

RE: 25-3703-A SGR-LOT 74 ROOF Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: 25-3703-A Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 40 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	174047316	CJ01	6/9/2025	21	174047336	T06	6/9/2025
2	174047317	HG01	6/9/2025	22	174047337	T07	6/9/2025
3	174047318	J01	6/9/2025	23	174047338	V01	6/9/2025
4	174047319	J02	6/9/2025	24	174047339	V02	6/9/2025
5	174047320	M01	6/9/2025	25	174047340	V03	6/9/2025
6	174047321	M01A	6/9/2025	26	174047341	V04	6/9/2025
7	174047322	M02	6/9/2025	27	174047342	V05	6/9/2025
8	174047323	PB01	6/9/2025	28	174047343	V06	6/9/2025
9	174047324	PB01GE	6/9/2025	29	174047344	V07	6/9/2025
10	174047325	T01	6/9/2025	30	174047345	V08	6/9/2025
11	174047326	T01G	6/9/2025	31	174047346	V09GE	6/9/2025
12	174047327	T01GE	6/9/2025	32	174047347	V10	6/9/2025
13	174047328	T02	6/9/2025	33	174047348	V11	6/9/2025
14	174047329	T02G	6/9/2025	34	174047349	V12	6/9/2025
15	174047330	T02GE	6/9/2025	35	174047350	V13	6/9/2025
16	174047331	T03	6/9/2025	36	174047351	V14	6/9/2025
17	174047332	T03GE	6/9/2025	37	174047352	V15	6/9/2025
18	174047333	T03S	6/9/2025	38	174047353	V16	6/9/2025
19	174047334	T05	6/9/2025	39	174047354	V17	6/9/2025
20	174047335	T05A	6/9/2025	40	174047355	V18	6/9/2025

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Riverside Roof Truss.

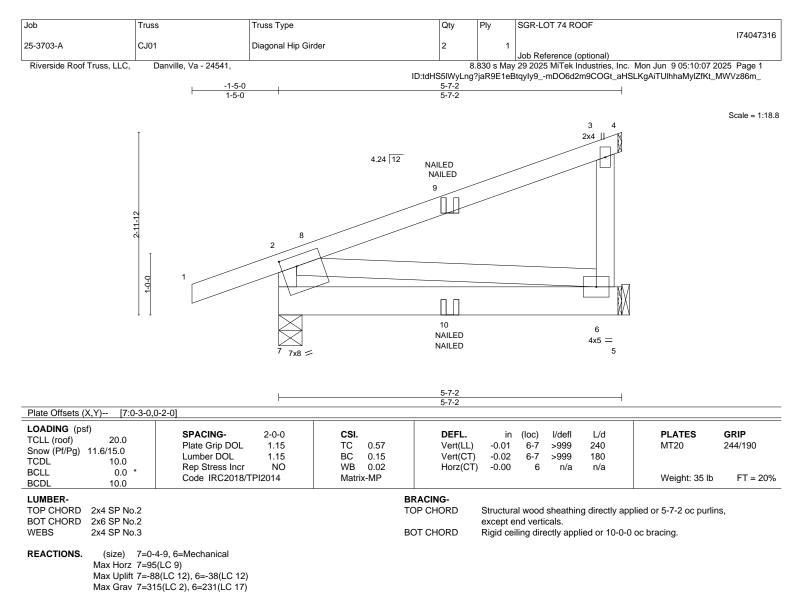
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-7=-263/94

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Ct=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 6.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- initial rules is designed in accordance with the 2016 international Residential Code sections RS02.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
   indicates 3-10d (0.148\*x3") or 2-12d (0.148\*x3.25") toe-nails per NDS guidlines.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

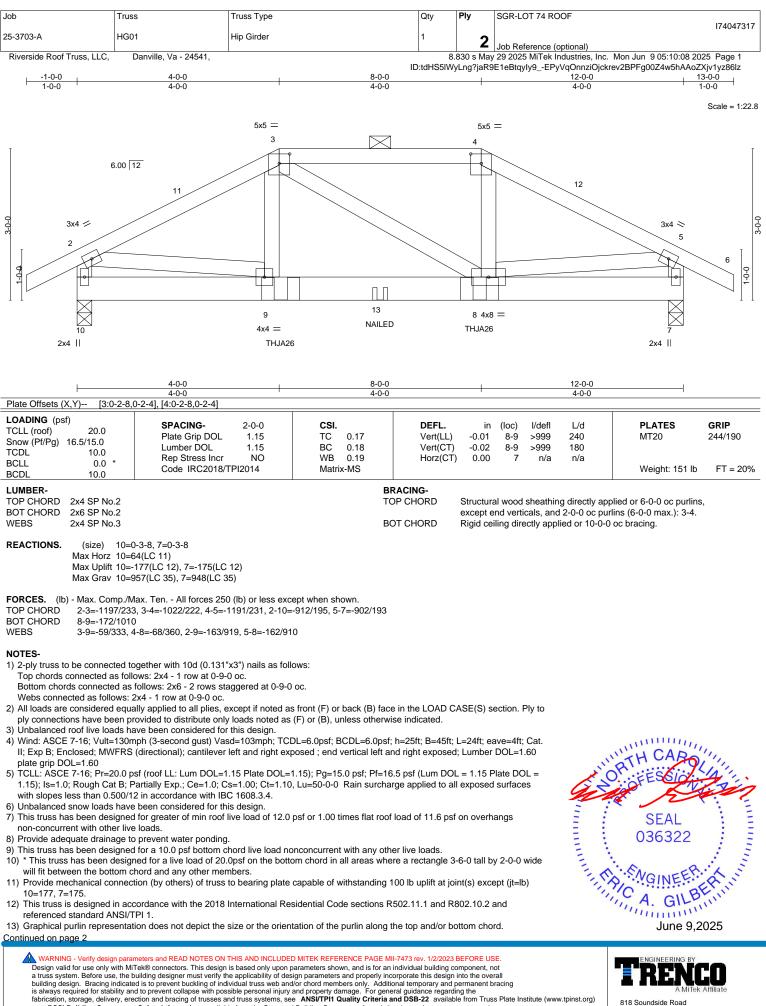
#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-43, 2-3=-43, 3-4=-43, 5-7=-20



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 74 ROOF
					174047317
25-3703-A	HG01	Hip Girder	1	2	
				<b>_</b>	Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	.830 s May	29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:08 2025 Page 2
		ID	:tdHS5IW	yLng?jaR9	E1eBtqyIy9EPyVqOnnziOjckrev2BPFg00Z4w5hAAoZXjv1yz86lz

NOTES-

14) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Left Hand Hip) or equivalent at 4-0-6 from the left end to connect truss(es) to front face of bottom chord.

15) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Right Hand Hip) or equivalent at 7-11-10 from the left end to connect truss(es) to front face of bottom chord.

16) Fill all nail holes where hanger is in contact with lumber.

17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-43, 2-3=-43, 3-4=-53, 4-5=-43, 5-6=-43, 7-10=-20

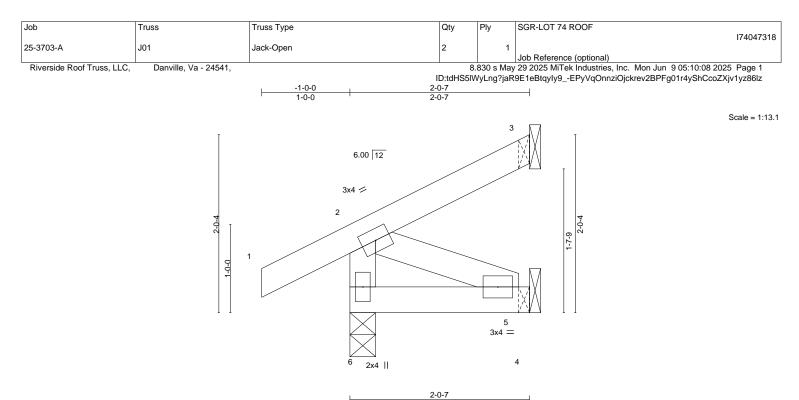
Concentrated Loads (lb)

Vert: 9=-336(F) 8=-336(F) 13=-134(F)



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			2-0-7	
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.09 BC 0.03 WB 0.03	DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         -0.00         6         >999         240         MT20         244/19           Vert(CT)         -0.00         5-6         >999         180         MT20         244/19           Horz(CT)         -0.00         3         n/a         n/a         16	10
BCDL 10.0	Code IRC2018/TPI2014	Matrix-MP	Weight: 12 lb FT =	= 20%

## LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 2-0-7 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 6=0-3-8, 3=Mechanical, 5=Mechanical Max Horz 6=69(LC 16) Max Uplift 6=-24(LC 16), 3=-10(LC 13), 5=-14(LC 16) Max Grav 6=167(LC 21), 3=34(LC 21), 5=40(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

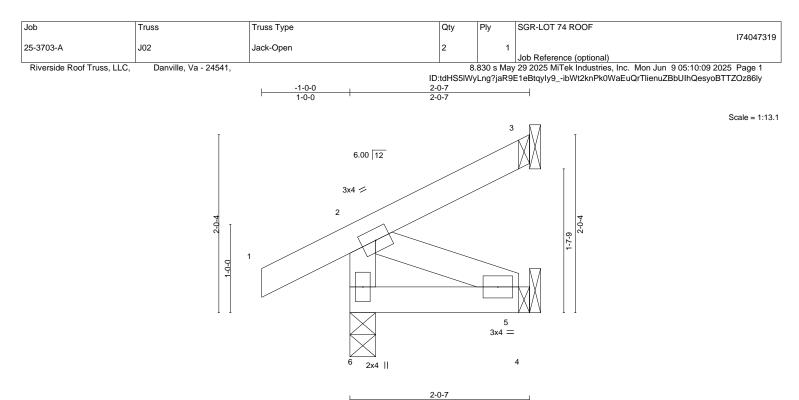
8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3, 5.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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		I	2-0-7		1				
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.09 BC 0.03 WB 0.03 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) 6 5-6 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 12 lb	<b>GRIP</b> 244/190 FT = 20%

### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 2-0-7 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 6=0-3-8, 3=Mechanical, 5=Mechanical Max Horz 6=69(LC 16) Max Uplift 6=-24(LC 16), 3=-10(LC 13), 5=-14(LC 16) Max Grav 6=167(LC 21), 3=34(LC 21), 5=40(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

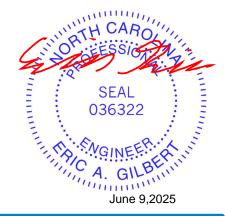
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

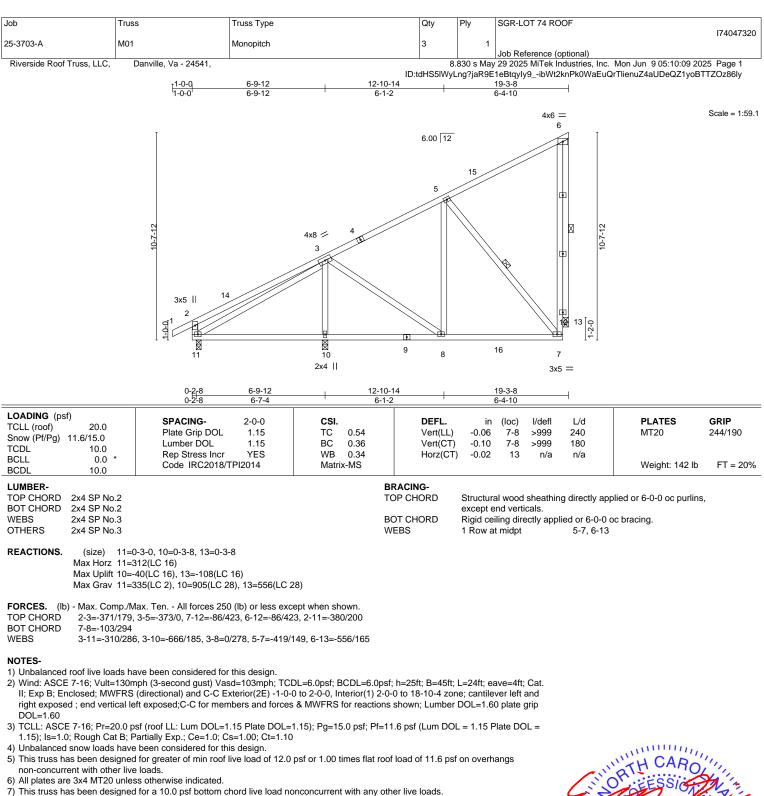
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3, 5.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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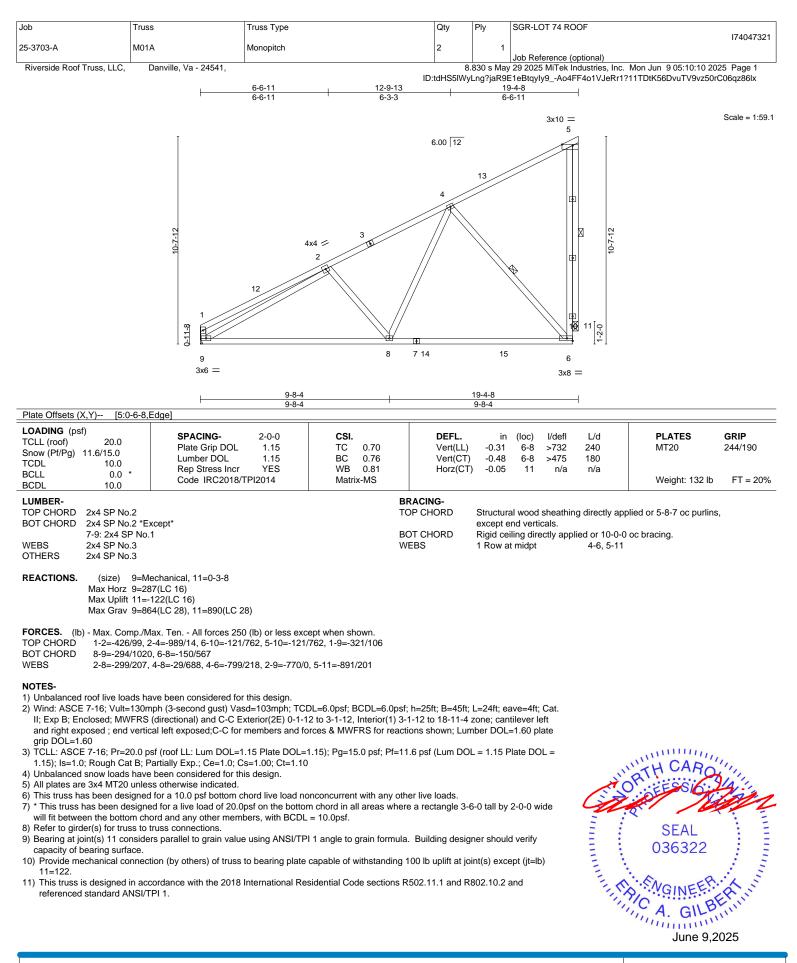




- 8) \* This trust has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 13=108.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

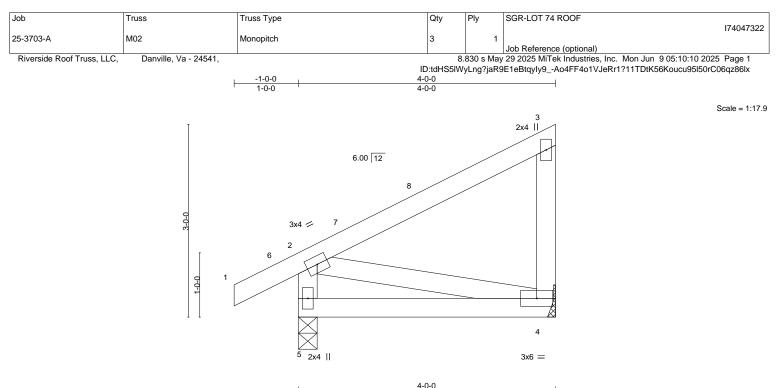


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			4-0-0						
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCDL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.25 BC 0.16 WB 0.06 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 -0.00	(loc) 4-5 4-5 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 24 lb	<b>GRIP</b> 244/190 FT = 20%

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 4=Mechanical Max Horz 5=98(LC 15) Max Uplift 5=-42(LC 16), 4=-26(LC 13)

Max Grav 5=228(LC 2), 4=154(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

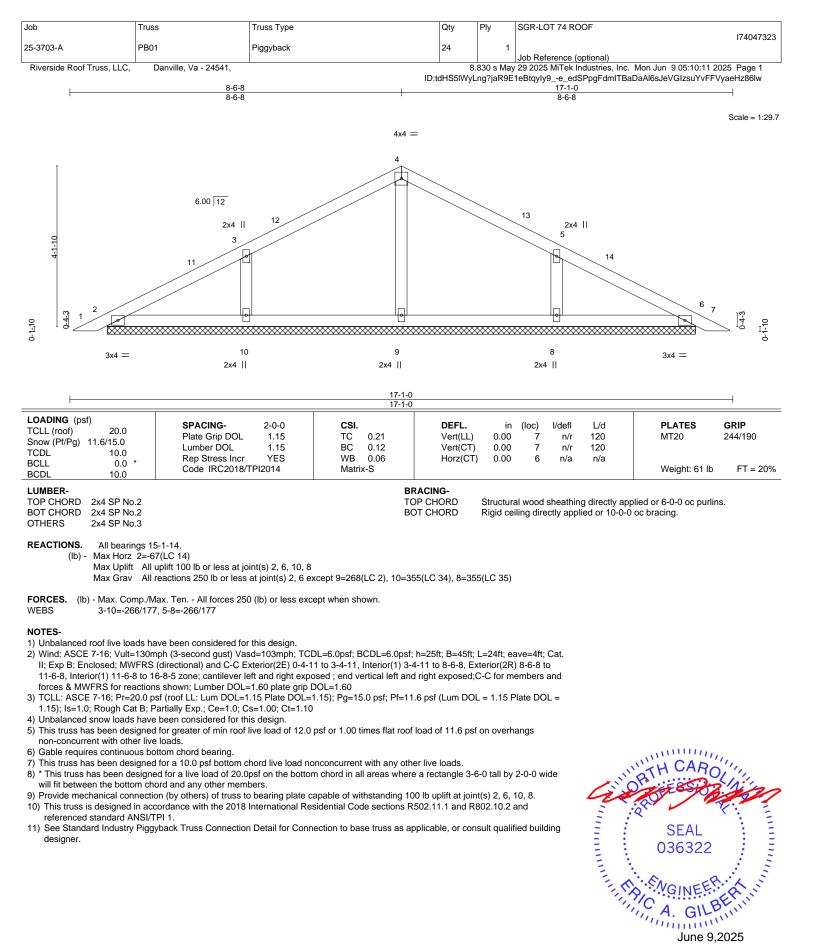
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

# SEAL 036322 June 9,2025

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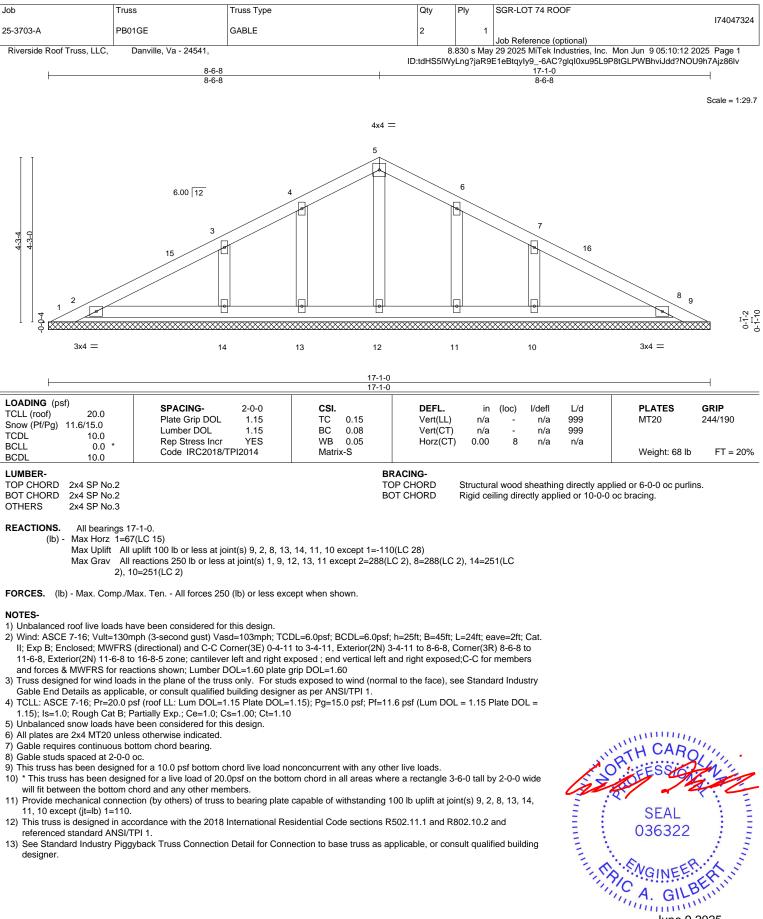




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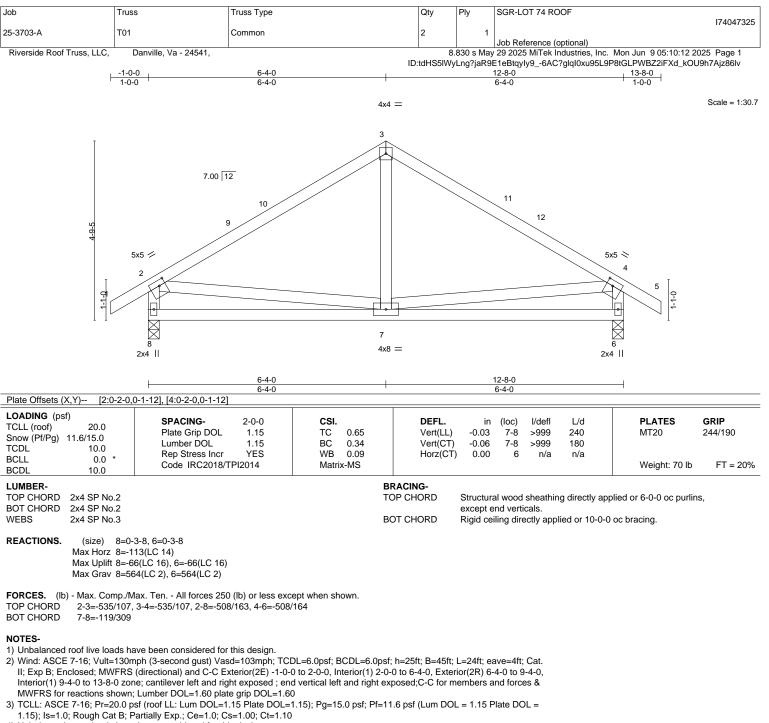
818 Soundside Road



June 9,2025

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bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Trus	S	Truss Type		Qty	Ply	SGR-LOT 74	ROOF		
25-3703-A	T01	G	Common Girder		1	2				174047326
Riverside Roof T	russ, LLC,	Danville, Va - 24541,			8.		Job Reference y 29 2025 MiT		c. Mon Jun 9 05:10:13	2025 Page 1
		3-3-12		6-4-0	9-4-	4	E1eBtqyly9	12-8-0	iVkcibnaykknk5aMMMS	KjpRgj9z86lu
		3-3-12	I	3-0-4	3-0-4	4	1	3-3-12		
					4x4 ≡					Scale = 1:29.5
	Т				3					
		7	00 12							
			3x5 📁			$\overline{\ }$	3x5 ≷ ∕ 4			
			2				A	12		
	4-9-5	1	1				$\rightarrow$			
		x5 1/2						$\searrow$	3x5 💸	
		1							5	
	1-1-0								1-1-0	
	11	13		14	15			16		
		13 10 LUS26	9 3x6 =	111000	8 = 13 3x8 = LUS26		73x6 = LUS26	LUS26	6	
	42	(4	LUS26		5.0 -		20020		4x4	
		3-3-12	I	6-4-0	9-4-	4	I.	12-8-0	1	
Plate Offsets (X	,Y) [6:Edge,	<u>3-3-12</u> 0-3-8]		3-0-4	3-0-4	4	1	3-3-12	1	
LOADING (psf)		SPACING-	2-0-0	CSI.	DEFL.	in	(loc) l/de	efl L/d	PLATES	GRIP
TCLL (roof) Snow (Pf/Pg) 1	20.0 1.6/15.0	Plate Grip DOL	1.15	TC 0.47	Vert(LL)	-0.02	8-9 >99	9 240	MT20	244/190
TCDL BCLL	10.0 0.0 *	Lumber DOL Rep Stress Incr	1.15 NO	BC 0.37 WB 0.44	Vert(CT) Horz(CT		8-9 >99 6 n			
BCDL	10.0	Code IRC2018/	TPI2014	Matrix-MS					Weight: 171 I	o FT = 20%
LUMBER- TOP CHORD	2x4 SP No.2				BRACING- TOP CHORD	Structur	ral wood shea	thing directly a	pplied or 6-0-0 oc purli	ns.
BOT CHORD	2x6 SP No.2 2x4 SP No.3				BOT CHORD	except e	end verticals.	applied or 10-0		,
					BOT CHORD	Trigiu ce	and a rective		-0 oc bracing.	
REACTIONS.	(size) 10= Max Horz 10=	0-3-8, 6=0-3-8 ·97(LC 35)								
	Max Uplift 6=-1 Max Grav 10=	27(LC 12) 2634(LC 3), 6=2350(LC	3)							
		fax. Ten All forces 25	,	ot when shown						
TOP CHORD	1-2=-2868/0, 2	2-3=-2145/95, 3-4=-214	· · /							
BOT CHORD		8-9=0/2434, 7-8=-73/2								
WEBS	3-8=-44/1958 5-7=-56/2024	, 4-8=-613/129, 4-7=-95	/558, 2-8=-812/0, 3	2-9=0/784, 1-9=0/214	41,					
NOTES-										
1) 2-ply truss to		ogether with 10d (0.131 ows: 2x4 - 1 row at 0-9-		WS:						
Bottom chord	s connected as	follows: 2x6 - 2 rows st		IC.						
		2x4 - 1 row at 0-9-0 oc. ally applied to all plies, e	except if noted as f	front (F) or back (B) f	ace in the LOAD C	ASE(S) s	section. Ply to			
		rovided to distribute only ave been considered for		-) or (B), unless othe	rwise indicated.					111
4) Wind: ASCE	7-16; Vult=130r	nph (3-second gust) Va (directional); cantilever	sd=103mph; TCDL					3	RTHUAR	China -
plate grip DO	L=1.60		<b>U</b> .		<b>U</b> .				CT. FESSO	13 m
		osf (roof LL: Lum DOL=1 Partially Exp.; Ce=1.0; C		15); Pg=15.0 pst; Pt=	11.6 pst (Lum DO	L = 1.15 I	Plate DOL =	y		
		e been considered for th d for a 10.0 psf bottom o		concurrent with any o	other live loads.			Ξ	SEAL	E E
		ed for a live load of 20.0 ord and any other mem		chord in all areas wh	ere a rectangle 3-0	6-0 tall by	2-0-0 wide		036322	1 E
9) Provide mech		on (by others) of truss to		bable of withstanding	100 lb uplift at joir	it(s) exce	pt (jt=lb)	THE REAL PROPERTY OF THE PARTY		1.3
		cordance with the 2018	International Resi	dential Code sections	8 R502.11.1 and R	802.10.2	and	11	A NGINEE	A ST
	standard ANSI/ n Strong-Tie LU	「PI 1. JS26 (4-10d Girder, 3-1	Od Truss) or equiv	alent spaced at 2-0-0	oc max. starting a	it 1-2-12 i	from the left	9	A. GIL	Shinin .
end to 11-2-	12 to connect to	russ(es) to back face of ger is in contact with lun	bottom chord.						June 9	
Continued on page	ge 2								Julie 9	,2020
	- Verify design para	meters and READ NOTES ON							ENGINEERING B	
a truss syste	m. Before use, the	Tek® connectors. This design ouilding designer must verify the ad is to provent buckling of income	ne applicability of desig	n parameters and properly	incorporate this design	into the ove	erall		REN	CO
is always rec	quired for stability ar	ed is to prevent buckling of ind to prevent collapse with pos	sible personal injury ar	d property damage. For g	eneral guidance regard	ing the			A MIT	ek Affiliate

billing design. Dialog indicating the billing of the billing of individual dust were alreed of the method in the state of the billing and permethold and billing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 74 ROOF	
					174047326	
25-3703-A	T01G	Common Girder	1	2		
				2	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,	8.830 s May 29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:13 2025 Page 2				

8.830 s May 29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:13 2025 Page 2 ID:tdHS5IWyLng?jaR9E1eBtqyIy9\_-aMmOt5rwnE00iVkcibnaykknk5aMMMSXjpRgj9z86lu

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-5=-43, 6-10=-20

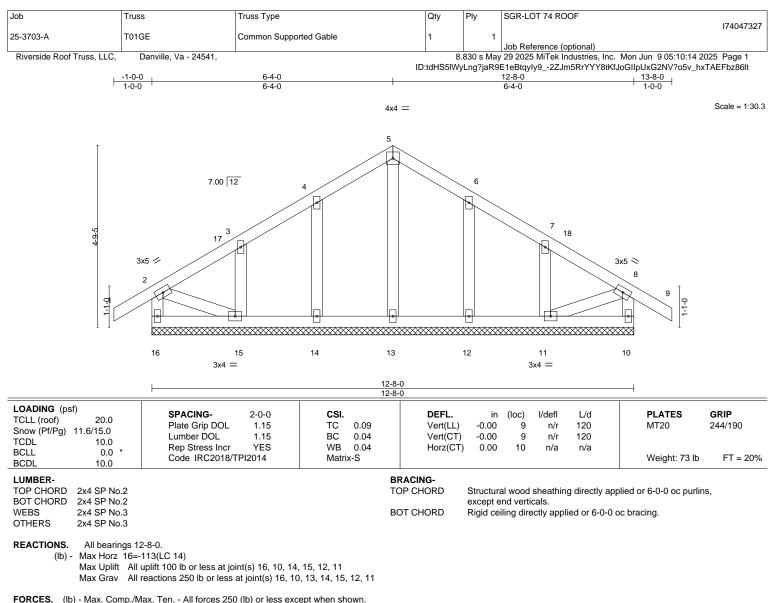
Concentrated Loads (lb)

Vert: 7=-474(B) 9=-595(B) 13=-595(B) 14=-474(B) 15=-474(B) 16=-474(B)



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#### NOTES-

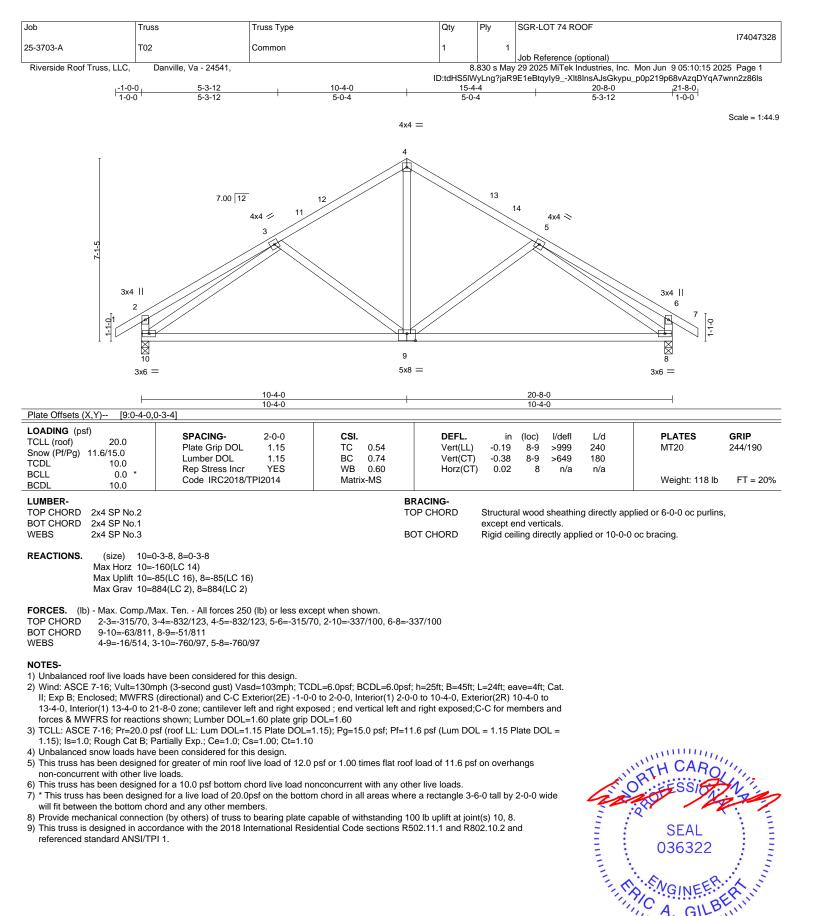
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 6-4-0, Corner(3R) 6-4-0 to 9-4-0, Exterior(2N) 9-4-0 to 13-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
   Orbita study presended 0.0.0 presented at 0.0.0 presented against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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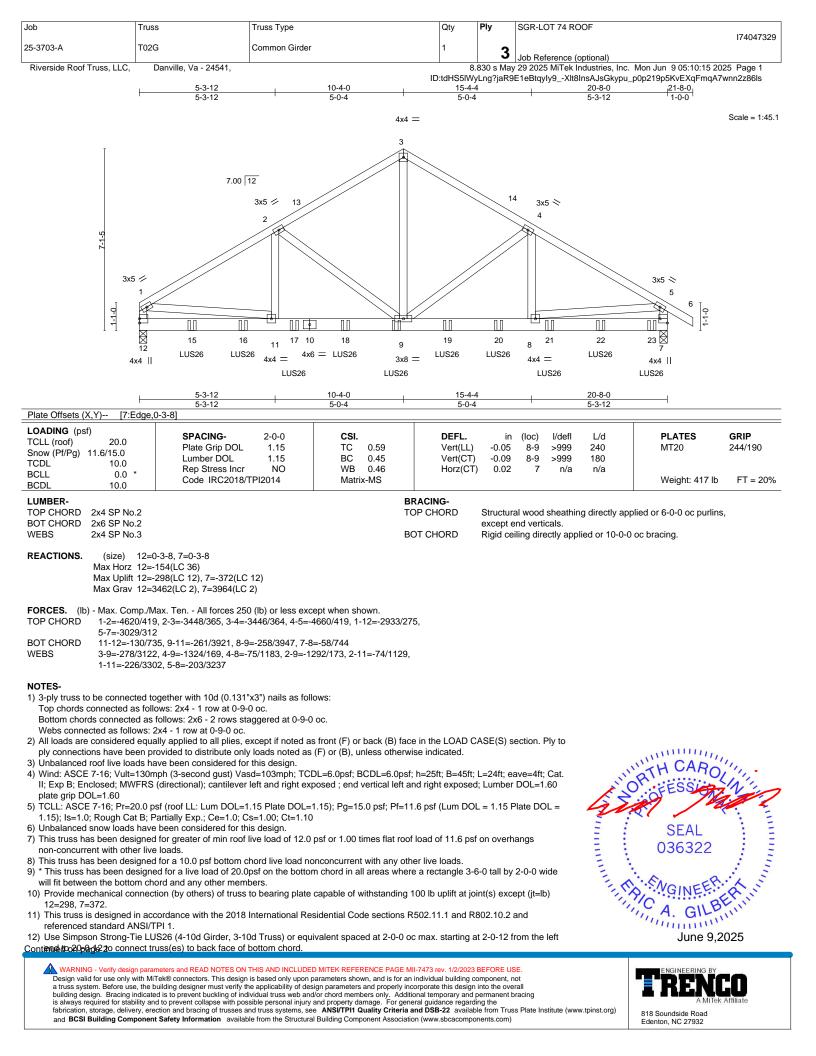


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minin June 9,2025



818 Soundside Road



Job	Truss	Truss Type	Qty	Ply	SGR-LOT 74 ROOF
					174047329
25-3703-A	T02G	Common Girder	1	2	
				3	Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	830 s May	29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:16 2025 Page 2

ID:tdHS5IWyLng?jaR9E1eBtqyly9\_-?xRWW7to49ObZzTBNjKHZMMG4JamZi?\_PnfKJUz86Ir

#### NOTES-

13) Fill all nail holes where hanger is in contact with lumber.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

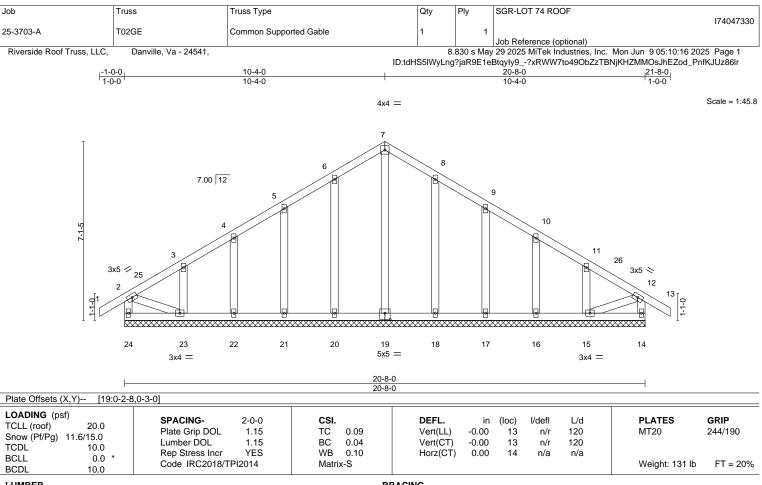
Vert: 1-3=-43, 3-5=-43, 5-6=-43, 7-12=-20 Concentrated Loads (lb)

Vert: 9=-469(B) 15=-474(B) 16=-469(B) 17=-469(B) 18=-469(B) 19=-469(B) 20=-469(B) 21=-469(B) 22=-469(B) 23=-480(B)



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LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
OTHERS	2x4 SP No.3		6-0-0 oc bracing: 23-24,14-15.

#### REACTIONS. All bearings 20-8-0.

(lb) - Max Horz 24=-160(LC 14)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

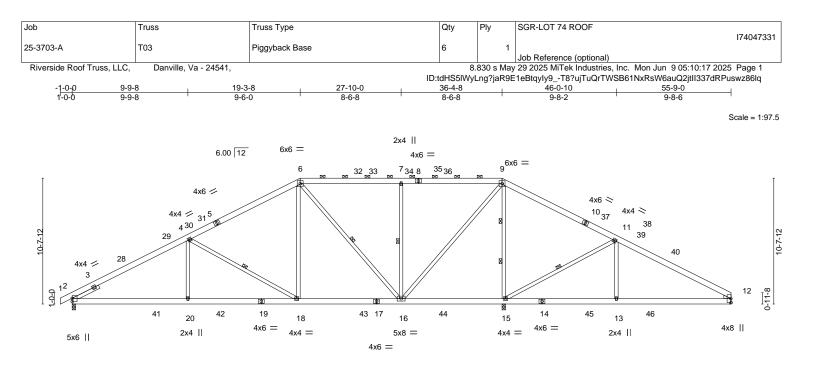
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 10-4-0, Corner(3R) 10-4-0 to 13-4-0, Exterior(2N) 13-4-0 to 21-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Max Uplift All uplift 100 lb or less at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15



9-9-8	19-3-8	27-10-0	36-4-8	36-6-4	46-0-10	55-9-0	
9-9-8 LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	9-6-0 SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Rep Stress Incr YES	8-6-8 <b>CSI.</b> TC 0.70 BC 0.69 WB 0.85 WB 0.85	8-6-8 DEFL. Vert(LL) Vert(CT) Horz(CT)	0-1 <sup>1</sup> 12 in (loc) -0.12 18-20 -0.23 18-20 0.05 15	>999 180	PLATES MT20	<b>GRIP</b> 244/190
BCDL         10.0           LUMBER-         TOP CHORD         2x6 SP No.2           BOT CHORD         2x6 SP No.2         WEBS         2x4 SP No.3           WEDGE         Right: 2x4 SP No.3         WEDGE	Code IRC2018/TPI2014	T	RACING- OP CHORD OT CHORD	except 2-0-0 oc purlin	s (6-0-0 max.): 6-9. rectly applied or 10	Veight: 407 lb v applied or 4-4-13 oc purlir -0-0 oc bracing, Except:	FT = 20%

WEBS

1 Row at midpt

2 Rows at 1/3 pts

SLIDER Left 2x4 SP No.3 2-6-0

REACTIONS. (size) 2=0-3-8, 15=0-3-8, 12=Mechanical Max Horz 2=206(LC 15) Max Uplift 2=-124(LC 16), 15=-124(LC 16), 12=-53(LC 16)

Max Grav 2=1668(LC 28), 15=2954(LC 29), 12=688(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2540/258, 4-6=-1636/263, 6-7=-858/260, 7-9=-858/260, 9-11=0/718,

- 101010102 2-4-2040/200, 4-0=-1030/203, 0-7=-008/200, 7-9=-858/260, 9-11: 11-12=-726/143
- BOT CHORD
   2-20=-144/2329, 18-20=-144/2329, 16-18=0/1435, 15-16=-511/138, 13-15=-25/577, 12-13=-25/577

   WEBS
   4-20=0/448, 4-18=-1055/175, 6-18=0/920, 6-16=-984/46, 7-16=-741/173, 9-16=-137/1927, 9-15=-2081/217, 11-15=-1209/188, 11-13=0/493

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=56ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 4-6-14, Interior(1) 4-6-14 to 19-3-8, Exterior(2R) 19-3-8 to 27-2-2, Interior(1) 27-2-2 to 36-4-8, Exterior(2R) 36-4-8 to 44-3-2, Interior(1) 44-3-2 to 55-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 9) Potent windprice for the truth of the truth of

9) Refer to girder(s) for truss to truss connections.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=124, 15=124.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Continuities and a continuity of the continuity of

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



4-18, 6-16, 7-16, 11-15

9-15



	Job	Truss	Truss Type	Qty	Ply	SGR-LOT 74 ROOF	
						174047331	
	25-3703-A	T03	Piggyback Base	6	1		
						Job Reference (optional)	
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	830 s May	29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:17 2025 Page 2	
ID:tdHS5IWyLng?jaR9E1eBtqyly9T8?ujTuQrTWSB61NxRsW6auQ2jtII337dRPuswz							

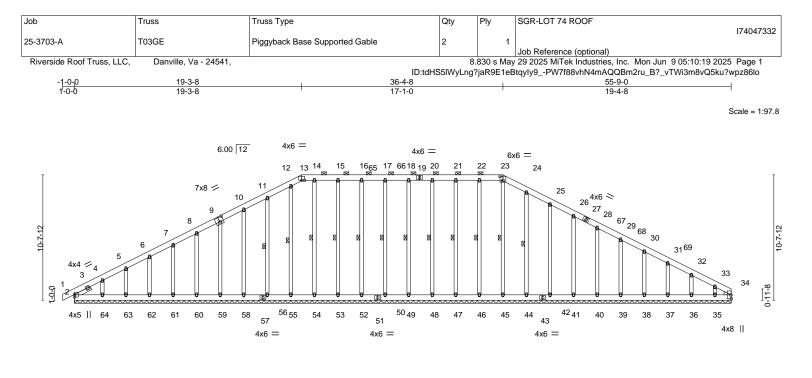
#### NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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		55-9- 55-9-						
Plate Offsets (X,Y) [9:0-4-0,0	0-4-8], [23:0-3-0,0-4-0]		.0					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.07 BC 0.03 WB 0.19 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 1 -0.00 1 0.01 34	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 542 lb	<b>GRIP</b> 244/190 FT = 20 <sup>6</sup>
LUMBER-           TOP CHORD         2x6 SP No.2           BOT CHORD         2x6 SP No.2           OTHERS         2x4 SP No.3           WEDGE         Right: 2x4 SP No.3           SLIDER         Left 2x4 SP No.3	3 1-6-4	Т	BRACING- TOP CHORD BOT CHORD VEBS	Structural wood 2-0-0 oc purlins Rigid ceiling di 1 Row at midpi	s (6-0-0 m rectly app	ax.): 13-23. lied or 10-0-0 23-45, 22	blied or 6-0-0 oc purlins oc bracing. -46, 21-47, 20-48, 18-4 -53, 14-54, 12-55, 11-5	9, 17-50,
58, 5 Max Grav All r 54, 5 34 FORCES. (lb) - Max. Comp./M TOP CHORD 11-12=-113/28 16-17=-111/28		, 38, 37, 36, 35, 34 46, 47, 48, 49, 50, 52, 5 2, 41, 40, 39, 38, 37, 36, ept when shown. I-15=-111/287, 15-16=-1	35, 11/287,					
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads ha</li> <li>2) Wind: ASCE 7-16; Vult=130m</li> <li>II; Exp B; Enclosed; MWFRS</li> <li>24-10-6, Exterior(2N) 24-10-6</li> <li>exposed ; end vertical left and grip DOL=1.60</li> <li>3) Truss designed for wind loads</li> <li>Gable End Details as applicate</li> <li>4) TCLL: ASCE 7-16; Pr=20.0 ps</li> <li>1.15); Is=1.0; Rough Cat B; P</li> <li>applied to all exposed surface</li> <li>5) Unbalanced snow loads have</li> </ul>	ve been considered for this design. ph (3-second gust) Vasd=103mph; TCI (directional) and C-C Corner(3E) -1-0-0 to 36-4-8, Corner(3R) 36-4-8 to 41-11- a right exposed; C-C for members and for in the plane of the truss only. For stud- ble, or consult qualified building designes of (roof LL: Lum DOL=1.15 Plate DOL=- artially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 is with slopes less than 0.500/12 in acco been considered for this design. for greater of min roof live load of 12.0 loads. prevent water ponding. s otherwise indicated. ttom chord bearing.	to 4-4-8, Exterior(2N) 4 6, Exterior(2N) 41-11-6 rces & MWFRS for read s exposed to wind (norm r as per ANSI/TPI 1. 1.15); Pg=15.0 psf; Pf=1 , Lu=50-0-0; Min. flat roo protance with IBC 1608.3	-4-8 to 19-3-8, Cc to 55-7-4 zone; cc titions shown; Lurr nal to the face), s 6.5 psf (Lurn DOI of snow load gove 8.4.	orner(3R) 19-3-8 antilever left and ober DOL=1.60   ee Standard Ind _ = 1.15 Plate D erns. Rain surch	to right plate	Community Co	SEAL 036322	ERIT

- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSI/TPI Quality Criteria and DSB-22</u> available from Truss Plate Institute (www.tpinst.org) and <u>Before Building Component Scient Information</u> available from the Structural Building Component Science (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road Edenton, NC 27932

June 9,2025

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 74 ROOF		
					174047332		
25-3703-A	T03GE	Piggyback Base Supported Gable	2	1			
					Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	830 s May	29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:20 2025 Page 2		
		ID:tdHS5IWyLng?jaR9E1eBtqyly9tjh1LUwJ8Ou02amycZPDkCW4Dw2IVb9ZKOdYSFz86In					

#### NOTES-

12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

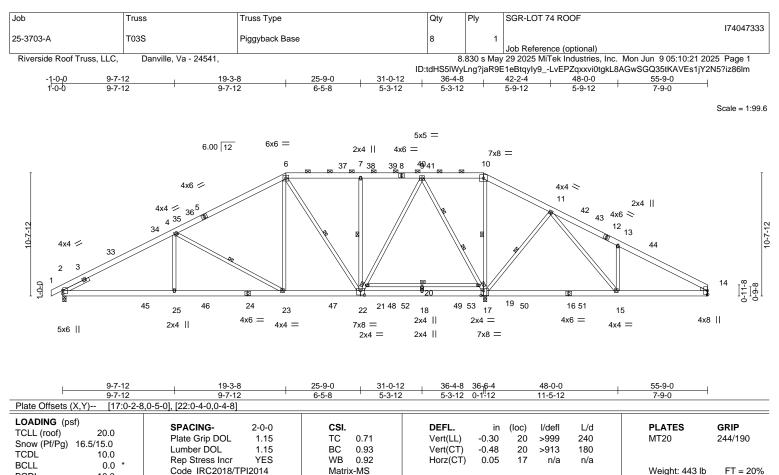
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 46, 47, 48, 49, 50, 52, 53, 56, 58, 59, 60, 61, 62, 63, 64, 44, 42, 41, 40, 39, 38, 37, 36, 35, 34.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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BCDL	10.0						
LUMBER-			B	RACING-			
TOP CHORD	2x6 SP No.2		т	OP CHORD	Structural wood sheathin	ng directly app	lied or 4-6-1 oc purlins, except
BOT CHORD	2x6 SP No.2 *E:			2-0-0 oc purlins (6-0-0 max.): 6-10.			
	19-21: 2x4 SP No.2		B	OT CHORD	Rigid ceiling directly app	c bracing. Except:	
WEBS	2x4 SP No.3 *E	xcept*			6-0-0 oc bracing: 19-21		
	9-17: 2x4 SP No	o.1	W	/EBS	1 Row at midpt	4-23, 6-22	, 7-22, 10-17, 11-17
WEDGE					2 Rows at 1/3 pts	9-19	

Right: 2x4 SP No.3

#### SLIDER Left 2x4 SP No.3 2-6-0

- REACTIONS. (size) 2=0-3-8, 14=Mechanical, 17=0-3-8 Max Horz 2=206(LC 15) Max Uplift 2=-106(LC 16), 14=-53(LC 16), 17=-24(LC 16) Max Grav 2=1600(LC 28), 14=444(LC 55), 17=3699(LC 29)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD
   2-4=-2424/210, 4-6=-1478/220, 6-7=-901/200, 7-9=-901/200, 9-10=0/939, 10-11=0/1122, 11-13=-462/218, 13-14=-470/184

   BOT CHORD
   2-25=-105/2228, 23-25=-105/2228, 22-23=0/1295, 18-22=-84/277, 17-18=-84/277, 10-18=-84/270, 10-18=-84/270, 1
- 2-23=103/2220, 22-23=0/1293, 16-22=-64/277, 17-16=-64/277 15-17=-555/52, 14-15=-129/341 WEBS 4-25=0/477, 4-23=-1088/173, 6-23=-11/780, 6-22=-794/99, 7-22=-459/126.
- 4-25=0/477, 4-23=-1060/173, 6-23=-11/760, 6-22=-194/99, 7-22=-459/126, 21-22=-34/1604, 9-21=0/1726, 9-19=-2068/126, 17-19=-2191/91, 10-17=-708/50, 11-17=-846/192, 11-15=-96/1099, 13-15=-419/185, 18-20=-255/0

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=56ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 4-6-14, Interior(1) 4-6-14 to 19-3-8, Exterior(2R) 19-3-8 to 27-2-2, Interior(1) 27-2-2 to 36-4-8, Exterior(2R) 36-4-8 to 44-3-2, Interior(1) 44-3-2 to 55-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

#### Confine the dog inder (s) for truss to truss connections

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[	Job	Truss	Truss Type	Qty	Ply	SGR-LOT 74 ROOF
						174047333
	25-3703-A	T03S	Piggyback Base	8	1	
						Job Reference (optional)
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	830 s May	29 2025 MiTek Industries, Inc. Mon Jun 9 05:10:21 2025 Page 2

ID:tdHS5IWyLng?jaR9E1eBtqyIy9\_-LvEPZqxxvi0tgkL8AGwSGQ35tKAVEs1jY2N5?iz86Im

NOTES-

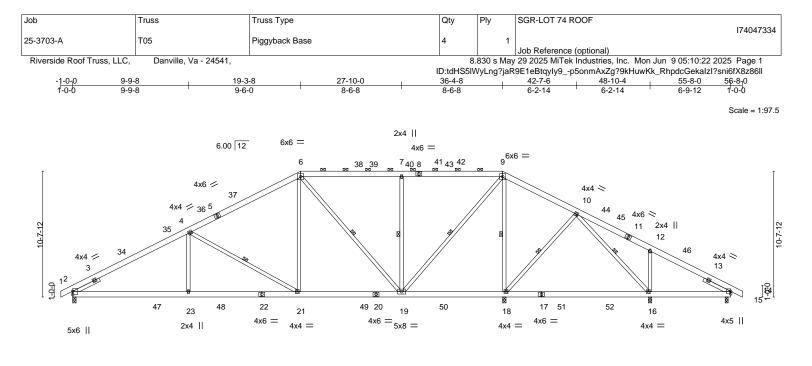
10) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 17 except (jt=lb) 2=106.
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) Graphical purplic protocontation does not depict the size as the origential code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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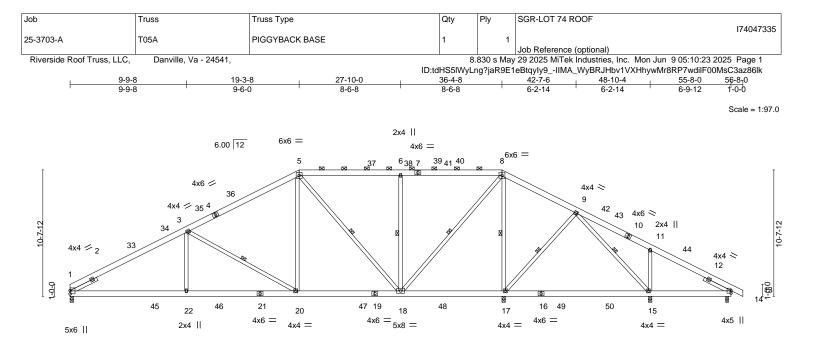


⊢	<u>9-9-8</u> 9-9-8	<u>19-3-8</u> 9-6-0	<u>27-10-0</u> 8-6-8	36-4-8 8-6-8	<u>36-6-4</u> 0-1-12	<u>48-10-4</u> 12-4-0	55-5-8	<u>55</u> -8-0 0-2-8
		3-0-0	0-0-0	0-0-0	0-1-12	12-4-0	0-7-4	0-2-0
LOADING (ps TCLL (roof)	st) 20.0	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP
Snow (Pf/Pg)		Plate Grip DOL 1.15	TC 0.71	Vert(LL)	-0.19 16-18	>784 240	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.70	Vert(CT)	-0.29 16-18	>505 180		
BCLL	0.0 *	Rep Stress Incr YES	WB 1.00	Horz(CT)	0.05 18	n/a n/a		
BCDL	10.0	Code IRC2018/TPI2014	Matrix-MS				Weight: 420 ll	b FT = 20%
LUMBER-		-	F	BRACING-				
TOP CHORD	2x6 SP No.2			TOP CHORD	Structural wood	sheathing directly ap	plied or 4-5-1 oc purli	ns. except
BOT CHORD	2x6 SP No.2					(6-0-0 max.): 6-9.	F	,
WEBS	2x4 SP No.3		E	BOT CHORD		ectly applied or 6-0-0	oc bracing.	
SLIDER	Left 2x4 SP No.	3 2-6-0, Right 2x4 SP No.3 2-6-0	١	WEBS	1 Row at midpt		9, 7-19, 9-19, 9-18, 10	0-18, 10-16
(lb) -	Max Grav All	08(LC 14) uplift 100 lb or less at joint(s) 16, 14 e reactions 250 lb or less at joint(s) exo 345(LC 55)	1 1 1	( /	5(LC 29),			
TOP CHORD BOT CHORD WEBS	2-4=-2540/261 2-23=-122/233 4-23=0/445, 4	lax. Ten All forces 250 (lb) or less c 1, 4-6=-1638/268, 6-7=-841/267, 7-9= 35, 21-23=-122/2335, 19-21=0/1442, -21=-1051/175, 6-21=0/928, 6-19=-9 96, 9-18=-1993/191, 10-18=-505/187	841/267, 9-10=0/624 18-19=-436/151, 16-18=-2 76/41, 7-19=-743/180,					
NOTES-								
1) Unbalanced	d roof live loads ha	ave been considered for this design.						
		nph (3-second gust) Vasd=103mph; 7						
		(directional) and C-C Exterior(2E) -1						
		6-4-8, Exterior(2R) 36-4-8 to 44-3-0,				osed ;		
	left and right expe	osed;C-C for members and forces &	MWFRS for reactions show	vn; Lumber DOL=1	1.60 plate grip			
DOL=1.60							TH CAR	11.
		sf (roof LL: Lum DOL=1.15 Plate DO				)L =	"TH CAR	Sugar.
		Partially Exp.; Ce=1.0; Cs=1.00; Ct=1 es with slopes less than 0.500/12 in a			ms. Rain surch	arge	R	Ling
		been considered for this design.	accordance with IBC 1008.	5.4.			FESS	7 Vie
		for greater of min roof live load of 12	2 0 psf or 1 00 times flat ro	of load of 11 6 pef	on overbangs	125	1 12	
	rent with other live			01 10 20 01 11.0 031	on overhangs	- N	R / /	· · · · ·
		prevent water ponding.					SEAL	
,		for a 10.0 psf bottom chord live load	I nonconcurrent with any of	ther live loads		E 1	SEAL	1 1
8) * This truss	has been designe	ed for a live load of 20.0psf on the bo ord and any other members, with BC	ttom chord in all areas whe		-0 tall by 2-0-0 v	vide	036322	11
	chanical connection	on (by others) of truss to bearing plat		100 lb uplift at joint	(s) 16, 14 excep	t III	N. END - CE	1.3
v /	,						A CHUNNER	
	d standard ANSI/T	cordance with the 2018 International PI 1.	Residential Code sections	R502.11.1 and R8	02.10.2 and		SICINES	AL TIM
referenced	d standard ANSI/T					vide It	A. GIL	

June 9,2025

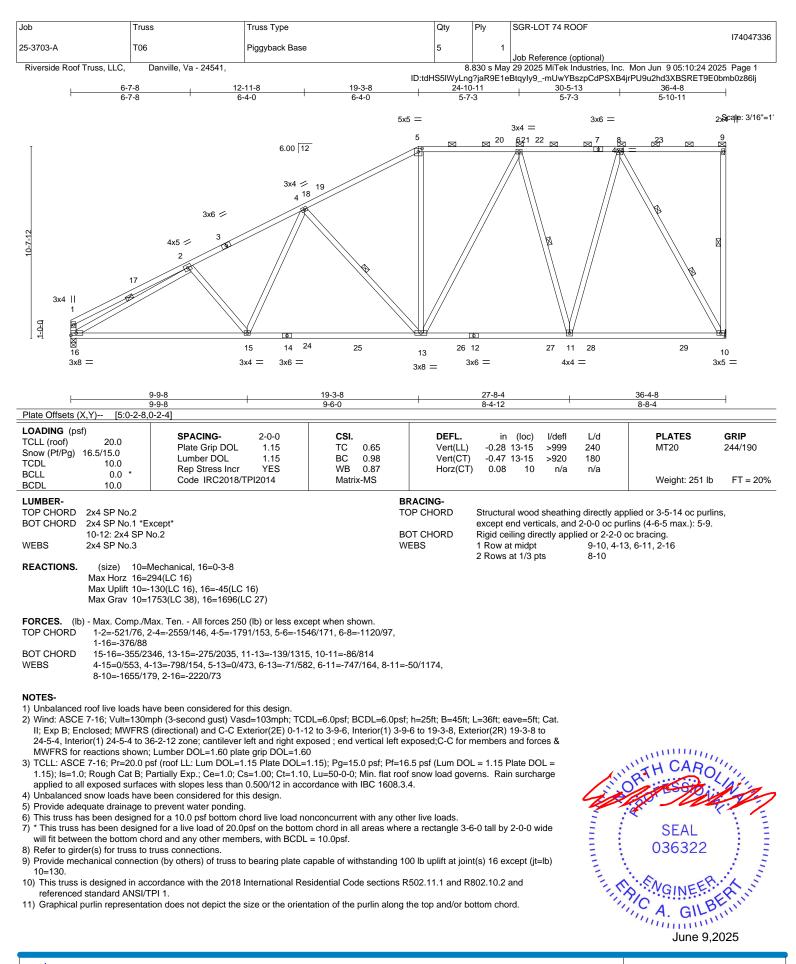
PREPARENCE BY A MITEK Affiliate 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

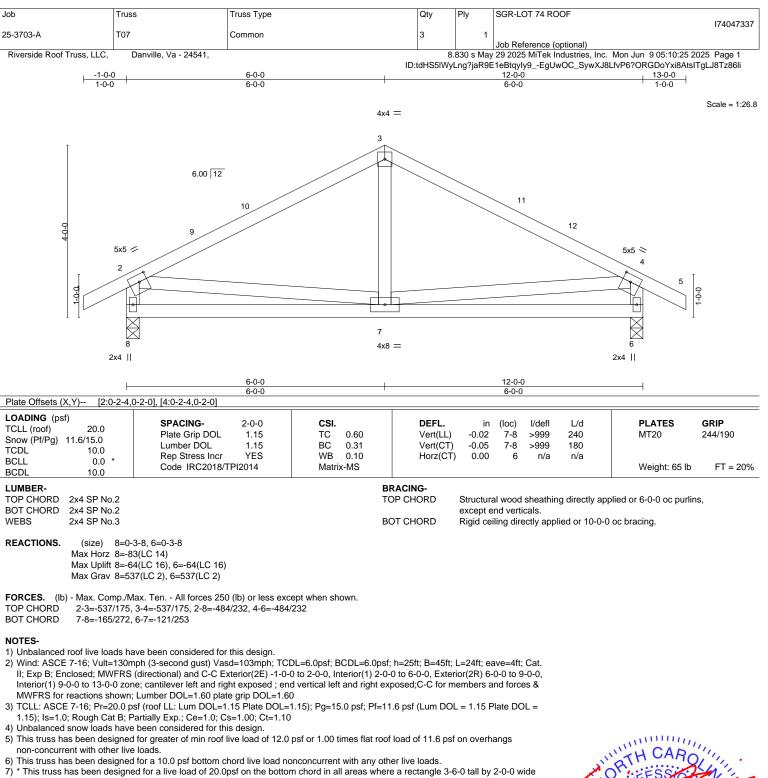


9-9-8	<u> </u>	27-10-0 8-6-8	36-4-8 8-6-8	36-6-4 0-1-12	48-10-4 12-4-0		5-8-0 0-2-8
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         16.5/15.0           TCDL         10.0           BCLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.71 BC 0.69 WB 1.00 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 15-17 -0.29 15-17 0.05 17	l/defl L/d >784 240 >505 180 n/a n/a	PLATES MT20 Weight: 418 lb	<b>GRIP</b> 244/190 FT = 20%
REACTIONS. All bearings 0- (Ib) - Max Horz 1=-20 Max Uplift All 0 Max Grav All 1	3 2-6-0, Right 2x4 SP No.3 2-6-0 3-8 except (jt=length) 13=0-3-0. 05(LC 14) uplift 100 lb or less at joint(s) 1, 15, 13 e reactions 250 lb or less at joint(s) excep 145(LC 55)	T B W except 17=-103(LC 16)	OT CHORD VEBS	2-0-0 oc purlins Rigid ceiling dire 1 Row at midpt	(6-0-0 max.): 5-8. ectly applied or 6-0-0 o	plied or 4-5-2 oc purlins oc bracing. 8, 6-18, 8-18, 8-17, 9-1	
TOP CHORD         1-3=-2547/266           BOT CHORD         1-22=-122/234           WEBS         3-22=0/446, 3-	lax. Ten All forces 250 (lb) or less exc 5, 3-5=-1642/274, 5-6=-842/271, 6-8=-8- 11, 20-22=-122/2341, 18-20=0/1446, 17 -20=-1054/176, 5-20=0/929, 5-18=-975/ 95, 8-17=-1991/191, 9-17=-504/186, 9-1	-18=-433/151, 15-17=-25 /40, 6-18=-744/181,					
<ol> <li>Wind: ASCE 7-16; Vult=130m II; Exp B; Enclosed; MWFRS 27-2-0, Interior(1) 27-2-0 to 3 end vertical left and right expo DOL=1.60</li> <li>TCLL: ASCE 7-16; Pr=20.0 p: 1.15); Is=1.0; Rough Cat B; P applied to all exposed surface</li> <li>Unbalanced snow loads have</li> <li>This truss has been designed non-concurrent with other live</li> <li>Provide adequate drainage to 7) This truss has been designed</li> <li>*This truss has been designed</li> <li>This truss has been designed</li> <li>This truss has been designed</li> <li>This truss is been designed</li> <li>*This truss has been designed</li> </ol>	p prevent water ponding. I for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the botton ord and any other members, with BCDL on (by others) of truss to bearing plate o cordance with the 2018 International Re	0 to 5-6-13, Interior(1) 5-6 erior(1) 44-3-0 to 56-8-0 VFRS for reactions show 1.15); Pg=15.0 psf; Pf=11 , Lu=50-0-0; Min. flat roc ordance with IBC 1608.3 psf or 1.00 times flat roo onconcurrent with any oth m chord in all areas when = 10.0psf. capable of withstanding 1 sidential Code sections F	5-13 to 19-3-8, Ext zone; cantilever le rn; Lumber DOL=1 6.5 psf (Lum DOL of snow load gover 6.4. If load of 11.6 psf her live loads. re a rectangle 3-6- 00 lb uplift at joint R502.11.1 and R8	erior(2R) 19-3-8 ff and right expo .60 plate grip = 1.15 Plate DC ns. Rain surchan on overhangs 0 tall by 2-0-0 w (s) 1, 15, 13 exco 02.10.2 and	to used ; uL = urge ide ept	SEAL 036322	

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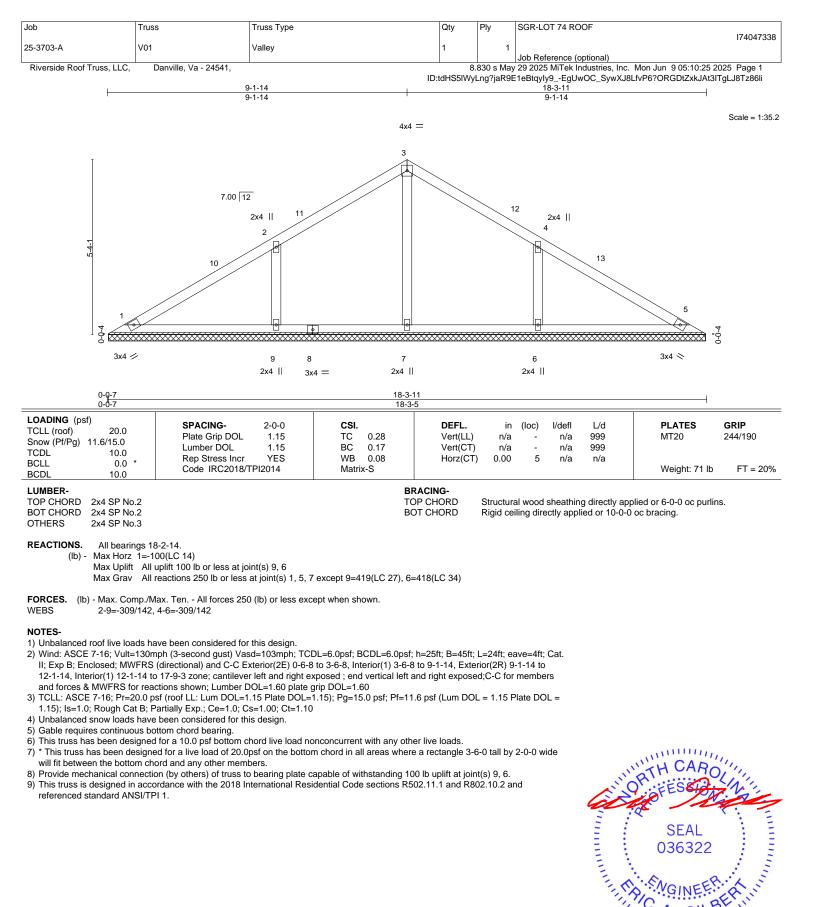


will fit between the bottom chord and any other members. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



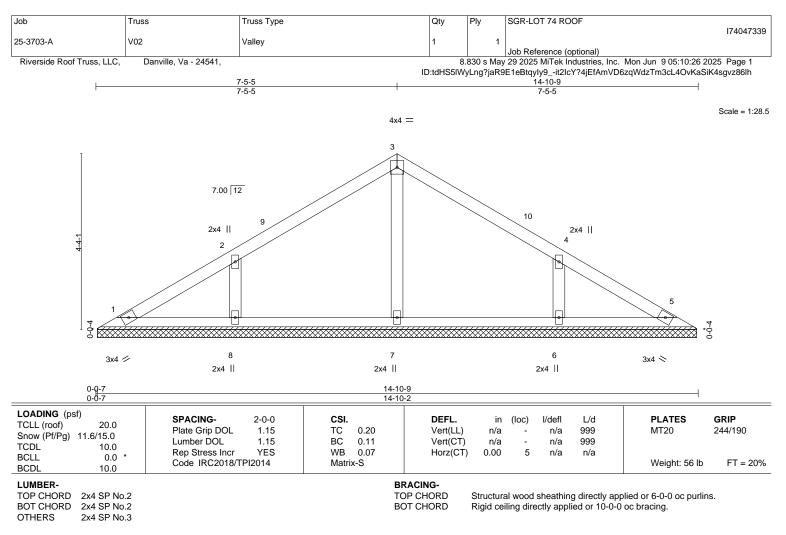
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GILU.... June 9,2025



REACTIONS. All bearings 14-9-11. (Ib) - Max Horz 1=80(LC 15

- Max Horz 1=80(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=269(LC 2), 8=328(LC 33), 6=328(LC 34)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-5-5, Interior(1) 3-5-5 to 7-5-5, Exterior(2R) 7-5-5 to 10-5-5, Interior(1) 10-5-5 to 14-4-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

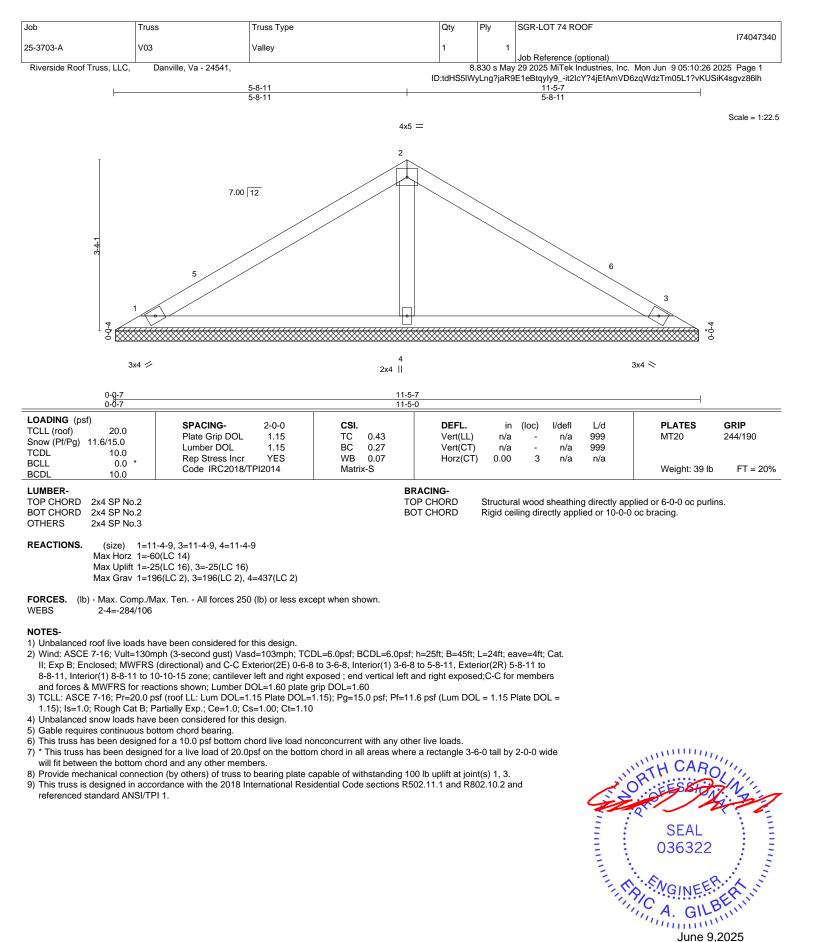
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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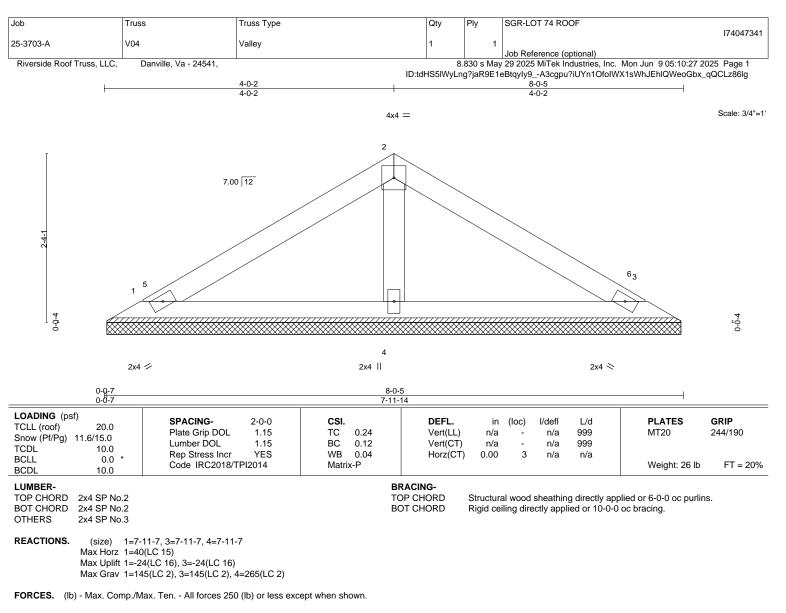


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TRENCO A Mi Tek Affilia 818 Soundside Road



#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-0-2, Exterior(2R) 4-0-2 to 7-0-2, Interior(1) 7-0-2 to 7-5-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

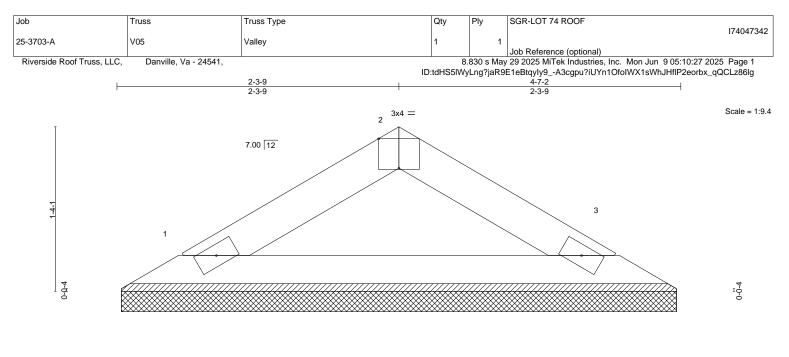
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818 Soundside Road



2x4 💋

2x4 📎

Structural wood sheathing directly applied or 4-7-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

0- <u>0-</u> 7 0-0-7		4-7-2 4-6-1						
Plate Offsets (X,Y) [2:0-2-0,	Edge]	+01	•					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 POUL 0.0 t	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.05 BC 0.15 WB 0.00	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (l- n/a n/a 0.00	- n	efl L/d /a 999 /a 999 /a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-P					Weight: 13 lb	FT = 20%
LUMBER-		BR	ACING-					

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=4-6-5, 3=4-6-5 Max Horz 1=-20(LC 14) Max Uplift 1=-9(LC 16), 3=-9(LC 16) Max Grav 1=141(LC 2), 3=141(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

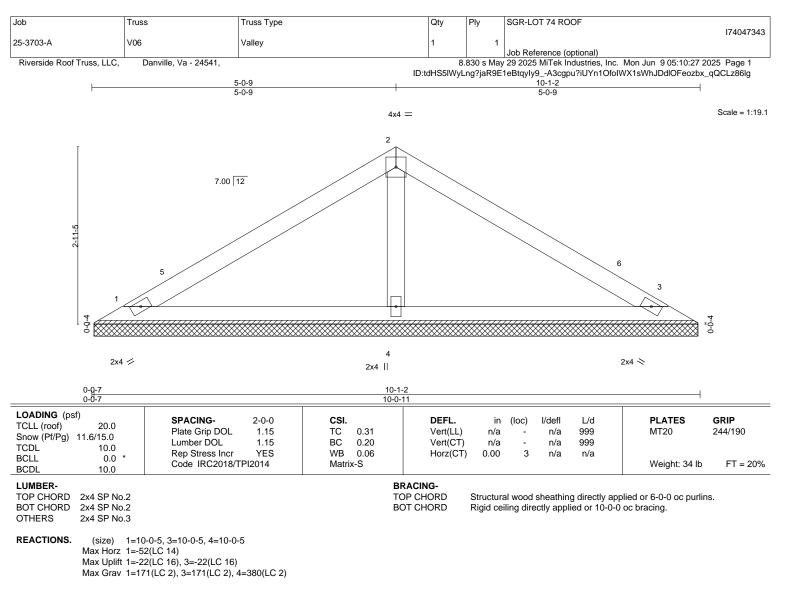
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818 Soundside Road



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

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2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 5-0-9, Exterior(2R) 5-0-9 to 8-0-9, Interior(1) 8-0-9 to 9-6-10 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

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5) Gable requires continuous bottom chord bearing.

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8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

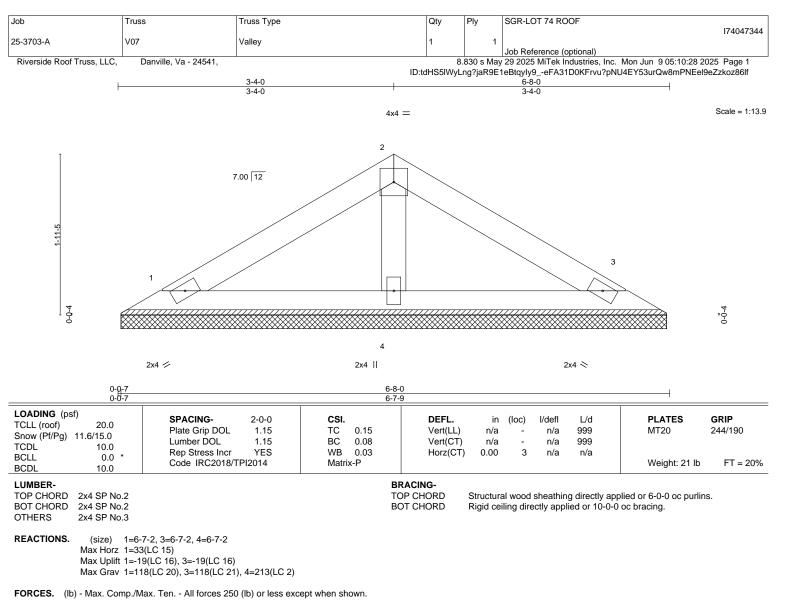
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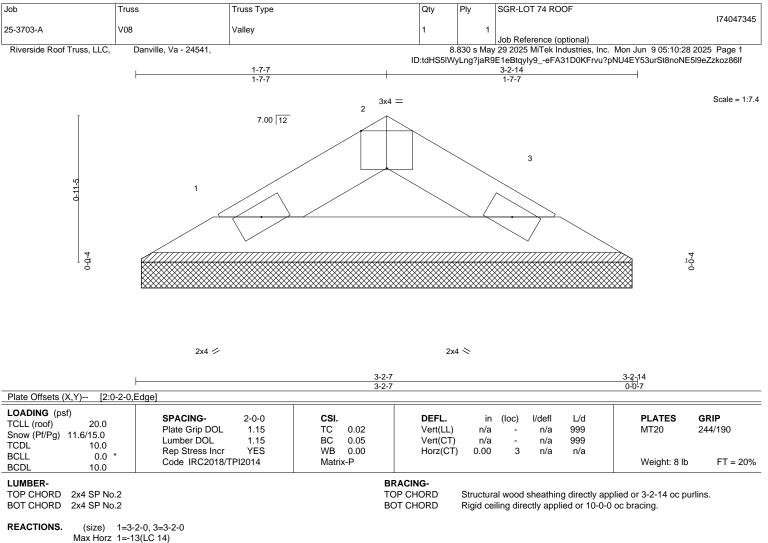
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Max Uplift 1=-5(LC 16), 3=-5(LC 16)

Max Grav 1=86(LC 2), 3=86(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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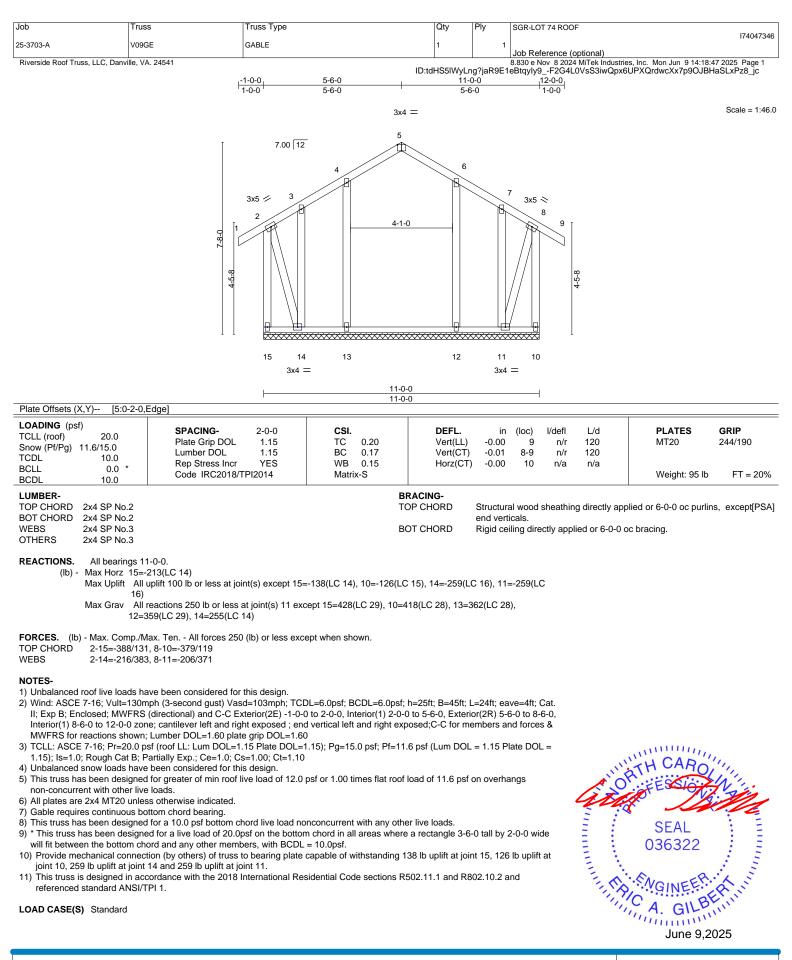
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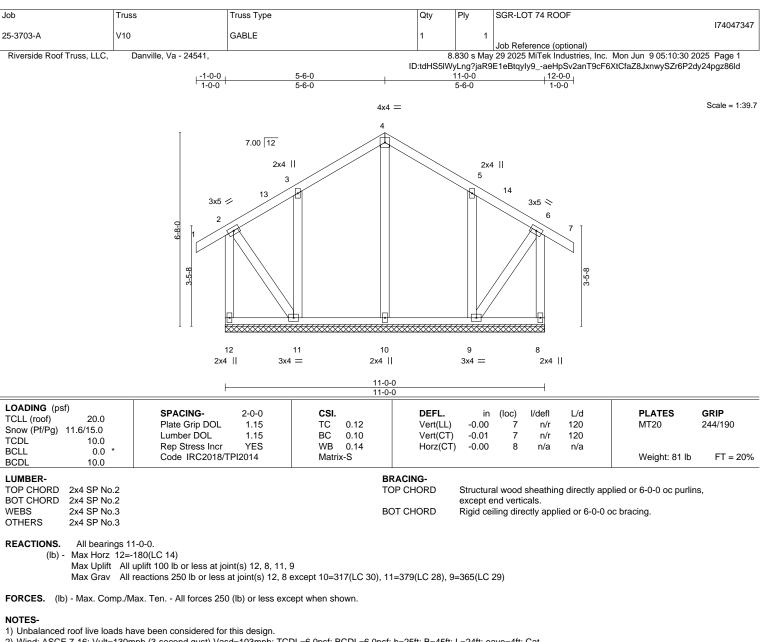
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818 Soundside Road Edenton, NC 27932



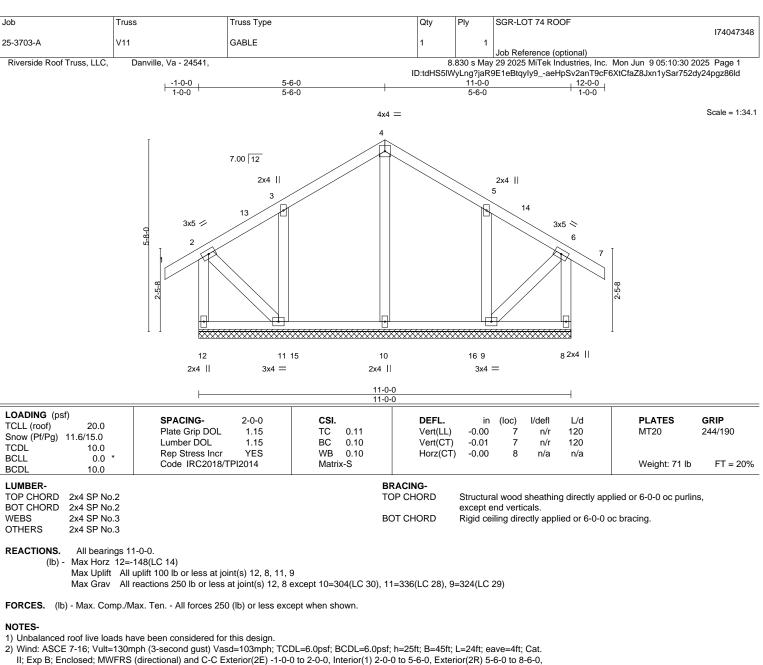
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 5-6-0, Exterior(2R) 5-6-0 to 8-6-0, Interior(1) 8-6-0 to 12-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 8, 11, 9.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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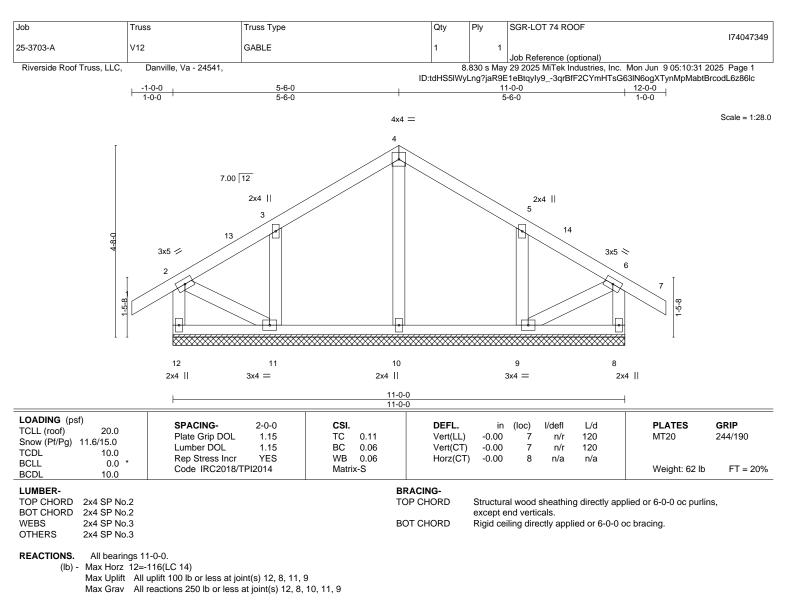


- In Exp B, Enclosed, MWFRS (directional) and C-C Extend(22) 1-0-0 to 2-0-0, intend(1) 2-0-0 to 5-6-0, Extend(2R) 5-6-0 to 3-6-0. Interior(1) 8-6-0 to 12-0-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

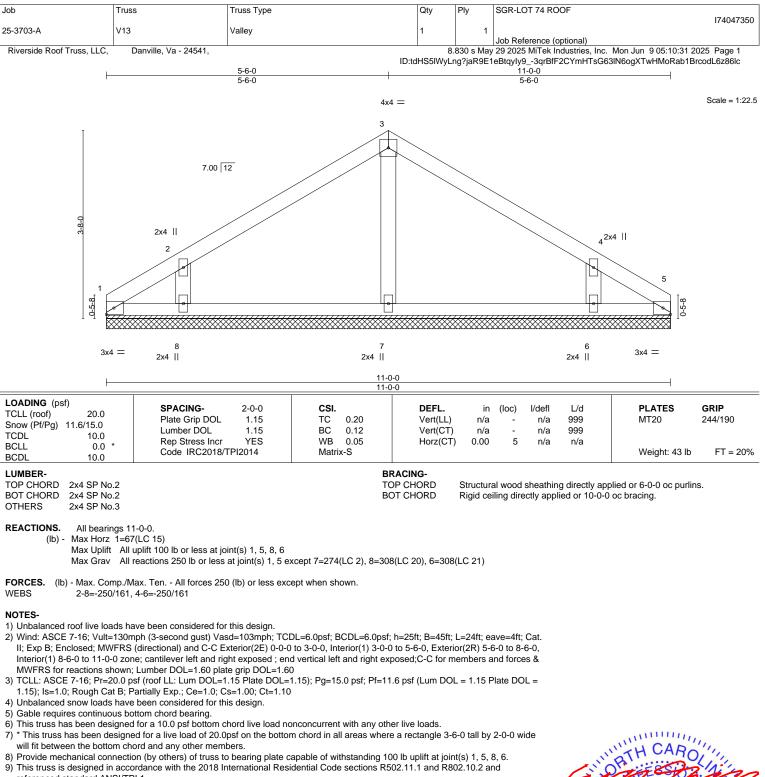
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 5-6-0, Exterior(2R) 5-6-0 to 8-6-0, Interior(1) 8-6-0 to 12-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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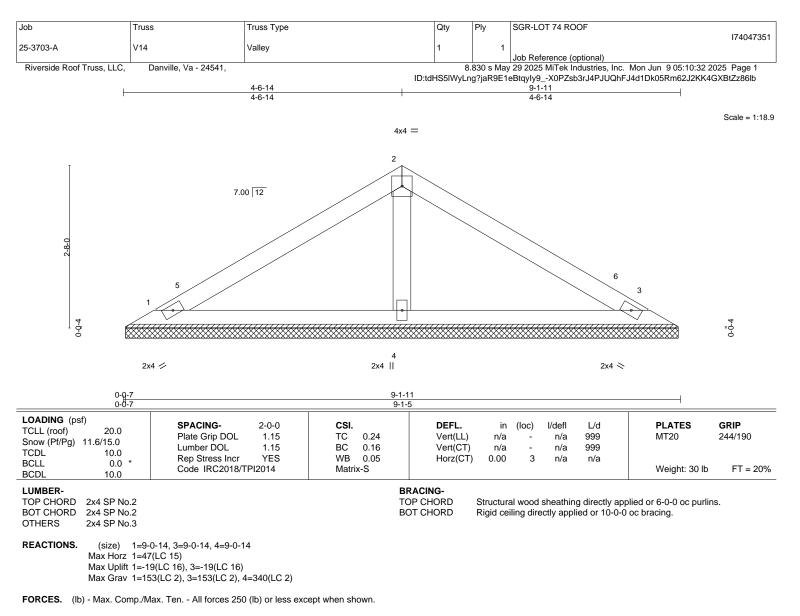
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818 Soundside Road



#### NOTES-

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2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-6-14, Exterior(2R) 4-6-14 to 7-6-14, Interior(1) 7-6-14 to 8-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

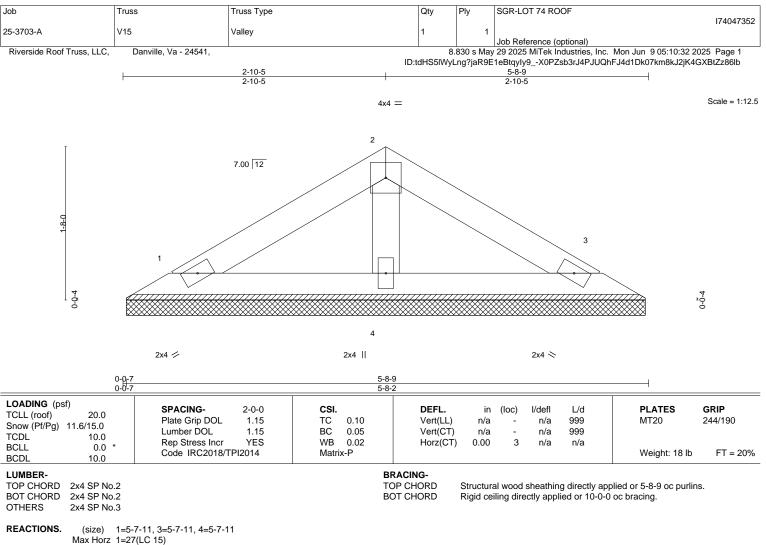
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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A MiTek Affili 818 Soundside Road



Max Horz 1=27(LC 15)Max Uplift 1=-16(LC 16), 3=-16(LC 16)

Max Grav 1=97(LC 2), 3=97(LC 2), 4=177(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

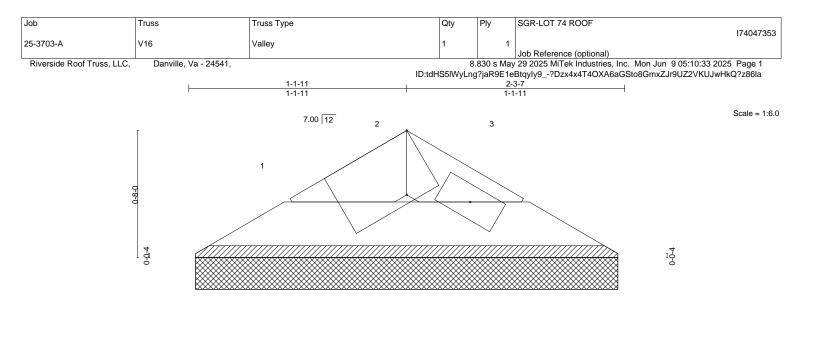
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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818 Soundside Road



4x6 💋

2x4 🔍

2-3-7 0<u>101</u>7 0-0-7 2-3-0 Plate Offsets (X,Y)-- [1:0-2-1,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 тс 0.01 Vert(LL) 999 MT20 244/190 n/a n/a Snow (Pf/Pg) 11.6/15.0 Lumber DOL 1.15 BC 0.02 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 \* Code IRC2018/TPI2014 FT = 20% Matrix-P Weight: 5 lb

BRACING-

TOP CHORD

BOT CHORD

# BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=2-2-9, 3=2-2-9 Max Horz 1=-7(LC 14) Max Uplift 1=-3(LC 16), 3=-3(LC 16) Max Grav 1=48(LC 2), 3=48(LC 2)

10.0

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



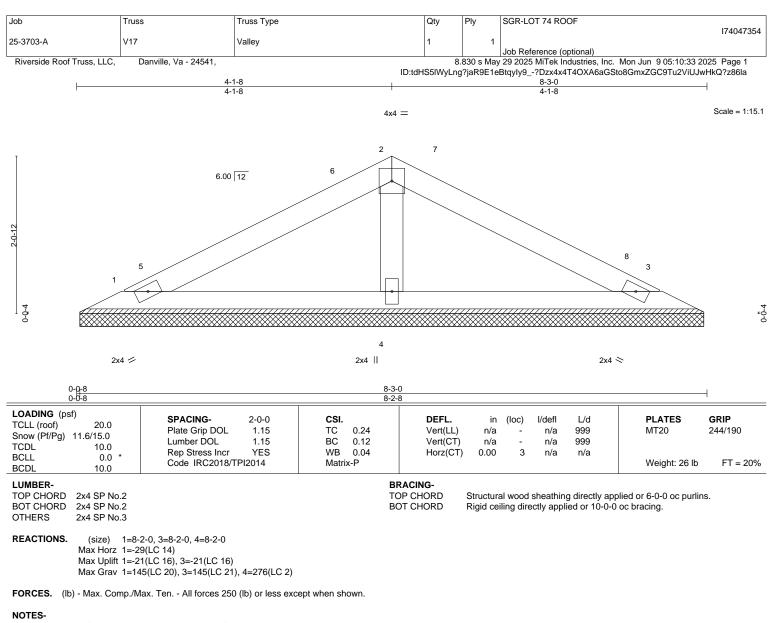
Structural wood sheathing directly applied or 2-3-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road



1) Unbalanced roof live loads have been considered for this design.

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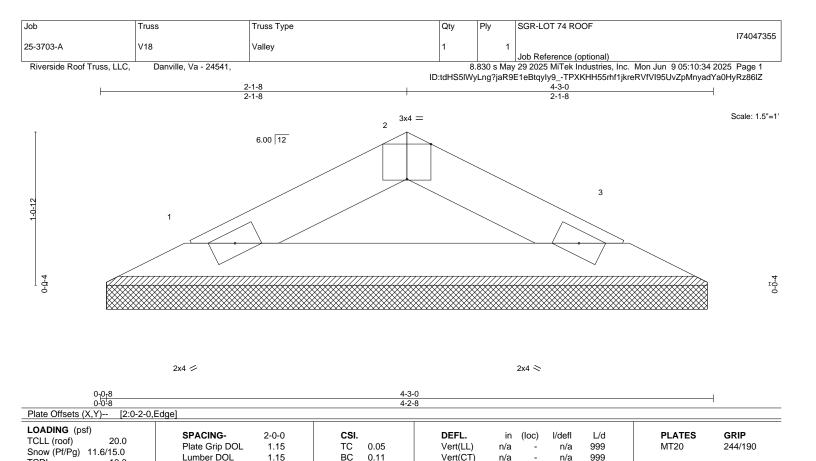
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818 Soundside Road Edenton, NC 27932



FORCES.	(lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
NOTES-	

TCDL

BCLL

BCDL

LUMBER-

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

1=4-2-0, 3=4-2-0

Max Uplift 1=-7(LC 16), 3=-7(LC 16) Max Grav 1=120(LC 2), 3=120(LC 2)

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

YES

WB

Matrix-P

0.00

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.00

3

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

Structural wood sheathing directly applied or 4-3-0 oc purlins.

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

10.0

10.0

(size) Max Horz 1=12(LC 15)

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

0.0 \*

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Rep Stress Incr

Code IRC2018/TPI2014

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

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FT = 20%

Weight: 11 lb

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