### DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
	()	( /	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

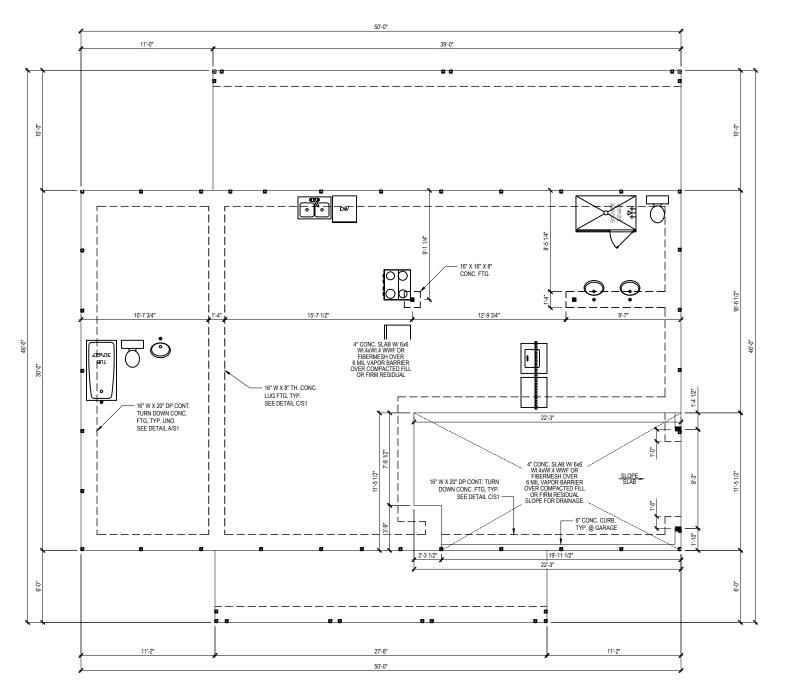
- STRUCTURAL NOTES:

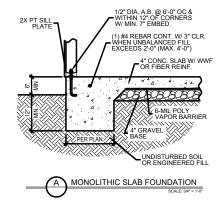
  1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS. ALL LIMBERS EVAIL IS EXCEPTED.
- ALL LUMBER SHALL BE SYP #2 (UNO)
- ALL LUMBER SHALL BE SYP #2 (UNO)
  ALL LUL LUMBER TO BE 1.75 "WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PS; E = 1.9M PS! (OR GREATER)
  (I.E. ILEVEL MICROLAM)
  ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PS!) (OR GREATER)
  ALL PSL LUMBER IS TO BE 1.8E (Fb = 2.400 PS!) (OR GREATER)
  ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2±10 W
  (1) 24 JACK STUD (UN C.) AND KING STUDS PER TABLE R802.75, AND TOGETHER W (2) 10d NAILS @ 8° OC. PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 8°-3", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 15". MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 15". MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 15". ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2±10 (UN O.) REFER TO TABLES R802.7(1) AND R802.7(2).
  ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2±10 (UN O.) REFER TO TABLES R802.7(1) AND RR02.7(2). TRUSHERS R802.7(1) AND RR02.7(2) FOR JACK STUD REQUIREMENTS FOR HEADERS PSMANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS

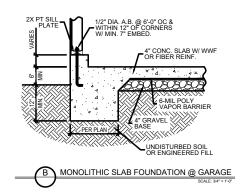
- FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
  REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS O'VER 10-0" IN HEIGHT.
  ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
  Fy = 50 KSI MIN. (UNO)
  ALL EXTERIOR LUMBER TO BE #2 SYP PT
  ALL CONCRETE, (b= 3000 PSI MIN.
  PRESUMPTIVE BEARING CAPACITY = 2000 PSF
  1/2"00 ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE
  THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
  PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3-0" O.C.
  FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
  MASONRY. MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
  PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
- NCRC.
  15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- LEAST HURIZONTAL DIMENSION.

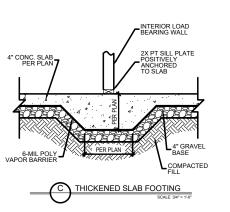
  UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
  ANCHORED TO THE FOUNDATION.

  METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.







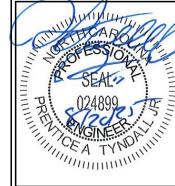


**FOUNDATION PLAN** 

Engineers seal does not include construct \* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
\* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. Will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



ABIGAIL PEARSON 8521 CHRISTIAN LIGHT RD. FUQUAY-VARINA, NC 27526 PEARSON RESIDENCE LOT #9 BLOCK #12 JASMINE RD. FUQUAY VARINA, NC 27!



Project #: 2501-010185
Date: 08/19/25
Engineered By: JA
DWG. Checked By: PTII
Scale: SEE PLAN
PELITORONIO

	REVISIONS				
No.	Date:	Remarks			
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**S**1

1 of 4

#### **DESIGN LOADS**

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
		( - /	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10 5 L/240 L				
EXTERNAL BALCONY	40 10 L/360 L				
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

- STRUCTURAL NOTES:

  1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

  2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYPIGALL SECRETURES RESPONSIBILITY TO MAIN STATEMENT OF THE PROPERTY OF ENGINEERING & DESIGN. PA IS NOT RESPONSIBLE FOR DIMENSIONS
- ENDINEERING & DESIGN, PLAS NOT RESPONSIBLE FOR INITIATIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
  ALL LUMBER SHALL BE SYP #2 (UNO)
  ALL LUL LUMBER TO BE 1.75 WIDE (ACTUAL) EACH SINGLE MEMBER AND
  Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)
- (I.E. ILEVEL MICROLAM)
  ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
- ALL LSL LUMBER IS TO BE 1:86: (Fb = 2:325 FS) (NG KEAL HE)
  ALL PSL LUMBER IS TO BE 1:86: (Fb = 2:400 PS) (NG GREATER)
  ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (?) 2:x10 w
  (1) 2:x4 JACK STUD (JAN.) AND KING STUDS PET TABLE R602.7.5, AND
  TOGETHER w! (2) 10d NAILS @ 8° O.C., PROVIDED THAT THE TOP OF THE
  WINDOW HEICHT IS 6:3°. MINIMUM BOTTOM OF THE WINDOW HEIGHT IS
  1:6°. OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- (UNO)
  REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
  OF ALL WALLS OVER 10-0" IN HEIGHT.
  ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fv = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT

- ALL CONCRETE, (s 3000 PSI MIN. PRESUMPTIVE BEARING CAPACITY = 2000 PSF 1/270 ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
- FOR BASEMENTS, ANCHOR BOLT SHALL EXTEND? "INTO CONCRETE OR MASONRY.

  PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9-0" (UNO)
  PROVIDE A MINIMUM OF 500# UPLIET & LATERAL CONNECTION AT TOP
  AND BOTTOM OF PORCH COLUMNS, (U.N.O.)
  PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
- NGRC.

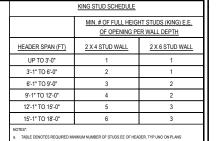
  15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

  16) UPLIFT LOADS GREATER THAN SOOR SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.

  17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

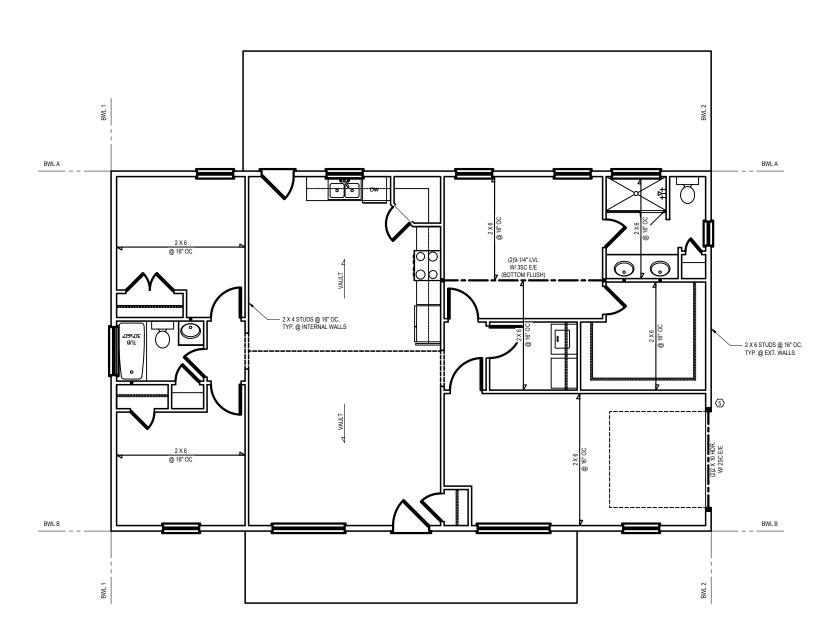
## STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
   WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
   BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.
   REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING
- CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE W 5 COOLER NAILS (OR EQUAL PER TABLE 87'02.3.5) SPACED 69 7" O.C. AT PANEL EDGES, NCLUDING TOP AND BOTTOM PLATES 8.7" O.C. AT INTERMEDIATE SUPPORTS
- 3) 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE W/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ALL SHEAL HABLE SURFACES OF EARIENDY WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 7/16'. SHEATHING SHALL BE SECURED WITH AMINIMUM BA COMMON OR GALVANIZED BOX NAILS (2-1/2' LONG X 0.113" DIA) SPACED AT 6" OL. AT PANEL EDGES AND SPACED AT CROAD AND AND STRUCTURE OF THE PARTY OF THE P 6" O.C. AT INTERMEDIATE SUPPORTS.
- MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
- MINIMOM BRACED WALL PANEL LENGTHS WITH CS-WSP
  BE AS FOLLOWS:
   24" ADJACENT TO OPENINGS NOT MORE THAN
  67% OF WALL HEIGHT
   30" ADJACENT TO OPENINGS GREATER THAN
  - 67% AND LESS THAN 85% OF WALL HEIGHT. 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- (4) SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, THERR AIN. 48' BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800S HALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR
- 5 MINIMUM 800# HOLD-DOWN DEVICE



- THES.

  THALE DENOTES REQUIRED MANAMA NUMBER OF STUDS EE OF HEADER, TYP UND ON PLANS
  NUMBER OF KING STUDS LISTED ABOVE ARE BASED IN NOMINAL WALL HEIGHT, STUD SPACING OF I
  OC., AND LITAMEN WAS RESED OF IS DUINE PEOPORIRE BY
  HEADER SPANS IN TABLE ARE BASED ON ROUGH OPENINES, INTERPOLATION BE WIFENEN SPAN VALUE
  SPERMITTER, ROUGH DUINEMERS OF KORS STUDS, ESTRAPOLATION SPONBITTED. CONTACT
  TYPIOALL ENCANCERNIG AND DESIGN IF HEADER SPANS EXCEED TABLE VALUES



FIRST FLOOR PLAN 1/8" = 1'-0"

BWL B = 6.17 FT BWL 1 = 10.25 FT BWL 2 = 10.25 FT

Engineers seal does not include conprocedures or safety precaution Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. Please review these doc

Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendetc. presented in these documents were



. PEARSON RISTIAN LIGHT RD. (-VARINA, NC 27526



E RD. ' VARINA, NC 27526

Project #: 2501-010185 Date: 08/19/25 SEE PLAN

REVISIONS

Sheet Number

**S**2

#### STRUCTURAL NOTES

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	, ,	, ,	LL	TL	
ALL FLOORS	40	10	L/360	L/240	
ATTIC (w/ walk up stairs)	30	10	L/360	L/240	
ATTIC (pull down access)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	SEISMIC ZONES A, B & C				

- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE, (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO. ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.) ALL LSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.) ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2"Ø x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES. AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE. 11)
- 12) WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE. ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS: 39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12 36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12 \*\*MEAN ROOF HEIGHT 30'-0" OR LESS

WALL AND ROOF CLADDING VALUES:

- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION. 16)
- REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA. 17)
- PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION. 20)
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

CLIMATE ZONES	FENESTRATION U-FACTOR b,j	SKYLIGHT <sup>b</sup> U-FACTOR	GLAZED FENESTRATION SHGC <sup>b,<u>k</u></sup>	CEILING <sup>m</sup> R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE <sup>i</sup>	FLOOR R-VALUE	BASEMENT <sup>c,o</sup> WALL R-VALUE	SLAB <sup>d</sup> R-VALUE AND DEPTH	CRAWL SPACE <sup>C</sup> WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5	5/13 or 5/10 cont	19	<u>5/13</u> <sup>f</sup>	0	5/13
4	0.35	0.55	0.30	38 or 30 cont j	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	<u>10/15</u>	10	<u>10/15</u>
5	0.35	0.55	NR	38 or 30 cont	<sup>n</sup> 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 <sup>g</sup>	<u>10/15</u>	10	10/19

## TABLE N1102.1 CLIMATE ZONES 3-5

- a. R-VALUES ARE MINIMUMS, U-FACTORS AND SHGC ARE MAXIMUMS, WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.
- b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.
- c. "10/15" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME
- OR R-15 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL.
- d. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM
- f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE N1101.7 AND TABLE N1101.7.
- g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM
- h. THE FIRST VALUE IS CAVITY INSULATION. THE SECOND VALUE IS CONTINUOUS INSULATION, SO \*13+5" MEANS R-13 CAVITY INSULATION PILUS R-5 INSULATED. SHEATHING, "15+3" MEANS R-15 CAVITY INSULATION, PLUS R-3 INSULATED SHEATHING, IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR, INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVIT
- i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL.
- $\downarrow$  IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE
- PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE
- PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- I. R.30 SHALL BE DEEMED TO SATISFY THE CELLING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R.30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. DITEMPT BY AS INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH OF THE ATTIC ROOF DECK.
- m. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF, THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE.

  8. 1-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY, FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED
  AND INSTALLED IN A 224 WALLE IS NOT DEEMED TO COMPLY.
- Q. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

## **DEFINITIONS FOR COMMON ABBREVIATIONS**

ALT	=	ALTERNATE	MANUF	=	MANUFACTURER
CANT	=	CANTILEVER	MAX	=	MAXIMUM
CJ	=	CEILING JOIST	MIN	=	MINIMUM
CMU	=	CONCRETE MASONRY UNIT	NOM	=	NOMINAL
COL	=	COLUMN	O.C.	=	ON CENTER
CONC	=	CONCRETE	PL	=	POINT LOAD
CONT	=	CONTINUOUS	PT	=	PRESSURE TREATED
CT	=	COLLAR TIE	REINF	=	REINFORCED
DBL	=	DOUBLE	REQ'D	=	REQUIRED
DIA	=	DIAMETER	RJ	=	ROOF JOIST
DJ	=	DOUBLE JOIST	RS	=	ROOF SUPPORT
DR	=	DOUBLE RAFTER	SC	=	STUD COLUMN
DSP	=	DOUBLE STUD POCKET	SCH	=	SCHEDULE
EA	=	EACH	SPEC	=	SPECIFIED
EE	=	EACH END	TH	=	THICK
FJ	=	FLOOR JOIST	TJ	=	TRIPLE JOIST
FND	=	FOUNDATION	TRTD	=	TREATED
FTG	=	FOOTING	TSP	=	TRIPLE STUD POCKET
GALV	=	GALVANIZED	TYP	=	TYPICAL
HORIZ	=	HORIZONTAL	UNO	=	UNLESS NOTED OTHERWISE
HT	=	HEIGHT	W	=	WIDE FLANGE BEAM
JSC	=	JACK STUD	WWF	=	WELDED WIRE FABRIC
KS	=	KING STUD	XJ	=	EXTRA JOIST

### 1500 SQ. FT. OF ATTIC / 300 = 5.0 SQ. FT. INLETS/OUTLETS REQUIRED

- CALCULATION BASED ON VENTILATORS USED AT LEAST 3'-0" ABOVE HE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED



Engineers seal does not include con procedures or safety precaution

- plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engi
- Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommen



- PEARSON IRISTIAN LIGHT RD. Y-VARINA, NC 27526

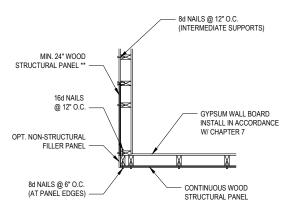


Project #: 2501-010185	
<u>Date:</u> 08/19/25	
Engineered By: JA	
DWG. Checked By: PTII	
Scale: NOT TO SCALE	

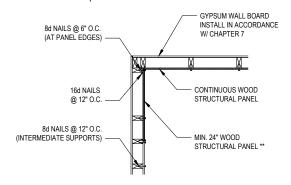
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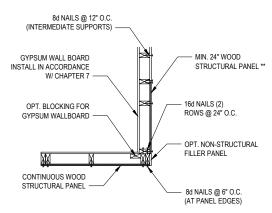


# a) OUTSIDE CORNER DETAIL



# b) INSIDE CORNER DETAIL

\*\* IN LIEU OF THE 24" (MIN.) CORNER RETURN, A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE CORNER STUD AND TO THE FOUNDATION OR FRAMING BELOW.



c) GARAGE DOOR CORNER

# **B1: TYPICAL EXTERIOR CORNER** FRAMING FOR CONTINUOUS SHEATHING

## STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL
  BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR
  WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING)
- 3 3/8" WOOD STRUCTURAL PANEL )WSP) SECURE W/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5. EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 7/16". SHEATHING SHALL BE SECURED WITH MINIMUM 8D COMMON OR GALVANIZED BOX NAILS (2-1/2" LONG X 0.113" DIA.) SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 6" O.C. AT INTERMEDIATE SUPPORTS.
- MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
  - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
  - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT
  - 48" FOR OPENINGS GREATER THAN 85% OF WALL
- 4 SHEATH INTERIOR AND EXTERIOR
- FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3 (4). IN LIEU OF A CORNER RETURN, EITHER A MINIMUM 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW
  - 5 MINIMUM 800# HOLD-DOWN DEVICE

REQUIRED BRACED WALL PANEL CONNECTIONS					
	REQUIRED CONNECTION			CONNECTION	
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS	
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 6" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.	

\*\*OR EQUIVALENT PER TABLE R702.3.5

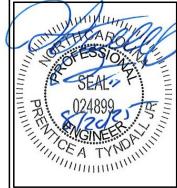
# **B3: BRACE WALL PANEL CONNECTIONS**

NO SCALE



Engineers seal does not include co procedures or safety precaution

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