

RE: 4699687 - JSJ, Belford Prime A/B (5-13-25)

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Project Customer: JSJ Builders Project Name:
Lot/Block: 80 Subdivision: DUCKS LANDING
Address:
City: LILLINGTON State: NC

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City, County: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.8
Wind Code: ASCE 7-10 Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-10
Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 3 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Job ID#	Truss Name	Date
1	177446980	4699687	A06	10/31/25
2	177446981	4699687	A07	10/31/25
3	177446982	4699687	A08	10/31/25

The truss drawing(s) referenced above have been prepared by
Truss Engineering Co. under my direct supervision based on the parameters
provided by Builders FirstSource-Sumter,SC.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



October 31, 2025

Gilbert, Eric

RE: \$JOBNAME - \$JOBDESC

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Project Customer: \$SI_CUSTOMER Project Name: \$SI_JOBNAME
Lot/Block: \$SI_LOTNUM Subdivision: \$SI_SUBDIV
Address: \$SI_SITEADDR
City, County: \$SI_SITECITY State: \$SI_SITESTATE

RE: \$JOBNAME - \$JOBDESC

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

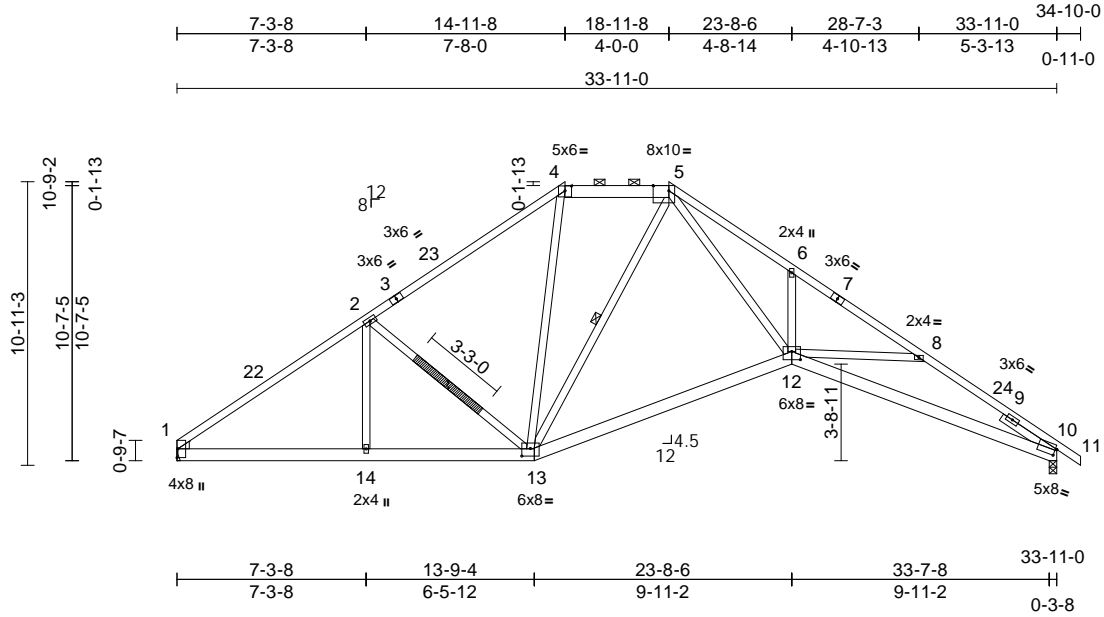
Project Customer: \$SI_CUSTOMER Project Name: \$SI_JOBNAME
Lot/Block: \$SI_LOTNUM Subdivision: \$SI_SUBDIV
Address: \$SI_SITEADDR
City, County: \$SI_SITECITY State: \$SI_SITESTATE

Job	Truss	Truss Type	Qty	Ply	JSJ, Belford Prime A/B (5-13-25)	
4699687	A06	Hip	1	1	Job Reference (optional)	I77446980

Builders FirstSource (Sumter, SC), Sumter, SC - 29153,

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ID:JBYSAsVECGGoPzEjH6uDL?dzWi0H-RfC?PsB70Hq3NSgPqnL8w3uITxbGKWrcDoi7J4zJC?r

Page: 1



Scale = 1:88.8

Plate Offsets (X, Y): [1:Edge,0-0-6], [4:0-3-0,0-2-3], [5:0-7-5,Edge], [10:0-0-11,0-3-2], [12:0-4-0,0-3-12], [13:0-4-0,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.56	Vert(LL)	-0.20	12-13	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.43	12-13	>937	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.25	10	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MS		Wind(LL)	0.19	12-13	>999	240	Weight: 231 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E or 2x4 SP DSS or 2x4 SP
SS *Except* 4-5:2x6 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E or 2x6 SP DSS
WEBS 2x4 SP No.3 *Except* 12-5:2x4 SP No.2
WEDGE Left: 2x4 SP No.3
SLIDER Right 2x4 SP No.2 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or
2-9-9 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
bracing.
WEBS 1 Row at midpt 5-13

REACTIONS

(size) 1= Mechanical, 10=0-3-8
Max Horiz 1=344 (LC 10)
Max Uplift 1=323 (LC 12), 10=353 (LC 13)
Max Grav 1=1356 (LC 1), 10=1412 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=1945/542, 2-4=1543/551,
4-5=1281/538, 5-6=3454/968,
6-8=3438/781, 8-10=3673/939, 10-11=0/31
BOT CHORD 1-14=457/1564, 13-14=457/1564,
12-13=140/1455, 10-12=703/3163
WEBS 6-12=409/340, 8-12=311/380, 2-14=0/222,
5-12=534/2522, 4-13=113/504,
5-13=555/101, 2-13=613/386

NOTES

- 1) Repair Condition: web has 0-3-0 long break centered at 3-10-9 below joint 2.
- 2) Apply 39" long 2x4 SP No.2 scab to front side(s) of truss centered on damage located 3-10-9 below joint 2 with 2 row(s) of 10d (0.131"x3") nails spaced 2" o.c. from front face. Minimum 0-3-0 end distance.

3) N/A.

- 4) Unbalanced roof live loads have been considered for this design.
- 5) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) 0-0-0 to 3-4-11, Interior (1) 3-4-11 to 14-11-8, Exterior (2) 14-11-8 to 23-8-6, Interior (1) 23-8-6 to 34-10-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 1 and 353 lb uplift at joint 10.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



October 31,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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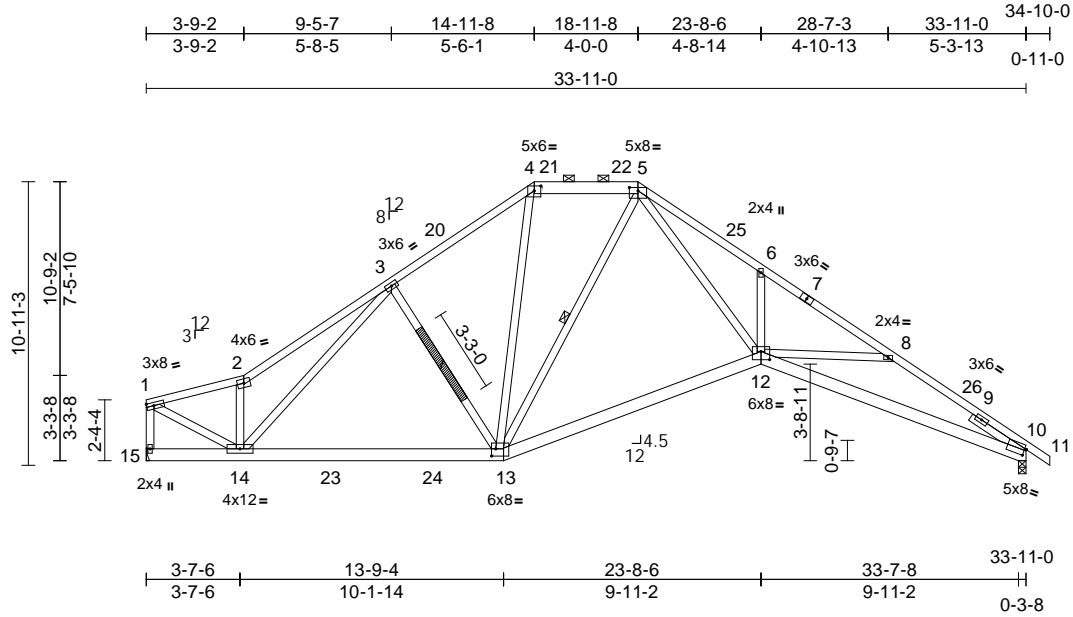
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	JSJ, Belford Prime A/B (5-13-25)	
4699687	A07	Piggyback Base	1	1	Job Reference (optional)	I77446981

Builders FirstSource (Sumter, SC), Sumter, SC - 29153,

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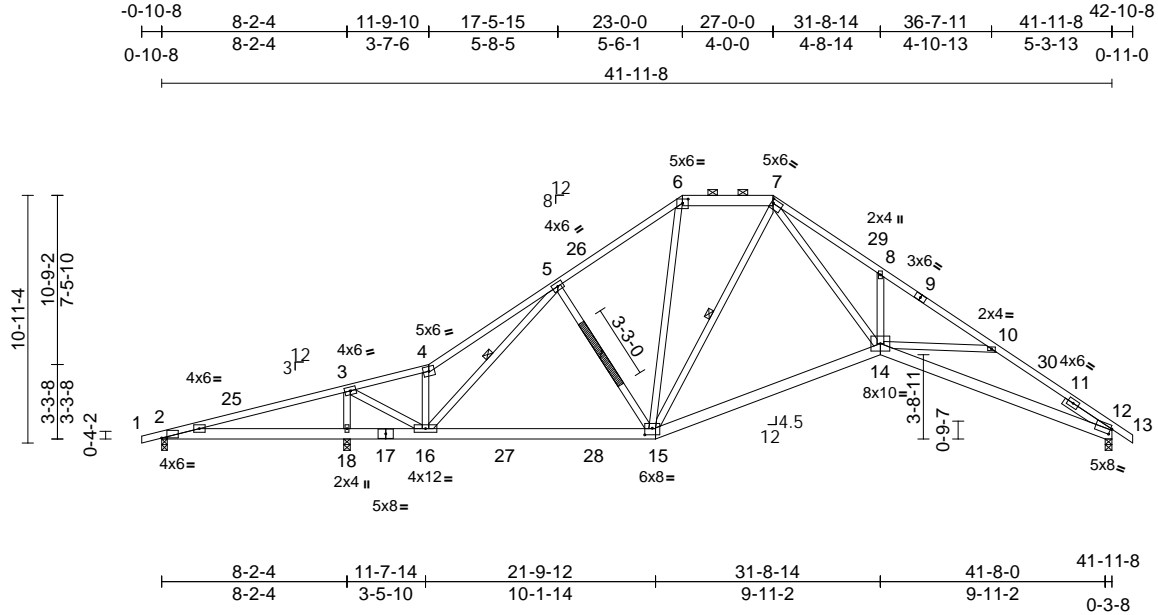


Job	Truss	Truss Type	Qty	Ply	JSJ, Belford Prime A/B (5-13-25)	
4699687	A08	Piggyback Base	3	1	Job Reference (optional)	177446982

Builders FirstSource (Sumter, SC), Sumter, SC - 29153,

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Page: 1



Scale = 1:101.7

Plate Offsets (X, Y): [2:0-2-12,0-0-2], [6:0-3-0,0-2-3], [7:0-1-0,0-2-4], [12:0-0-11,0-3-2], [15:0-4-0,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.48	Vert(LL)	-0.19	14-15	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.33	Vert(CT)	-0.40	14-15	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.98	Horz(CT)	0.22	12	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MS		Wind(LL)	0.17	14-15	>999	240	Weight: 279 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E or 2x4 SP DSS or 2x4 SP
SS *Except* 6-7:2x6 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E or 2x6 SP DSS
WEBS 2x4 SP No.3
SLIDER Right 2x4 SP No.2 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or
3-0-14 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc
bracing.
WEBS 1 Row at midpt 5-16, 7-15

REACTIONS

(size) 2=0-3-0, 12=0-3-8, 18=0-3-8
Max Horiz 2=364 (LC 11)
Max Uplift 2=-277 (LC 8), 12=-340 (LC 13),
18=-575 (LC 12)
Max Grav 2=82 (LC 23), 12=1317 (LC 1),
18=2134 (LC 1)

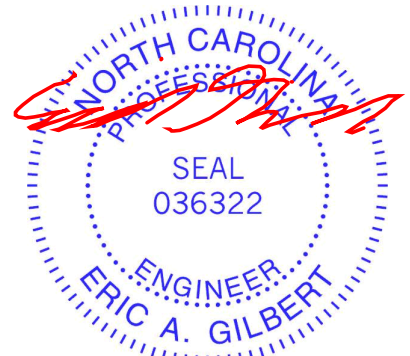
FORCES

(lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=0/15, 2-3=-363/1292, 3-4=-663/123,
4-5=-811/239, 5-6=-1318/494,
6-7=-1075/467, 7-8=-3141/848,
8-10=-3103/656, 10-12=-3367/825,
12-13=0/31
BOT CHORD 2-18=-1269/424, 16-18=-1269/424,
15-16=-260/1130, 14-15=-82/1262,
12-14=-606/2903
WEBS 3-18=-1830/524, 8-14=-446/350,
10-14=-346/384, 4-16=-484/250,
3-16=-389/2062, 5-15=-273/308,
5-16=-713/265, 6-15=-143/495,
7-15=-584/99, 7-14=-471/2368

NOTES

- Repair Condition: web has 0-3-0 long break centered at 3-6-8 below joint 5.
- Apply 39" long 2x4 SP No.2 scab to front side(s) of truss centered on damage located 3-6-8 below joint 5 with 2 row(s) of 10d (0.131"x3") nails spaced 2" o.c. from front face. Minimum 0-3-0 end distance.
- N/A.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) -0-10-8 to 3-3-14, Interior (1) 3-3-14 to 23-0-0, Exterior (2) 23-0-0 to 31-2-6, Interior (1) 31-2-6 to 42-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 2, 340 lb uplift at joint 12 and 575 lb uplift at joint 18.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



October 31,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompoments.com)

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Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

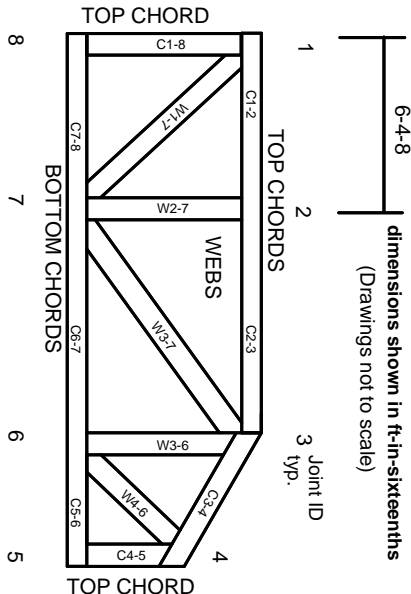
BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:
ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:
ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.
Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023