

RE: 25-3702-A SGR-LOT 78 ROOF Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer:Project Name: 25-3702-ALot/Block:MorAddress:SubCity:Sta

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.7 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	171333174	PB01	2/11/2025	21	171333194	V07	2/11/2025
2	171333175	PB01GE	2/11/2025				
3	171333176	T01	2/11/2025				
4	171333177	T01GE	2/11/2025				
5	171333178	T02	2/11/2025				
6	171333179	T03	2/11/2025				
7	171333180	T03AS	2/11/2025				
8	171333181	T03GE	2/11/2025				
9	171333182	T03S	2/11/2025				
10	171333183	T04	2/11/2025				
11	171333184	T04G	2/11/2025				
12	171333185	T04GE	2/11/2025				
13	171333186	T05	2/11/2025				
14	171333187	T05GE	2/11/2025				
15	171333188	V01	2/11/2025				
16	171333189	V02	2/11/2025				
17	171333190	V03	2/11/2025				
18	171333191	V04	2/11/2025				
19	171333192	V05	2/11/2025				
20	171333193	V06	2/11/2025				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Riverside Roof Truss.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





February 11,2025



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



		12-0-0						
		12-0-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.09 BC 0.05 WB 0.05 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lo n/a n/a 0.00	oc) l/defl - n/a - n/a 6 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 39 lb	GRIP 244/190 FT = 20%
LUMBER-	1	BR	ACING-					

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. All bearings 12-0-0.

Max Horz 1=30(LC 15) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 2, 6, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) 0-5-7 to 3-5-7, Exterior(2N) 3-5-7 to 6-0-0, Corner(3R) 6-0-0 to 9-0-0, Exterior(2N) 9-0-0 to 11-6-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) Gable requires continuous bottom chord bearing.

- Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 9) will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 2, 6, 10, 8. 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

- referenced standard ANSI/TPI 1.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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7-11-4	15-10-8	23-9-12	30-6-0	36-6-0	47-2-2		57-10	-4 60-8-8	3	
/-11-4	/-11-4	/-11-4	0-0-4	0-0-0	10-0-2		10-8	-2 2-10-4	+	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC 0.50	DEFL. Vert(LL)	in (loc) -0.16 17-19	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190	
TCDL 10.0 BCLL 0.0 *	Rep Stress Incr Code IRC2018/	1.15 YES TPI2014	WB 0.85 Matrix-MS	Horz(CT)	-0.28 17-19 0.04 15	>999 n/a	180 n/a	Weight: 490 lb	FT = 20%	
BCDL 10.0										
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.	3 2-6-0, Right 2x4 SP N	lo.3 2-6-0		BRACING- TOP CHORD BOT CHORD WEBS	RD Structural wood sheathing directly applied or 5-8-6 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 7-9. RD Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 3-23, 5-22, 6-22, 7-22, 8-21, 9-21, 10-19, 12-16					
REACTIONS. All bearings 0-3-8 except (jt=length) 15=Mechanical. (lb) - Max Horz 1=192(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 1, 16, 15 except 22=-175(LC 16) Max Grav All reactions 250 lb or less at joint(s) 15 except 1=725(LC 27), 16=1733(LC 28), 22=3282(LC 27)										
FORCES. (lb) - Max. Comp.//W TOP CHORD 1-3=-962/116, 0.10-055/222	1ax. Ten All forces 25 3-5=-286/126, 5-6=0/9	0 (lb) or less exce 56, 6-7=0/641, 7-8	pt when shown. 3=-365/202, 8-9=-365/	202,						
BOT CHORD 1-25=-30/996, 16-17=-85/146	23-25=-30/996, 21-22= 51	-686/187, 19-21=	0/798, 17-19=-37/127	6,						
VEBS 3-25=0/311, 3-23=-903/133, 5-23=0/726, 5-22=-1211/152, 6-22=-849/214, 7-22=-1476/81, 7-21=-146/1755, 8-21=-507/136, 9-21=-1141/74, 9-19=-21/1011, 10-19=-839/166, 10-17=0/480, 12-16=-1774/139, 13-16=-398/186										
 WEBS 3-25=0/311, 3-23=-903/133, 5-23=0/726, 5-22=-1211/152, 6-22=-849/214, 7-22=-1476/81, 7-21=-146/1755, 8-21=-507/136, 9-21=-1141/174, 9-19=-21/1011, 10-19=-839/166, 10-17=0/480, 12-16=-1774/139, 13-16=-398/186 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=61ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 6-0-14, Interior(1) 6-0-14 to 24-6-0, Exterior(2R) 24-6-0 to 33-1-0, Interior(1) 33-1-0 to 36-6-0, Exterior(2R) 36-60 to 45-1-0, Interior(1) 45-1-0 to 60-8-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.5) [s=1-0; Rough Cat B; Partially Exp:: Ce=1.0; Cs=-1.0; Cl=-1.0, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) Unbalanced snow loads have been considered for this design. 5) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restrainbbracing and the permanent individual truss member restrainbracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. 6) Provide adequate drainage to prevent water ponding. 6) This truss has been designed for a live load of 20.0psf on th										



Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF	
						71333176
25-3702-A	T01	Piggyback Base	6	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:00 2025 F	Page 2
		ID:07Iz9	xtbY5He8	mIUVbML	?fzV1Kn-aanp9ShHek93DfHDTIAhsXsmMowbDAej2KYksFz	mVeH

10) Bearing at joint(s) 22 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 16, 15 except (jt=lb) 22=175.
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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		00-0-0							
Plate Offsets (X,Y) [16:0-3-	0,0-4-4], [22:0-3-0,0-4-4], [45:0-3-0,0-1-4	4], [54:0-3-0,0-1-4]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.18 BC 0.17 WB 0.18 Matrix-S	DEFL. ir Vert(LL) 0.00 Vert(CT) 0.00 Horz(CT) -0.01	n (loc) l/defl) 1 n/r) 1 n/r 38 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 580 lb	GRIP 244/190 FT = 20%		
			ACING						
LUMBERTOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.3OTHERS2x4 SP No.3SLIDERLeft 2x4 SP No	0.3 1-6-5	TO BC WE	P CHORD Structu except T CHORD Rigid c EBS 1 Row	ral wood sheath end verticals, ar eiling directly ap at midpt	ing directly app Id 2-0-0 oc purl plied or 6-0-0 o 22-49, 21- 16-56, 15-	lied or 6-0-0 oc purlins ins (10-0-0 max.): 16-2 c bracing. 50, 20-51, 19-52, 18-5 57, 14-58, 23-48, 24-4	, 22. 4, 17-55, 7		
REACTIONS. All bearings 58-0-0. (lb) - Max Horz 2=212(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 2, 51, 52, 54, 57, 58, 59, 60, 62, 63, 64, 65, 66, 67, 68, 48, 47, 46, 45, 43, 42, 41, 40, 39, 38 Max Grav All reactions 250 lb or less at joint(s) 2, 49, 50, 51, 52, 54, 55, 56, 57, 58, 59, 60, 62, 63, 64, 65, 66, 67, 48, 47, 46, 45, 43, 42, 41, 40, 39 except 68=259(LC 28), 38=542(LC 2)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-157/296, 4-5=-126/259, 5-6=-120/258, 6-7=-111/251, 14-15=-91/278, 15-16=-104/310, 16-17=-99/303, 17-18=-99/303, 18-19=-99/303, 19-20=-99/303, 20-21=-99/303, 21-22=-99/303, 22-23=-104/310, 23-24=-91/278 WEBS 33-38=-284/214									
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=61ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -011-0 to 5-1-14, Exterior(2N) 5-1-14 to 24-6-0, Corner(3R) 24-6-0 to 30-6-0, Exterior(2R) 30-6-0 to 36-6-0, Corner(3R) 36-6-0 to 42-6-0, Exterior(2N) 42-6-0 to 60-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Cable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15; Is=1-0; Rough Cat B; Partially Exp.; Ce=-1.0; Cs=-1.00; Ct=-1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 5) Unbalanced snow loads have been considered for this design. 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads. 7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses (FGSCY), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes									
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building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF	
					171	333177
25-3702-A	T01GE	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
Riverside Roof Truss 11 C	Danville, Va - 24541		8	730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:03 2025 Pac	ae 2

ID:07lz9xtbY5He8mIUVbML?fzV1Kn-_9TxnUj9xfYe46?o8tkOU9UMZ?2uQik9kImOTazmVeE

NOTES-

8) Provide adequate drainage to prevent water ponding.

9) All plates are 2x4 MT20 unless otherwise indicated.

10) Gable studs spaced at 2-0-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 51, 52, 54, 57, 58, 59, 60, 62, 63, 64, 65, 66, 67, 68, 48, 47, 46, 45, 43, 42, 41, 40, 39, 38.

14) N/A

15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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	8-2-0	16-4-0	23-8-0	24-6-0	30-6-0	36-6-0	43-8-	-2	50	-10-4	60-8-8	
Plate Offsets (X Y)-		0-2-0	7-4-0	0-10-0	6-0-0	6-0-0	1-2	2	1	-2-2	9-10-4	
	[21.0 0 0,	020]										
LOADING (pst)	20.0	SPACING-	2-0-0	CS	Ι.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
I ULL (FOOT) Show (Pf/Pa) 16 5	20.0	Plate Grip DOL	1.15	тс	0.47	Vert(LL)	-0.06	15-31	>999	240	MT20	244/190
	10.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.13	15-31	>928	180		
BCU	0.0 *	Rep Stress Incr	YES	WE	3 0.92	Horz(CT) 0.03	15	n/a	n/a		
BCDL	10.0	Code IRC2018/TF	PI2014	Ma	trix-MS						Weight: 473 lb	FT = 20%
							Ctructur		abaathin	a directly on		avaant
	SP NO.2					TOP CHORD		al wood	(6 0 0 m		plied of 6-0-0 oc putlins	, except
WEBS 2v4	ISP No.2						Rigid ce	ilina dir	ectly annl	ax.). 7-9. ied or 10-0-() oc bracing Except:	
SLIDER Left	t 2x4 SP No 3	2-6-0 Right 2x4 SP No	3 2-6-0			BOT ONORD	6-0-0 00	hracing	19-21 r		o oc bracing, Except.	
OLIDEIX LON			.0200			WEBS	1 Row a	at midpt	g. 10 <u>2</u> 1.	4-22.6-2	1. 7-21. 8-19. 9-19. 10-	18
REACTIONS. AI	II bearings 0-3	3-8 except (it=length) 14	=Mechanical.							,	,,,,	
(lb) - Ma	ax Horz 2=198	B(LC 15)										
Ma	ax Uplift All u	plift 100 lb or less at join	nt(s) 2, 15, 14 e	except 21=	-143(LC 16)	1						
Ma	ax Grav All re	eactions 250 lb or less a	t joint(s) excep	t 2=964(L0	C 28), 21=2	793(LC 28), 15=13	70(LC 29)),				
	14=59	94(LC 29)										
FORCES. (lb) - M	lax. Comp./Ma	ax. Ten All forces 250	(lb) or less exc	ept when s	shown.							
TOP CHORD 2-	-4=-1325/159,	, 4-6=-598/152, 6-7=0/57	(4, 7-8=-422/2)	32, 8-9=-42	22/232, 9-10)=-831/245,						
	0-12=-1068/22	26, 12-14=-500/195	0/567 10 21	274/144	19 10 0/60	10						
17 17	-24=-09/1334, 7 19_ 20/99/	15 17 97/151 11 15-	=0/307, 19-21= 97/461	=-374/141,	10-19=0/09	<i>1</i> 2,						
WEBS 4-	FRS 4.24-0/316 4.22873/117 6.22-0/201 6.211264/170 7.211614/190											
7-	-19=-125/1302	2 8-19=-489/132 9-19=	-751/57 9-18=	-11/608 1	0-18=-407/1	121						
12	2-17=0/572. 1	2-15=-1002/161		,	0.00.000	,						
	,											
NOTES-												
1) Unbalanced roof	live loads hav	ve been considered for t	his design.									
 Wind: ASCE 7-16 	6; Vult=130mp	ph (3-second gust) Vasd	l=103mph; TCl	DL=6.0psf;	BCDL=6.0	osf; h=25ft; B=45ft;	L=61ft; e	ave=7ft	; Cat.		MITTIN	
II; Exp B; Enclose	ed; MWFRS (directional) and C-C Ext	erior(2E) -0-11	-0 to 5-1-1	4, Interior(1) 5-1-14 to 24-6-0,	Exterior(2	2R) 24-6	6-0 to	-	"TH CARO	111
33-1-0, Interior(1) 33-1-0 to 36	6-6-0, Exterior(2R) 36-6-0	0 to 45-1-0, Int	erior(1) 45	-1-0 to 60-8	-8 zone; cantilever	left and ri	ight exp	osed ;	5	R	Sill.
	and right expo	sed;C-C for members an	nd forces & iviv	VFK5 101 1	eactions sho	Swn; Lumber DOL=	1.60 plat	e gnp			O FESSION	12 in
3) TCLL: ASCE 7-1	6. Pr-20.0 ps	f (roof LL : Lum DOL =1.1	15 Plate DOI -	1 15)· Da-	15 0 pef. Pf.	-16 5 pef (Lum DO	I _1 15 DI	lato		UT		
DOI =1 15) Is=1	0. Rough Cat	t B: Partially Exp · Ce=1	0. Cs=1 00. C	t=1 10 Lu:	=50-0-0: Mir	flat roof snow loa	d governs	s Rain		50	2	C
surcharge applie	d to all expos	ed surfaces with slopes	less than 0.50)/12 in acc	ordance wit	h IBC 1608.3.4.	a goronn	or reality			SEAL	: =
4) Unbalanced snov	w loads have l	been considered for this	design.							= :	JLAL	÷ =
5) This truss has be	een designed	for greater of min roof liv	e load of 12.0	psf or 1.00) times flat r	oof load of 11.6 pst	f on overh	nangs		- E - 1	036322	- E E
non-concurrent w	with other live	loads.										: :
WARNING: This	s long span tru	uss requires extreme car	re and experier	nce for pro	per and safe	e handling and erec	tion. For	general			· ~ ~	1. 2
handling and ere	ection guidanc	e, see Guide to Good Pi	ractice for Han	dling, Insta	Illing & Brac	ing of Metal Plate (Connecte	d Wood		11	NGINEEN	123
Trusses ("BCSI")), jointly produ	iced by SBCA and TPI.	I he building ov	vner or the	owner's au	thorized agent sha	Il contract	t with a		11	20	1 N N
qualified register	ed design pro	ressional for the design	and inspection	of the tem	iporary insta	allation restraint/bra	cing and	the	n or	1	A GILB	in the
bracing	idual truss me	mber restraint/bracing.	will ek assume	s no respu		iruss manufacture,	nanuling,	, erecuo	n, or		111111111	A.
 7) Provide adequate 	e drainage to	prevent water ponding									February 11.2	025
(8) nFiniseruss has be	en designed	for a 10.0 psf bottom ch	ord live load no	onconcurre	ent with any	other live loads.						020
Commence pages					and any							
🛕 WARNING - Ve	erify design param	eters and READ NOTES ON TH	HIS AND INCLUDE	D MITEK REF	ERENCE PAGE	E MII-7473 rev. 1/2/2023	BEFORE U	SE.			ENGINEERING BY	<u></u>
Design valid for u	use only with MiTe	ek® connectors. This design is	based only upon p	parameters sh	hown, and is for	r an individual building c	omponent, r	not				-n
building design.	Bracing indicated	is to prevent buckling of indivi	idual truss web and	l/or chord me	mbers only. A	dditional temporary and	permanent l	bracing				بوب
is always require	ed for stability and	to prevent collapse with possil	ble personal injury	and property	damage. For o	general guidance regard	ing the	- Plate In	etituto (vara	v toinet ora)	A MiTek	Anillate
and BCSI Build	ling Component	Safety Information available	from the Structura	Building Co	mponent Assoc	ciation (www.sbcacompo	nents.com)			p.not.org/	818 Soundside Road Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF	
						71333178
25-3702-A	T02	Piggyback Base	3	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:04 2025 F	Page 2
		ID:07lz9	9xtbY5He8	BmIUVbML	.?fzV1Kn-SM1J_qknhzgVhGa_iaFd1N0RtPKW9zXlzyWy?0zi	mVeD

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 10) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15, 14 except (jt=lb) 21=143.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust structure Bucking Component Advancement and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Scale = 1:106.8



	8-4-5	16-5-3	24-6-0	30-6-0	39-9-12	46-9-5	5 53-8-15	61-0-0	
Plate Offse	ts (X,Y) [20:0-4-0),0-4-0]	0-0-13	0-0-0	9-5-12	0-11-3	0-11-9	7-3-1	
LOADING TCLL (roof) Snow (Pf/P TCDL BCLL BCDL	(psf)) 20.0 g) 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir Code IRC20	2-0-0 DL 1.15 1.15 nor YES 18/TPI2014	CSI. TC 0.45 BC 0.77 WB 0.99 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.15 24-26 -0.28 24-26) 0.06 20	l/defi L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 484 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHOF BOT CHOF WEBS SLIDER REACTION	 RD 2x6 SP No.2 RD 2x6 SP No.2 2x4 SP No.3 *E 9-20: 2x4 SP No. Left 2x4 SP No. Left 2x4 SP No. IS. (size) 2=0- Max Horz 2=-2 Max Uplift 2=-1 Max Grav 2=10 	xcept* 5.2 3 2-6-0, Right 2x4 S 3-8, 15=0-3-8, 20=0 01(LC 14) 14(LC 16), 15=-59(L 641(LC 28), 15=579(P No.3 2-6-0 -3-8 .C 16), 20=-180(LC LC 55), 20=3518(LI	В Т V 16) С 30)	RACING- OP CHORD OT CHORD VEBS	Structural wood s except 2-0-0 oc purlins (Rigid ceiling dire 1 Row at midpt 2 Rows at 1/3 pts	sheathing directly ap 6-0-0 max.): 7-9. ctly applied or 6-0-0 4-24, 6-2: s 9-20	blied or 3-9-10 oc purlir oc bracing. 3, 7-21, 8-21, 10-20, 11	ıs, 20
FORCES. TOP CHOP BOT CHOP WEBS	(lb) - Max. Comp./M RD 2-4=-2845/24 9-10=0/1192, RD 2-26=-143/263 19-20=-443/1 4-26=0/273, 4 7-21=-1148/93 11-20=-1059/	Aax. Ten All forces 7, 4-6=-2191/267, 6- 10-11=0/1231, 11-1: 94, 24-26=-143/2694 19, 17-19=-77/559, 1 -24=-690/108, 6-24= 2, 8-21=-499/141, 9- 126, 11-19=0/643, 1	250 (lb) or less exc 7=-1284/260, 7-8=- 3=-20/509, 13-15=- 4, 23-24=-69/2076, 2 5-17=-77/559 0/671, 6-23=-1245/ 21=-130/1916, 9-20 3-19=-832/114, 13-	ept when shown. 604/232, 8-9=-604/232, 305/119 21-23=0/1166, 20-21=-4 166, 7-23=-39/1101, =-2384/154, 10-20=-460 17=0/294	37/161,)/164,				
NOTES- 1) Unbalan 2) Wind: A3 II; Exp B 33-1-8, I ; end ve DOL=1.(3) TCLL: A DOL=1.' 3) TCLL: A DOL=1.' 4) Unbalan 5) This trus non-con 6) WARNIN handling Trusses qualified permane bracing. 7) Provide (8) nthisedus	ced roof live loads has SCE 7-16; Vult=130r ; Enclosed; MWFRS interior(1) 33-1-8 to 3 ritical left and right ex 30 SCE 7-16; Pr=20.0 p 15); Is=1.0; Rough C ge applied to all expo ced snow loads have s has been designed current with other live NG: This long span t and erection guidan ("BCSI"), jointly proc registered design pr ant individual truss m adequate drainage to a bag been designed	ave been considered nph (3-second gust) (directional) and C-1 6-6-0, Exterior(2R) 3 posed;C-C for memb sf (roof LL: Lum DO at B; Partially Exp.; (sed surfaces with sk e been considered for d for graeter of min re e loads. russ requires extrem ice, see Guide to Go luced by SBCA and ofessional for the de ember restraint/brac o prevent water pono d for a 10.0 psf botto	I for this design. Vasd=103mph; TCI C Exterior(2E) -0-11 86-6-0 to 45-1-8, Int bers and forces & M L=1.15 Plate DOL= Ce=1.0; Cs=1.00; C opes less than 0.500 r this design. bof live load of 12.0 e care and experier od Practice for Han TPI. The building ov sign and inspection ing. MiTek assume ling. m chord live load no	DL=6.0psf; BCDL=6.0ps -0 to 5-2-3, Interior(1) 5- erior(1) 45-1-8 to 61-11- WFRS for reactions sho 1.15); Pg=15.0 psf; Pf=1 I=1.10, Lu=50-0-0; Min. 1 D/12 in accordance with I psf or 1.00 times flat roc nce for proper and safe H dling, Installing & Bracin, vner or the owner's auth of the temporary installa s no responsibility for tru	f; h=25ft; B=45ft; 2-3 to 24-6-0, Ex 0 zone; cantileve wn; Lumber DOL 6.5 psf (Lum DO) lat roof snow loa BC 1608.3.4. If load of 11.6 psf andling and erec g of Metal Plate (orized agent shal ation restraint/bra siss manufacture, her live loads.	L=61ft; eave=7ft; terior(2R) 24-6-0 t r left and right exp =1.60 plate grip L=1.15 Plate d governs. Rain con overhangs stion. For general Connected Wood I contract with a cing and the handling, erection	Cat. o osed	SEAL 036322	
WAI Desig a truss buildir is alwa fabrica and E	RNING - Verify design para n valid for use only with Mi s system. Before use, the I g design. Bracing indicat ays required for stability ar ation, storage, delivery, ere SCSI Building Component	meters and READ NOTES Tek® connectors. This de building designer must ver ed is to prevent buckling o id to prevent collapse with action and bracing of truss t Safety Information av	ON THIS AND INCLUDE sign is based only upon p ify the applicability of des f individual truss web and possible personal injury es and truss systems, se ailable from the Structura	D MITEK REFERENCE PAGE N parameters shown, and is for an ign parameters and properly in //or chord members only. Addi and property damage. For ger e ANSI/TP11 Quality Criteria I Building Component Associat	All-7473 rev. 1/2/2023 h individual building c corporate this design tional temporary and leral guidance regard and DSB-22 availab ion (www.sbcacompo	BEFORE USE. omponent, not into the overall permanent bracing ing the le from Truss Plate Insi nents.com)	titute (www.tpinst.org)	A MiTek 818 Soundside Road Edenton, NC 27932	Affiliate

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF	
						71333179
25-3702-A	T03	PIGGYBACK BASE	3	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:05 2025 F	Page 2
		ID:07Iz9>	ktbY5He8r	nIUVbML?	fzV1Kn-wYbiCAIPSGoLJQ9BGImsZaZdzpa0uPhSCcFVXSz	mVeC

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

a) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15 except (jt=lb) 2=114, 20=180.
This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust structure Bucking Component Advancement and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Scale = 1:108.7



	8-4-5	16-5-3 8-0-13	24-6-0	29-9-12 3	<u>34-9-12 39-9-1</u>	2 46-9-5	53-8-15	61-0-0		
Plate Offse	ets (X,Y) [19:0-4-0),0-4-0]	0-0-13	5-5-12	3-0-0 3-0-0	0-11-3	0-11-3	7-5-1		
LOADING TCLL (roof Snow (Pf/F TCDL BCLL BCDL	(psf)) 20.0 2g) 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip D Lumber DOI Rep Stress Code IRC2	2-0-0 OOL 1.15 L 1.15 Incr YES 018/TPI2014	CSI. TC 0.46 BC 0.79 WB 0.97 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l -0.24 22 > -0.38 22 > 0.06 19	/defl L/d 999 240 999 180 n/a n/a	PLATES MT20 Weight: 496 lb	GRIP 244/190 FT = 20%	
LUMBER- TOP CHOI BOT CHOI WEBS SLIDER REACTION	RD 2x6 SP No.2 RD 2x6 SP No.2 *E 21-23: 2x4 SP N 2x4 SP No.3 *E 9-24: 2x4 SP No. Left 2x4 SP No. NS. (size) 2=0- Max Horz 2=1 Max Uplift 2=-1	xcept* No.2 xcept* o.2, 9-19: 2x4 SP D 3 2-6-0, Right 2x4 3 3-8, 15=0-3-8, 19= 99(LC 15) 01(LC 16), 15=-32(ISS SP No.3 2-6-0 0-3-8 (LC 16), 19=-80(LC 1	16)	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sh except 2-0-0 oc purlins (6- Rigid ceiling direct 6-0-0 oc bracing: 2 1 Row at midpt 2 Rows at 1/3 pts	eathing directly ap -0-0 max.): 7-9. ly applied or 6-0-0 :1-23 4-27, 6-2 9-21	plied or 3-8-14 oc purlin oc bracing. Except: 6, 7-24, 8-24, 10-19, 11	ıs, I-19	
FORCES. TOP CHOI BOT CHOI WEBS	$\begin{array}{l} \mbox{Max Grav } 2=1664(LC 28), 15=497(LC 55), 19=3937(LC 30) \\ \mbox{FORCES.} (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. \\ \mbox{TOP CHORD} & 2-4=-2895/218, 4-6=-2250/234, 6-7=-1335/229, 7-8=-767/196, 8-9=-767/196, \\ & 9-10=0/1288, 10-11=0/1323, 11-13=0/621, 13-15=-559/179 \\ \mbox{BOT CHORD} & 2-29=-137/2735, 27-29=-137/2735, 26-27=-59/2126, 24-26=0/1208, 20-24=-322/168, \\ & 19-20=-322/168, 18-19=-545/93, 16-18=-132/516, 15-16=-132/516 \\ \mbox{WEBS} & 4-29=0/267, 4-27=-680/112, 6-27=0/680, 6-26=-1256/164, 7-26=-60/1021, \\ & 7-24=-996/116, 8-24=-500/142, 23-24=-107/1971, 9-23=-77/2134, 9-21=-2597/117, \\ & 19-21=-2746/85, 10-19=-452/160, 11-19=-1033/135, 11-18=0/618, 13-18=-860/121, \\ & 13-16=0/309 \\ \end{array}$									
NOTES- 1) Unbalar 2) Wind: A II; Exp E 33-1-8, end vert DOL=1. 3) TCLL: A DOL=1. surcharg 4) Unbalar 5) This trus non-con 6) WARNIII handling Trusses qualified perman. Corbinediator	aced roof live loads his SCE 7-16; Vult=130r 3; Enclosed; MWFRS Interior(1) 33-1-8 to 3 icical left and right exp 60 SCE 7-16; Pr=20.0 p 15); Is=1.0; Rough C ge applied to all expo neced snow loads have shas been designed current with other live NG: This long span t g and erection guidan ("BCSI"), jointly proc d registered design pr ent individual truss m on page 2	ave been consideren nph (3-second gust (directional) and C 6-6-0, Exterior(2R) osed;C-C for memb osed;C-C for memb sef (roof LL: Lum DC at B; Partially Exp.; sed surfaces with s a been considered f d for greater of min a loads. russ requires extrer ice, see Guide to G luced by SBCA and tofessional for the d ember restraint/bra	ed for this design.) Vasd=103mph; TC -C Exterior(2E) -0-1 36-6-0 to 45-1-8, In bers and forces & MM DL=1.15 Plate DOL= Ce=1.0; Cs=1.00; C lopes less than 0.50 for this design. roof live load of 12.0 me care and experie ood Practice for Har i TPI. The building o lesign and inspectior cing. MiTek assume	DL=6.0psf; BCDL=6.0p I-0 to 5-2-3, Interior(1) 4 terior(1) 45-1-8 to 61-0- WFRS for reactions sho 1.15); Pg=15.0 psf; Pf= t=1.10, Lu=50-0-0; Min 0/12 in accordance with psf or 1.00 times flat ro nce for proper and safe idling, Installing & Braci wher or the owner's aut n of the temporary instal as no responsibility for th	sf; h=25ft; B=45ft; 5-2-3 to 24-6-0, Ex 0 zone; cantilever I wn; Lumber DOL= 16.5 psf (Lum DOL flat roof snow load h IBC 1608.3.4. bof load of 11.6 psf handling and erec nog of Metal Plate C horized agent shal llation restraint/brac russ manufacture,	L=61ft; eave=7ft; Ca terior(2R) 24-6-0 to eft and right expose 1.60 plate grip _=1.15 Plate d governs. Rain on overhangs tion. For general Connected Wood I contract with a cing and the handling, erection, o	at. ed ;	SEAL 036322	EH-1111	
WA Desig a trus buildii is alw fabric and	RNING - Verify design para In valid for use only with Mi s system. Before use, the I ng design. Bracing indicat ays required for stability ar ation, storage, delivery, ere BCSI Building Component	meters and READ NOTE Tek® connectors. This d building designer must w ed is to prevent buckling to prevent collapse wil cotion and bracing of trus t Safety Information	S ON THIS AND INCLUDE lesign is based only upon erify the applicability of de of individual truss web an th possible personal injury sees and truss systems, si vailable from the Structure	D MITEK REFERENCE PAGE parameters shown, and is for sign parameters and properly d/or chord members only. Ad and property damage. For g e ANS/TP11 Quality Criteri al Building Component Associ	MII-7473 rev. 1/2/2023 an individual building co incorporate this design ditional temporary and p eneral guidance regardi a and DSB-22 availabi ation (www.sbcacompol	BEFORE USE. omponent, not into the overall permanent bracing ng the le from Truss Plate Institu nents.com)	ite (www.tpinst.org)	A MITEK BASOUNDERING BY A MITEK 818 Soundside Road Edenton, NC 27932	LOO Affiliate	

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF
					171333180
25-3702-A	T03AS	PIGGYBACK BASE	1	1	
					Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:06 2025 Page 2
		ID:07lz9xtb	Y5He8mII	JVbML?fz\	/1Kn-Ok94PWm2DawCxakNq?H56o6nVCwydsAbQG?23vzmVeB

7) Provide adequate drainage to prevent water ponding.

8) All plates are 2x4 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

11) Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

12) Provide metal plate or equivalent at bearing(s) 19 to support reaction shown.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 19 except (jt=lb) 2=101.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





			6	51-0-0						_
I			6	61-0-0						1
Plate Offsets ((X,Y) [16:0-3-0	,0-4-4], [22:0-3-0,0-4-4]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YEE Code IRC2018/TPI2014	CSI. TC 0.05 BC 0.03 WB 0.18 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.01	(loc) 36 36 36	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 586 lb	GRIP 244/190 FT = 20%
	10.0									
LUMBER-				BRACING-						
TOP CHORD	2x6 SP No.2			TOP CHORD	Structura	l wood	sheathin	g directly ap	oplied or 6-0-0 oc purlins	, except
BOT CHORD	2x6 SP No.2				2-0-0 oc j	purlins	(6-0-0 m	ax.): 16-22.		
OTHERS	2x4 SP No.3			BOT CHORD	Rigid ceil	ing dire	ctly appl	ied or 10-0-	0 oc bracing.	
SLIDER	Left 2x4 SP No.3	3 1-6-5, Right 2x4 SP No.3 1-6-5	5	WEBS	1 Row at	midpt	,	22-50, 2 ⁻ 16-57, 1	1-51, 20-52, 19-53, 18-5 5-58, 14-59, 23-49, 24-4	5, 17-56, 8
REACTIONS.	All bearings 61	-0-0.								
(lb)	- Max Horz 2=-2	00(LC 14)								

Max Uplift All uplift 100 lb or less at joint(s) 2, 52, 53, 55, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 69, 49, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38 Max Grav All reactions 250 lb or less at joint(s) 2, 50, 51, 52, 53, 55, 56, 57, 58, 59, 60, 61, 63, 64, 65,

66, 67, 68, 69, 49, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38, 36

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 14-15=-115/287, 15-16=-128/319, 16-17=-122/311, 17-18=-122/311, 18-19=-122/311, 19-20=-122/311, 20-21=-122/311, 21-22=-122/311, 22-23=-128/319, 23-24=-115/287

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=61ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 5-2-3, Exterior(2N) 5-2-3 to 24-6-0, Corner(3R) 24-6-0 to 30-6-0, Exterior(2N) 30-6-0 to 36-6-0, Corner(3R) 36-6-0 to 42-6-0, Exterior(2N) 42-6-0 to 61-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 8) Provide adequate drainage to prevent water ponding.
- 9) All plates are 2x4 MT20 unless otherwise indicated.
- 10) Gable requires continuous bottom chord bearing.

OdihtiGableontustagepaced at 2-0-0 oc.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





lah	Truco	Truco Turo	Otre	DIV				
JOD	Tiuss	Truss Type	QIY	Piy	SGR-LUT 76 KUUF			
					171333181			
25-3702-A	T03GE	PIGGYBACK BASE SUPPO	1	1				
					Job Reference (optional)			
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:09 2025 Page 2			
		ID:07Iz9xtbY5He8mIUVbML?fzV1Kn-pJqC1XowWVIno1TyV8rokQkP7Q8YqQD16EDiqEzmVe8						

- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 52, 53, 55, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 69, 49, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Scale = 1:108.8



⊢	8-4-5	16-5-3	24-6-0	29-9-12	34-9-1	2 39-9-12	2	46-9-5		53-8-15	61-0-0	
	8-4-5	8-0-13	8-0-13	5-3-12	5-0-0	5-0-0		6-11-9		6-11-9	7-3-1	
Plate Uffsets (X,Y) [20:0-4-0,0-4-0]												
LOADING (psi	f)							<i>a</i> ,				
TCLL (roof)	20.0	SPACING-	2-0-0	CSI.		DEFL.	in	(IOC)	l/defl	L/d	PLATES	GRIP
Snow (Pf/Pg)	16 5/15 0	Plate Grip DOL	1.15	TC 0.46		Vert(LL)	-0.24	23	>999	240	MT20	244/190
	10.0/10.0	Lumber DOL	1.15	BC 0.79		Vert(CT)	-0.38	23	>999	180		
RCU	0.0 *	Rep Stress Incr	YES	WB 0.96		Horz(CT)	0.06	20	n/a	n/a		
DOLL	0.0	Code IRC2018/	TPI2014	Matrix-MS							Weight: 498 lb	FT = 20%
BCDL	10.0										_	
LUMBER-					BRA	CING-						
TOP CHORD	2x6 SP No.2				TOP	CHORD	Structura	al wood	sheathin	a directly ap	plied or 3-8-14 oc purlir	ıs.
BOT CHORD	2x6 SP No.2 *E	xcept*					except			5		- /
	22-24 2x4 SP N	10.2					2-0-0 00	nurlins	(6-0-0 m	ax)· 7-9		
WEBS	2x4 SP No 3 *F	vcent*			BOT	CHORD	Rigid cei	iling dire	ctly annl	ied or 6-0-0 (oc bracing Except	
WEBO	0-25: 2v4 SP No	2 9-20: 224 50 055			DOI	ONORD	6-0-0 00	bracino	· 22-24		oc bracing. Except.	
	J off 2v4 SD No.	2.2.6.0 Diabt 2v4 SD N	0 2 2 6 0				1 Down	t midnt	. 22-24	1 20 6 2	7 7 75 8 75 10 70 11	20
SLIDER	Len 2x4 SP NO.	5 2-0-0, Right 2x4 SP N	10.5 2-0-0		VVED	.5	I KOW a		-	4-20, 0-2	7, 7-25, 6-25, 10-20, 11	-20
							2 Rows	at 1/3 p	S	9-22		
REACTIONS.	(size) 2=0-	3-8, 15=0-3-8, 20=0-3-8	3									
	Max Horz 2=-2	01(LC 14)										
	Max Uplift 2=-1	03(LC 16), 15=-64(LC 1	16), 20=-75(LC 10	6)								
	Max Grav 2=16	63(LC 28), 15=552(LC	55), 20=3938(LC	; 30)								
FORCES. (lb) - Max. Comp./M	lax. Ten All forces 250) (lb) or less exce	ept when shown.								
TOP CHORD	2-4=-2893/223	8, 4-6=-2248/239, 6-7=-	1335/234, 7-8=-7	67/201, 8-9=-767/	201,							
	9-10=0/1293,	10-11=0/1327, 11-13=0	/626, 13-15=-550)/182								
BOT CHORD	2-30=-121/273	37, 28-30=-121/2737, 27	7-28=-43/2129, 2	5-27=0/1210, 21-2	25=-325/1	79,						
	20-21=-325/17	79. 19-20=-546/108. 17-	19=-135/507.15	-17=-135/507								
WEBS	4-30=0/267.4	-28=-680/112, 6-28=0/6	80. 6-27=-1256/	64.7-27=-60/102	1.							
	7-25=-97/114 8-25=-500/42 24-25=-105/197 9-24=-74/2135 9-22=-2600/112											
	20-22=-2749/79, 10-20=-452/160, 11-20=-1032/135, 11-19=0/616, 13-19=-855/110.											
	13-17-0/308	3, 10 20- 432/100, 11	20= 1002/100, 1	1 13=0/010, 10 10	= 000/11	0,						
	13-17-0/300											
NOTES												
NUIE3-	l reaf live leads be	we have sevel-level for	this design									
1) Unbalanced	roof live loads ha	ave been considered for	this design.			050 D 450 I	0.14		• •		"TH CARO	111
2) Wind: ASCE	= 7-16; Vult=130m	ph (3-second gust) Vas	sd=103mph; ICL	L=6.0psf; BCDL=	6.0pst; h=	=25ft; B=45ft; L	_=61ft; ea	ave=/ft;	Cat.	1	A	City.
I; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 5-2-3, Interior(1) 5-2-3 to 24-6-0, Exterior(2R) 24-6-0 to												
33-1-8, Inter	rior(1) 33-1-8 to 3	6-6-0, Exterior(2R) 36-6	6-0 to 45-1-8, Inte	rior(1) 45-1-8 to 6	1-11-0 zc	one; cantilever	left and r	right exp	osed	123	NO NI	· MA
; end vertica	al left and right exp	posed;C-C for members	and forces & M	WFRS for reaction	s shown;	Lumber DOL=	=1.60 pla	te grip		- un		
DOL=1.60												12
TCLL: ASCE	E 7-16; Pr=20.0 p	sf (roof LL: Lum DOL=1	.15 Plate DOL=1	.15); Pg=15.0 psf;	Pf=16.5	psf (Lum DOL:	.=1.15 Pla	ate		- E - E	SEAL	
DOL=1.15);	Is=1.0; Rough Ca	at B; Partially Exp.; Ce=	1.0; Cs=1.00; Ct	=1.10, Lu=50-0-0;	Min. flat i	roof snow load	l governs	. Rain		- E - E	OLAL	- E -
surcharge a	pplied to all expos	sed surfaces with slope	s less than 0.500	/12 in accordance	with IBC	1608.3.4.	-			- E - 1	036322	
4) Unbalanced	l snow loads have	been considered for th	is desian.									
5) This truss ha	as been designed	for greater of min roof	live load of 12.0	osf or 1.00 times fl	at roof loa	ad of 11.6 psf (on overh	ands				1. 2
non-concurr	ent with other live	loads									S. A.	1 A S
6) WARNING:	This long span to	uss requires extreme c	are and experien	ce for proper and	safe hand	tling and erect	ion For (reneral		1.5	& GINEF!	as
bandling and	d oroction quidan	co soo Guido to Good	Bractico for Hand	lling Installing 8 P	racing of	Motol Ploto C	onnoctor	J.Wood		11	10	EN N
	CSIII) isistly prod	used by SPCA and TPL	The building out	ning, mstannig & D	authoria	ad agent shall	oontroot	with o			A GILD	11
nusses (Bl	vor , joinuy prou	aced by SBCA and TPI		of the temporer :	auuiuiiz	su ayerit Sridli		willa			"HILLING WILLING	
quaimed reg	jistereu uesign pr	oressional for the design	MiTels each and the section	or the temporary li	istaliatiof		ang and t				Echnicary 44.0	0.05
permanent i	naiviauai truss m	emper restraint/bracing.	IVITTEK assumes	ino responsibility	IOF TRUSS I	nanuracture, h	ianaiing,	erection	i, or		repruary 11,2	2025
Contriagencion pa	age 2											
A						170		-				
Design voli	id for use only with Mi	TIELETS AND KEAD NUTES ON	inis AND INCLUDED	arameters shown and	AGE MII-74	ividual building cou	moonent n	et.			ENGINEERING BY	
a truss svs	tem. Before use, the b	puilding designer must verify the	he applicability of desi	anameters shown, and an parameters and pro	perly incom	orate this design in	nto the over	rall				
building de	building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing											
is always re	is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the											
tabrication,	, storage, delivery, ere	ction and bracing of trusses a	na truss systems, see	Puilding Component A	riteria and	DSB-22 available	e from Trus	s Plate In:	stitute (www	/.tpinst.org)	818 Soundside Road	
and DUSI	Banang Componen	concerv information availab	ne nom me Structural	building component A	ออบบเสแบท (www.sucacompone	iciiio.com)				Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF				
					17133318	82			
25-3702-A	T03S	PIGGYBACK BASE	6	1					
					Job Reference (optional)				
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	.730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:10 2025 Page 2				
		ID:07Iz9xtbY5He8mIUVbML?fzV1Kn-HWObFtpYHpQePB183rM1GeGTVqHuZqFBLuzGCqzmVe7							

7) Provide adequate drainage to prevent water ponding.

8) All plates are 2x4 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

11) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

12) Provide metal plate or equivalent at bearing(s) 20 to support reaction shown.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 20 except (jt=lb) 2=103.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





February 11,2025





Job	Truss	Truss Type	Qty	Ply	SGR-LOT 78 ROOF	
						171333184
25-3702-A	T04G	COMMON GIRDER	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8	.730 s Dec	5 2024 MiTek Industries, Inc. Mon Feb 10 15:09:11 2025	Page 2

ID:07lz9xtbY5He8mIUVbML?fzV1Kn-liyzSDqA26YV1LcLcZtGprpZ9DepIBfKaYipl6zmVe6

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-43, 2-4=-43, 4-6=-43, 7-12=-20

Concentrated Loads (lb)

Vert: 10=68(B) 9=68(B) 12=-417 15=68(B) 16=68(B) 17=68(B) 18=68(B) 19=-395(B) 20=-395(B) 21=-395(B)

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818 Soundside Road





BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 19=46(LC 15)

2x4 SP No.3

2x4 SP No.3

Max Uplift All uplift 100 lb or less at joint(s) 19, 12, 17, 18, 14, 13

Max Grav All reactions 250 lb or less at joint(s) 19, 12, 16, 17, 18, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

WEBS

OTHERS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-1-0, Exterior(2N) 2-1-0 to 6-0-0, Corner(3R) 6-0-0 to 9-0-0, Exterior(2N) 9-0-0 to 12-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 12, 17, 18, 14, 13.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



ute (www.tpinst.org)

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcaccomponents.com)





818 Soundside Road





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- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 6-2-5, Exterior(2R) 6-2-5 to 9-2-5, Interior(1) 9-2-5 to 11-10-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





818 Soundside Road



- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-9-2, Exterior(2R) 4-9-2 to 7-9-2, Interior(1) 7-9-2 to 8-11-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



818 Soundside Road



Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat.
 II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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A MiTek Affi 818 Soundside Road

