ELEVATIONS



ROOFING NOTE: ADD SNOW AND ICE SHIELD BENEATH ROOF PITCHES THAT ARE LESS THAN  $\frac{4}{12}$ 

ROOFING NOTE PER 2018 NC BUILDING CODE R905.2: TWO LAYERS OF UNDERLAYMENT MUST BE APPLIED TO ROOF PITCHES THAT ARE LESS THAN  $\frac{4}{12}$ 

# FRONT ELEVATION SCALE = \frac{1}{4}" = 1'-0"



ELEVATIONS



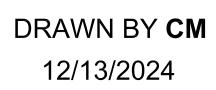
RIGHT ELEVATION

SCALE = \frac{1}{4}" = 1'-0"



REAR ELEVATION

SCALE = \frac{1}{4}" = 1'-0"

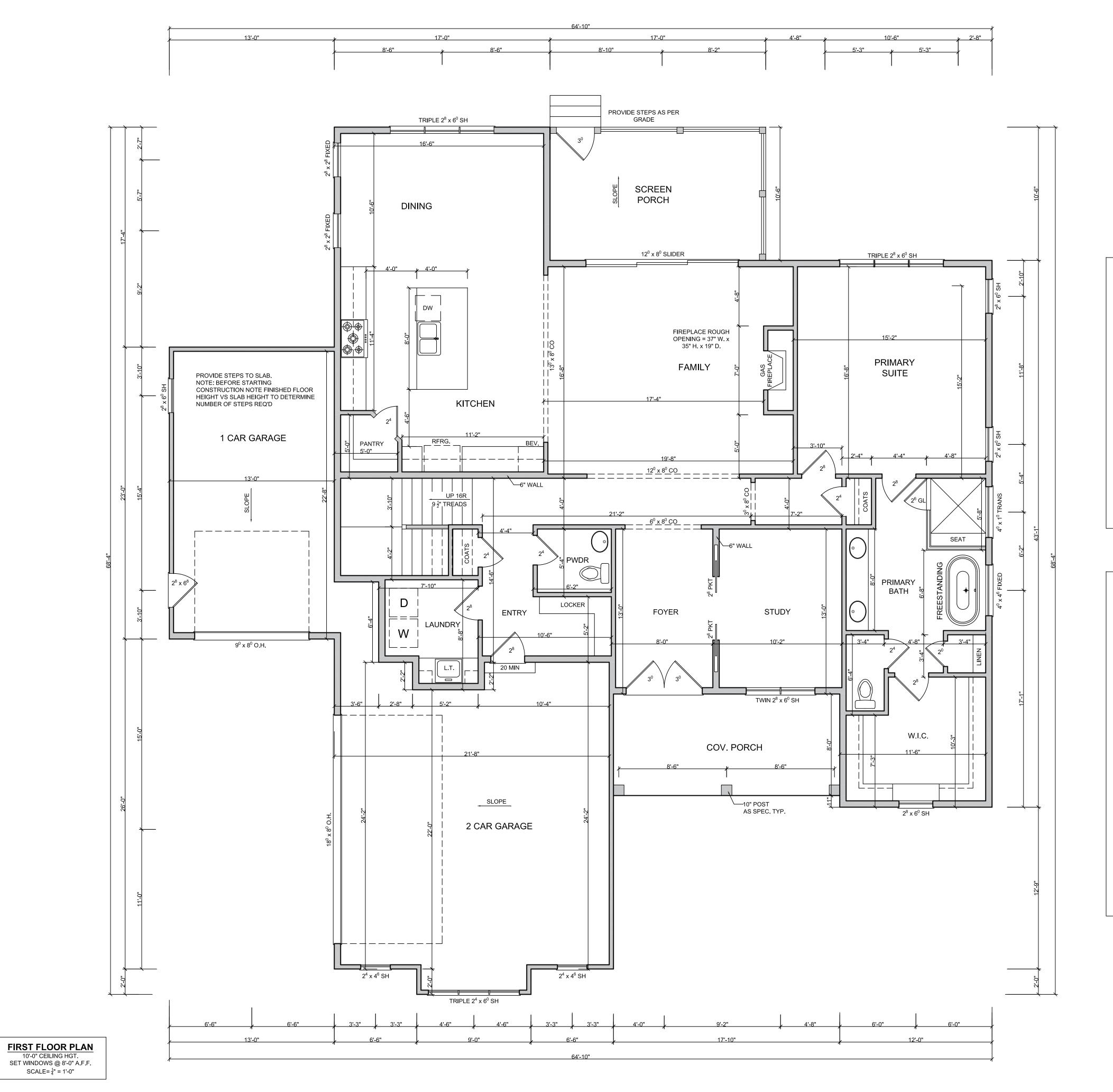


**REVISION DATE** 

SHEET NO.

3

FLOOR PLAN



# AREA CALCULATIONS

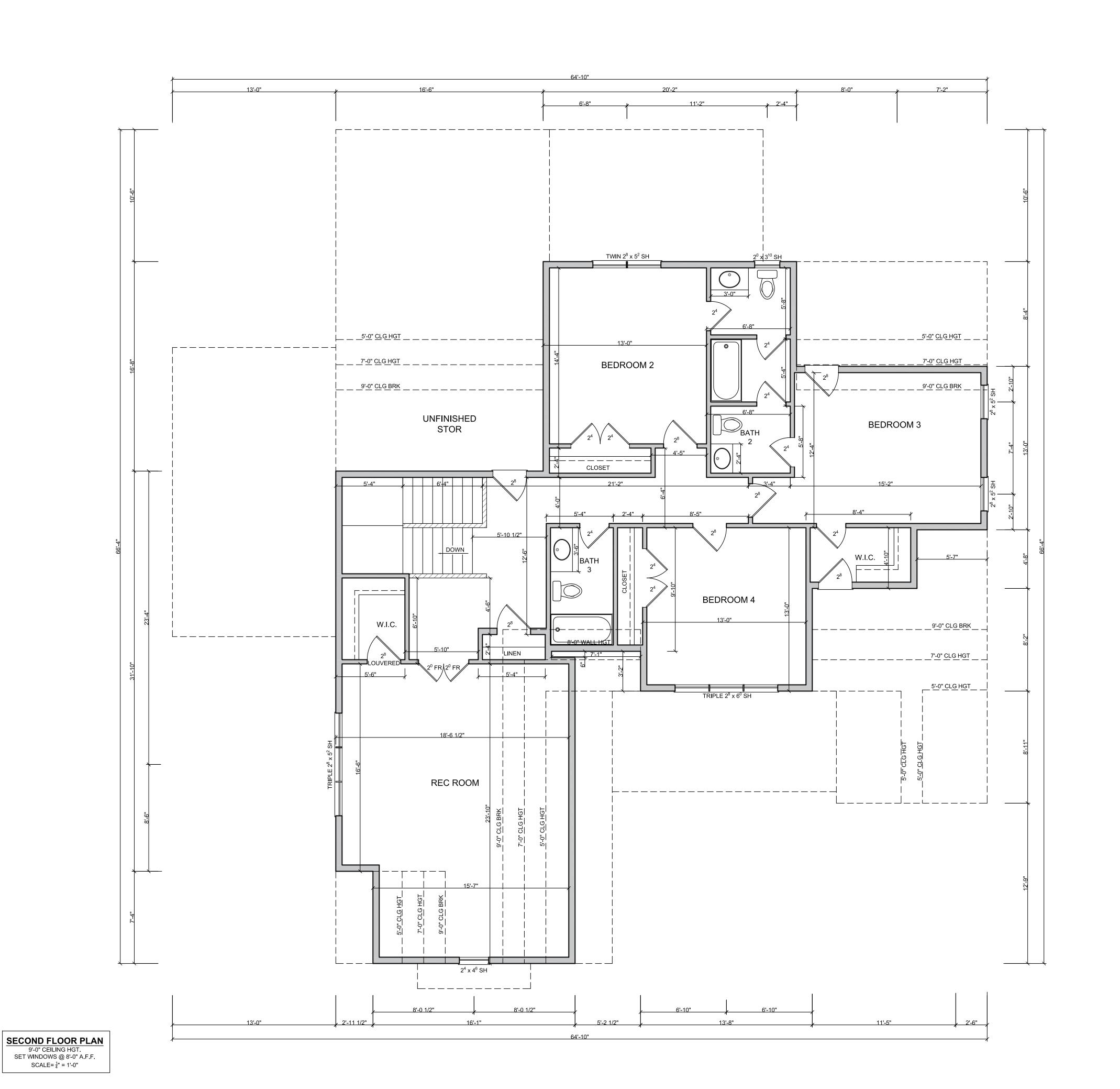
HEATED (SQ.FT.)		UNHEATED (SQ.FT.)	
BASEMENT 1ST FLOOR SECOND FLOOR	N/A 1992 1521	GARAGE PORCH SCREEN	858 140 179
TOTAL	3513	TOTAL	332 1509

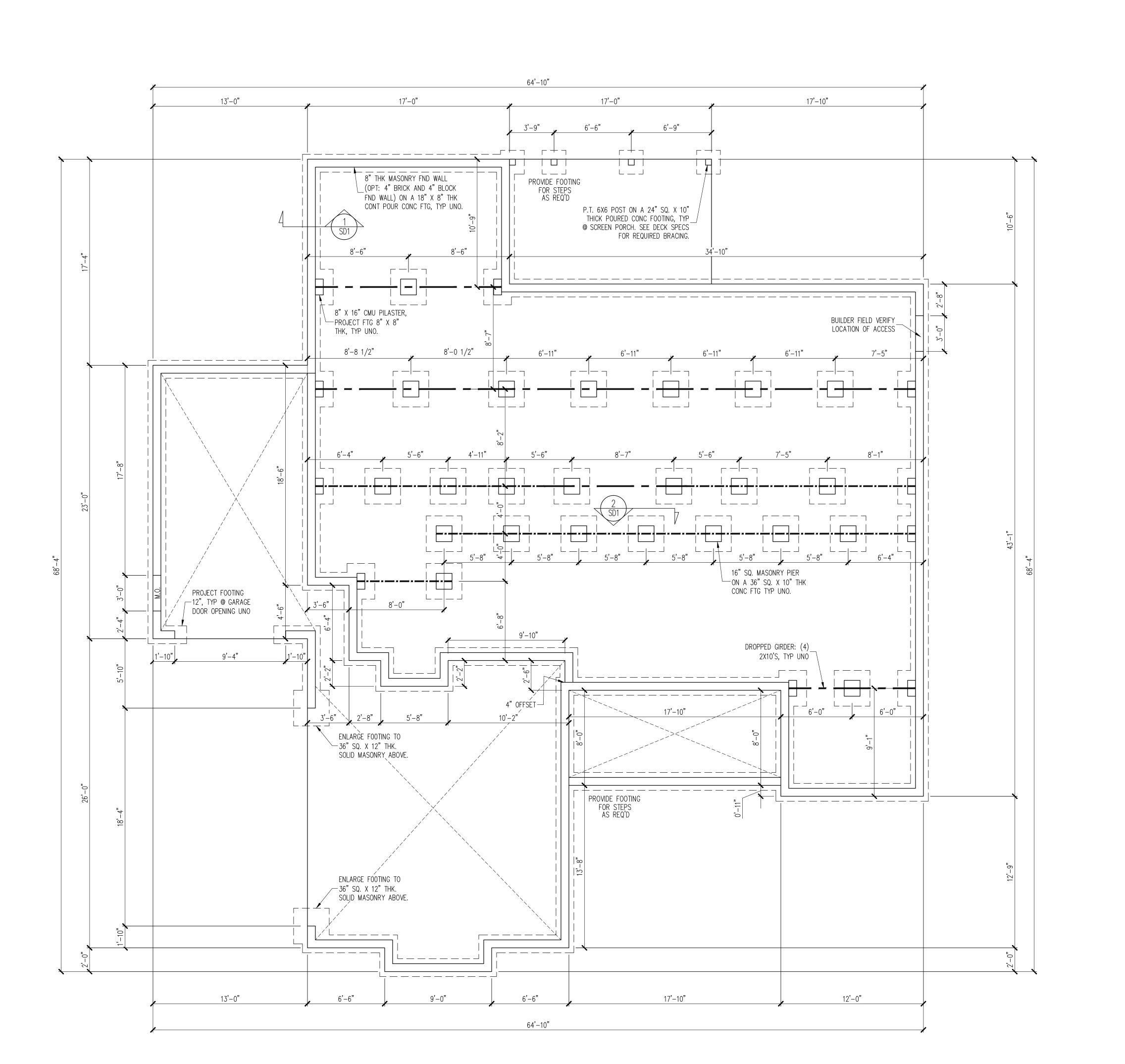
#### NOTES

- PLANS DESIGNED UNDER 2018 NORTH CAROLINA RESIDENTIAL CODE.
- ALL ANGLED WALLS ARE 45° UNLESS NOTED OTHERWISE.
- FINISH DOOR AND WINDOW HEADS SHALL ALIGN EXCEPT AS NOTED.
- ALL MATERIALS AND COMPONENTS MUST BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS.
- GRADES SHOWN ARE ILLUSTRATIVE, NATURAL GRADE WILL DETERMINE FINISH GRADE.
- FLOOR PLAN NOTATIONS GOVERN OVER ELEVATION SCALE.
- VERIFY ALL WINDOW SIZES, RADIUS AND DETAILS WITH CHOSEN MANUFACTURER.
- ALL HABITABLE ROOMS SHALL MEET LIGHT/VENTILATION & EGRESS REQUIREMENTS.
- TEMPERED GLASS TO BE USED AT ALL SAFETY REQUIRED LOCATIONS.
- ALL WINDOW GLAZING TO HAVE 0.35 U-FACTOR MIN.
- ALL CABINET DESIGNS/LAYOUTS TO BE VERIFIED WITH SHOP DRAWINGS FROM CABINET MANUFACTURER.
- PER 2018 N.C. RESIDENTIAL BUILDING CODE TABLE R302.6: SHEETROCK ON GARAGE CEILING TO BE  $\frac{5}{8}$ " TYPE "X".

4

FLOOR PLAN





ENG: TRB/MEB DATE: 7/9/2025

> PLAN **BRIGHTON**

PROJECT NO. 25-27-081

SHEET NO.

of 7

FOUNDATION PLAN

PLAN DESIGNED UNDER

2018 NORTH CAROLINA

RESIDENTIAL CODE

-HEIGHT AND BACKFILL LIMITATIONS FOR
FOUNDATION WALLS ARE TO BE GOVERNED
BY THE NCSBC, LATEST EDITION.
REINFORCEMENT AND GROUTING SHALL BE
DETERMINED BY FINAL SITE CONDITIONS.

-BUILDER TO FIELD LOCATE CRAWLSPACE

ACCESS OPENING WITH MINIMUM DIMENSIONS OF 18X24. DO NOT LOCATE ACCESS OPENING BELOW POINT LOADS FROM ABOVE WITHOUT ENGINEER APPROVAL.

CONSTRUCTION SPECIFICATIONS

INSTANT REFERENCES

REFER TO THE CONSTRUCTION SPECIFICATIONS

SECTIONS FOR THE FOLLOWING INFORMATION:

PART 1.01: <u>CURRENT GOVERNING CODE</u>

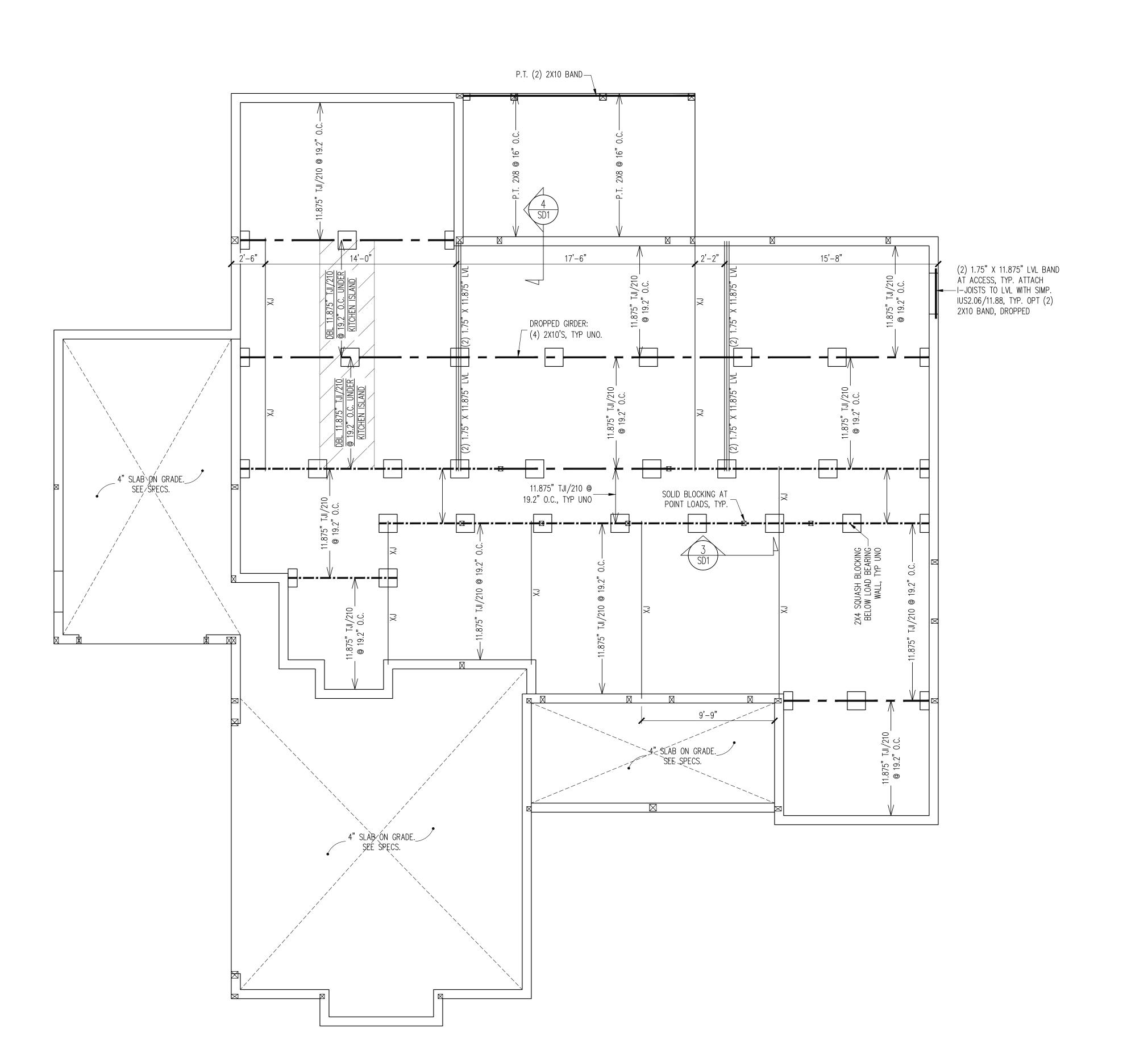
PART 14: <u>STUD SUPPORT FOR BEAMS</u>

PART 16.02: GENERAL WALL BRACING NOTES

PART 17: KING STUDS FOR EXTERIOR WALLS

SEE DETAIL / CONSTRUCTION SPECIFICATIONS SHEETS FOR I-JOISTS ALLOWABLE SUBSTITUTIONS

1/4" = 1'-0"



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DDENDUM

REV # REF PROJ # DATE

ASSO

SCOPE STRUCTU LOC 1 PINEHURST

ENG: TRB/MEB

DATE: 7/9/2025
PLAN

BRIGHTON
PROJECT NO.

25-27-081 SHFFT NO.

SHEET NO.

 $\frac{1/4" = 1'-0"}{2} \quad \boxed{2 \quad \text{of} \quad 7}$ 

CRAWL SPACE FRAMING

TRUSS SUBSTITUTION

11.875" I—JOISTS PERMITTED TO BE SUBSTITUTED WITH 12" FLOOR TRUSSES.

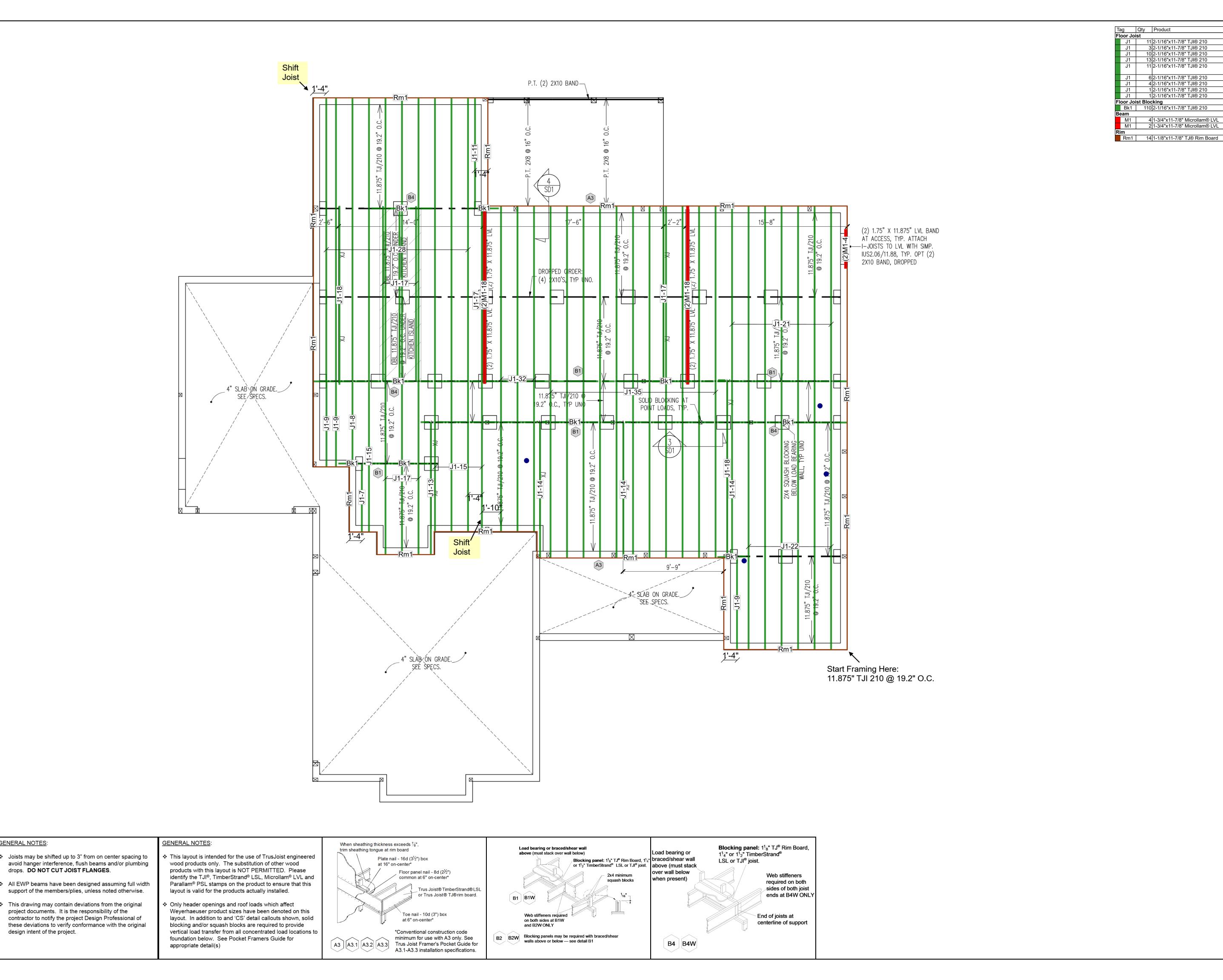
MAINTAIN MINIMUM SPACING AS CALLED OUT ON PLANS.

SIMP. IUS/ITS2.06 HANGERS TO BE SUBSTITUTED WITH SIMP. IUS/ITS3.56 HANGER WHEN FLOOR TRUSSES HAVE BEEN INSTALLED.

GIRDER LEGEND

ALL GIRDERS (4) 2X10'S, TYP UNO. BOLTING NOT REQUIRED FOR DROPPED GIRDERS.

I-JOIST SQUASH BLOCKING REQUIRED



**GENERAL NOTES**:

design intent of the project.



Len Cut Logic

R/L

16' 14/16

Dickersor y Dicke Pinehurst  $\mathbf{\Omega}$  $\Theta$ OMO

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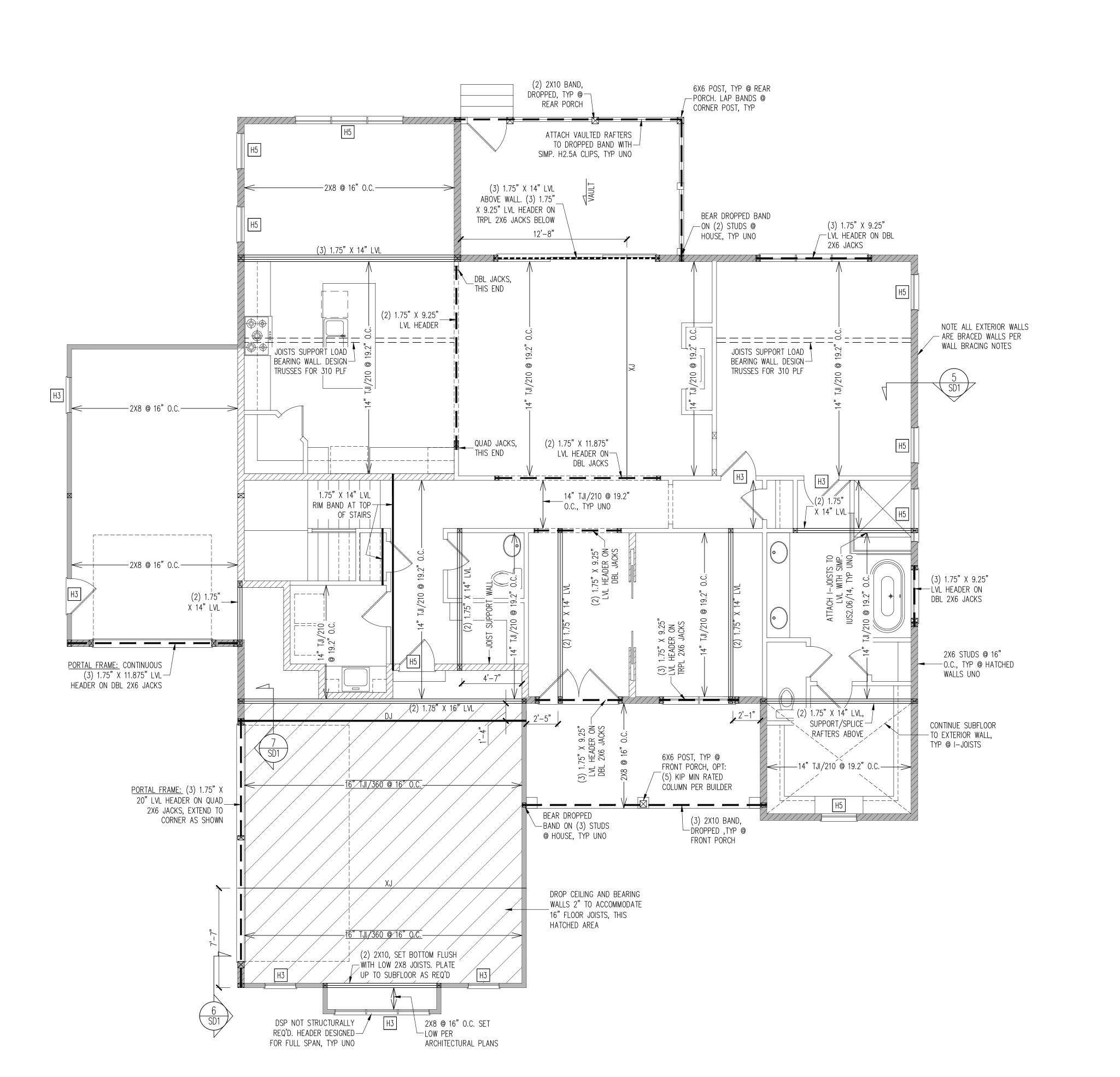
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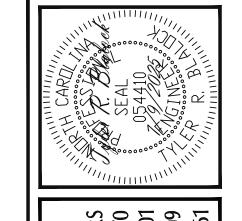
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Architectural Date: 12/13/2024

Structural Date: 7/9/2025 stimator: **EMP** racking: JMP7986

Sheet 1 of 2





MAINTAIN MINIMUM SPACING AS CALLED OUT ON SIMP. IUS/ITS2.06 HANGERS TO BE SUBSTITUTED WITH SIMP. IUS/ITS3.56 HANGER WHEN FLOOR

TRUSS SUBSTITUTION

14" I-JOISTS PERMITTED TO BE SUBSTITUTED WITH

16" I-JOISTS PERMITTED TO BE SUBSTITUTED WITH

14" FLOOR TRUSSES.

16" FLOOR TRUSSES.

## WALL BRACING

TRUSSES HAVE BEEN INSTALLED.

CS - ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD. \_\_\_\_\_

SHADED WALLS:

-PROVIDED CONTINUOUS SHEATHING = 294' MIN.

\_\_\_\_\_

### HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.

\_\_\_\_\_

(C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

-HEADERS IN NON LOAD BEARING INTERIOR

WALLS ARE NOT LABELED.

1ST FLOOR FRAMING PLAN

WALLS AND CEILING 1/4" = 1'-0"

3 of 7

ENG: TRB/MEB

DATE: 7/9/2025

PLAN

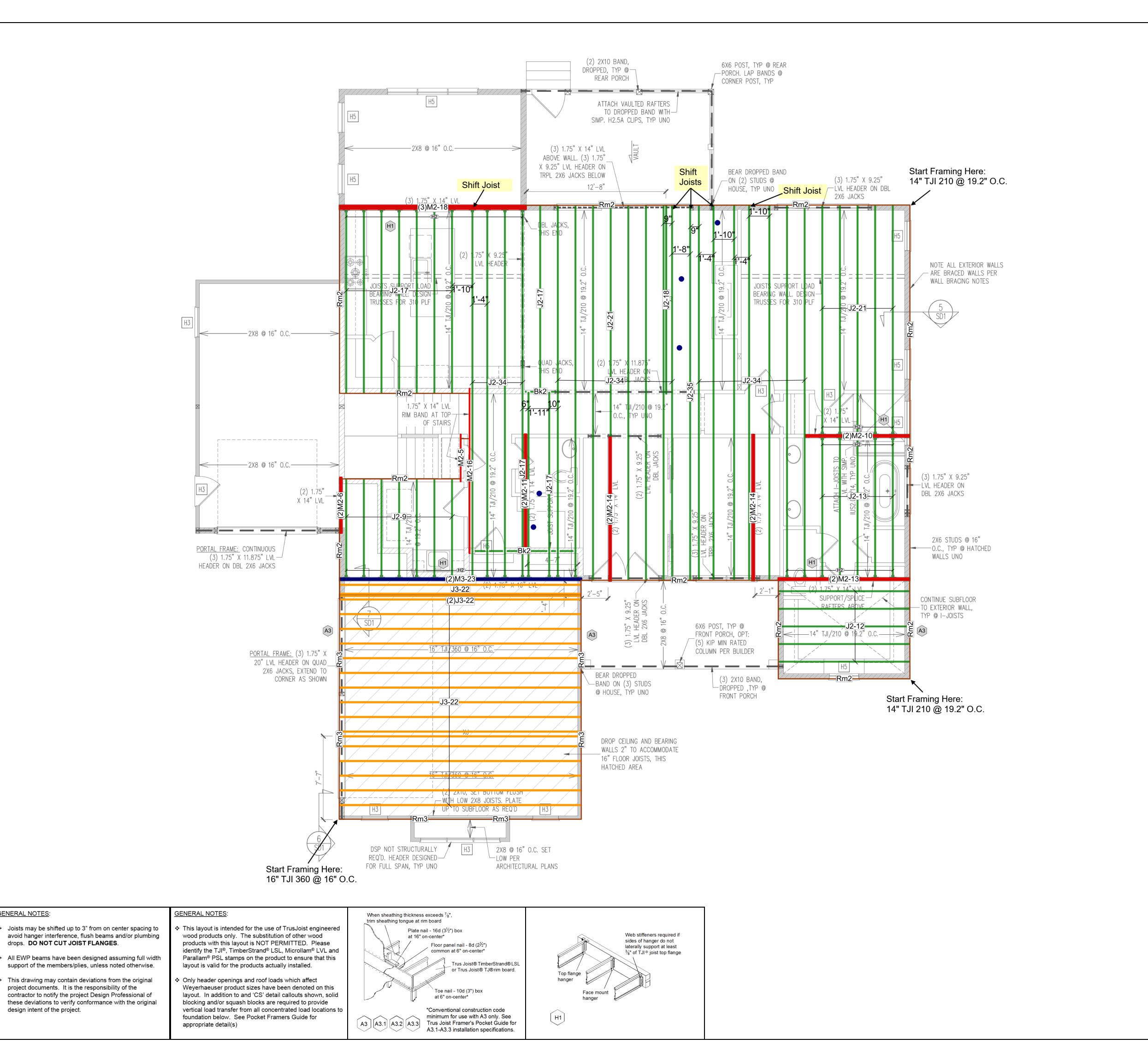
**BRIGHTON** 

PROJECT NO.

25-27-081

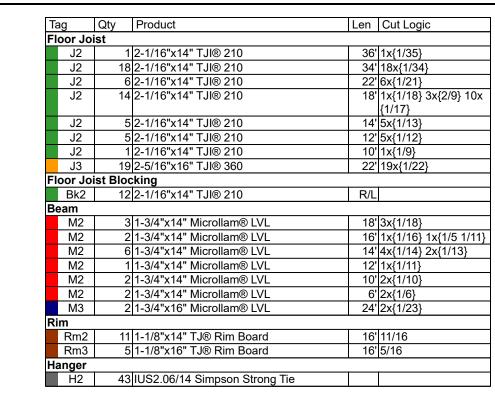
SHEET NO.

**S**3



GENERAL NOTES:

design intent of the project.





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Layor

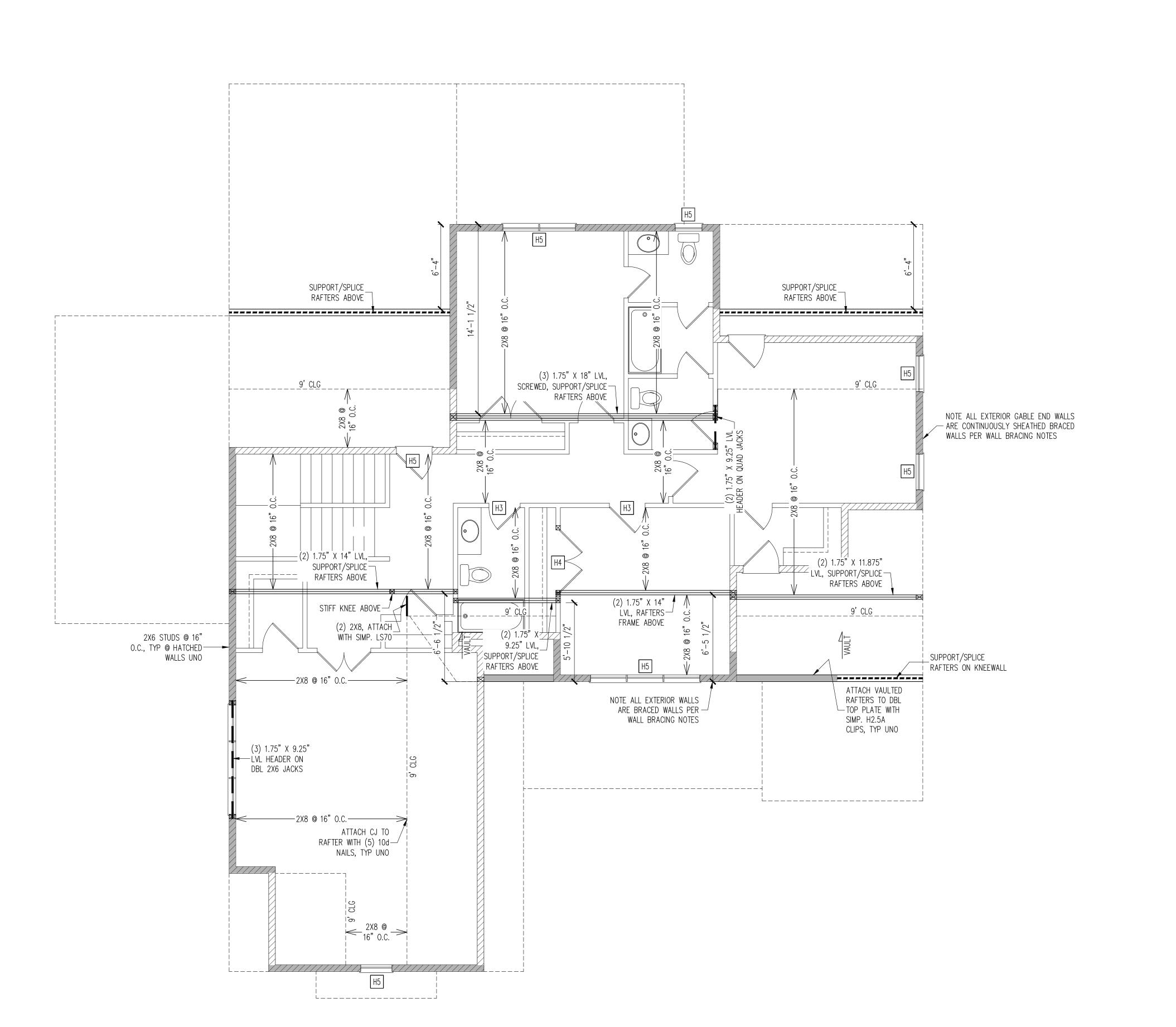
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7

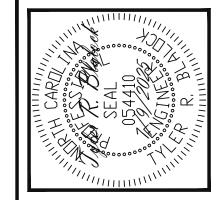
Architectural Date: 12/13/2024 Structural Date: 7/9/2025

stimator: **EMP** racking: JMP7986

Sheet 2 of 2



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PLAN

**BRIGHTON** 

PROJECT NO.

25-27-081

SHEET NO.

-PROVIDED CONTINUOUS SHEATHING = 139' MIN.

HEADER SCHEDULE

WALL BRACING

CS - <u>ALL</u> EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH

7/16 APA RATED OSB NAILED TO STUDS WITH

8d NAILS @ 6" O.C. AT PANEL EDGES, 12"

\_\_\_\_\_

H1 SINGLE 2X4 TURNED FLAT (A)

O.C. IN PANEL FIELD.

SHADED WALLS:

- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.

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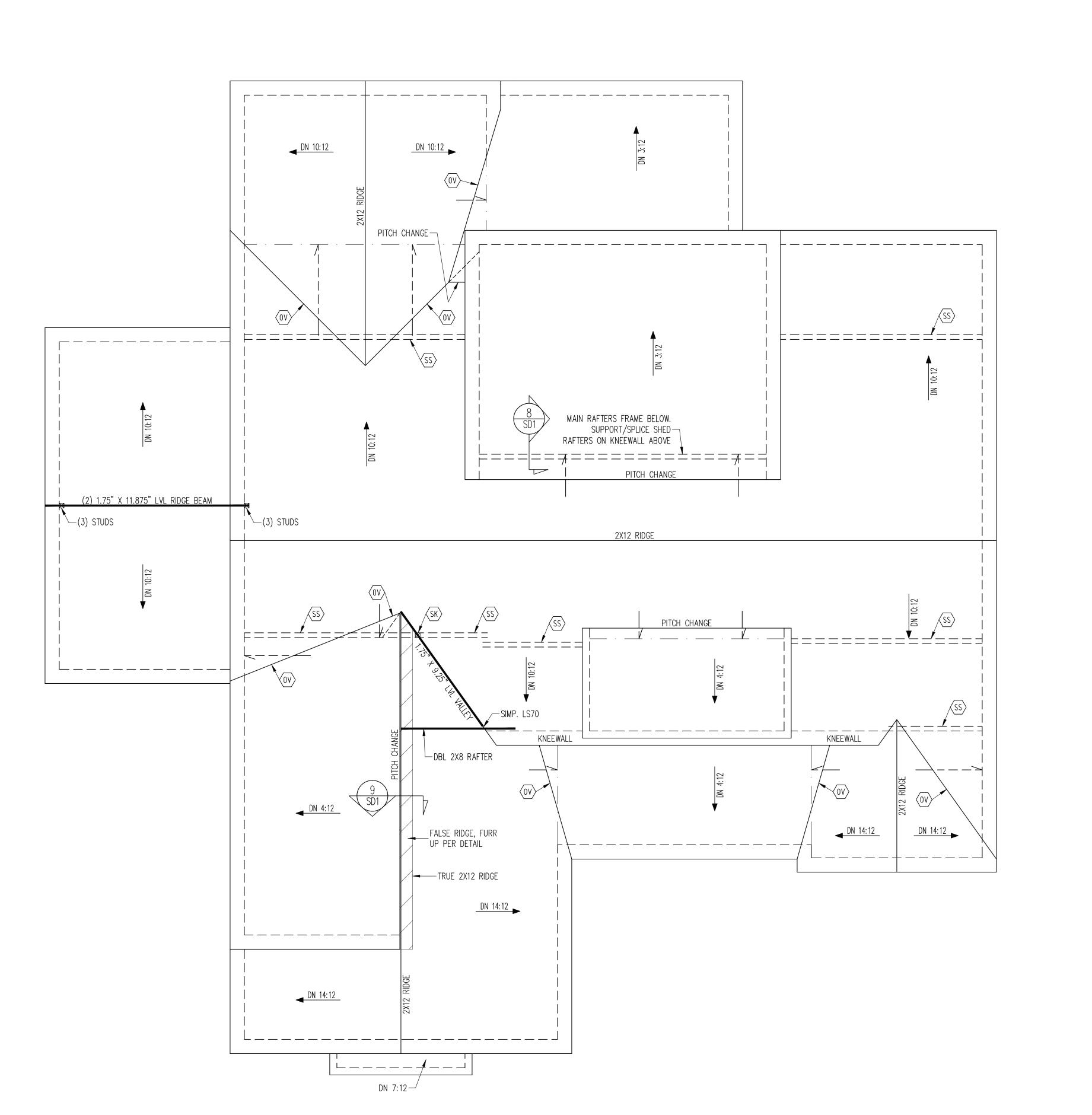
(C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

DTES:

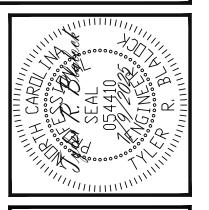
-HEADERS IN NON LOAD BEARING INTERIOR WALLS ARE NOT LABELED.

2ND FLOOR FRAMING PLAN
WALLS AND CEILING 1/4" = 1'-0"

 $\frac{\text{CEILING } 1/4" = 1'-0"}{\text{4 of } 7}$ 



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HURST

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BRIGHTON

PROJECT NO. 25-27-081

SHEET NO.

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1/4" = 1'-0"

ROOF FRAMING PLAN

FRAMING NOTES

ROOF ONLY

-COMMON RAFTERS 2X8 @ 16" O.C. TYP U.N.O.
-COLLAR TIES 2X4 EVERY 3RD SET OF RAFTERS
TYP U.N.O.
-VERIFY ROOF PITCHES, OVERHANG LENGTHS, AND
KNEEWALL FRAMING HGTS WITH ARCHITECTURAL
DRAWINGS, TYPICAL.

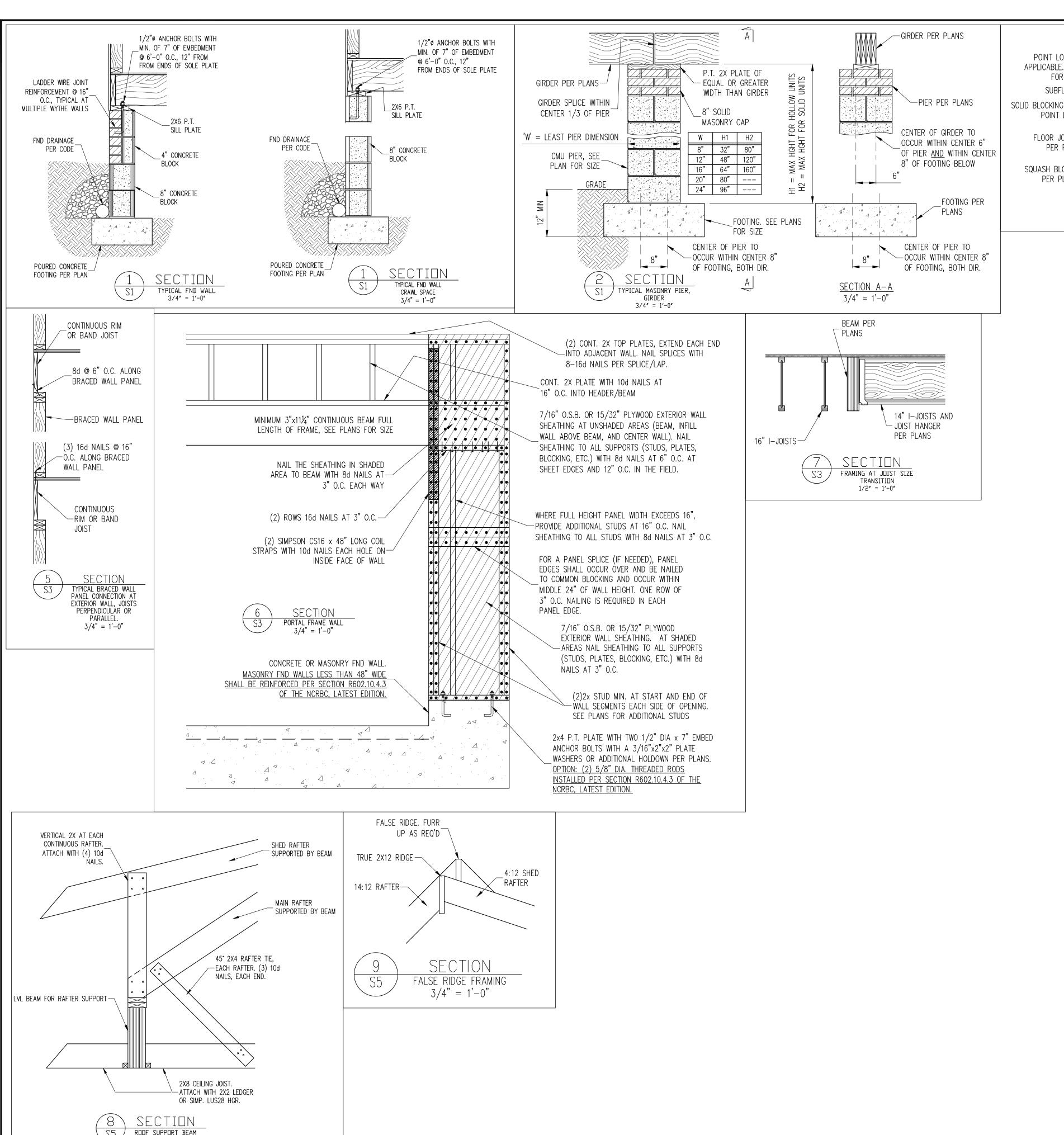
FRAMING SCHEDULE

ROOF ONLY

OV OVERFRAME VALLEY ( 2X10 SLEEPER )

SS SUPPORT/SPLICE RAFTERS ON KNEEWALL BELOW

SK DBL 2X4 STIFF KNEE



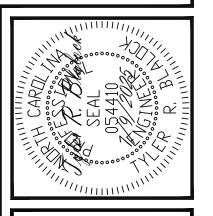
AT SHED ROOF 3/4'' = 1'-0''

STUD WALL ABOVE, DECK BAND FASTENED TO RIM JOIST WITH BOLTS AND FROM ENDS WHERE APPLICABLE. NAILS. SEE DECK SPECS. POINT LOAD, WHERE APPLICABLE. SEE PLAN— FOR LOCATION. RIM JOIST-SUBFLOOR — SOLID BLOCKING FOR 2X10 P.T. SECTION A-A POINT LOAD DECK BAND 3/4" = 1'-0" P.T. 2X2 LEDGER DECK BAND FASTENED TO RIM JOIST WITH FLOOR JOISTS -TWO ROWS OF SIMPSON SDWS22400DB PER PLAN 3" TO 6" (OR EQUAL). SEE DECK SPECS. ─FLASHING FROM ENDS d/2 SQUASH BLOCKS PER PLANS SECTION SECTION

DECK CONNECTION AT SIDING, TYP.

3/4" = 1'-0" S2 TYPICAL DROPPED GIRDER 3/4" = 1'-0" SECTION A-A OPTIONAL 3/4" = 1'-0"

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SCOPE STRUCTURAL ADDENDUM
LOC: 1 PINEHURST

LOC: 1 PINEHURST

LOC: 1 PINEHURST

REV # REF PROJ #

ENG: TRB/MEB
DATE: 7/9/2025

PLAN BRIGHTON

PROJECT NO. 25-27-081

SHEET NO.

6 of 7

#### DECK SPECIFICATIONS A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO PART 1: GENERAL JOIST SPAN DECKING A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED USING THESE PROVISIONS. 1" S4S 12" O.C 16" O.C. 1" T&G SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING. 24" O.C. 11/4" S4S 32" O.C. 2" S4S WHEN ATTACHED TO A STRUCTURE. THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING 9. MAXIMUM HEIGHT OF DECK SUPPORT POSTS IS AS FOLLOWS: SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED POST SIZE MAX POST HEIGHT FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE 6X6 INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK ENGINEERED STRUCTURE. NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT WITH THE BRICK NOTES: 1) THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. 2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT. WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE 3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER. STRUCTURE, THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE DECK BAND TO THE STRUCTURE: DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING A. ALL STRUCTURES EXCEPT BRICK STRUCTURES A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-O" AND THE DECK IS ATTACHED TO JOIST LENGTH THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED. UP TO 8' MAX. UP TO 16' MAX. B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. REQUIRED | ONE- 5/8" Ø BOLT @ 42" O.C. AND | ONE- 5/8" Ø BOLT @ 20" O.C. AND THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF FASTENERS (2) ROWS OF 12d NAILS @ 8" O.C. OR (3) ROWS OF 12d NAILS @ 6" O.C. OR THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED TWO ROWS OF SIMPSON SDWS22400DB TWO ROWS OF SIMPSON SDWS22400DB BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE ATTACHED AT THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8" BOLT A . BRICK VENEER STRUCTURES C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE | 2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH. JOIST LENGTH WITH THE FOLLOWING: UP TO 16' MAX. UP TO 8' MAX. POST SIZE | TRIBUT. AREA | POST HEIGHT | EMB. DEPTH | CONC. DIAM. RFQUIRFD ONE- 5/8" Ø BOLT @ 16" O.C. ONE- 5/8" Ø BOLT @ 28" O.C. FASTENERS 6'-0" IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT. D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO JOISTS TO A TREATED STRUCTURE BAND THE POSTS WITH ONE -5/8" $\phi$ BOLT AT EACH END OF THE BRACE. GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED. WITH 2- 5/8" Ø BOLTS 2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2". 3) NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF 1 1/2". FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE FOR BUILDINGS. MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS: PART 4: WELDING PART 7: MASONRY SUBCONTRACTORS THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER

PART 2: DESIGN LOADS USE BALCONIES, DECKS, ATTICS WITH FIXED STAIR PART 3: STRUCTURAL STEEL AWS CERTIFIED WELDER PART 6: REBAR AND WIRE REINFORCEMENT THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (EOR) BEFORE PROCEEDING IF THE FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION: 1) THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR 2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION ANY FRRORS DUE TO A FAILURE TO FOLLOW THE ABOVE PROCEDURES SHALL NOT BE THE RESPONSIBILITY OF THE EOR. FURTHERMORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO ENSURE THAN ANY REVISIONS ISSUED BY THE EOR ARE PROMPLY DISTRIBUTED TO THE

2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:

CONSTRUCTION SPECIFICATIONS

CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL

1.02 DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.

1.05 METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF 7.04 MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530 THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

LIVE LOAD (PSF) DEAD LOAD (PSF)

ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STAIRS, FIRE ESCAPES GARAGES (PASSENGER CARS ONLY) ATTICS (NO STORAGE, LESS THAN 5' HEADROOM) ATTICS (WITH STORAGE) 20 10 (15 FOR VAULTS)

OTES: — INDIVIDUAL STAIR TREADS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS. BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINISHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER

2 INTERIOR WALLS: 5 PSF LATERAL.

.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).

WIDE FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM

3.02 SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM

5.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE

3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE 3.05 STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL

1.01 WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN 🛭 13.01 FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN

PART 5: CONCRETE AND SLABS ON GRADE 5.01 CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 4-6% AIR ENTRAINMENT, FOR

EXTERIOR CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO. ALL ITEMS NOTED AS 'CONCRETE' ARE TO BE CAST IN PLACE,

5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION.

03 SLABS ON GRADE, IF ANY, SHALL BE CAST IN PLACE, CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS. FIBER LENGTH 1 1/2". DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 4" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS

6.01 REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO 6.02 LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO

6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064.

CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING.

TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW

ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE. FINAL FL PL FLITCH PLATE

7.01 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT,

f'M = 1,500 PSI MIN

7.02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW

7.03 MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.

7.05 LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS

PART 8: BOLTS AND LAG SCREWS

8.01 BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD MEMBERS. HOLES FOR BOLTS SHALL BE AISC STANDARD HOLES UNO

LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO NDS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR

ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554-15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO

PART 9: DRIVEN FASTENERS

PART 10: DIMENSIONAL LUMBER

9.01 NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667- 05. NAILS ARE TO BE COMMÓN WIRE OR BOX

10.01 SOLID SAWN WOOD FRAMING DESIGN IS BASED ON NO. 2 SPRUCE PINE FIR OR SYP #2 FOR JOISTS, RAFTERS, GIRDERS, BEAMS, STUDS, ETC. MINIMUM ALLOWABLE DESIGN PROPERTIES ARE AS FOLLOWS: = 1,400,000 PSI,  $F_c$  perp = 425 PSI,  $F_v$  = 135 PSI, SPECIFIC GRAVITY = 0.42 MIN  $F_b = 875$  PSI FOR 2X4, 2X6, 2X8.  $F_b = 800$  PSI FOR 2X10'S, 750 PSI FOR 2X12'S PART 11: ENGINEERED LUMBER

11.01 LVL OR PSL MINIMUM ALLOWABLE DESIGN PROPERTIES ARE AS FOLLOWS: = 1,900,000 PSI,  $F_b$  = 2600 PSI,  $F_v$  = 285 PSI,  $F_c$  perp = 750 PSI SL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS:

= 1.3 X 10E6 PSI,  $F_b$  = 1700 PSI,  $F_v$  = 400 PSI,  $F_c$  perp = 680 PSI 11.02 LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS

PART 12: PRESSURE TREATED LUMBER 2.01 LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER

SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A) PART 13: STEEL FLITCH PLATE BEAMS

TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER USING 1/2" Ø BOLTS SPACED AT 16" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 16" MAX FROM EACH END OF THE BEAM. TYP UNO PART 14: STUD SUPPORTS FOR BEAMS

4.01 STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:

-WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR <u>FULL WIDTH</u> ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS WIDE AS THE TRUE WIDTH OF THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UNO. FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON

-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED COLUMN TYP UNO.

4.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS: -WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL. THE BEAM SHALL BEAR <u>FULL WIDTH</u> ON THE SUPPORTING WALL INDICATED (LESS 1 1/2" TO ALLOW FOR A CONTINUOUS RIM JOIST WHERE APPLICABLE) AND SHALL BE SUPPORTED BY A GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS). FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLÚMN IS CENTERED ON THE BEAM

**ABBREVIATIONS** 

TJ TRIPLE JOIST

TYP TYPICAL TRPL TRIPLE

FND FOUNDATION

FTG FOOTING

HDG HOT DIPPED

HGR HANGER

GALVANIZED

LVL LAMINATED VENEER

PSL PARALLEL STRAND

PT PRESSURE TREATED

SP SPACE (OR SPACING)

SSP SINGLE STUD POCKET

LUMBER

NTS NOT TO SCALE

O.C. ON CENTER

QJ QUAD JOIST

SQ SQUARE

B. BOTH

BTWN BETWEEN

CONC CONCRETE

DIA DIAMETER

DBL DOUBLE

EQ EQUAL

EA EACH

FLG FLANGE

B.F. BOTH FNDS

CIP CAST IN PLACE

DJ DOUBLE JOIST

DSP DBL STUD POCKET

CS CONTINUOUS SHEATHING

-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN

14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD. 14.04 STUDS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS WITHIN

THE COLUMN NAILED TOGETHER WITH ONE ROW OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL RE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH

FLOOR LEVELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH OF THE STUD COLUMN

PART 15: NAILING OF MULTI PLY WOOD BEAMS

WITHIN THE CAVITY FORMED BY THE

SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS @ 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER. STAGGER ROWS 5" MIN.

D2 LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP

PART 16: WALL FRAMING AND BRACING

16.01 STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING R ROOF. NO INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITIES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL BE CONTINUOUS, TYP UNC MAX ALLOWABLE WALL HEIGHTS FOR EXTERIOR STUD WALLS, INCLUSIVE OF SOLE PLATE AND DBL TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 2X6 PURLINS AT 8' HEIGHT (AND AT 16' HEIGHT FOR TALL WALLS), TYP UNO: 2X4 @ 16" O.C.: 11'-1 1/2" 2X6 @ 16" O.C.: 17'-0" 2X4 @ 12" O.C.: 12'-1 1/2" 2X6 @ 12" O.C.: 18'-8" DBL 2X4 @ 16" O.C.: 13'-4" DBL 2X6 @ 16" O.C.: 21'-0'

16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY -BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO -WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION 602.10 OF THE 2018 NCRC. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10 OF THE 2018 NCRC HAS BEEN MET AND EXCEEDED -BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO PROVIDE CONTINUOUS PANEL UPLIET RESISTANCE AND COMPLIANCE WITH NCRBC R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS. -MAY SUBSTITUTE WSP FOR GB -SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO. PART 17: KING STUDS

OI KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS:

NUMBER OF KING STUDS MAX OPENING WIDTH 5'-0" 9'-0" 13'-0" 17'-0" 21'-0"

PART 18: SUBSTITUTIONS

18.01 MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS. UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

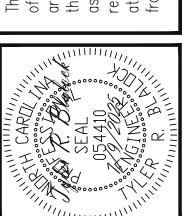
PART 19: OWNERSHIP OF STRUCTURAL DESIGN

19.01 THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETA

ALLOWABLE I-JOIST SUBSTITUTION NOTE: MAINTAIN JOIST DEPTH, DIRECTION, AND SPACING SPECIFIED ON

TRPL TRIPLE TSP TRIPLE STUD POCKET UNO UNLESS NOTED		MANUFACTURER	DEPTH	SERIES	SIMPSON FACE MOUNT HGR	SIMPSON TOP FLANGE HGR
	OTHERWISE EXTRA JOIST	BLUELINX BOISE CASCADE BOISE CASCADE INTERNATIONAL BEAMS	11.875" 11.875" 11.875" 11.875"	BLI 40 BCI 5000s BCI 6000s IB 400	IUS2.56/11.88 IUS2.06/11.88 IUS2.37/11.88 IUS2.56/11.88	ITS2.56/11.88 ITS2.06/11.88 ITS2.37/11.88 ITS2.56/11.88
		LP CORP NORDIC ROSEBURG WEYERHAEUSER WEYERHAEUSER	11.875" 11.875" 11.875" 11.875" 11.875"	LPI 20+ NI 40X RFPI 40s TJI 210 EEI-20	IUS2.56/11.88 IUS2.56/11.88 IUS2.56/11.88 IUS2.06/11.88 IUS2.37/11.88	ITS2.56/11.88 ITS2.56/11.88 ITS2.56/11.88 ITS2.06/11.88 ITS2.37/11.88
		BLUELINX BOISE CASCADE BOISE CASCADE LP CORP NORDIC ROSEBURG WEYERHAEUSER WEYERHAEUSER	14" 14" 14" 14" 14" 14" 14" 14"	BLI 40 BCI 5000s BCI 6000S LPI 20+ NI 40X RFPI 40s TJI 210 EEI-20	IUS2.56/14 IUS2.06/14 IUS2.37/14 IUS2.56/14 IUS2.56/14 IUS2.56/14 IUS2.06/14 IUS2.37/14	ITS2.56/14 ITS2.06/14 ITS2.37/14 ITS2.56/14 ITS2.56/14 ITS2.56/14 ITS2.06/14 ITS2.73/14
		BOISE CASCADE LP CORP LP CORP NORDIC ROSEBURG WEYERHAEUSER WEYERHAEUSER	16" 16" 16" 16" 16" 16"	BCI 60s LP 36 LP 42+ NI 70 RFPI 70 TJI 360 EEI-30	IUS2.37/16 IUS2.37/16 IUS2.56/16 IUS2.56/16 IUS2.37/16 IUS2.37/16 IUS2.37/16	ITS2.37/16 ITS2.37/16 ITS2.56/16 ITS2.56/16 ITS2.37/16 ITS2.37/16 ITS2.73/16
		i e				

JOISTS NOT LISTED IN THE ABOVE TABLE MAY BE USED PROVIDED THEY MEET OR EXCEED THE PROPERTIES OF THOSE LISTED. SUBSTITUTE USP BRAND HANGERS WITH EQUIVALENT VALUES AS DESIRED.



ENG: TRB/MEB

PLAN **BRIGHTON** 

DATE: 7/9/2025

PROIECT NO. 25-27-081

SHEET NO.

of /

