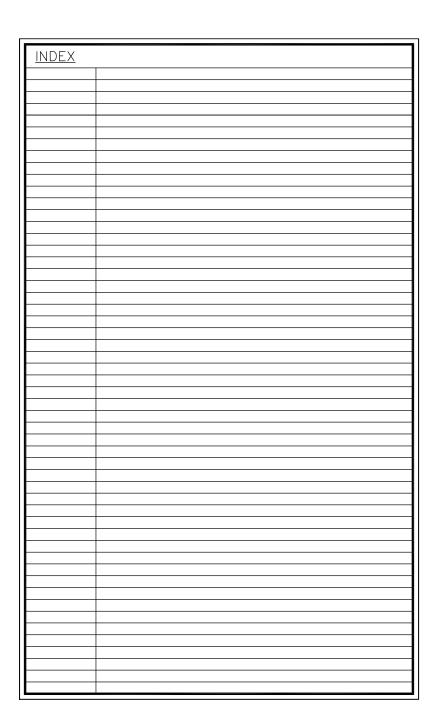
STONEHAVEN-RALE

RALEIGH- LOT 00.0076 BLAKE POND SF

(MODEL# 3656) ELEVATION 7 - GR





ELEVATION 7 ELEVATION 7 FIRST FLOOR GARAGE GARAGE FRONT PORCH — ELEVATION 7 SECOND FLOOR OPTIONS 3RD CAR GARAGE SCREENED PORCH BEDROOM 5 MORNING ROOM TOTAL TOTAL TOTAL COVERED / UNHEATED UNCOVERED UNC				
ELEVATION 7	ADEA CALCULATIONS			
ELEVATION 7	AREA CALCULATIONS		COVERED /	
FIRST FLOOR 1703 SF GARAGE 487 SF FRONT PORCH — ELEVATION 7 189 SF SECOND FLOOR 1918 SF OPTIONS 3RD CAR GARAGE 264 SF SCREENED PORCH 150 SF BEDROOM 5 26 SF MORNING ROOM 251 SF		HEATED	LINHEATED /	LINCOVERED
GARAGE			ONITEATED	ONCOVERED
## FRONT PORCH — ELEVATION 7 189 SF SECOND FLOOR 1918 SF OPTIONS 264 SF SCREENED PORCH 150 SF BEDROOM 5 26 SF MORNING ROOM 251 SF		1703 SF	407.05	
SECOND FLOOR				
OPTIONS 3RD CAR GARAGE SCREENED PORCH BEDROOM 5 MORNING ROOM 26 SF MORNING ROOM 251 SF	FRONT PORCH - ELEVATION /		189 SF	
OPTIONS 3RD CAR GARAGE SCREENED PORCH BEDROOM 5 MORNING ROOM 26 SF MORNING ROOM 251 SF				
3RD CAR GARAGE 264 SF SCREENED PORCH 150 SF BEDROOM 5 26 SF MORNING ROOM 251 SF	SECOND FLOOR	1918 SF		
3RD CAR GARAGE 264 SF SCREENED PORCH 150 SF BEDROOM 5 26 SF MORNING ROOM 251 SF				
SCREENED PORCH 150 SF BEDROOM 5 26 SF MORNING ROOM 251 SF	OPTIONS			
SCREENED PORCH 150 SF BEDROOM 5 26 SF MORNING ROOM 251 SF	3RD CAR GARAGE		264 SF	
BEDROOM 5				
MORNING ROOM 251 SF		26 SF	. 50 0.	
TOTAL 3898 SF 1090 SF	MORNING ROOM	231 31	+	
TOTAL 3898 SF 1090 SF				
TOTAL 3898 SF 1090 SF				
	TOTAL	3898 SF	1090 SF	
			+	
			1	
			+	
			+	1
				1

160 Bronze Leaf Drive

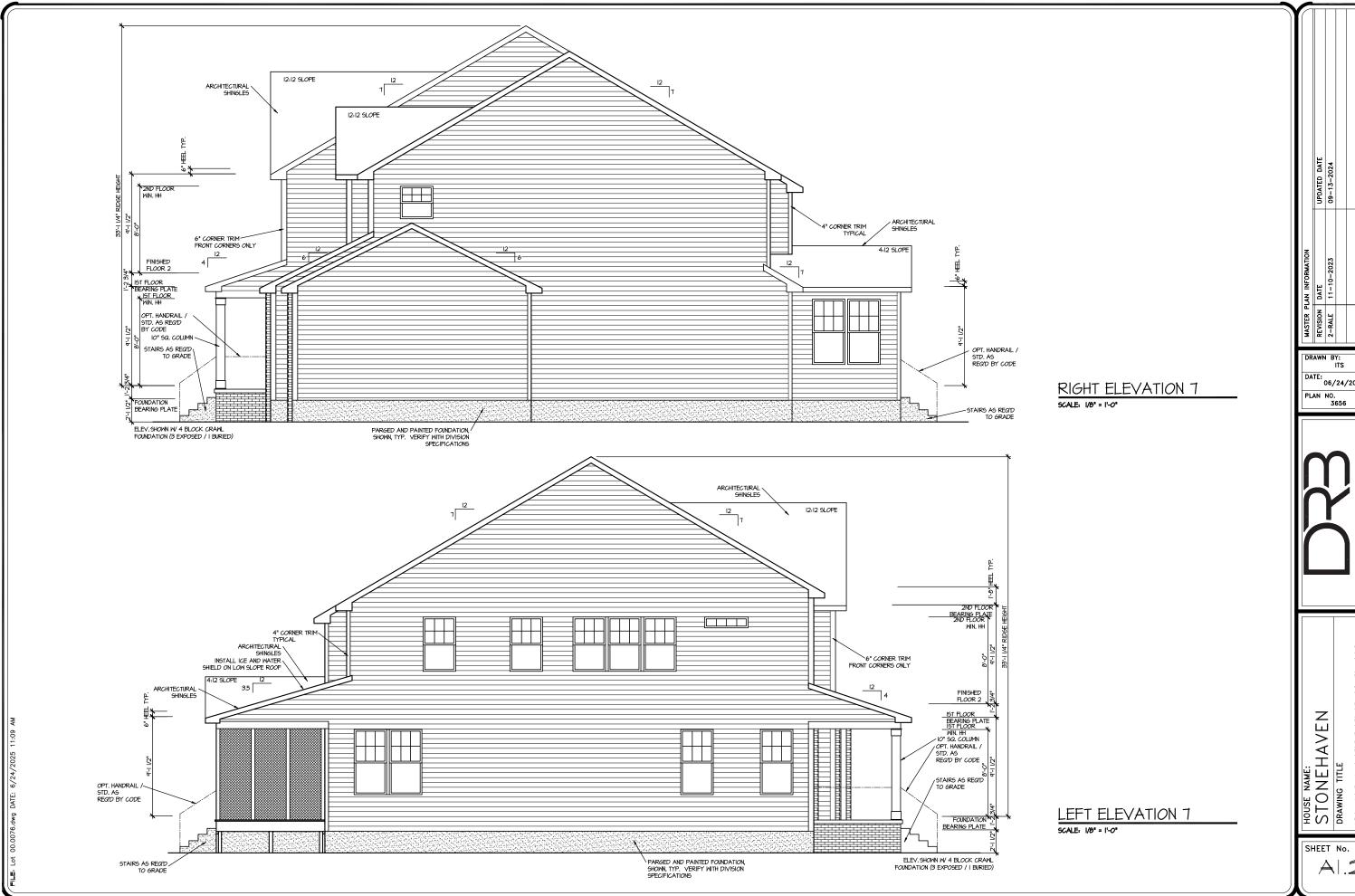
LOT	SPECIFIC	
1	LOT 00.0076	BLAKE POND SF
		STONEHAVEN R2 ELEVATION 7
2	ADDRESS	160 BRONZE LEAF DRIVE LILLINGTON, NC 27546
		· · · · · · · · · · · · · · · · · · ·



DATE: 06/24/2025



SHEET No.





DATE: 06/24/2025



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SHEET No.

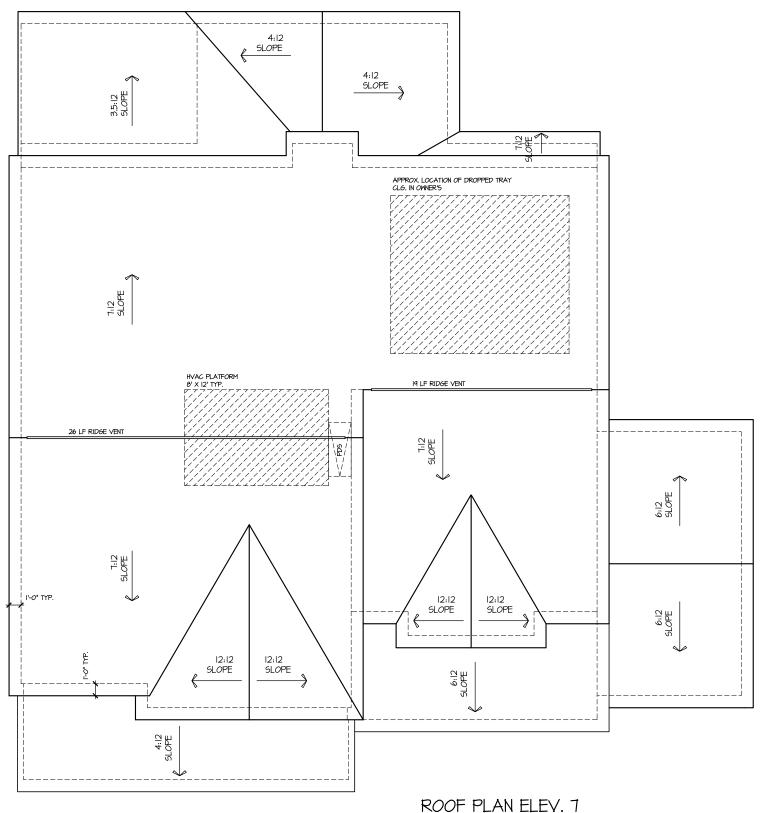
ROOF VENTILATION CALCULATIONS:
ROOF AREA = 2021 50, FT.
OVERAL BURSED, 100 50, FT.
1 TO 300 = 10.5 50, FT.
1 TO 300 = 10.5 50, FT.
50-60% IN TOP THIRD = 3.55 - 5.67 50, FT. (1 TO 300)
HET FREE AREA OF WINTED SOFTI = 5.15 60, IN / LINEAR FT.
NET FREE AREA OF RIDGE VENTI = 10 50, IN / LINEAR FT.

CMER VENTING, (BOTTOM 2/3 RDS).

THI LINEAR REET OF SOFFIT X 51 50, IN. = 3.13 50, FT.
LINEAR VENTING, (TOP 1/3 BD).

45 LINEAR PET OF RIDGE X 19 50, IN = 5.63 50, FT.
5.65 50, T. ENTERED 50, 50, 60, 60

TOTAL ROOF VENTILATION, 6.16 50, FT. > 1.09 50, FT. (ROTD).

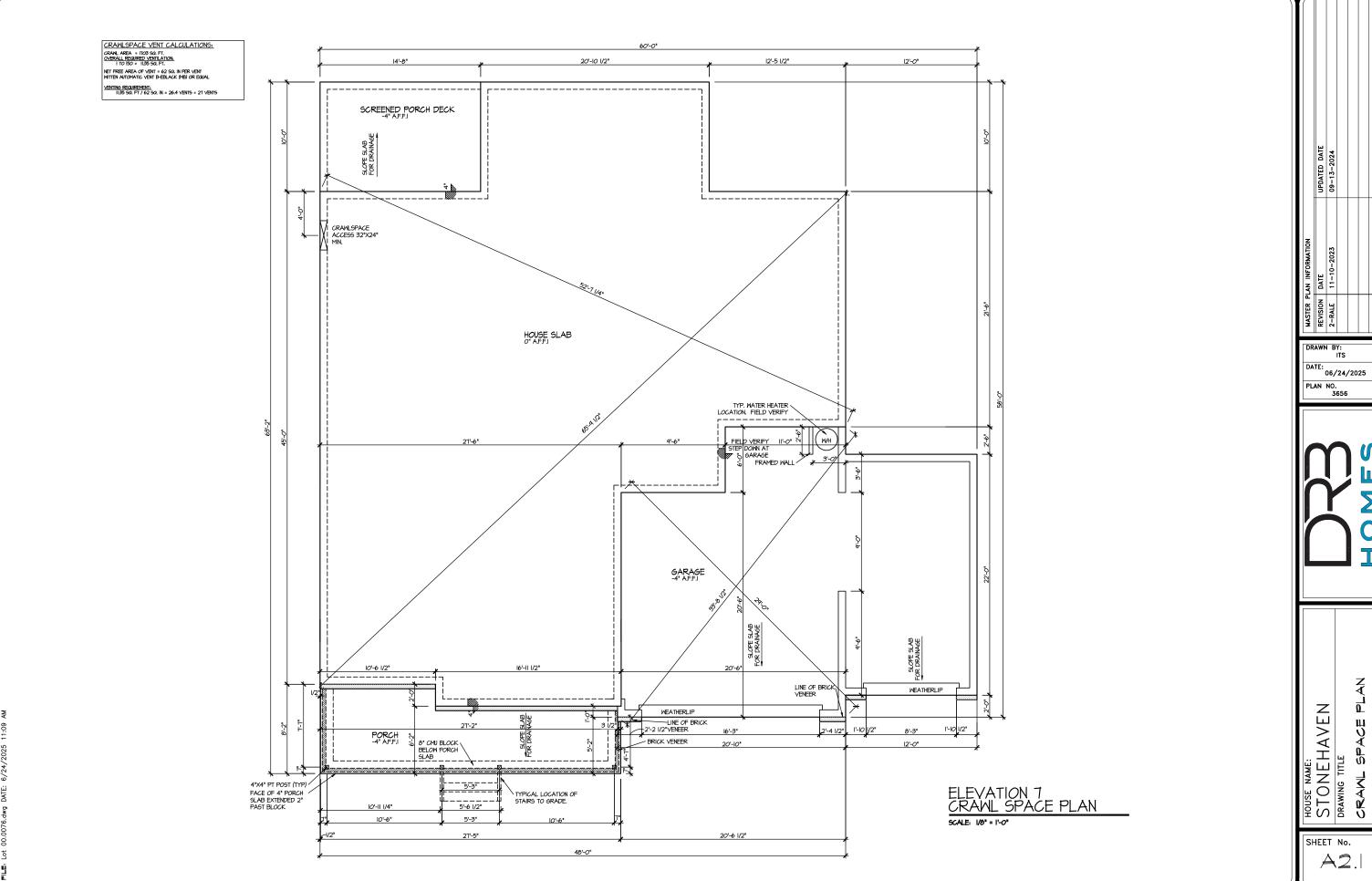


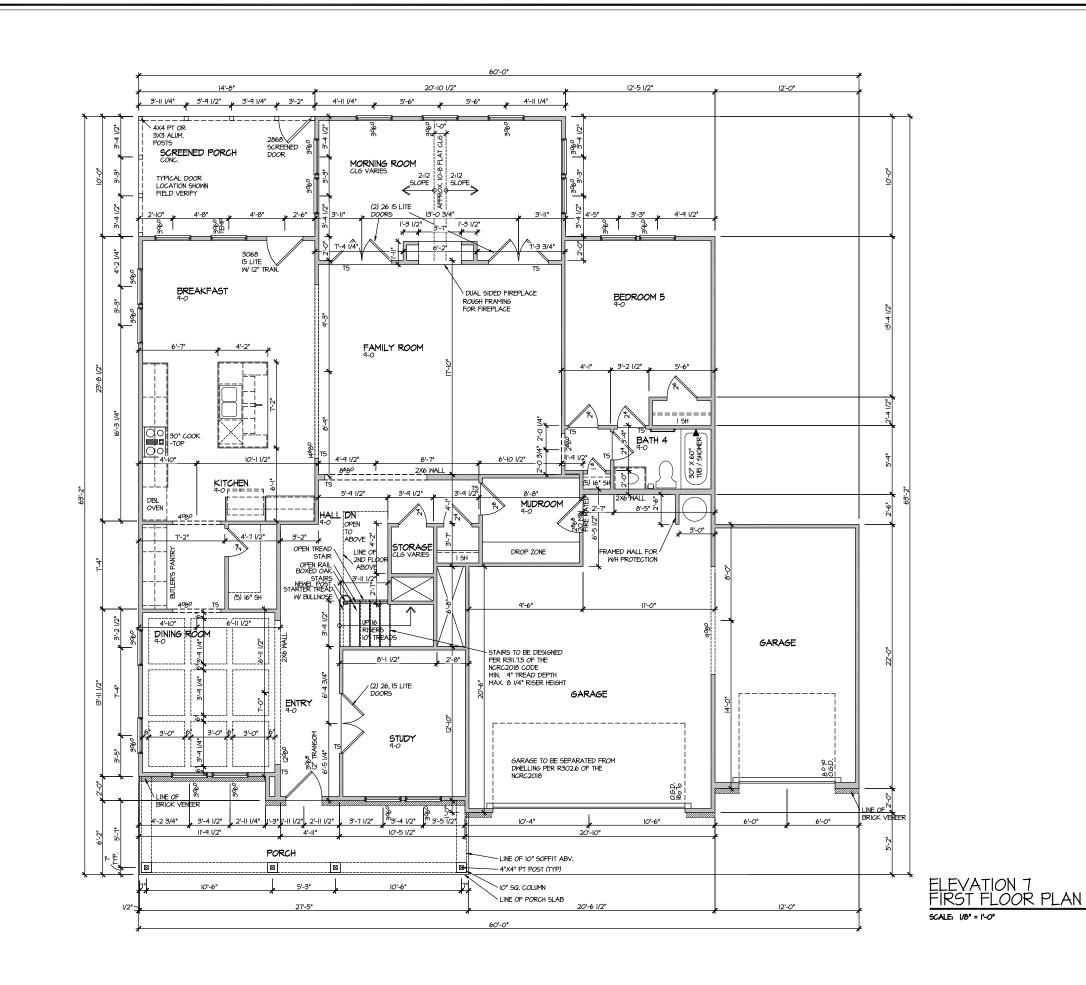
ROOF PLAN ELEV. 7

HOUSE NAME:
STONEHAVEN
DRAWING TITLE
ROOF PLAN SHEET No.

DRAWN BY:

DATE: 06/24/2025 PLAN NO. 3656



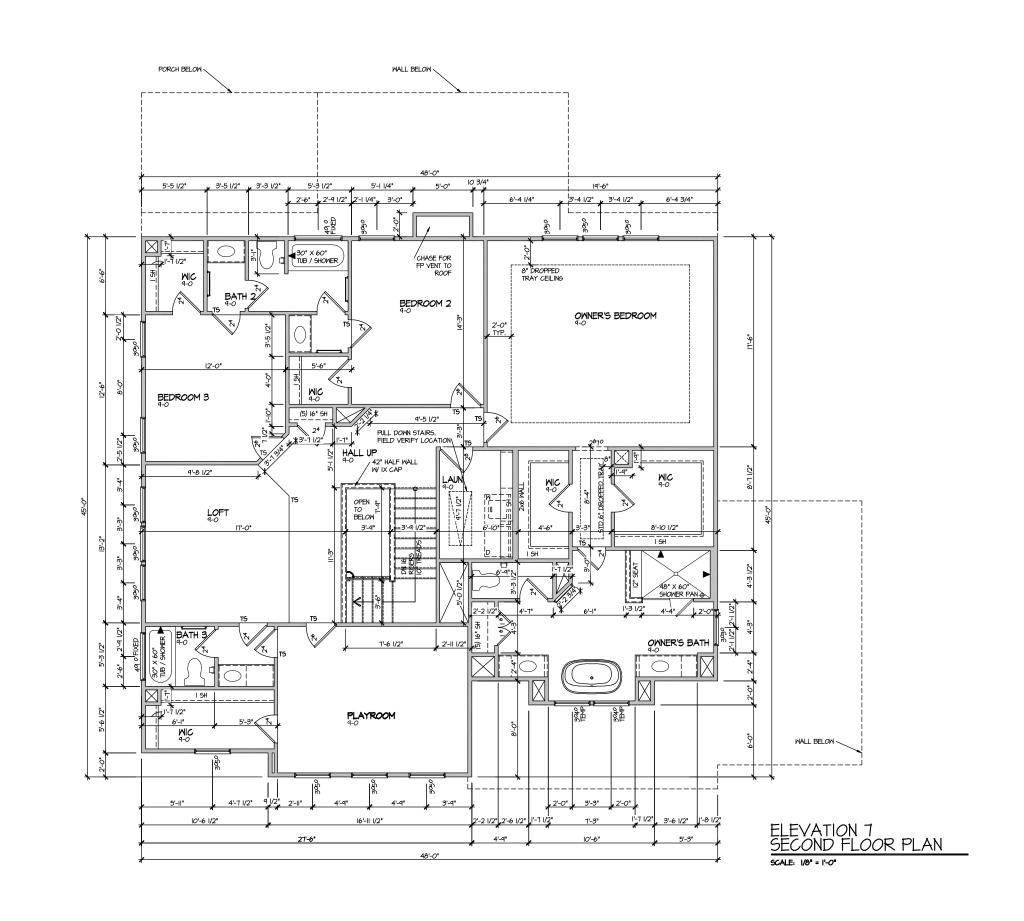


DRAWN BY: DATE: 06/24/2025 PLAN NO. 3656



HOUSE NAME:
STONEHAVEN
DRAWING TITLE

SHEET No. A3.



DRAWN BY:

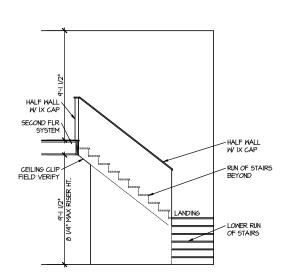
DATE: 06/24/2025 PLAN NO. 3656



HOUSE NAME:
STONEHAVEN
DRAWING TITLE
SECOND FLOOR P

SHEET No.

A3.2



STAIR SECTION SCALE: 1/8" = 1'-0"

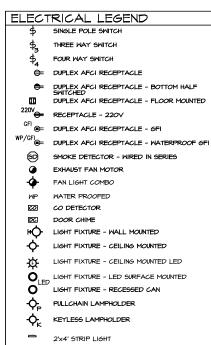
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DATE: 06/24/2025 PLAN NO. 3656

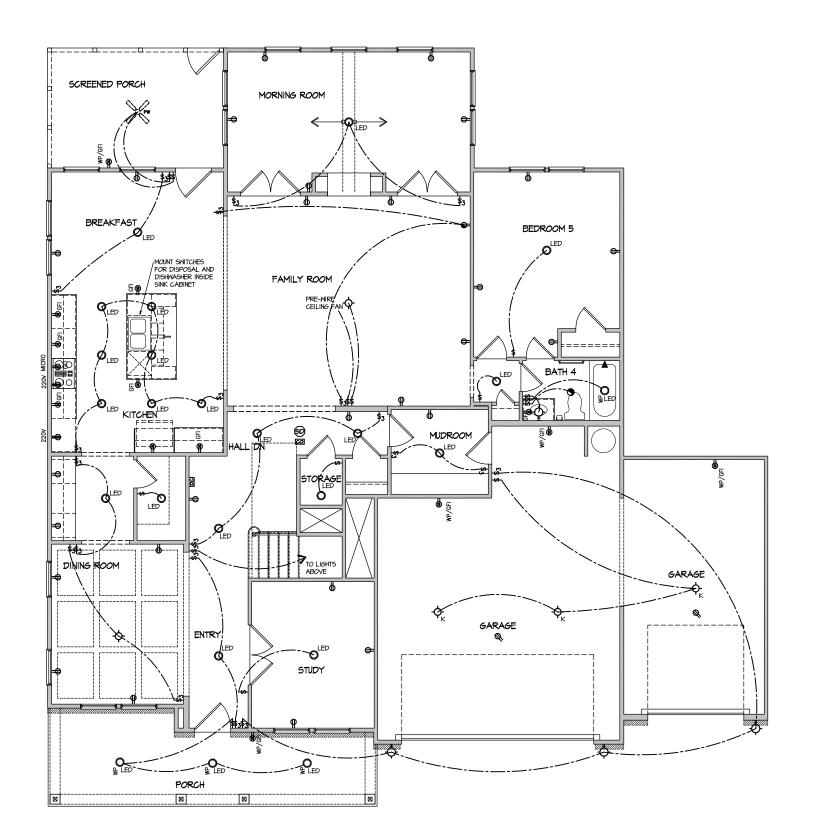


HOUSE NAME:
STONEHAVEN
DRAWING TITLE
BUILDING SECTION

SHEET No. A4.1



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



DRAWN BY:

DATE: 06/24/2025 PLAN NO. 3656

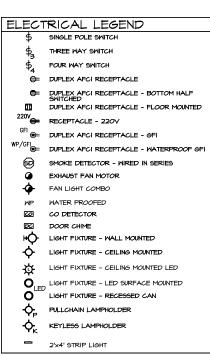


HOUSE NAME:
STONEHAVEN
DRAWING TITLE
FIRST FLOOR ELE

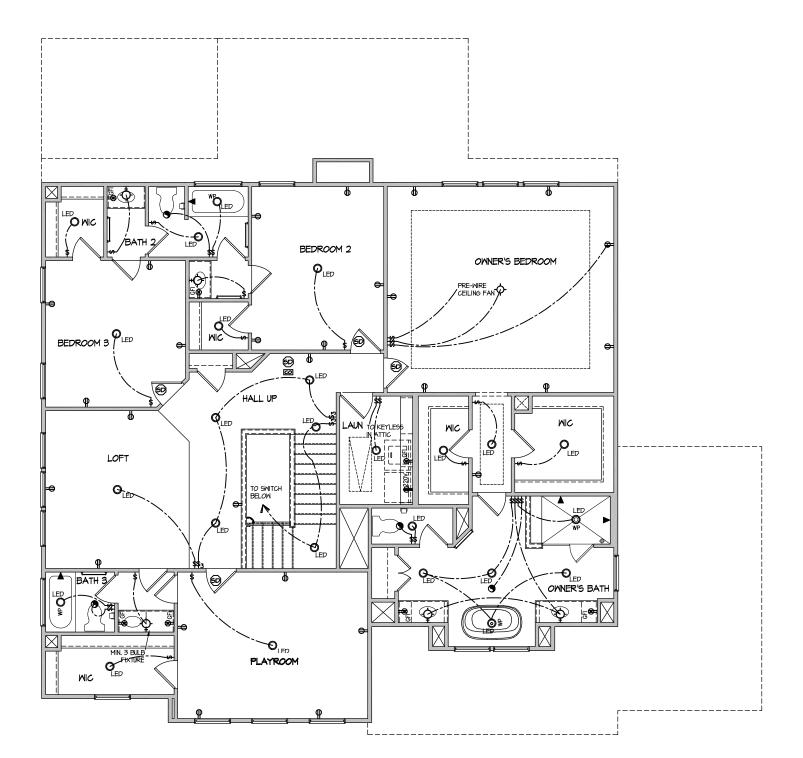
ELECTRICAL PLAN FIRST FLOOR - ELEV. 7

SCALE: 1/8" = 1'-0"

SHEET No.



MOTE, ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.

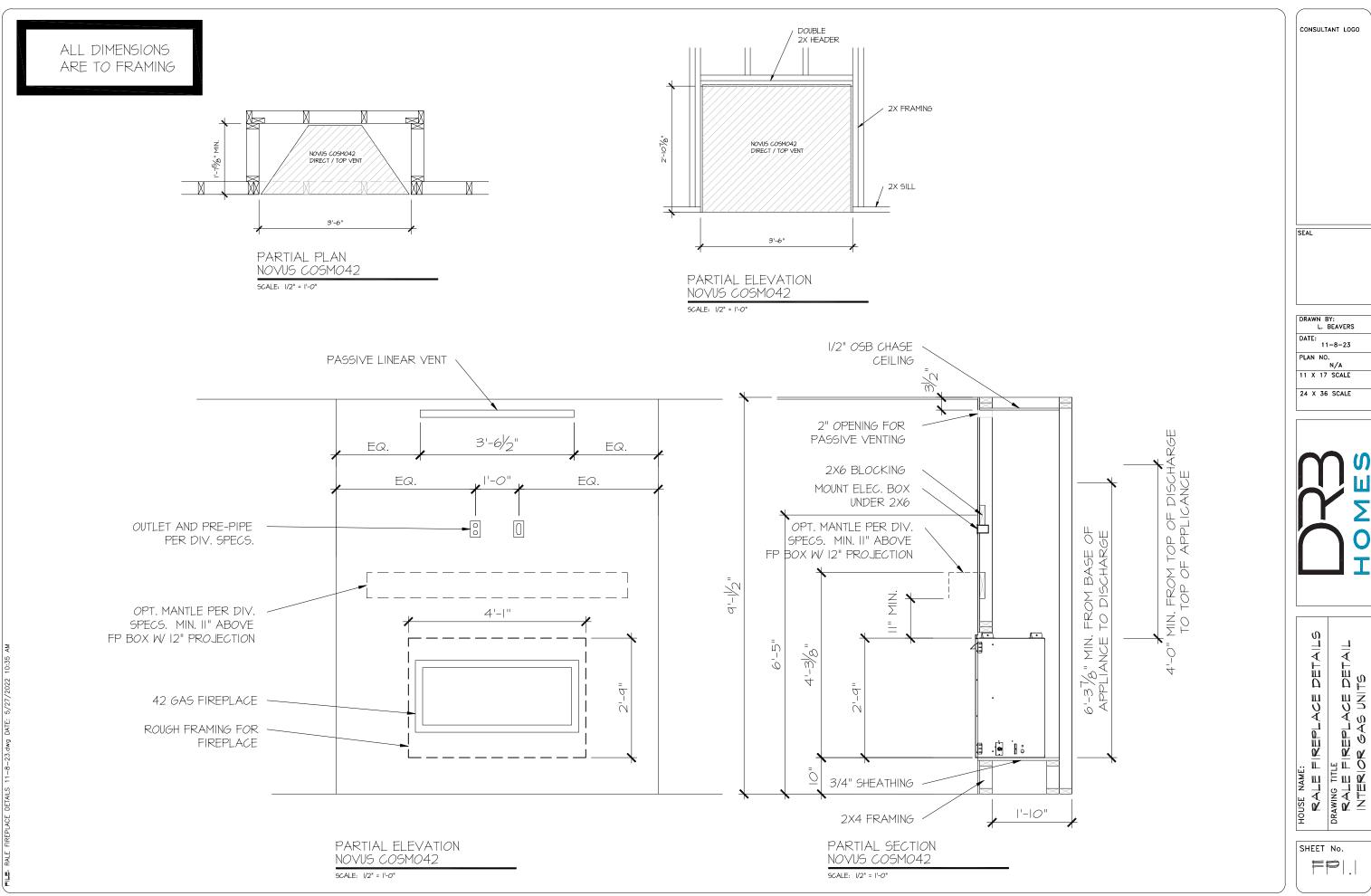


ELECTRICAL PLAN SECOND FLOOR - ELEV. 7 SCALE: 1/6" = 1'-0" HOUSE NAME:
STONEHAVEN
DRAWING TITLE
SECOND FLOOR FLECTRICAL
SECOND FLOOR FLECTRICAL

SHEET No.

DRAWN BY:

FILE: Lot 00.0076.dwg DATE: 6/24/2025 11:09



CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 11-8-23 PLAN NO. 11 X 17 SCALE



SHEET No.

DESCRIPTION OF BLDG. ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS 🛭 4" O.C.	(3) NAILS 💇 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS • 8" O.C.	TOENAILS @ 6" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS ⊘ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" o.c.	NAILS @ 16" O.C.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP ② CORNERS \$ INTERSECTING WALLS	(2) NAILS	(2) NAILS

* 2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE * ARE SHOWN)

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENT IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS IOTED OTHERWISE ON PLAN

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO
- FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

ROOF DEAD = 7 PSF T.C., 10 PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION, ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP. U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
 I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (I)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15). ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- 'LSL' Fb=2325 psi; Fv=3i0 psi; E=1.55xi0^6 psi
- 'LVL' Fb=2600 psi; Fv=265 psi; E=2.0x10^6 psi 'PSL' - FB=2400 PSI; FV=240 PSI; E=2.0XIO^6 PSI
- M+K SHALL BE FILLY INDEMNIFIED FOR ANY AND ALL ISSUES OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALL ATION.
- FOR 2 \$ 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x31/5" SIMPSON SDS SCREMS (OR 3½" TRUSSLOK SCREMS) @ 16" O/C. USE A MINIMUM OF 3 ROMS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION, LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM FDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF $\frac{1}{4}$ "x6" SIMPSON SDS SCREMS (OR 6 $\frac{3}{4}$ " TRUSSLOK SCREMS) • 16" O/C. USE A MINIMUM OF 4 ROMS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE. A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND/BEARING. BLOCKING TO MATCH POST ABOVE FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH
- P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MEK FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I 1/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131" NAILS @ 6"04. @ PANEL EDGES & @ 12"04. FIELD. - 2 🖣 × 0.120" NAILS • 4" O.C. • PANEL EDGES € • 8" O.C. FIELD.
- 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN, SCREMS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO:
 - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS w/ (3) 3"x0.131" NAILS • 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" × 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12" O.C. FIELD. - w/ 2 3 × 0.120" NAILS • 4"o.c. • PANEL EDGES € • 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"0.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.

VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size
3'-0"	20 FT. MAX	L3"x3"x/4"
	3 FT, MAX	L3"x3"x/4"
6'-0"	I2 FT. MAX	L4"x3"x/4"
	20 FT. MAX	L5"x31½"x7%"
ð'-O"	3 FT, MAX	L4"x4"x¼" *
	I2 FT. MAX	L5"x3½"x¾"
	I6 FT. MAX	L6"x3%"x3%"
4'-6"	I2 FT. MAX	L6"x3½"x%;"
16'-0"	2 FT. MAX	L7"x4"x½" **
IO-0	3 FT, MAX	L8"x4"x½" **

SHALL SUPPORT 2 3/8" - 3 1/2" VENEER W/ 40 paf MAXIMUM WEIGHT < 16' SHALL HAVE 4" MIN. BEARING

- = 16' SHALL HAVE 8" MIN, BEARING (16' SHALL NOT BE FASTENED BACK TO HEADER.
- 16' SHALL BE FASTENED BACK TO WOOD HEADER IN WALL @48"O.
- W/5" DIA. x 3 1/5" LONG LAG SCRENG IN 2" LONG VERTICALLY SLOTTED HOLES. MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
 WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE
- HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 1/2" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT
- FINE-THIS.

 SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT BICOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL FASTENED BACK TO BEAM, FASTENERS SHALL MAINTAIN A 25' (MINIMAN) CLEAR DISTANCE ROYAL BOTTOM OF BEAM.
- FOR QUEEN VENEER USE L4x3x FOR 3%" VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RO

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301211) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NOSEC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST

THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

EXT. WALL SHEATHING SPECIFICATION

- 1/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ・ALT. STAPLE CONNECTION SPEC: 1 名" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD.

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 3/8" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 3/4" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C IN FIELD, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN. IT WILL BE SPECIFICALLY NOTED ON PLAN.
- * DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- <u>PRE-MANUFACTURED PANELIZED WALLS:</u> FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWAL OR 3" O.C. OSB SHEARWALL.

► INDICATES HOLDOWN BELOW

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED, BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING
- 1/2" DIA. ANCHOR BOLTS 6'-0" O.C, 7" MIN. EMBEDMENT
- (CONC), 15" MIN, EMBEDMENT (CMU)
- SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC) SIMPSON MAB23 ANCHOR STRAPS 2'-8" O.C. (CMU)
- (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER. • CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN
- THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: FOUNDATION WALLS 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE
- 60,000 psi BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
- TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (91/3" FOR 10" THICK WALL).
- * BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC, ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED PRIOR TO BACKELLING BY ADEQUATE TEMPORARY BRACING OR INSTALL IST FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.
 - FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 w/ (2)2x6 JACK STUDS, U.N.O
- LARGER OPENINGS SHALL BE PER PLAN. • ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS
- THAN 5% OR MORE THAN 7% AIR ENTRAINMENT ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- * PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP.
 - JOINTS SHALL BE LOCATED 10'-0" O.C. (RECOMMENDED) OR 15'-O" OC (MAXIMUM)
- JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I.I RATIO), WITH A MAXIMUM OF I.I.5 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fim=1500 psi). MORTAR SHALL BE ASTM C270, TYPE 5. CMU DESIGN PER ACI 530 \$ 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x6 (MIN.) x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE FASTENED PER ANCHORAGE SPECIFICATIONS NOTED ABOVE. TOP 2 COURSES (MIN.) OF PIER TO BE GROUTED SOLID (8 COURSE MAX, PIER HEIGHT).
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE. TOP 2 COURSES (MIN.) OF WALL TO BE GROUTED SOLID (8 COURSE MAX. WALL HEIGHT)
- * DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR BE DETERMINED BY PEST CONTROL COMPANY.

HOLD-DOWN SCHEDULE

5YMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC66B3 ALTERNATE
▶ нр-з	SIMPSON STHDI4/STHDI4RJ

* UTILIZE THE 95TB24 ANCHOR BOLT ● ALL MONOSLAB & INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED.

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN
THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 15" (FOR 7/8" DIA.) MIN. EMBEDMENT INTO CONCRETE, INSTALL PER MANUF, INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D.

DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE

NOFESSIO, SEPH T. R

CAR

O'



^{2al:} 7/11/25

l&K project numbe 126-2306

JTR rawn by: ssue date: 07-11-2

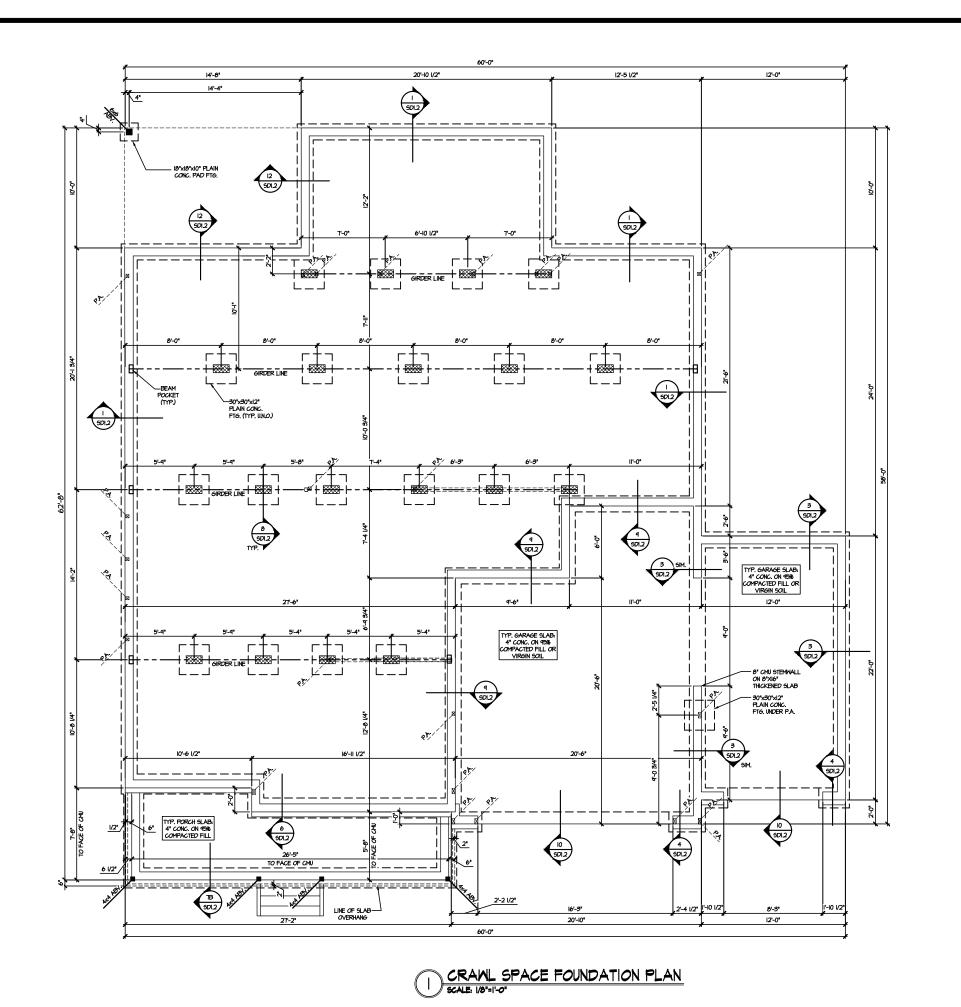
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POND COMMUNI STONEHAVEN 7 . NC 0 ЩП

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BLA LOT RAL



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O Symple In SERN S NUP
Sural Engineering, Inc.

MULHERNHKUL RESIDENTIAL STRUCTURAL ENGINEERI SUBMISSIER & BAILER (* FARING PATRICE)

M&K project number: 1 26-2306 1

project mgr: JTR
drawn by: GTK
issue date: O7-1 1-25

REVISIONS:

date: initial

M N

LEGEND

- INTERIOR BEARING WALL
- ==== BEARING WALL ABOVE
- ---- BEAM / HEADER
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

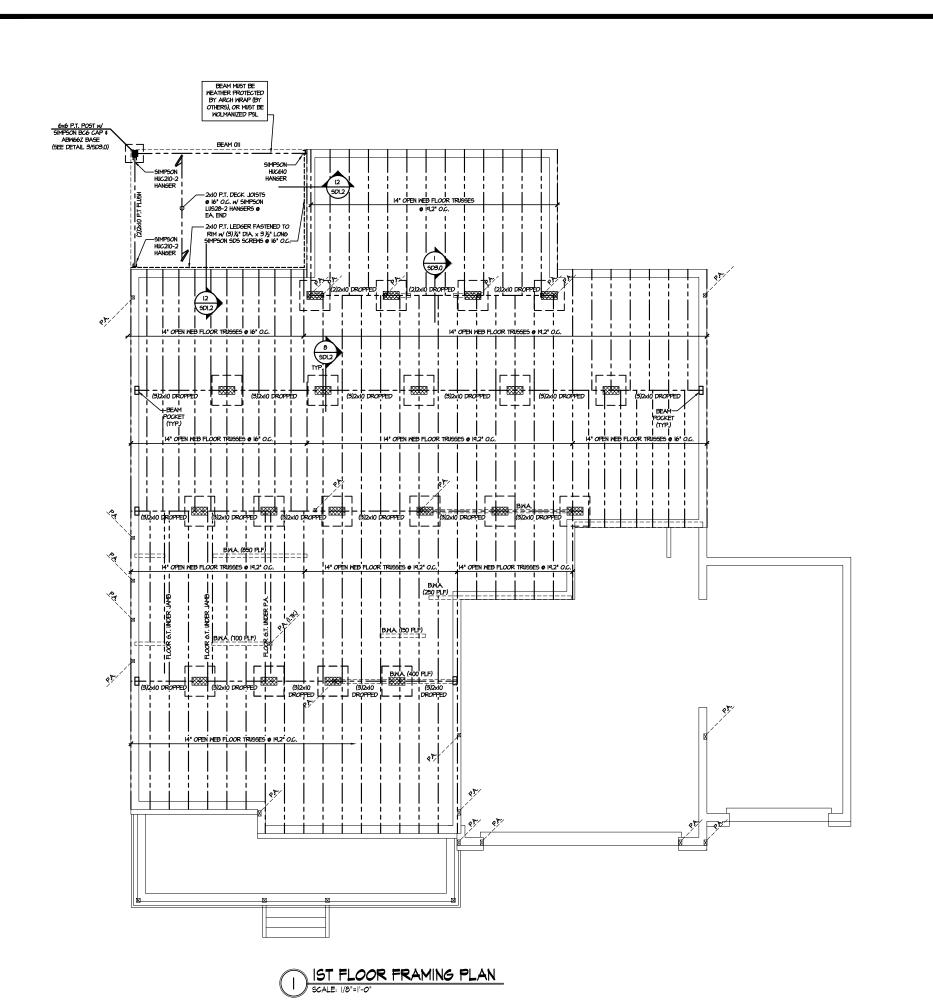
REFER TO SO.O FOR
TYPICAL STRUCTURAL NOTES

\$ SCHEDULES

SD2.IJ/SD2.IT REFERS TO
SD2.IJA/SD2.ITA FOR LYL/PSL/LSL
BEAMS OR SD2.IJB/SD2.ITB FOR
FLITCH BEAMS OR
SD2.IJC/SD2.ITC FOR STEEL
BEAMS

BLAKE POND COMMUNIT Lot 76 - Stonehaven 7 Raleigh, nc

OUNDATION PLANS



LEGEND

INTERIOR BEARING WALL

● □===□ BEARING WALL ABOVE

• --- BEAM / HEADER

• = = INDICATES SHEAR WALL & EXTENT

• EXTENT OF OVERFRAMING

JL METAL HANGER

* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

Engineered beam material schedule					
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)134"x914" - F	3½"x9¼" - F	N/A	(2)2xl0 + (I) ¼"xq¼" STEEL FLITCH PLATES - F	W8xIO − F
002	(2)194"x18" - FB	3½"xl8" - FB	N/A	(2)2xi2 + (I) I"xII¼" STEEL FLITCH PLATES - F	WI2xI9 - F
002A	(3)13/4"x16" - FB	5¼"xl6" - FB	N/A	(3)2xi2 + (2) 片"xi以" STEEL FLITCH PLATES - F	WI2xI9 - F
003	(2)134"×14" - F	3½"x 4" - F	(2)1¾"x14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F	WI2xI4 - F
004	(2)134"×14" - F	3½"x14" - F	(3)13/4"×14" - F	(2)2xi2 + (i) %"xil/4" STEEL FLITCH PLATES - F	WI2xI4 - F
005	(4)1¾"×18" - D	7"x 8" - D	N/A	(4)2xi2 + (3) %"xilk" STEEL FLITCH PLATES - D	WI2x30 - D
006	(2)1¾"×11%" - H	3½"xII%" - H	(3)134"×1176" - H	N/A	N/A
001	(2)13/4"×14" - H	3½"x14" - H	(3)134"x14" - H	(2)2xi2 + (I) %"xi以" STEEL FLITCH PLATES - H	N/A
000	(3)13/4"×14" - F	5¼"×14" - F	N/A	(3)2xi0 + (2) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F
004	(3)1¾"×18" - H	5¼"×18" - H	N/A	(3)2xi2 + (2) ½"xii以" 9TEEL FLITCH PLATES - H	N/A
010	(2)194"×944" - H	3½"x9¼" - H	(2)134"×944" - H	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - H	MBxIO - H
OII	(3)1¾"x4¼" - F	54"x94" - F	(3)13/4"×9/4" - F	(3)2x10 + (2) ¼"x11¼" STEEL FLITCH PLATES - F	M8x10 - F
012	(4)1¾"×11½" - D	T'xii%" - D	N/A	(4)2xl2 + (3) 以"xl以" STEEL FLITCH PLATES - D	WI2xI4 - D
013	(3)13/4"x16" - FT	5¼"xl6" - FT	N/A	(3)2xi2 + (2) ½"xi以" STEEL FLITCH PLATES - F	WI2xI9 - F
014	(2)134"×14" - F	3½"x 4" - F	(3)13/4"×14" - F	(2)2xl2 + (I) %"xl以" STEEL FLITCH PLATES - F	WI2xI4 - F

- BEAM NOTATION:

 "F" INDICATES FLUSH BEAM
 "FT" INDICATES FLUSH TOP BEAM
 "FT" INDICATES FLUSH BOTTOM BEAM
 "P" INDICATES DEVELOPED OPENING HEADER
 "P" INDICATES DROPPED OPENING HEADER
 REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
 REFER TO DETAIL E/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
 ROFER TUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN
 PLATES IN SUCCESSION W (2) 3*X01.20" NAILS © 8" O.C.
- FIGHTLES IN SUCCESSION W (2) 3 AVIJLO MAILS & OU.

 POR FLUSH BOTTOM BEAM PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D, FASTEN PLATES IN SUCCESSION W (2) 3 AVIJLO" NAILS & 8" O.C.

MULHERN+KUL RESIDENTIAL STREETURAL ENGINEERI

eal: 7/11/25

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SEPH T. R

1&K project number:

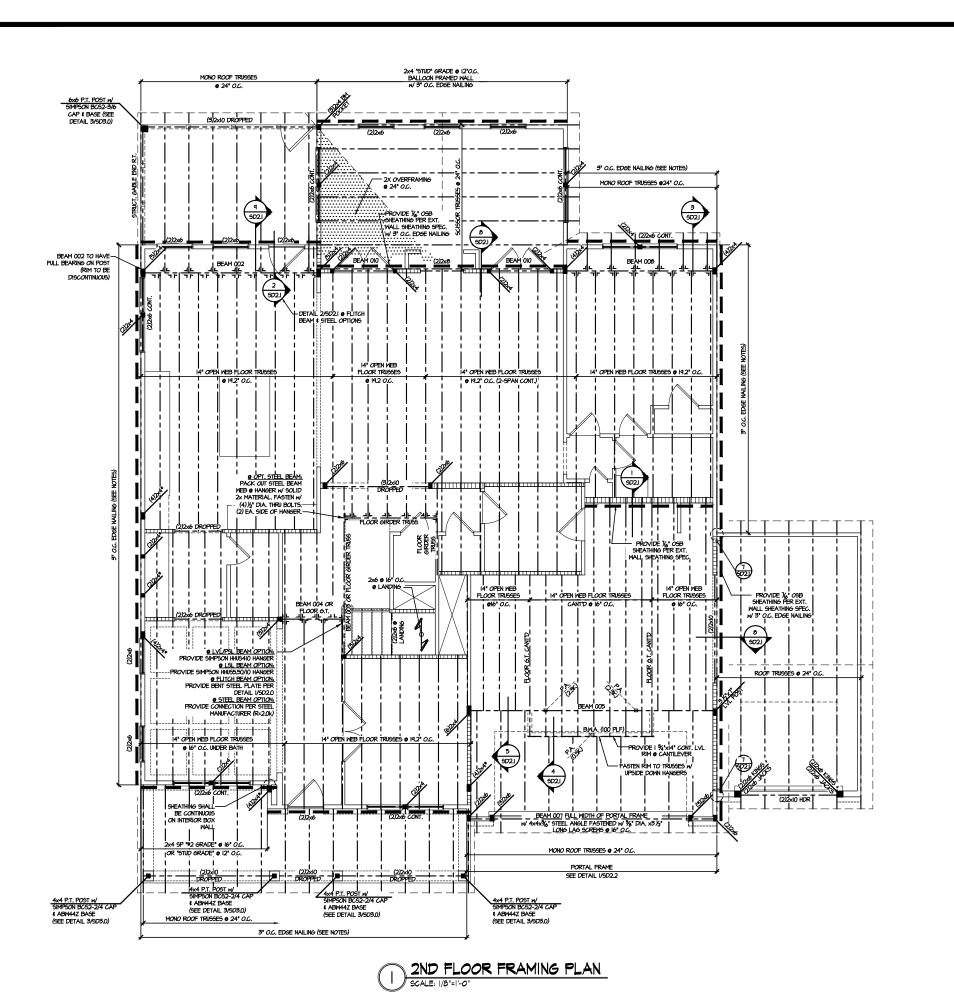
126-23061 **JTR**

frawn by: **GTK** issue date: 07-11-25

REVISIONS initial:

OR

BLAKE POND COMMUNITY Lot 76 - Stonehaven 7 Raleigh, nc



SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LEGEND

- □==== BEARING WALL ABOVE
- --- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT

- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

ENGINEERED BEAM MATERIAL SCHEDULE						
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION	
001	(2)13/4"x91/4" - F	3½"×9¼" - F	N/A	(2)2xi0 + (i) ¼"xq¼" STEEL FLITCH PLATES - F	₩8×10 - F	
002	(2)134"x18" - FB	3½"×18" - FB	N/A	(2)2xl2 + (1) I"xll¼" STEEL FLITCH PLATES - F	WI2xI9 - F	
002A	(3)13/4"x16" - FB	5¼"x16" - FB	N/A	(3)2x12 + (2)½"x1½" STEEL FLITCH PLATES - F	WI2xI9 - F	
003	(2)13/4"×14" - F	3½"x 4" - F	(2)1 ³ / ₄ "x14" - F	(2)2xi2 + (i) ¼"xil¼" Steel Flitch Plates - F	WI2xl4 - F	
004	(2)19/4"x14" - F	3½"×14" - F	(3)19/4"×14" - F	(2)2xi2 + (i) %"xi以" STEEL FLITCH PLATES - F	WI2xI4 - F	
005	(4)13/4"x18" - D	7"xl8" - D	N/A	(4)2x12 + (3) %"xII¼" STEEL FLITCH PLATES - D	WI2x30 - D	
006	(2)134"x1136" - H	3½"xII%" - H	(3)1¾"x11%" - H	N/A	N/A	
001	(2)19/4"×14" - H	3½"×14" - H	(3)1¾"×14" - H	(2)2xl2 + (l) %"xl以" STEEL FLITCH PLATES - H	N/A	
000	(3)1¾"x14" - F	5¼"xl4" - F	NA	(3)2xi0 + (2) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F	
P00	(3)13/4"x18" - H	514"×18" - H	N/A	(3)2xi2 + (2)½"xi以" STEEL FLITCH PLATES - H	N/A	
010	(2)134"×94" - H	3½"x9¼" - H	(2)134"×94" - H	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - H	MBxIO - H	
OII	(3)134"×94" - F	5¼"x4¼" - F	(3)1¾"×9¼" - F	(3)2x10 + (2) ¼"x1¼" STEEL FLITCH PLATES - F	M8xI0 - F	
012	(4)19/4"x11%" - D	7"xII%" - D	N/A	(4)2xi2 + (3)以"xi以" STEEL FLITCH PLATES - D	WI2xI4 - D	
013	(5)134"x16" - FT	5¼"xl6" - FT	N/A	(3)2xi2 + (2)½"xi以" STEEL FLITCH PLATES - F	WI2xI9 - F	
014	(2)1¾"x14" - F	3½"x 4" - F	(3)1¾"x14" - F	(2)2xi2 + (i) 3/2xil/4" STEEL FLITCH PLATES - F	WI2xI4 - F	

- BEAM NOTATION: "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM
 "FB" INDICATES FLUSH BOTTOM BEAM
 "D" INDICATES DROPPED BEAM

PLATES IN SUCCESSION W/ (2) 3"x0.120" NAILS @ 8" O.C.

- "IN" INDICATES DROPPED OPENING HEADER
 REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
 REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
- FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN PLATES IN SUCCESSION W (2) 3"XXXIDE XX STACKED PLATES ATOP BEAM AS REQUD. FASTEN

- INTERIOR BEARING WALL

- EXTENT OF OVERFRAMING
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

eal: 7/11/25

MULHERN+KUL Residential Structural Engineeri

1&K project number:

frawn by:

FVISIONS

126-2306

JTR

GTK issue date: 07-11-2

initial:

CAR

SEPH T. R

OOR

BLAKE POND COMMUNIT Lot 76 - Stonehaven 7 raleigh, nc

S3.0

STEEL OPTION

W6xIO - F

WI2xI9 - F

WI2xI9 - F

WI2xI4 - F

WI2xI4 - F

N/A

WI2xI4 - F

NΑ

W8xIO-F

WI2xI4 - D

WI2xI9 - F

WI2xI4 - F

MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEERI

1&K project number: 126-23061

roject mgr: **JTR** rawn by: ssue date: 07-11-25

FVISIONS initial:

RAMING

BLAKE POND COMMUNITY Lot 76 - Stonehaven 7 Raleigh, nc OOF

ROOF TRUSSES @ 24" O.C. - ROOF TRUSSES @ 24" O.C. - EXTEND EXTERIOR SHEATHING DOWN TO 2ND FLR SOLE PLATE PRIOR TO THE INSTALLATION OF LOW ROOF (TYP.) 3" O.C. EDGE NAILING (SEE NOTES) @ 24" O.C.



REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL D/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W/ (2) 3"X0,120" NAILS @ 8" O.C. FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D, FASTEN PLATES IN SUCCESSION w/ (2) 3"XO.120" NAILS @ 8" O.C.

ENGINEERED BEAM MATERIAL SCHEDULE

LSL OPTION

N/A

NΑ

(2)13/4"x14" - F

(3)13/4"×14" - F

(3)13/4"×113/6" - H

(3)13/4"x14" - H

N/A

(2)134"×94" - H

(3)13/4"x91/4" - F

N/A

NΑ

(3)134"x14" - F

FLITCH OPTION

(2)2x10 + (1) ¼"x4¼" 5TEEL FLITCH PLATES - F

(2)2xl2 + (1) I"xlK4" STEEL FLITCH PLATES - F

(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F

(2)2xl2 + (1) %"xlk" Steel Flitch Plates - F

(4)2xi2 + (3) %"xii¼" STEEL FLITCH PLATES - D

NΑ (2)2xl2 + (l) %"xllk" STEEL FLITCH PLATES - H

(3)2x12 + (2) ½"x11¼" STEEL FLITCH PLATES - H

(2)2xi0 + (1) ¼"x4¼" STEEL FLITCH PLATES - H

(3)2x10 + (2) ¼"x1¼" STEEL FLITCH PLATES - F

(4)2xi2 + (3) 从"xii以" STEEL FLITCH PLATES - D

(3)2xl2 + (2) ½"xll¼" STEEL FLITCH PLATES - F

(2)2xl2 + (I) %"xlK" STEEL FLITCH PLATES - F

LVL OPTION

(2)13/4"×9 1/4" - F

(2)13/4"×18" - FB

(2)13/4"×14" - F

(2)15/4"×14" - F

(4)13/4"x18" - D

(2)134"×1136" - H

(2)13/4"×14" - H

(3)13/4"×14" - F

(3)1¾"x18" - H

(2)13/4"×9/4" - H

(3)134"×944" - F

(4)13/4"×113/4" - D

(3)13/4"x16" - FT

(2)134"×14" - F

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM - "FB" INDICATES FLUSH BOTTOM BEAM - "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER

001

002

002A

003

004

006

001

008

010

OII

012

014

PSL OPTION

3½"x9¼" - F

3½"x18" - FB

5¼"x16" - FB

3½"xl4" - F

3%"xl4" - F

3½"xII%" - H

3½"x14" - H

5¼"xl4" - F

5¼"x18" - H

3½"×9¼" - H

5¼"x9¼" - F

7"x11%" - D

5¼"x16" - FT

3½"xl4" - F

LEGEND

INTERIOR BEARING WALL

● □===□ BEARING WALL ABOVE

BEAM / HEADER

• = = INDICATES SHEAR WALL & EXTENT

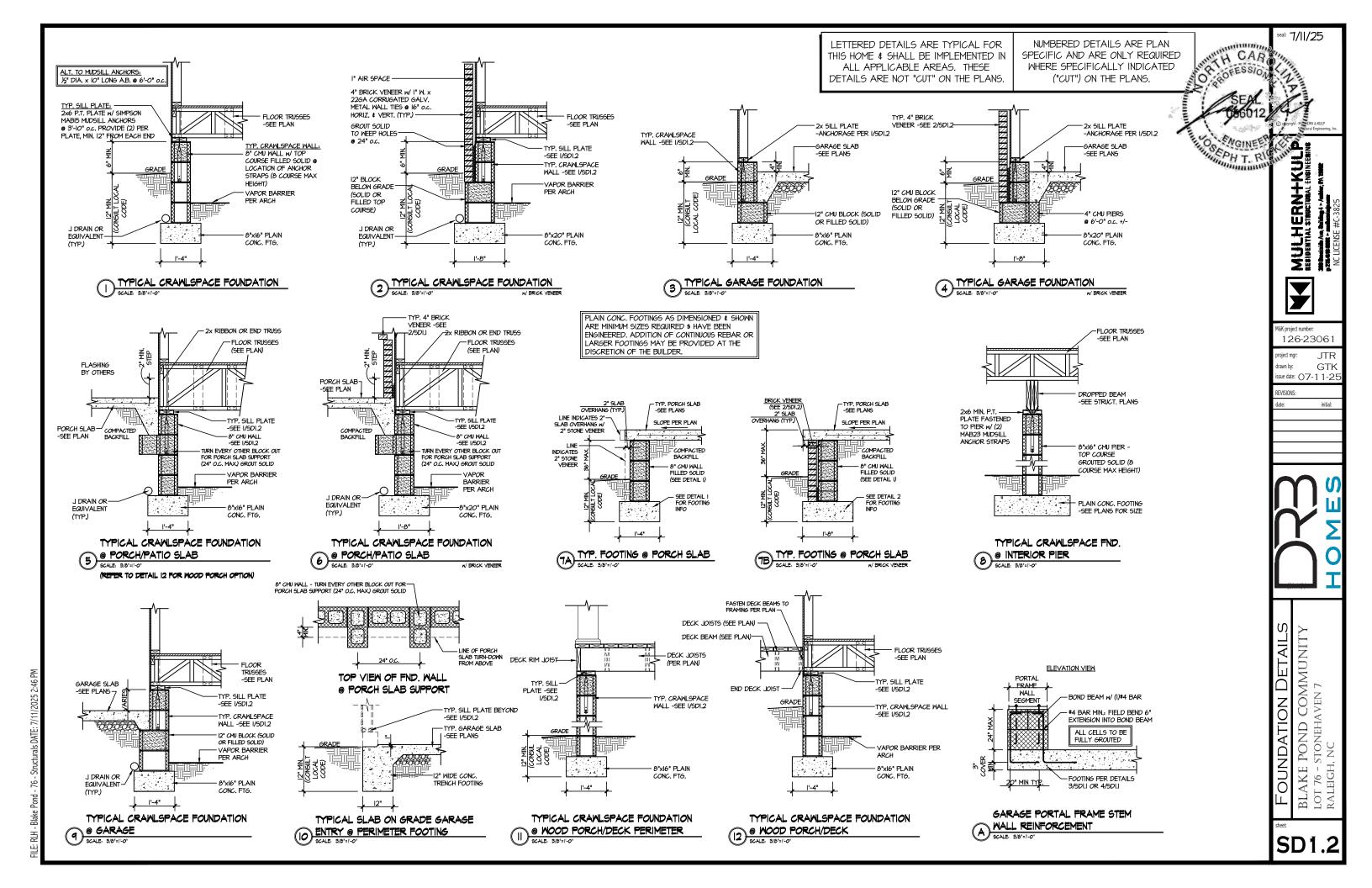
EXTENT OF OVERFRAMING

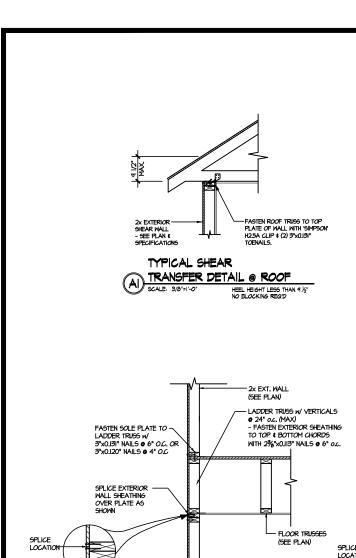
JL METAL HANGER

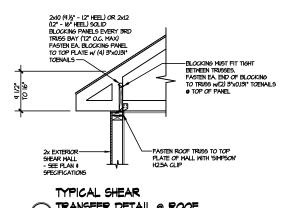
* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES









-2x EXT. WALL

RIBBON W/

-FLOOR TRUSSES

(SEE PLAN)

(SEE PLAN)

TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL

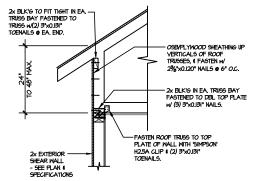
SHEATHING SPLICET

SOLE PLATE OR
 BOTTOM OF SOLE

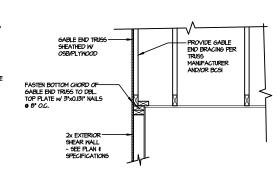
SPLICE EXTERIOR WALL SHEATHING OVER PLATE AS

FASTEN SOLE PLATE TO

3"x0.131" NAILS @ 6" O.C. OR 3"x0.120" NAILS @ 4" O.C







PACK OUT STEEL BEAM WEB W/-SOLID 2x MATERIAL & FASTEN

w/ (2) ROWS OF %" DIA. THRU-BOLTS • 16" O.C.



TYPICAL STEEL BEAM CONNECTION DETAIL SCALE 844-11-0*

-2x PLATE FASTENED TO TOP

-2x PLATE FASTENED TO TOP

FLANGE OF STL. BM. w/ (2) ROWS OF HILTI XU-47P8 PINS @ 16" O.C.



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eal: 7/11/25

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1&K project number 126-2306

JTR frawn by: issue date: 07-11-2!

REVISIONS

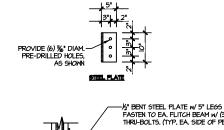
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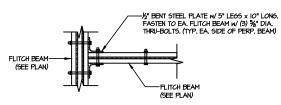
BLAKE POND COMMUNIT Lot 76 - Stonehaven 7 Raleigh, nc

("CUT") ON THE PLANS.

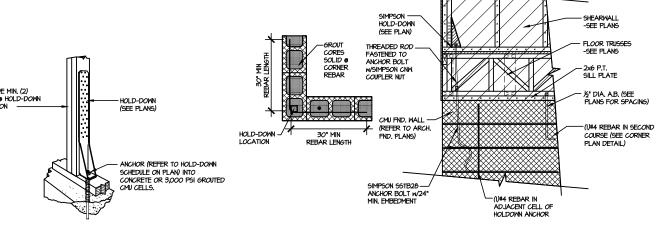
- STEEL FLITCH PLATE FASTEN WOOD BEAMS w/ FLITCH -PLATE W/ (2) ROWS OF 1/8" DIA. THRU-BOLTS @ 16" O.C. —2x wood beam (see plan) (SEE PLAN) - Steel Flitch Plate (See Plan) FASTEN WOOD BEAMS W/ FLITCH-PLATES w/ (2) ROWS OF %" DIA. THRU-BOLTS • 16" O.C. (SEE PLAN) (SEE PLAN) 2x WOOD BEAM (SEE PLAN) S-FLY OR MORE MOOD SEAM W/ (2 OR MORE) STEEL FLITCH FLATES







TYPICAL FLITCH BEAM TO FLITCH BEAM CONNECTION DETAIL SCALE: 5/4"=1"-0"



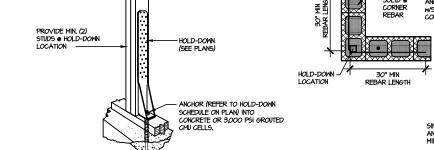
TYPICAL HOLD DOWN INSTALLATION

TYPICAL CORNER FOUNDATION HOLD-DOWN INSTALLATION

THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED

LETTERED DETAILS ARE TYPICAL FOR



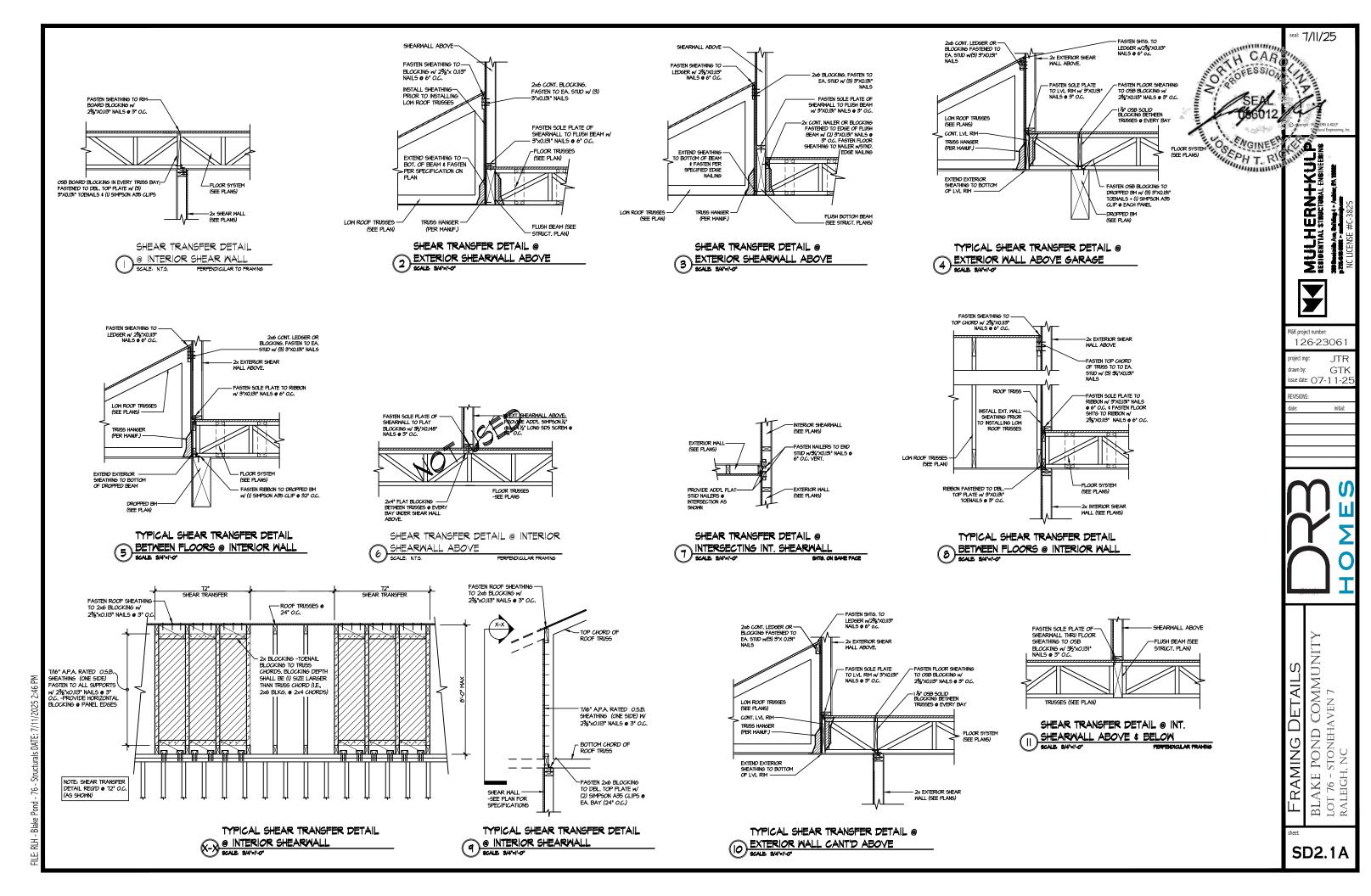
TYPICAL SHEAR TRANSFER DETAIL

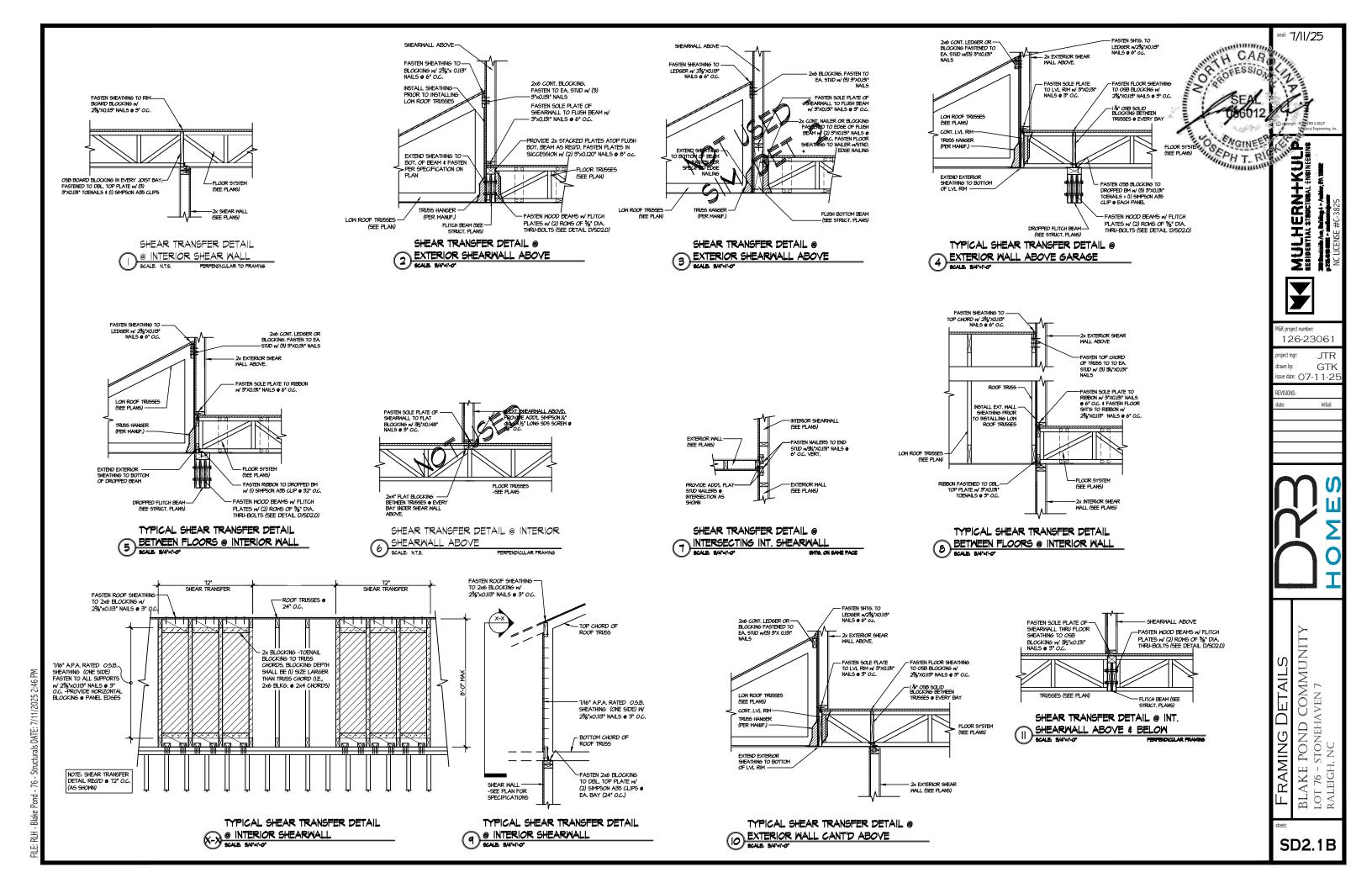
BETWEEN FLOORS @ EXTERIOR WALL

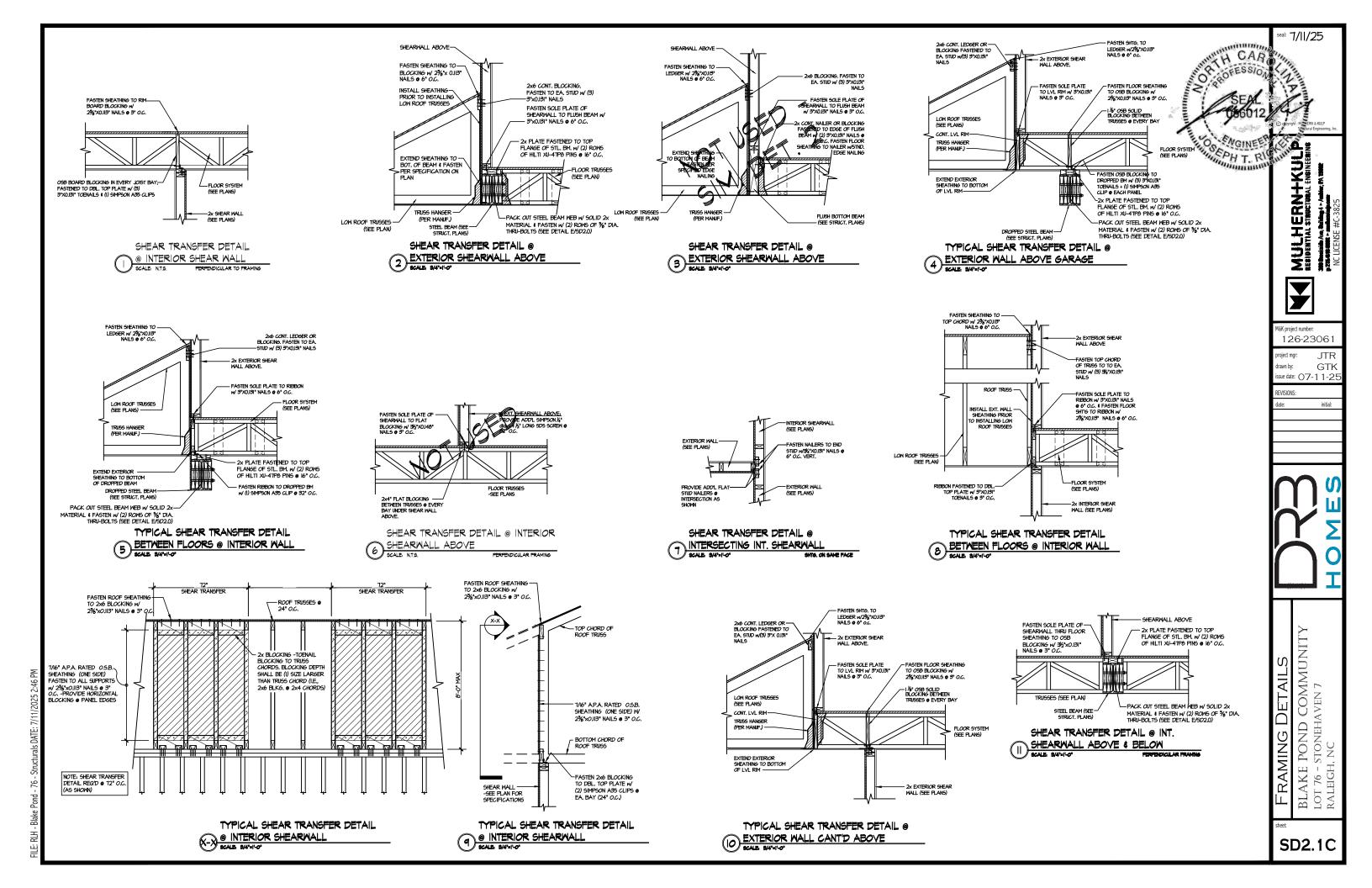
2x EXT. WALL

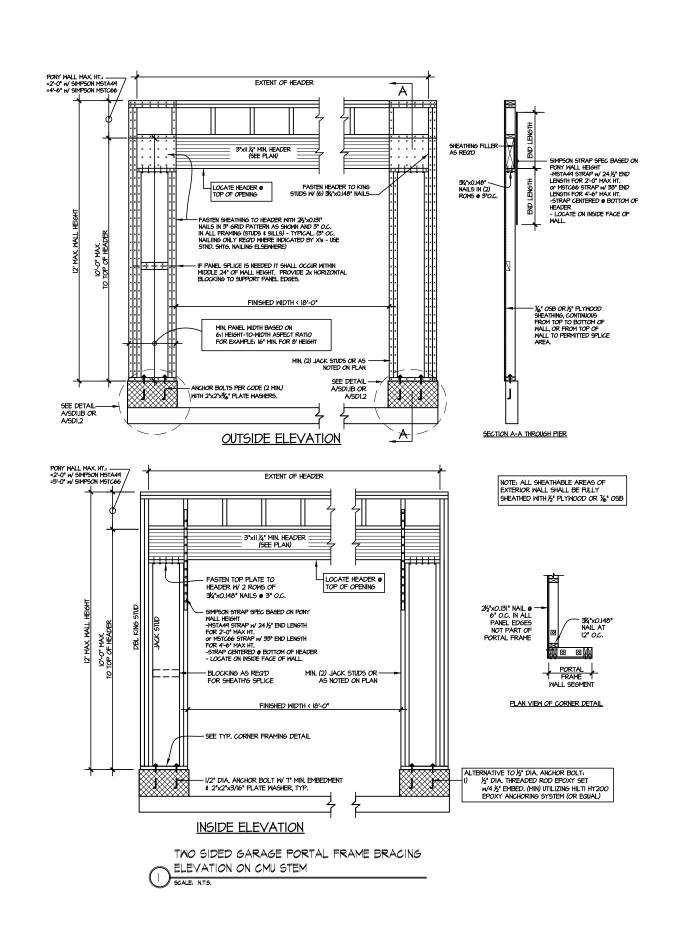
(SEE PLAN)

DETAILS Ü









eal: 7/11/25 MOR OFES OSEPH T. R

MULHERN+KUL Residential Structural Engineerin

M&K project number: 126-23061

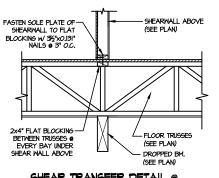
roject mgr: **JTR** drawn by: **GTK** issue date: 07-11-25

REVISIONS:

initial:

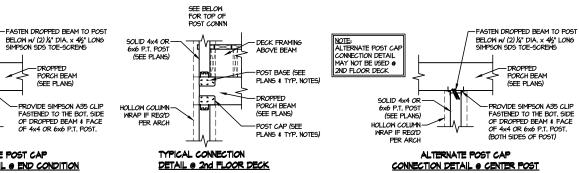
BLAKE POND COMMUNITY Lot 76 - Stonehaven 7 Raleigh, nc FRAMING DETAILS





SHEAR TRANSFER DETAIL @

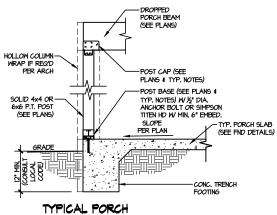
INTERIOR SHEARMALL ABOVE



HOLLOW COLUMN-WRAP IF REQ'D PER ARCH ALTERNATE POST CAP
CONNECTION DETAIL @ END CONDITION

NOTE: ALTERNATE POST CAP CONNECTION DETAIL MAY NOT BE USED @ 2ND FLOOR DECK

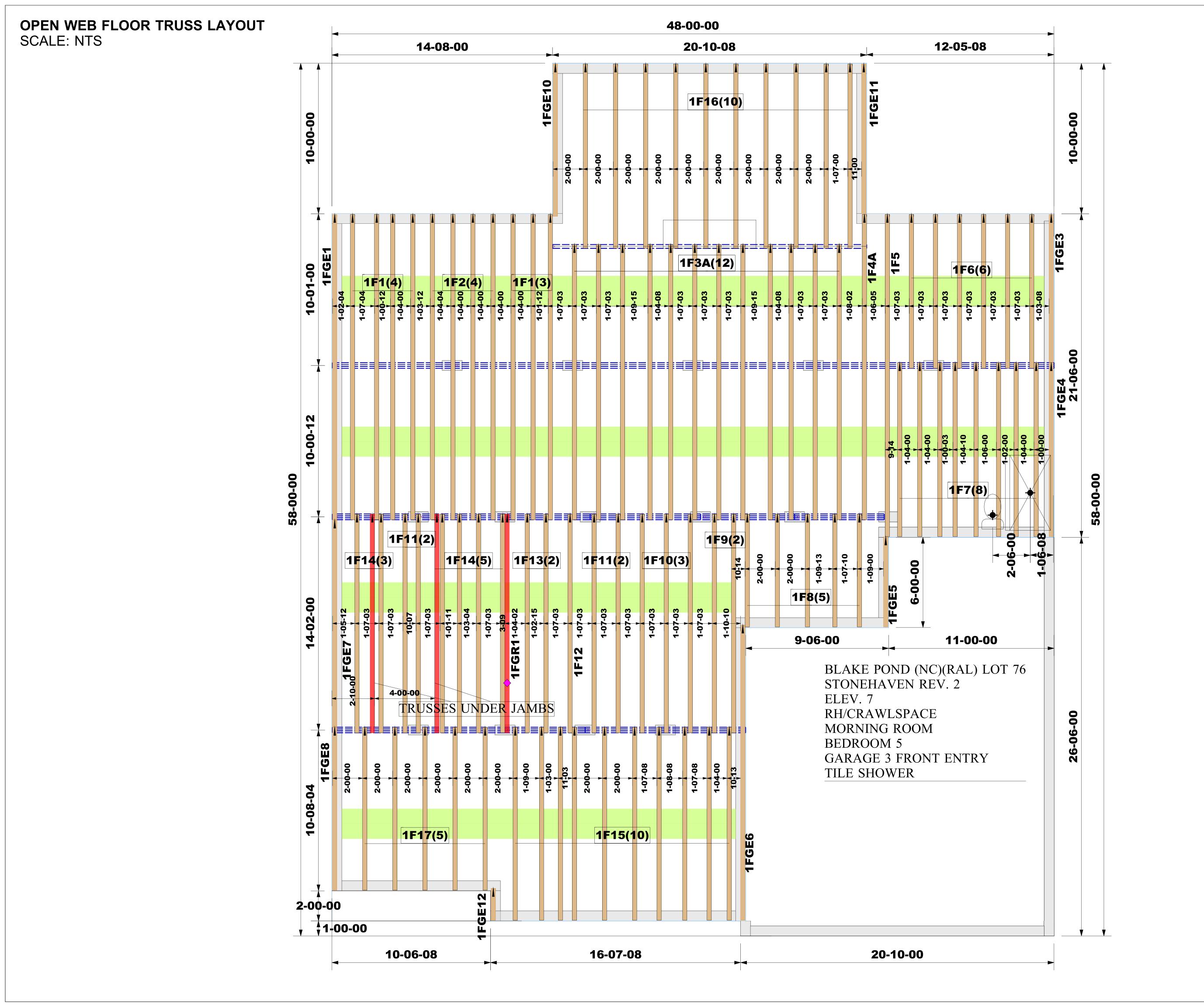
SOLID 4x4 OR — 6x6 P.T. POST (SEE PLANS)



DETAIL @ 2nd FLOOR DECK

3 POST CONNECTION DETAIL SLAB ON GRADE SHOWN (RAISED SLAB SIM.)

SD3.0





Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

Phone: 301-271-7591

Customer: DRB Raleigh Job Name: Blake Pond Lot 00.0076 OWF

Stonehaven

Model Name:

Third-Party Quality Assurance License
TPI Plant W974

NOTE:

9200.00

#

-ot

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR TH CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDIN THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.

WARNING: CONVENTIONAL FRAMING, ERECTION AND/OR PERMANE

BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS
DESIGNER, PLATE MANUFACTURER, OR THE TRUSS
MANUFACTURER. PERSONS ERECTING TRUSSES ARE
CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING TH
ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN
TOPPLING AND DOMINOING DURING ERECTION; AND
PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC
APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTAR
AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER

POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SWHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB

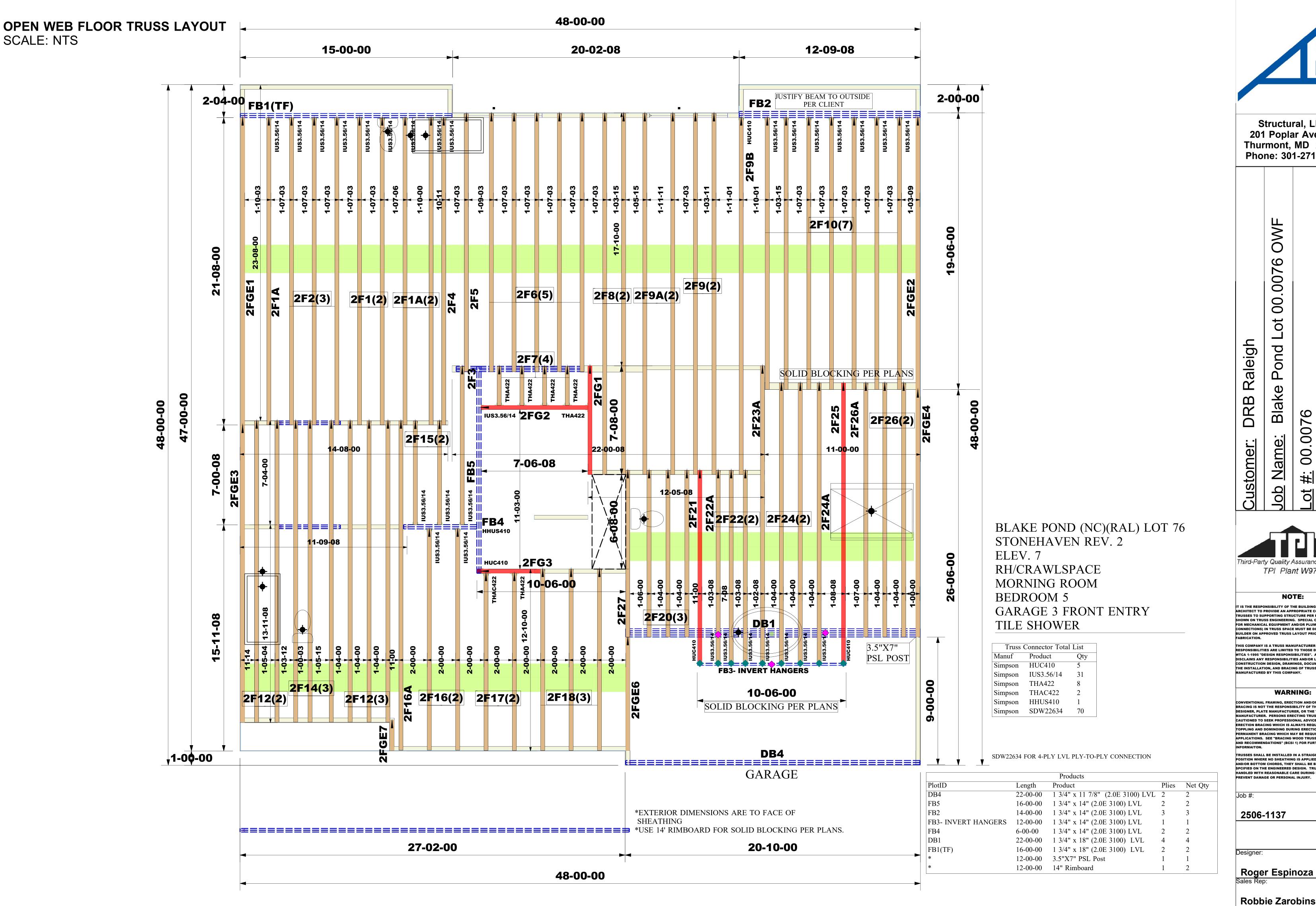
Job #:

2506-1137

Designer:

Roger Espinoza
Sales Rep:

Robbie Zarobinski





Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

9200.00

Stonehaven

Model Name:

TPI Plant W974

#

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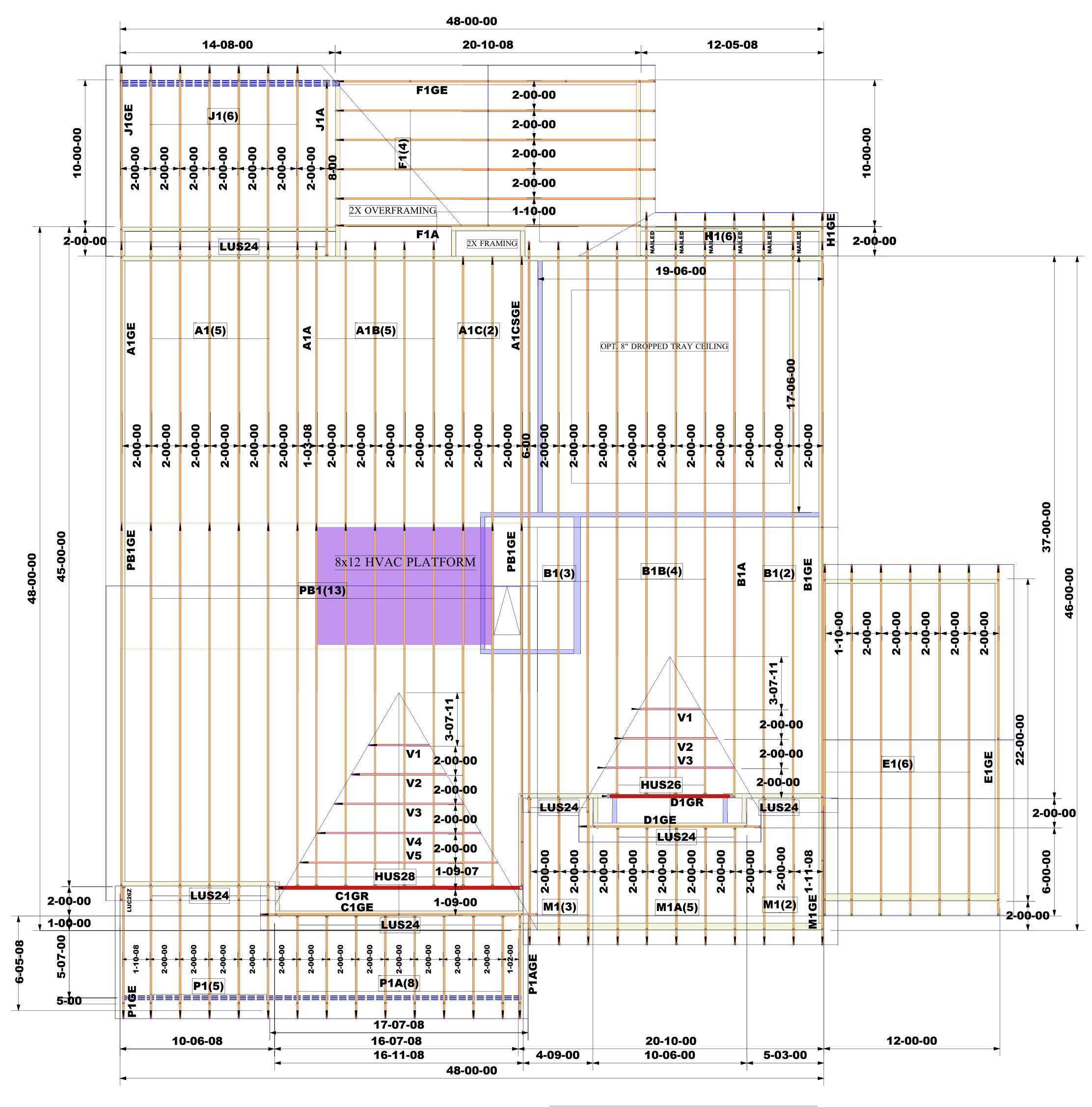
NOTE:

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY

WARNING:

BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS ANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC

Robbie Zarobinski



ROOF FRAMING PLAN

*EXTERIOR DIMENSIONS ARE TO FACE OF SHEATHING

*INSTALL SIMPSON H2.5A HURRICANE ANCHORS AT EACH BEARING POINT

Truss Connector Total List Product Simpson HUS26 LUC26Z LUS24 One H2.5A 140 Simpson HUS28

BLAKE POND SF LOT 00.0076

(NC)(RAL)

3656-1 - STONEHAVEN REV 2

EL. 7 OPT. MORNING ROOM

OPT. SCREEN PORCH

OPT. COVERED PORCH

OPT. GARAGE 3 CAR FRONT ENTRY

OPT. BEDROOM 5

OPT. FIRE PLACE

GARAGE RIGHT

Job #: 2506-1167	WARNING: CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE	NOTE: IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO	Customer:	DRB Raleigh		
	TRUSS DESIGNER, PLATE MANOFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	Job Name:	Blake Pond Lot 00.0076 ROOF	Third-Party Quality Assurance Licensee TPI Plant W974	
Designer: Sagar Banik Sales Rep:	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON. TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS. THEY	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE	<u>Lot #:</u> 00.00	076	Structural, LLC 201 Poplar Avenue Thurmont, MD 21788	
Sales Rep: Robbie Zarobinski	SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.	INCLUDING THE INSTALLATION, AND BRACING OF	Model Nam	e: Stonehaven Rev 2	Phone: 301-271-7591	

