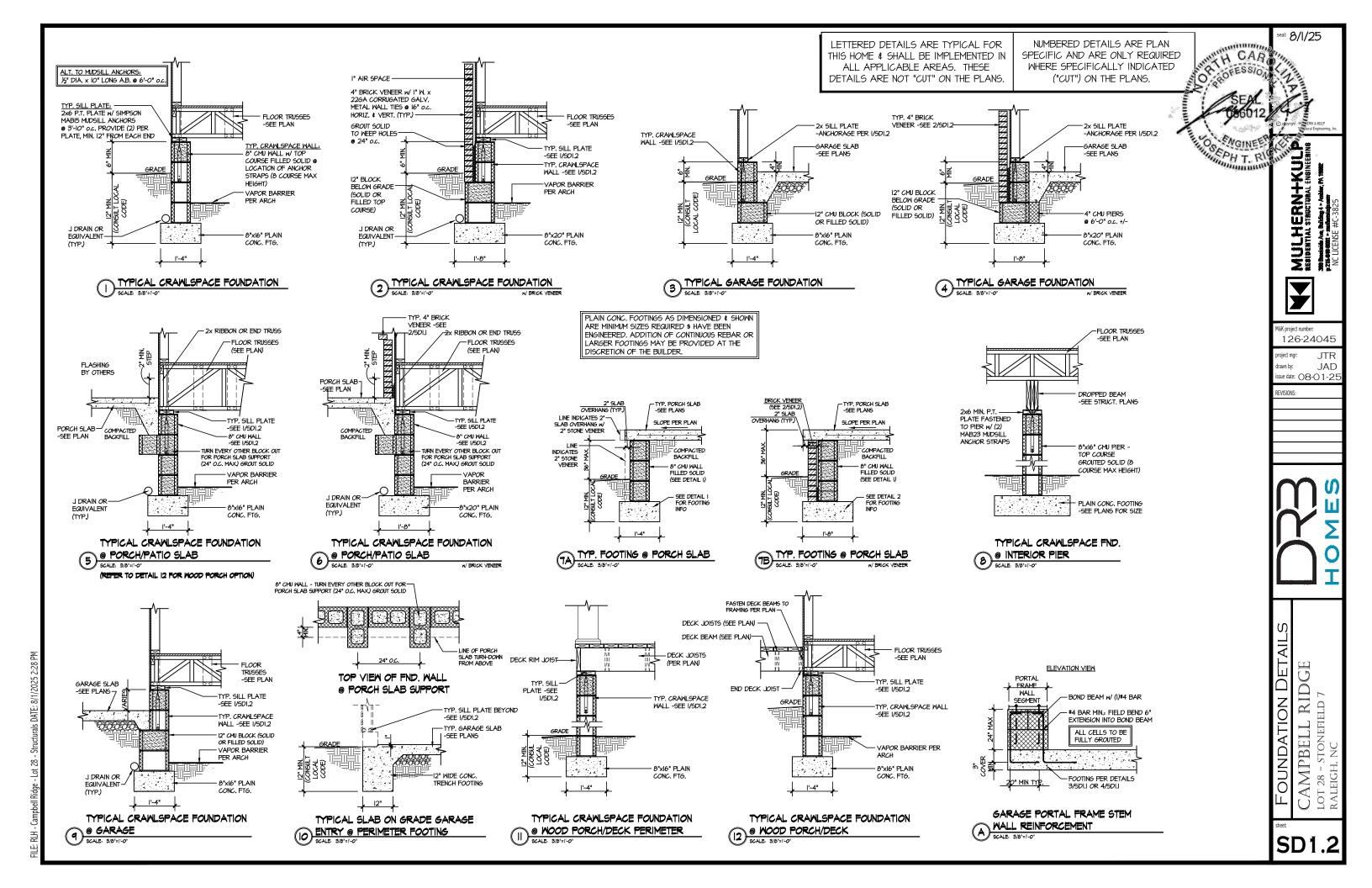
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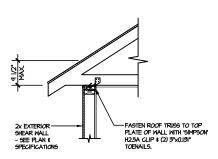
INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



ROOF

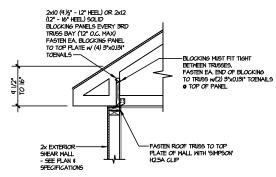




TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

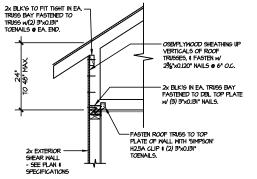
SCALE: 3/8'=1'-0' HEEL HEIGHT LESS THAN 9 ½"
NO BLOCKING REGD



TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

SCALE: 3/6"=1"-0" HEEL HEIGHT BETWEEN 9 ½" - 16"
BLOCKING REGID



TYPICAL SHEAR TRANSFER

A3 DETAIL @ RAISED HEEL TRUSS

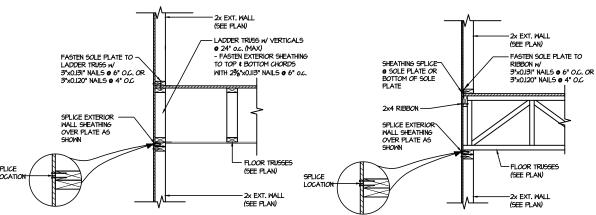
SCALE: 9/8/21-0/2 MPER LEED (1975)

2) EXTERIOR
99 FAR MAIL
98 FE PLANL
99 FECHICATIONS

TYPICAL GABLE END DETAIL

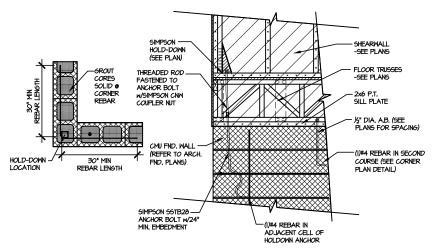
B SCALE: 3/8'=1-0'

FASTEN BOTTOM CHORD OF— GABLE END TRUSS TO DBL., TOP PLATE w/ 3"x0.131" NAILS © 8" O.C. PROVIDE GABLE
END BRACING PER
TRUSS
MANUFACTURER
AND/OR BCSI



TYPICAL SHEAR TRANSFER DETAIL
BETWEEN FLOORS @ EXTERIOR WALL
SCALE 300'-11-0" FARALLE PRIOR





TYPICAL CORNER FOUNDATION

HOLD-DOWN INSTALLATION

SCALE: NT.5.

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS. al: 8/1/25

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M&K project number:

drawn by:

REVISIONS:

126-24045

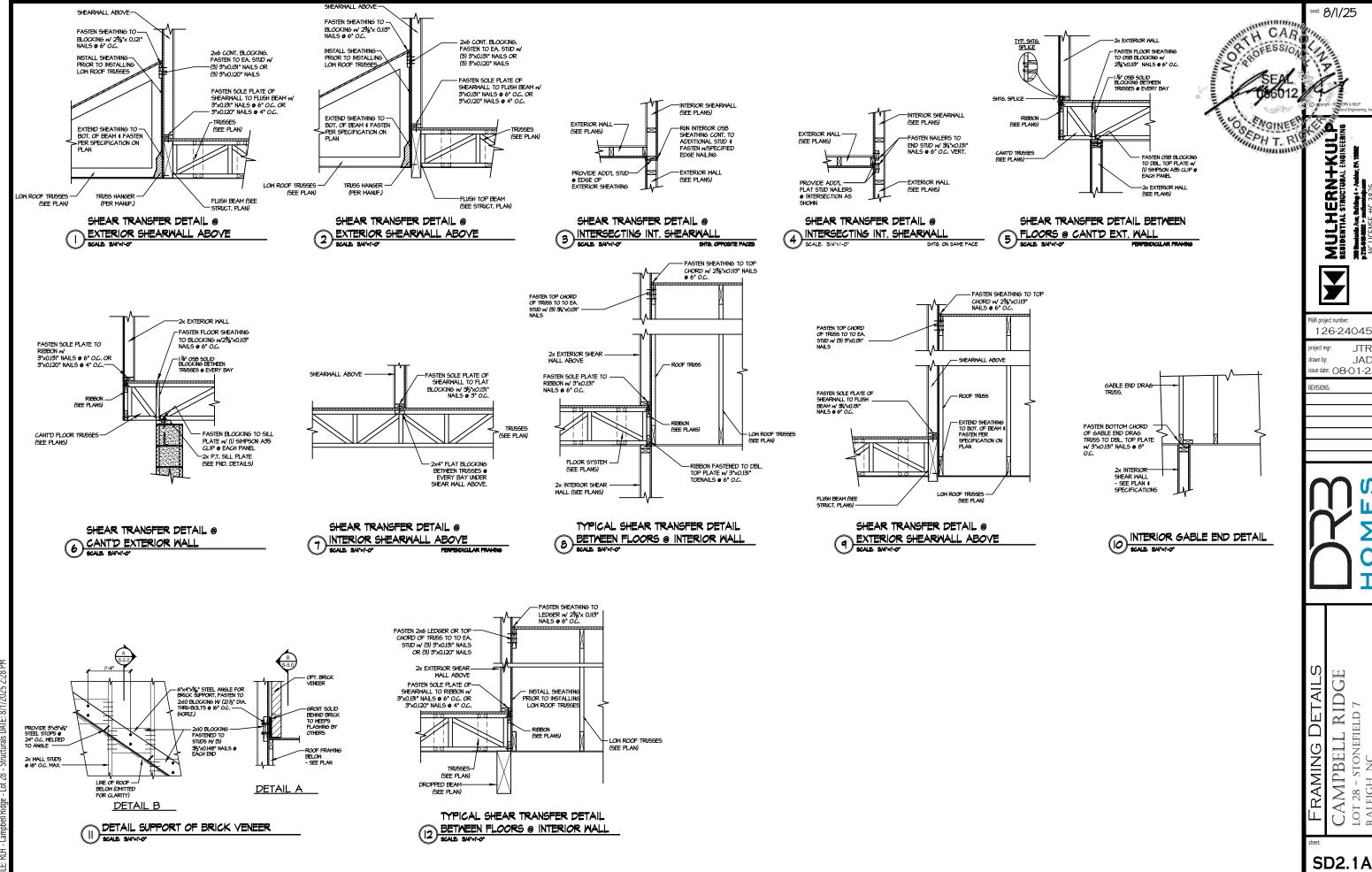
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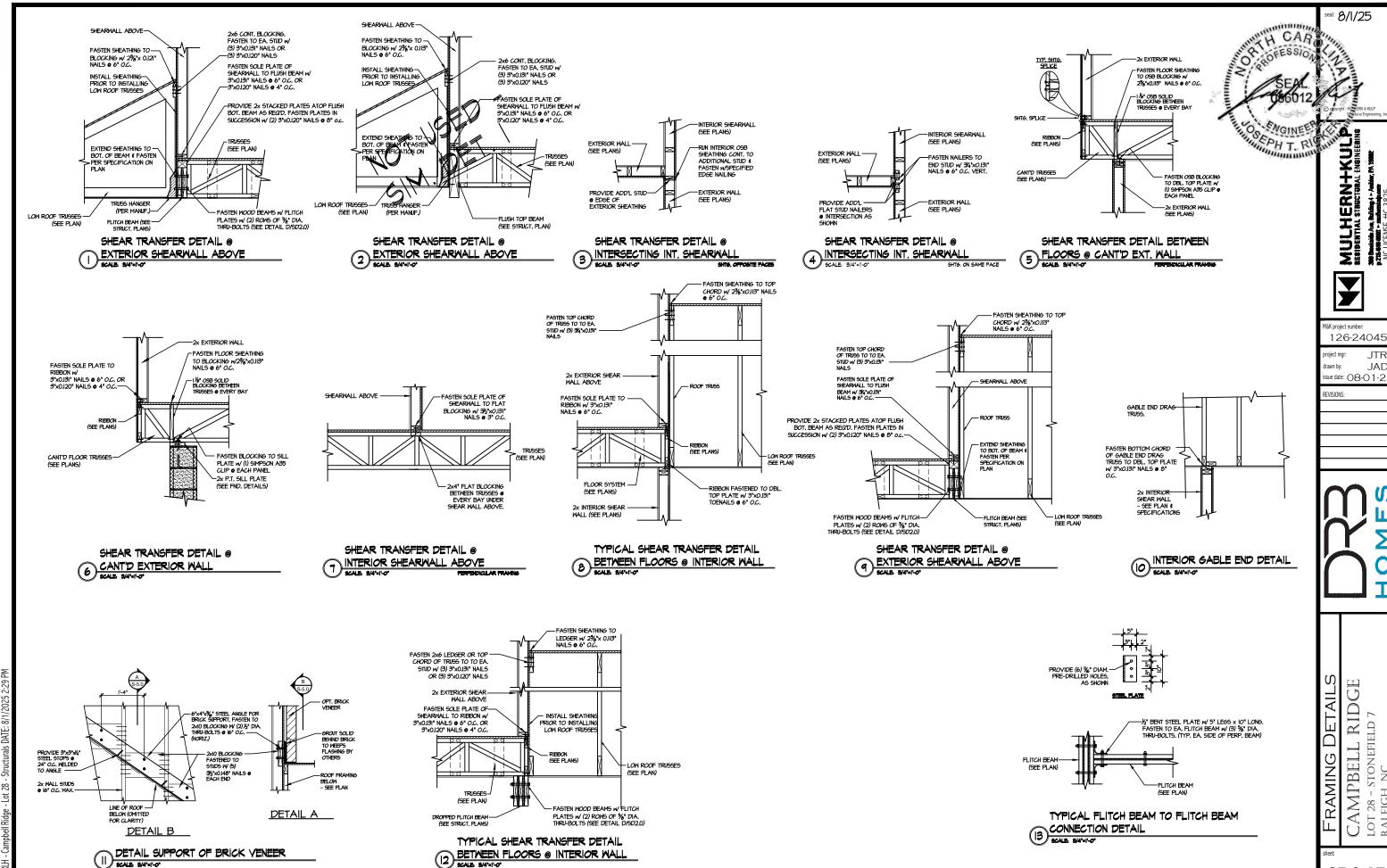
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TOP SOFES

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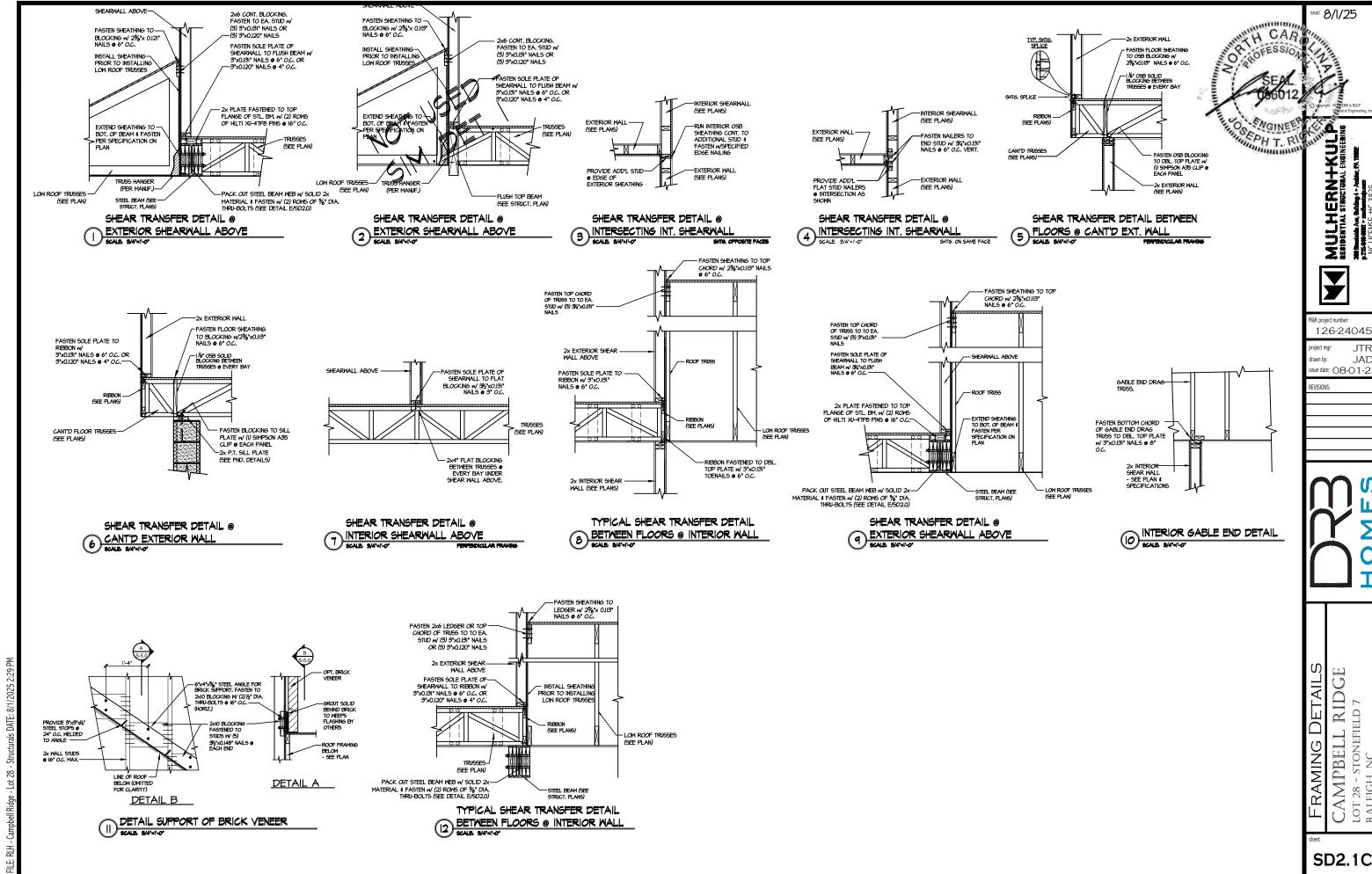


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SD2.1B

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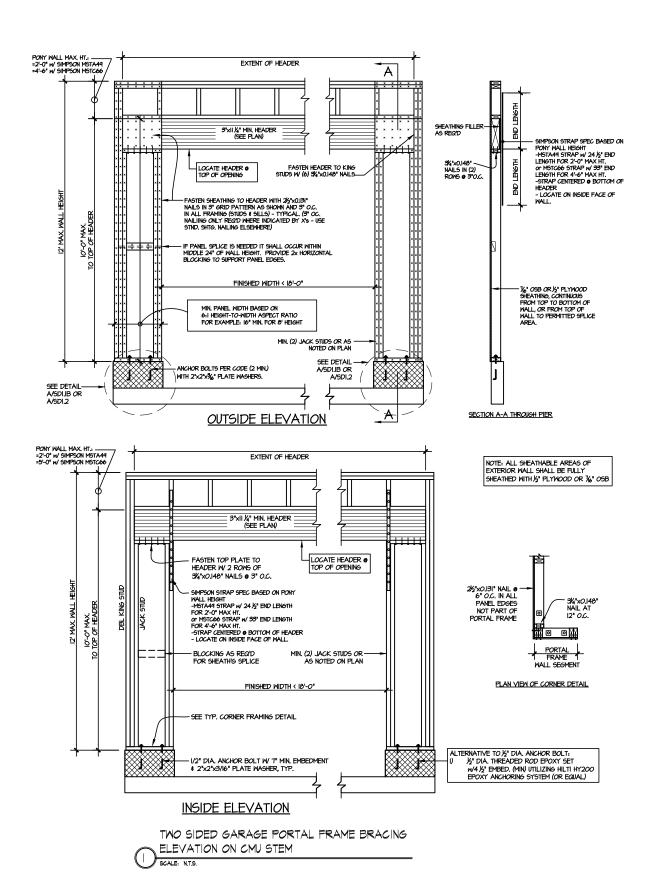
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SD2.1C

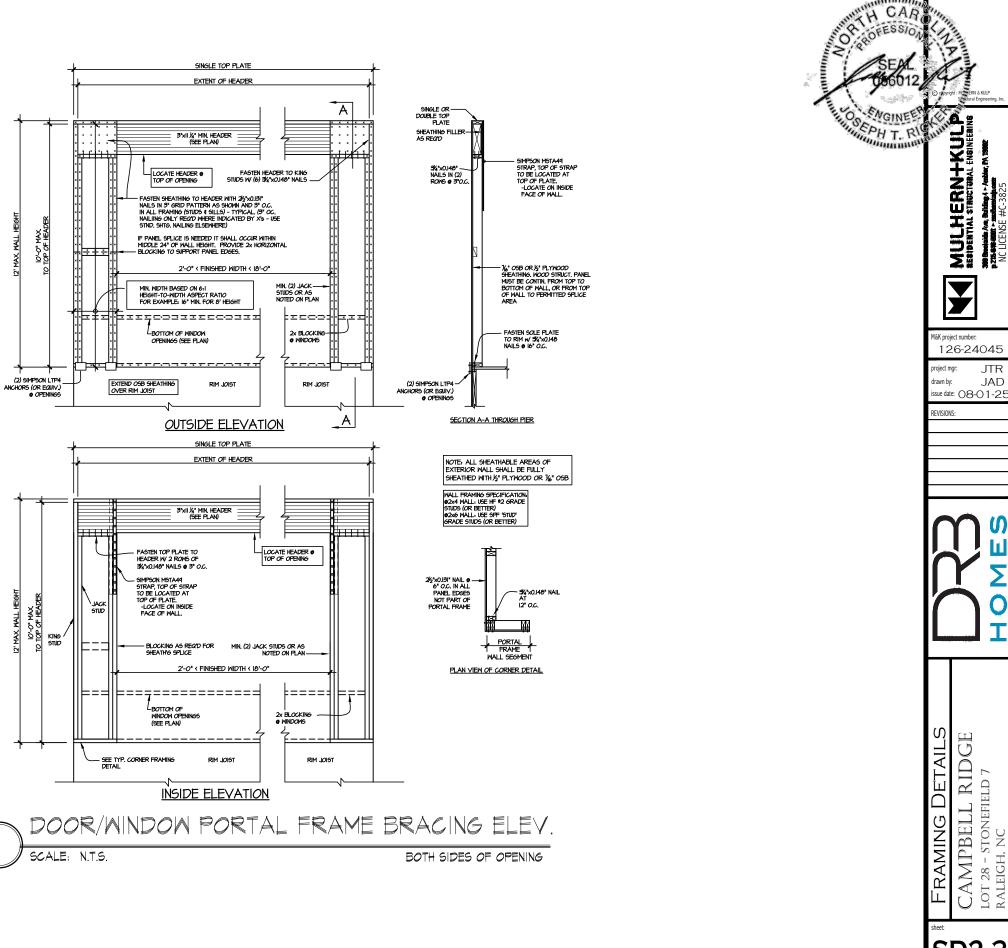


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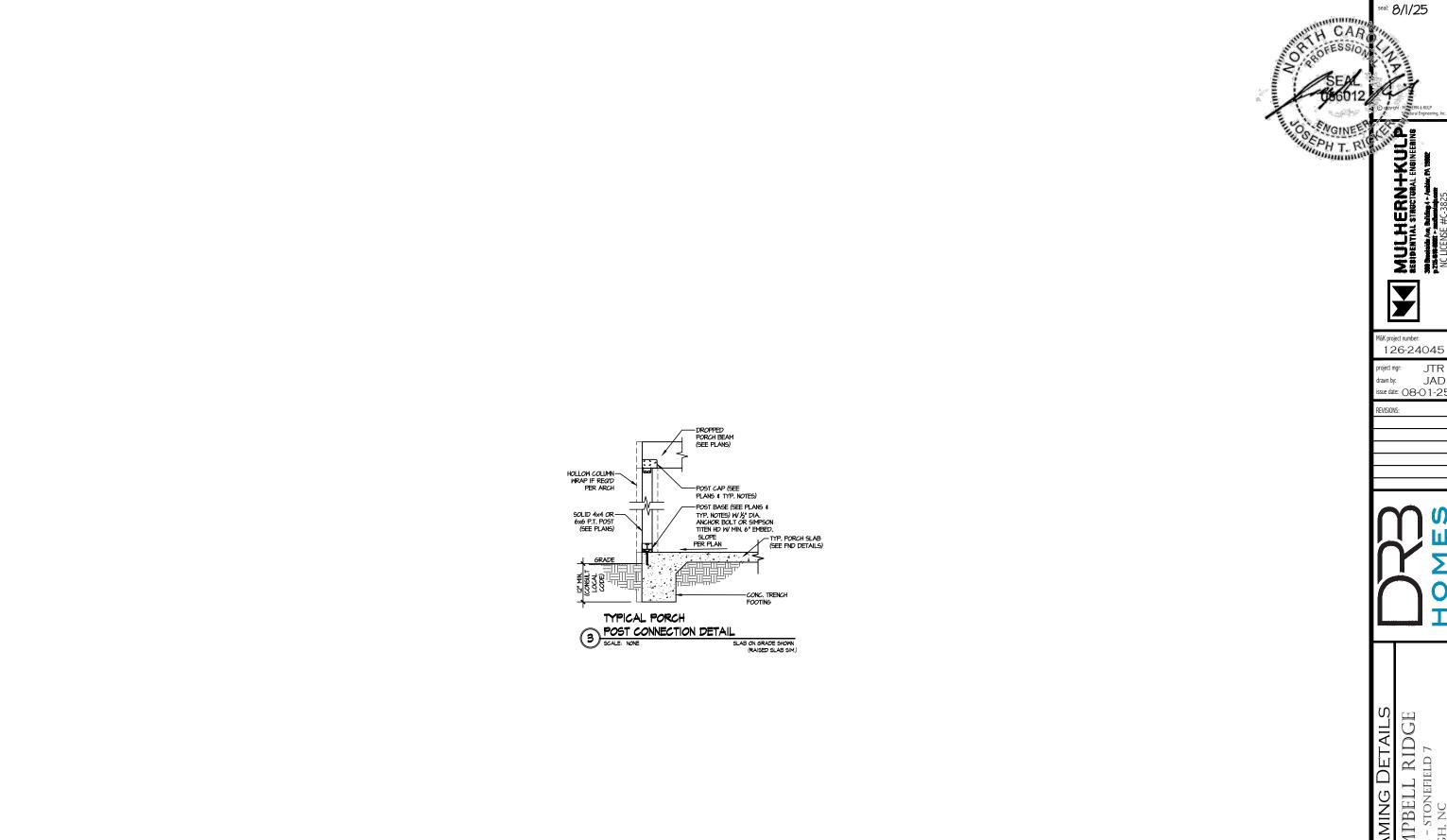
SD2.2



SD2.3

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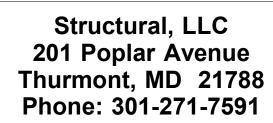


CAMPBELL RIDGE LOT 28 - STONEFIELD 7 RALEIGH, NC

JTR JAD

SD3.0





OWF 00.0028 Lot Campbell Ridge Raleigh

00.0028 lame: ot Stonefield Rev.3

Model Name:

TPI Plant W974

NOTE: IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR

ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR TH CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDIN THE INSTALLATION, AND BRACING OF TRUSSES

WARNING: CONVENTIONAL FRAMING, ERECTION AND/OR PERMANEN

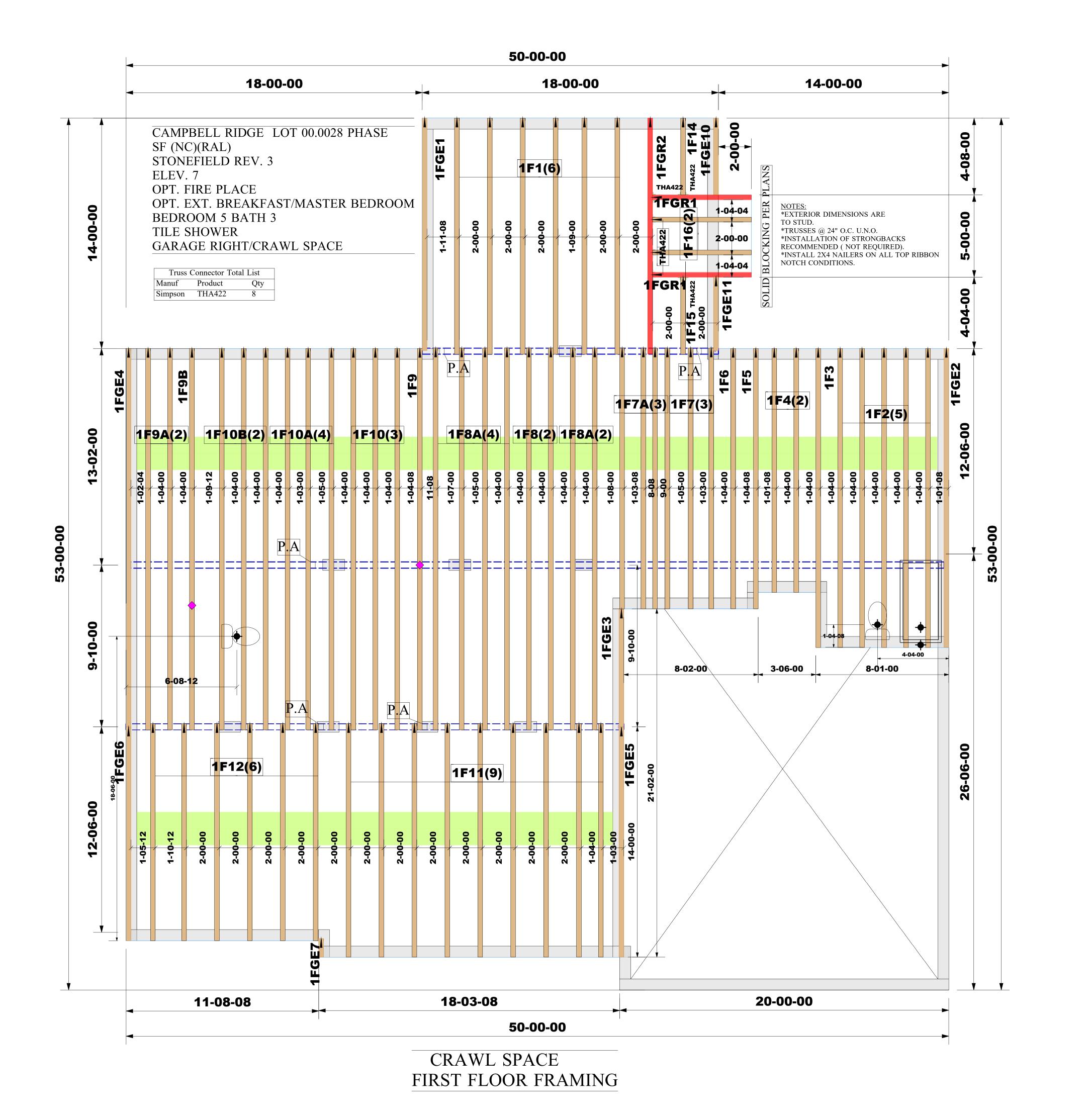
BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING TH **ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN** TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTAR AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY

2507-2763

Sayan Roy
Sales Rep:

Robbie Zarobinski



OWF TRUSS LAYOUT SCALE: NTS



Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

er: DRB Raleigh ne: Campbell Ridge Lot 00.0028 OWF

Stonefield Rev.3

Model Name:



00.0028

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NOTE:

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THI CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.

WARNING:

CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENTOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMATION.

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.

Job #:

2507-2763

Designer:

Sayan Roy
Sales Rep:

Robbie Zarobinski

