# STONEFIELD-RALE

RALEIGH - LOT 00.0007 BLAKE POND SF (MODEL# 1635)

**ELEVATION 1 - GL** 

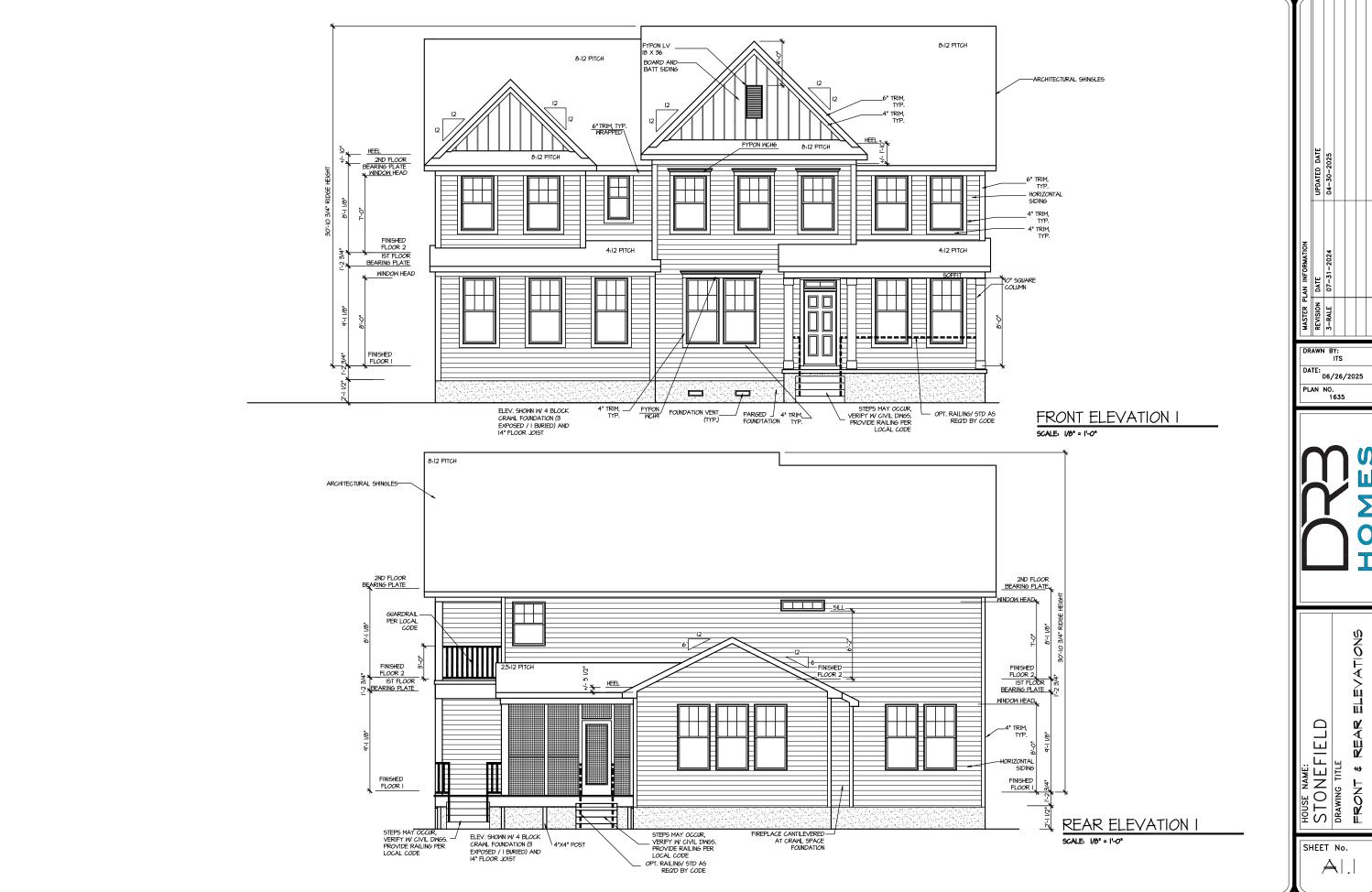


AREA CALCULATIONS	115.1.750	COVERED /	
ELEVATION 1	HEATED	UNHEATED	UNCOVERED
FIRST FLOOR	1542 SF		
GARAGE		496 SF	
FRONT PORCH — ELEVATION 1		103 SF	
SECOND FLOOR	1616 SF		
OPTIONS			
REAR SCREEN PORCH		144 SF	
FIREPLACE	10 SF		
BED 5 W/ BATH 3	55 SF	-55 SF	
SIDE PORCH		93 SF	
TOTAL	3223 SF	781 SF	

## 243 Bronze Leaf Drive

	<u>SPECIFIC</u>	
1	LOT 00.0007	BLAKE POND SF
		STONEFIELD REV. RALE-3 ELEVATION 1
2	ADDRESS	243 BRONZE LEAF DRIVE LILLINGTON, NC 27546
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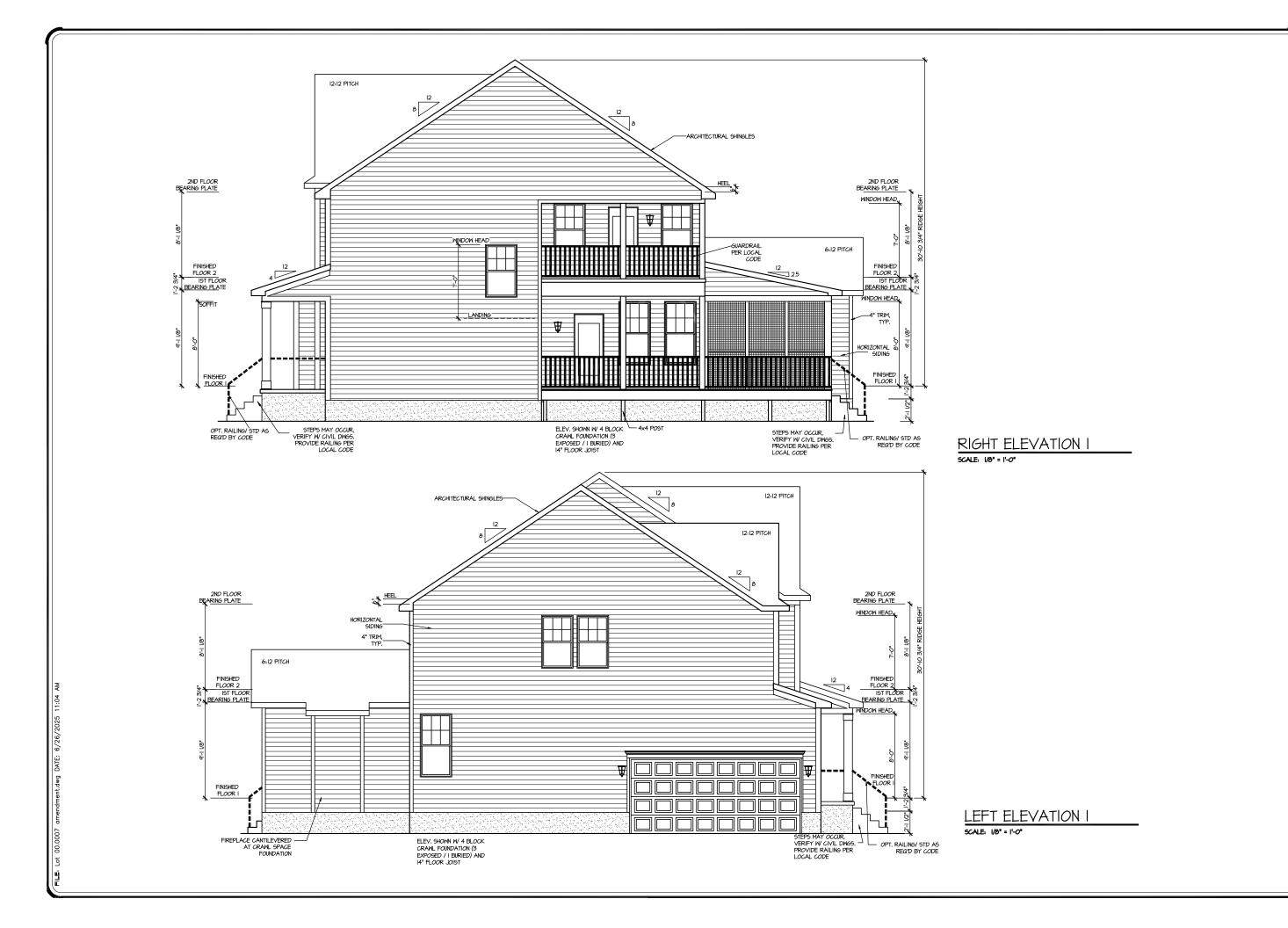
INDEX	



DATE: 06/26/2025

PLAN NO. 1635

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DRAWN BY:
ITS

DATE:
06/26/2025

PLAN NO.
1635



AWING TITLE GHT & LEFT ELEVATIONS

HOUSE NAME:
STONEFIELD
DRAWING TITLE

UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA I = 1784 50. FT.

OVERAL REQUIRED VENTILATION.

1 TO 800 = 11.421 50. FT.

1 TO 800 = 55.463 50. FT.

50% IN TOP THIRD = 2.48 50. FT. (1 TO 800)

NET FREE AREA OF NOTED SOFFTI = 31.50. IN / LINEAR FT.

NET FREE AREA OF RIDGE VENT = 18 50. IN / LINEAR FT.

LOWER VENTINS. (BOTTOM 2/3 RDS)

20 LINEAR FIET OF SOFFIT X 5.1 50. IN. = 3.245 50. FT.

IPPER VENTINS. (TOP 1/3 BD)

24 LINEAR FIET OF RIDSE X IB 50. IN. = 3 50. FT.

3 50. FT. AT 50%

(1 TO 300 ALLOYED)

UPPER ROOF VENTILATION CALCULATIONS:
ROOF AREA 3 = 95 SQ. FT.
OVERAL BEGINED VENTILATION.
10 300 SQ. FT.
10 300 SQ. FT.
506 IN TOP THERD = 0.005 SQ. FT. (1 TO 300)

NET FREE APEA OF VANIED SOFFIT = 5.1 SO, IN / LINEAR FT.
NET FREE APEA OF RIDGE VENT = 10 SQ, INV LINEAR FT.
LOWER VENTING. (BOTTOM 2/8 EDS)
25 LINEAR FEET OF COFFIT X 5.1 SQ, IN = 0.41 SQ, FT.
12PEX VANIES, LOTO 2/8 ED.
1 SQ, FT. SETWEN SO, IN = 1 SQ, FT.
1 SQ, FT. SETWEN SO, IN = 1 SQ, FT.
1 (TO 2/00 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS:
ROOF AREA 5 = 144 SQ. FT.
OVERALL REQUIRED VENTILATION.
1 TO 500 = 0.46 SQ. FT.
50% IN 100 FT.
50% IN 100 FT.
50% IN 100 FT.
60% IN 10

LOHER VENTING. (BOTTOM 2/3 RDS)

1 LINEAR RET OF SOFFIT X 5.1 50. IN. = 0.2T1 50. FT.

PURPER VENTING. (ROP 1/3 RD)

1 LINEAR RET OF RIDGE X IØ 50. IN = 0.25 50. FT.

1 SO. FT. AT 50%

(1 TO 300 ALLOHED)

UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA 2 = 25/2 50. FT.

OVERAL REGUIRED VENTILATION.

1 TO BO = 160 50. FT.

1 TO 300 = 0.04 50. FT.

50% IN TOP THRD = 0.42 50. FT. (1 TO 300)

LOWER VENTING. (BOTTON 2/3 RDS)

II LINEAR FIET OF SOFFIT X 5.7 5Q. IN. = 0.425 5Q. FT.

IPPER VENTING. (70P US BQ)

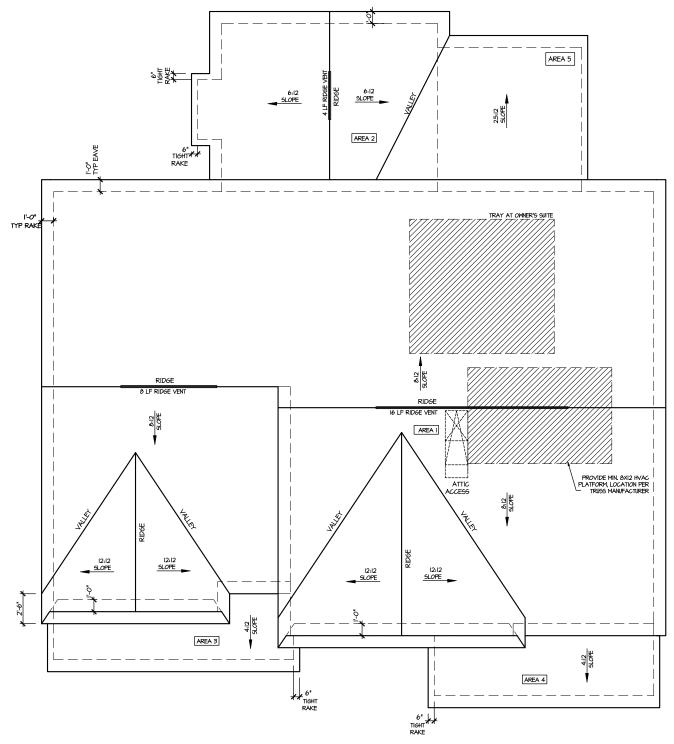
4 LINEAR FIET OF RIDGE X IB 5Q. IN = 0.5 5Q. FT.

0.5 5Q. FT. AT 50%

(1 TO 300 ALLOYED)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

NOTE: ROOF PLANS SHOWN W/ MIN. REQ'D RIDGE VENT LOCATIONS. ACTUAL RIDGE VENT LOCATIONS AND QUANTITY PER BUILDER



ROOF PLAN ELEV. I

| FILE: Lot 00.0007 amendment.dwg DATE: 6/26/2025 11:04 AM

HOUSE NAME:
STONEFIELD
DRAWING TITLE

DRAWN BY:

DATE: 06/26/2025 PLAN NO. 1635

SHEET No.

AI.3

CRAWL SPACE VENT CALCULATIONS: ELEV I CRAWL AREA = 1542 SQ. FT.

OVERALL REQUIRED VENTILATION:

I SQ. IN. PER I SQ. FT. = 1542 SQ. IN.

NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

<u>VENTING REQUIREMENT:</u> 1542 SQ. IN. / 72 SQ. IN. = 21.4 VENTS = 22 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED. VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1

CRAWL SPACE VENT CALCULATIONS: OPT. BED 5/BA 3
CRAWL AREA = 54 SQ. FT. OVERALL REQUIRED VENTILATION:

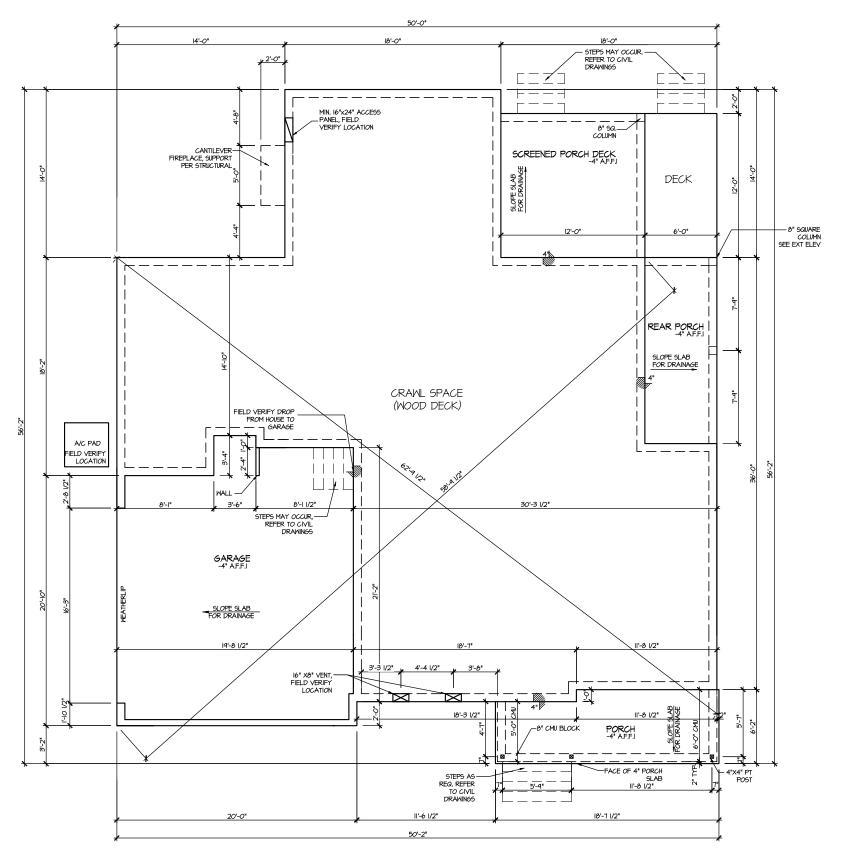
I SQ. IN. PER I SQ. FT. = 54 SQ. IN.

NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

#### VENTING REQUIREMENT:

54 SQ. IN. / 72 SQ. IN. = .8 VENTS = I VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED. VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1



ELEVATION I CRAWL SPACE PLAN

SCALE: 1/8" = 1'-0"

DRAWN BY:

PLAN NO. 1635

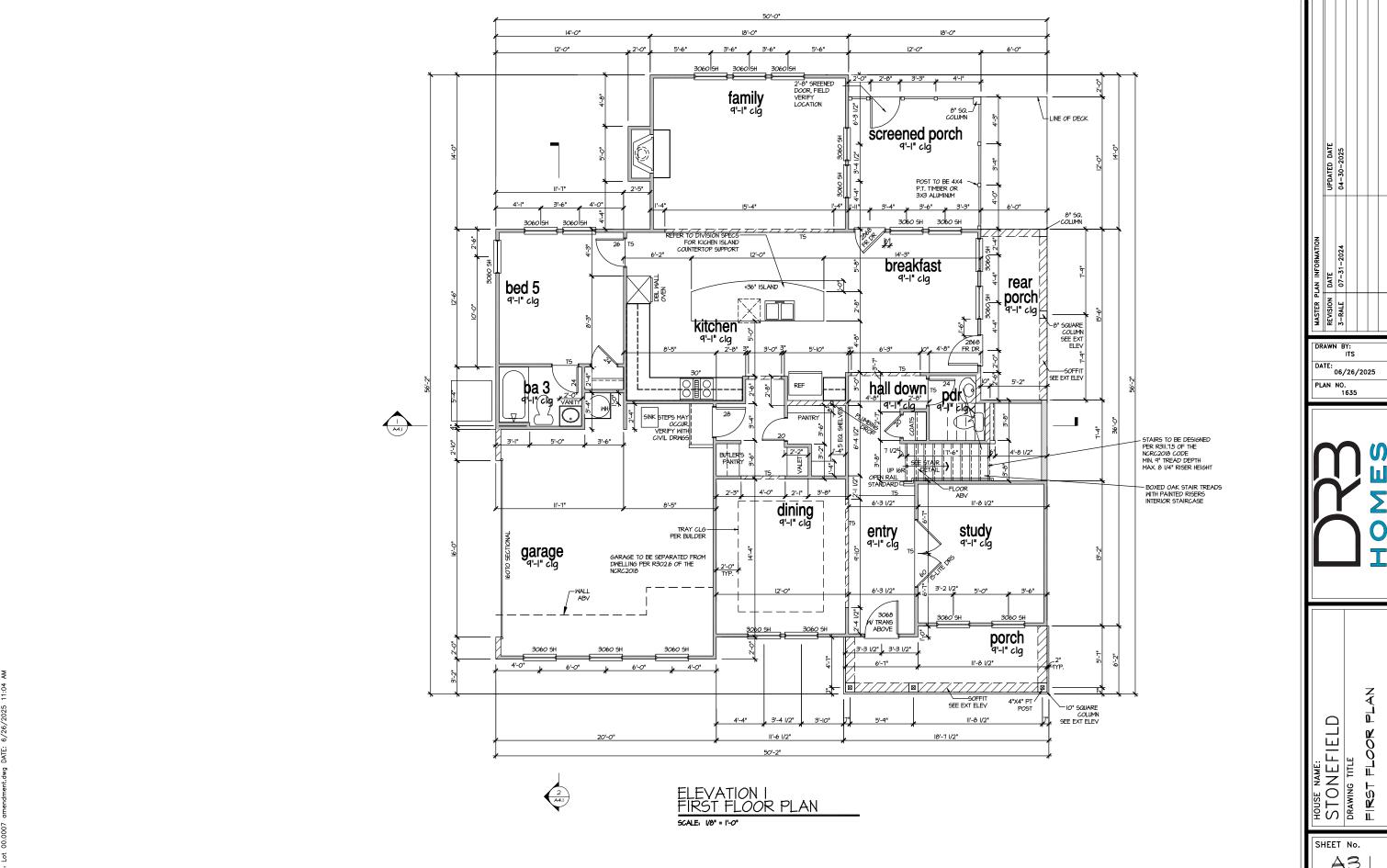
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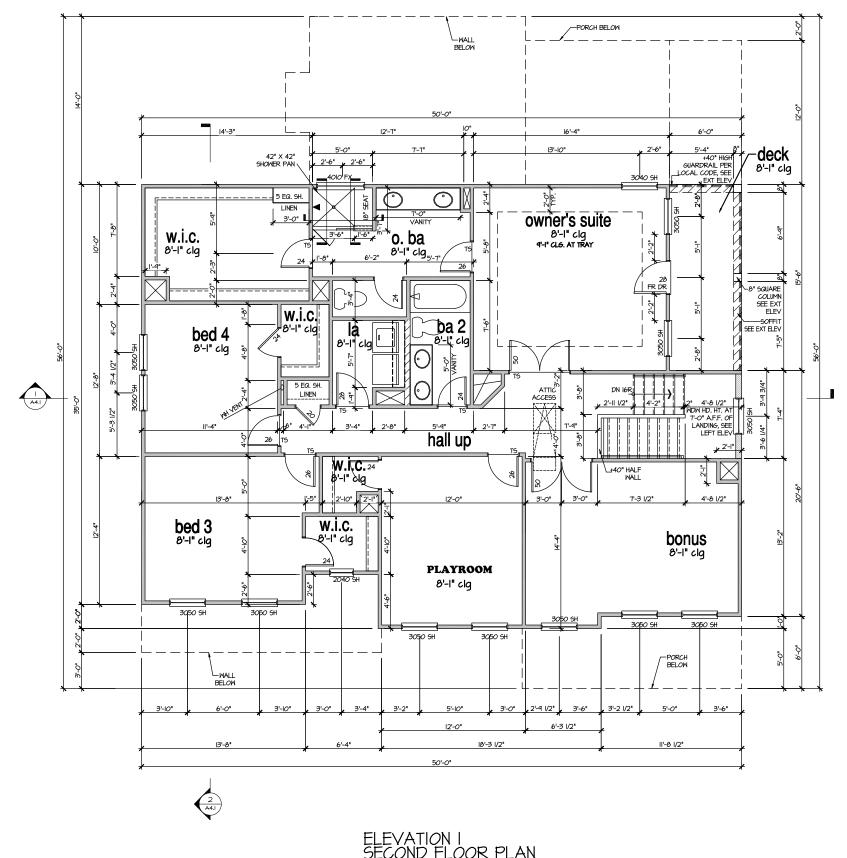
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HOUSE NAME: STONEFIE DRAWING TITLE

SHEET No. A2.



A3.



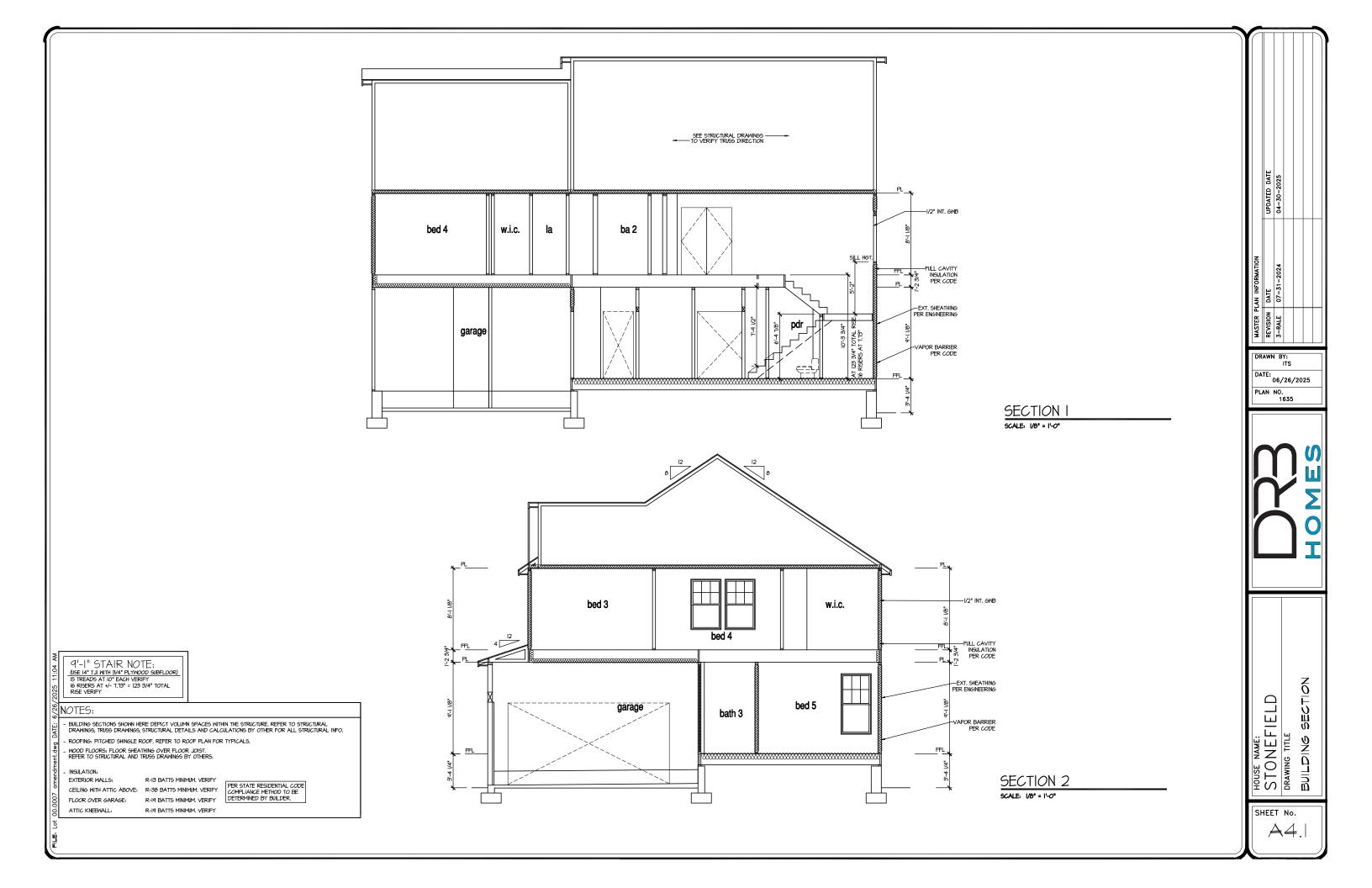
ELEVATION I SECOND FLOOR PLAN SCALE: 1/8" = 1'-0"

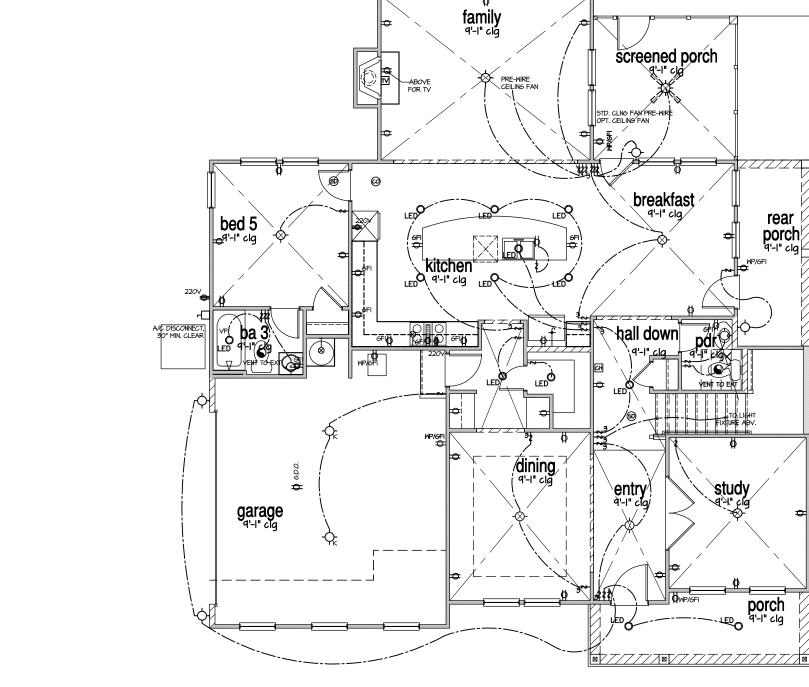
DRAWN BY: DATE: 06/26/2025 PLAN NO. 1635

HOUSE NAME:
STONEFIELD
DRAWING TITLE
SECOND FLOOR ∏ 00 N

SHEET No.

A3.2





NOTES: - PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL COD (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS. HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS. ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS. PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S MRITTEN INSTRUCTIONS.

CLOSET LIGHTS TO BE FLOURESCENT FIXTURES FOR NC & INCANDESCENT CLG. MOUNTED FIXTURES FOR ALL OTHER AREAS.

SWITCH AND RECEPTACLE BOXES

STANDARD ELECTRICAL BOX HEIGHTS

SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

LEGEND: CEILING FAN (PROVIDE ADEQUATE SUPPORT) CEILING MOUNTED INCANDESCEN AFCI DUPLEX OUTLET WEATHERPROOF OF AFOI DUPLEX OUTLET PUSHBUTTON SWITCH CEILING FAN WITH INCANDESCENT LIGHT FIXTURE (PROVIDE ADEGUATE SUPPORT) IIOV SMOKE DETECTOR W BATTERY BACKUP HALL MOUNTED INCANDESCENT HALF-SMITCHED AFCI DUPLEX OUTLET -D- RECESSED LIGHT FIXTURE \$220V 220 VOLT AFCI OUTLET ① THERMOSTAT ---- GAS SUPPLY WITH VALVE PH TELEPHONE REINFORCED JUNCTION BOX O<sup>LED</sup> LED SURFACE MOUNTED ▼ TELEVISION WALL SMITCH ──₩ HOSE BIBB EXHAUST FAN (VENT TO EXTERIOR ELECTRIC METER \$ 3 THREE-WAY SWITCH ELECTRIC PANEL -+CM 1/4" WATER STUB OUT \$4 FOUR-WAY SMITCH EXHAUST FANVLIGHT COMBINATION (VENT TO EXTERIOR) DISCONNECT SWITCH □□□ LED STRIP FIXTURE TECH HUB SYSTEM

ELECTRICAL PLAN FIRST FLOOR - ELEV. I SCALE: 1/8" = 1'-0"

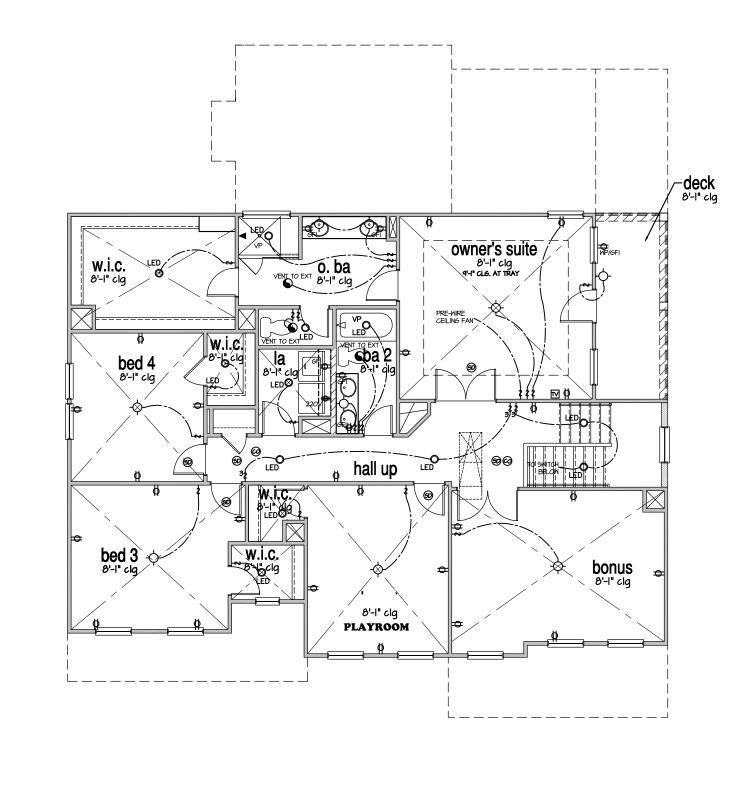
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STONEFIELE
DRAWING TITLE

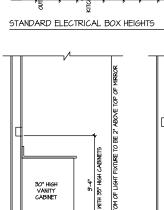
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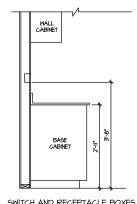
DRAWN BY:

DATE: 06/26/2025

PLAN NO. 1635







SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

NOTES: - PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING O PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRIC COD (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.

- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.

- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.

ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUM DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.

- PROVIDE POMER, LIGHT AND SMITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

CLOSET LIGHTS TO BE FLOURESCENT FIXTURES FOR NC & INCANDESCENT CLG. MOUNTED FIXTURES FOR ALL OTHER AREAS.

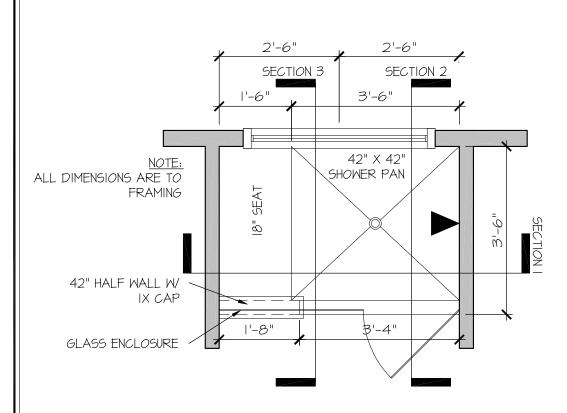
	LEGEND:						
,	AFCI DUPLEX OUTLET	ф	CEILING MOUNTED INCANDESCENT LIGHT FIXTURE	갦	CHIMES		CEILING FAN
CODES. ICAL	MWP/GFI MEATHERPROOF GFI AFCI DUPLEX OUTLET	-Q <sub>k</sub>	KEYLESS LIGHT	9	PUSHBUTTON SWITCH		(PROVIDE ADEQUATE SUPPORT)
	GROUND-FAULT CIRCUIT-INTERRUPTER AFCI DUPLEX OUTLET	ф	WALL MOUNTED INCANDESCENT	99	IIOV SMOKE DETECTOR W BATTERY BACKUP	CEILING FAN WITH INCANDESCENT	
	HALF-SMITCHED AFCI DUPLEX OUTLET	- :	LIGHT FIXTURE	0	CO2 DETECTOR	<b>M</b>	(PROVIDE ADEQUATE SUPPORT)
UMP PITS,	\$220V 220 VOLT AFCI OUTLET	ф	RECESSED LIGHT FIXTURE	Ð	THERMOSTAT	∞	CAC CHOOL A PILLITALING
5	REINFORCED JUNCTION BOX	OLED	LED SURFACE MOUNTED	PH	TELEPHONE		
•	\$ WALL SMITCH			M	TELEVISION	— ∳B HOSE BIBB	
	\$ 3 THREE-WAY SMITCH	•	EXHAUST FAN (VENT TO EXTERIOR)		ELECTRIC METER		
	4				ELECTRIC PANEL		
	\$4 FOUR-MAY SMITCH	-	EXHAUST FANLIGHT COMBINATION (VENT TO EXTERIOR)	-	DISCONNECT SWITCH	— ton	I/4" WATER STUB OUT
	□=□ LED STRIP FIXTURE		TECH HUB SYSTEM				

ELECTRICAL PLAN SECOND FLOOR - ELEV. I

SCALE: 1/8" = 1'-0"

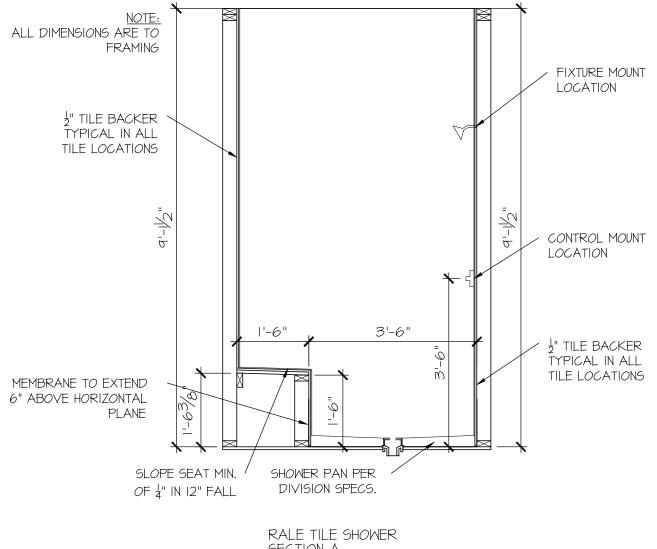
DRAWN BY: DATE: 06/26/2025 PLAN NO. 1635

ᇳ HOUSE NAME:
STONEFIELD
DRAWING TITLE
SECOND FLOOR



RALE TILE SHOWER 42" X 42" W 18" SEAT

SCALE: 1/2" = 1'-0"



SECTION A

SCALE: 1/2" = 1'-0"

CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE

24 X 36 SCALE



DETAIL SHOWER RALE



SEAL

DRAWN BY:
L. BEAVERS
DATE: 9/1/22

PLAN NO.

24 X 36 SCALE

~ "

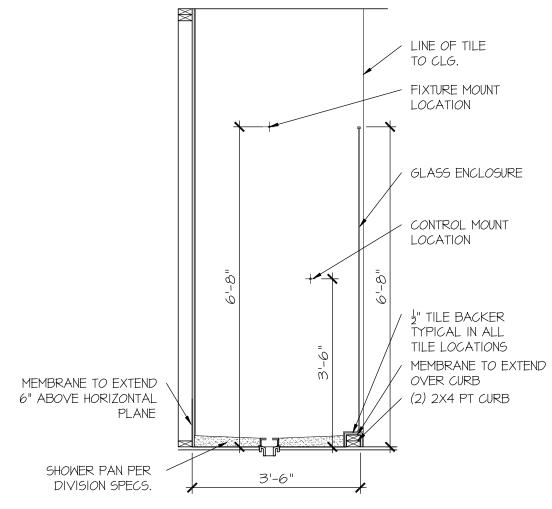


E ILE SHOWER DETAIL

OUSE NAME:

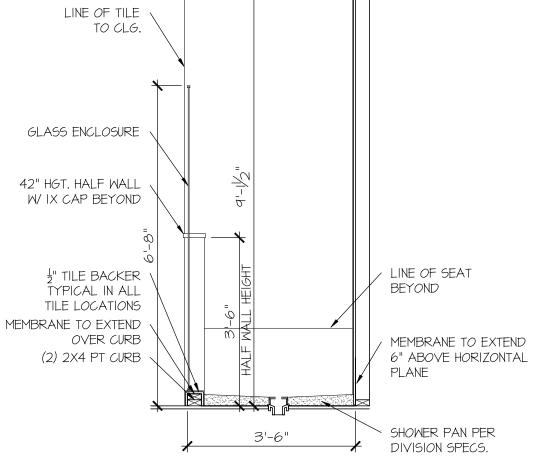
SHEET No.

P||.2



SCALE: 1/2" = 1'-0"





SCALE: 1/2" = 1'-0"

DESCRIPTION OF BLDG, ELEMENT	3"x0.131" NAILS	3"x0.120" NAIL5
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS <b>@</b> 4" o.c.	(3) NAILS • 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS @ 8" O.C.	TOENAILS @ 6" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS @ 24" o.c.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP @ CORNERS & INTERSECTING WALLS	(2) NAILS	(2) NAILS

2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE \* ARE SHOWN)

#### MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOT SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSIRE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO: FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES

#### ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS OTED OTHERWISE ON PLAN

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN AD IACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS:
- LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

#### GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

#### DESIGN LOADS

ROOF DEAD = 7 PSF T.C., IO PSF B.C.

LOAD DURATION FACTOR = 1.25

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN)

IO PSF T.C., 5 PSF B.C. (TRUSSES)

(ADD'L IO PSF @ TILE)

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

#### GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS, ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION. ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(I) FOR ALL CONNECTIONS, TYP, U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
   I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. • WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPF) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY, CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) • HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
  - (1)2x4/6 FLAT @ OPENINGS UP TO 4'. (2)2x4/6 FLAT UP TO 8'
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15). ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- "LSL" Fb=2325 psi: Fv=3I0 psi: E=I,55xI0^6 psi
- 'LVL' Fb=2600 psi; Fv=285 psi; E=2.0x10^6 psi
- 'PSL' FB=2900 PSI; FV=290 PSI; E=2.0XI0^6 PSI M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR
- INSTALLATION. FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x3/5" SIMPSON SDS SCREWS (OR 3½" TRUSSLOK SCREWS) & 16" O/C. USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF  $\frac{1}{4}$ "x6" SIMPSON SDS SCREWS (OR 6  $\frac{3}{4}$ " TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE, A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND,/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

#### FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT M&K FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131" NAILS @ 6"o.c. @ PANEL EDGES & @ 12"o.c. FIELD.
- 2 🖥 × 0.120" NAILS 4" O.C. PANEL EDGES € 8" O.C. FIELD. - 2 3" x 0,113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN. SCREWS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

#### ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) @ ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H2 5T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO
  - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS W/ (3) 3"x0.131" NAILS @ 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12" O.C. FIELD. - w/ 2 (8 × 0.120" NAILS @ 4"o.c. @ PANEL EDGES & @ 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"O.C. @ PANEL EDGES & @ 6" O.C. FIELD.

#### VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	STEEL ANGLE SIZE
3'-0"	20 FT. MAX	L3"x3"x¼"
	3 FT. MAX	L3"x3"x¼"
6'-0"	I2 FT. MAX	L4"x3"x¼"
20 FT. MAX		L5"x3½"x%"
	3 FT. MAX	L4"x4"x¼" *
8'-0"	I2 FT. MAX	L5"x3½"x%"
	I6 FT. MAX	L6"x31½"x3%"
9'-6"	I2 FT. MAX	L6"x3½"x¾6"
16'-0"	2 FT. MAX	L7"x4"x½" **
10-U	3 FT. MAX	L8"x4"x½" **

## · SHALL SUPPORT 2 3/8" - 3 1/2" VENEER W/ 40 psf MAXIMUM WEIGHT < 16' SHALL HAVE 4" MIN. BEARING

- IG' SHALL HAVE B" MIN BEARING
- 16' SHALL NOT BE FASTENED BACK TO HEADER
- 16' SHALL BE EASTENED BACK TO WOOD HEADER IN WALL 448" W/½" DIA. x 3 ½" LONG LAG SCRENG IN 2" LONG VERTICALLY SLOTTED HOLES.
- MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
  WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE
- HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE  $3\,\%$ " WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT FINISHING, SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL
- FASTENED BACK TO BEAM, FASTENERS SHALL MAINTAIN A 2½" (MINIMUM) CLEAR DISTANCE FROM BOTTOM OF BEAM. FOR QUEEN VENEER USE L4x3x FOR 3/5" VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RO

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301.2.1.1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NCSBC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED

THE CODE REQUIRED LATERAL FORCES.

#### SECTIONS R60235& R80211 EXT. WALL SHEATHING SPECIFICATION

TO RESIST THE WIND UPLIFT LOAD PATH PER

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES & ● 12" O.C. IN THE PANEL FIELD. TYP, U.N.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1½" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6" O.C IN FIELD.

#### BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 36" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 34" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C. IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2X HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

#### 3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING w/8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- PRE-MANUFACTURED PANELIZED WALLS FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

OR 3" O.C. OSB SHEARWALL.

► INDICATES HOLDOWN BELOW

INDICATES EXTENT OF INT. OSB SHEARWAI

#### GENERAL STRUCTURAL NOTES

#### **FOUNDATION**

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE 12" MAX FROM PLATE ENDS - LITTLIZING
- I/2" DIA. ANCHOR BOLTS € 6'-0" O.C, 7" MIN. EMBEDMENT
- (CONC.) 15" MIN EMBEDMENT (CMU) SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS 2'-8" O.C. (CMU)
- (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS) ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACL 318 CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: ...... FOUNDATION WALLS
  2,500 psi: ...... FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: ...... GARAGE & EXTERIOR SLABS ON GRADE
- 60,000 psi BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
- TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (91/5" FOR 10" THICK WALL).
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC, ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKELLING BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS
- FOR OPENINGS UP TO 36", PROVIDE MINIMUM IO" CONCRETE DEPTH OVER OPENING OR (3)2x10 W/ (2)2x6 JACK STUDS, U.N.O.
- LARGER OPENINGS SHALL BE PER PLAN. • ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS
- THAN 5% OR MORE THAN 7% AIR ENTRAINMENT • ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB. EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP.
  - JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" OC (MAXIMUM)
- JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I.I RATIO), WITH A MAXIMUM OF I.I.5 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- SI ABS CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi), MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 € 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x6 (MIN.) x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE FASTENED PER ANCHORAGE SPECIFICATIONS NOTED ABOVE. TOP 2 COURSES (MIN.) OF PIER TO BE GROUTED SOLID (8 COURSE MAX. PIER HEIGHT).
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE. TOP 2 COURSES (MIN.) OF WALL TO BE GROUTED SOLID (8 COURSE MAX, WALL HEIGHT)
- DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES, METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

#### HOLD-DOWN SCHEDULE

SYMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM U.N.O.) -OR- MSTC66B3 ALTERNATE
► HD-3	SIMPSON STHDI4/STHDI4RJ

\* UTILIZE THE SSTB24 ANCHOR BOLT @ ALL MONOSLAB & INTERIOR RAISED 5LAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE IO" (FOR 5/8" DIA.) OR 15" (FOR 7/8" DIA.) MIN. EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D.

<u>DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE</u>

<sup>2al:</sup> 7/9/25 CAR PROFESSION. O. MGINE SEPHT. R

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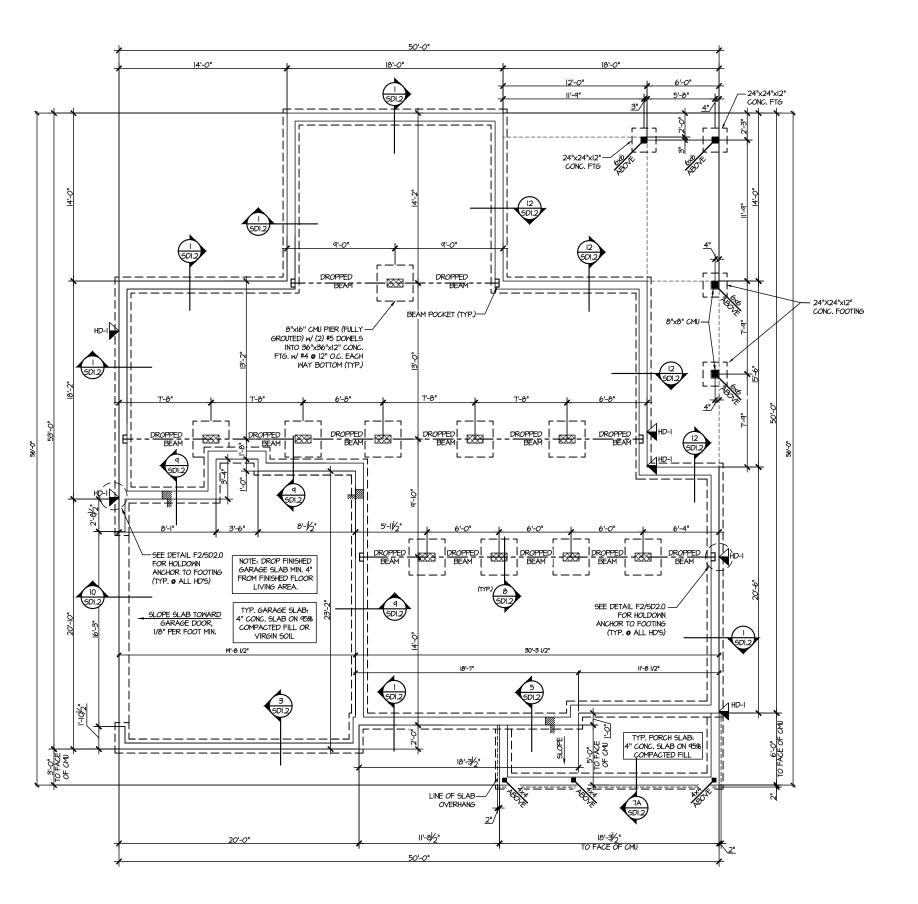
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CRAWL SPACE FOUNDATION PLAN SCALE 1/8"=1'-0"

eal: 7/9/25 H CAR SEPH T. R

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M&K project number 126-2306

JTR drawn by: issue date: 07-09-2

REVISIONS:

LEGEND

• = = INDICATES SHEAR WALL & EXTENT • EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

REFER TO SO.O FOR

TYPICAL STRUCTURAL NOTES & SCHEDULES

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

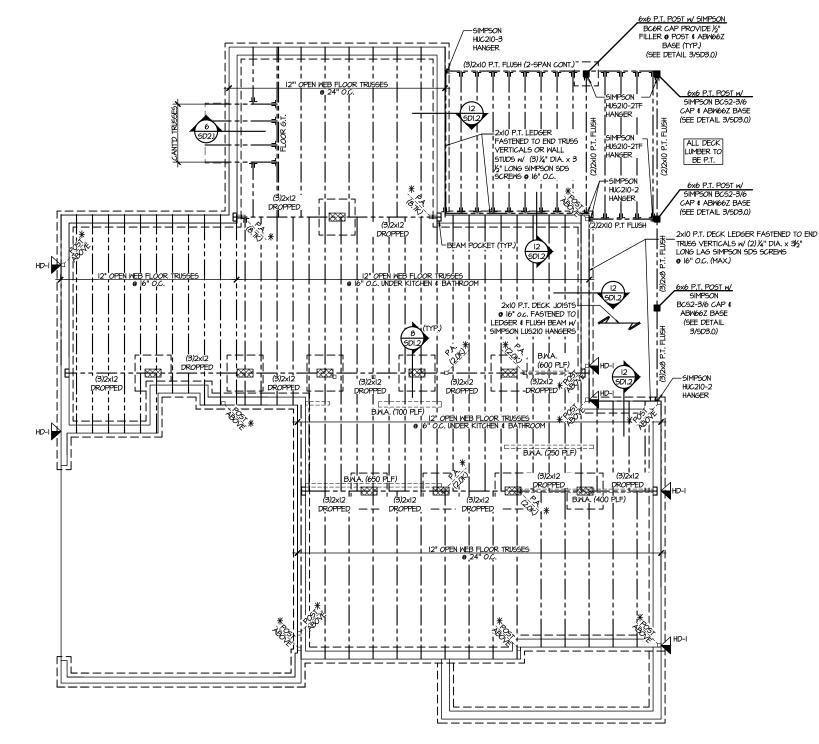
• IIIIII INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE

• BEAM / HEADER

JL METAL HANGER

BLAKE POND LOT 7 - STONEFIELD 1 RALEIGH, NC OUNDATION

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	ENGINEERED BEAM MATERIAL SCHEDULE				
STEEL OPTION	FLITCH OPTION	LSL OPTION	PSL OPTION	LVL OPTION	BEAM NUMBER
WI2xI4 - D	(2)2xl2 + (l) %"xll¼" STEEL FLITCH PLATES - D	N/A	3½"x16" - D	(2)13/4"×16" - D	001
WI2xI4 - D	(2)2xl2 + (l) 以"xli以" STEEL FLITCH PLATES - D	(3)13/4"×117/6" - D	3½"xll%" - D	(2)134"×1176" - D	002
WI2xl4 - F	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - F	(2)13/4"×14" - F	3½"x14" - F	(2)134"×14" - F	003
WI2xI4 - 5	(2)2xl2 + (l) %"xl以" STEEL FLITCH PLATES - F	(3)19¼"x14" - F	3½"x14" - F	(2)134"×14" - F	004
WI2x26 - F	(3)2xl2 + (2) %"xll4" STEEL FLITCH PLATES - F	N/A	5¼"×l8" - FT	(3)1¾"x18" - FT	005
WI2x26 - F	(3)2xl2 + (2) %"xll4" STEEL FLITCH PLATES - F	N/A	5¼"xl8" - FT	(3)13/4"x18" - FT	006
WI2xI4 - F	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - F	(2)13/4"×14" - F	3½"x14" - F	(2)13/4"×14" - F	007
WI2xI4 - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F	(2)13/4"×14" - F	3½"x14" - F	(2)1 <sup>5</sup> ¼"x14" - F	008
WI0x12 - D	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - D	(2)194"×94" - D	3½"×9¼" - D	(2)134"×94" - D	004
N/A	(3)2xl2 + (2) ½"xll¼" STEEL FLITCH PLATES - H	(3)134"x16" - H	3½"x16" - H	(2)13/4"x16" - H	010
WI2xI4 - D	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - D	(3)1¾"x11%" - D	3½"xII%" - D	(2)134"x1176" - D	OII
WI2xI9 - D	(3)2xl2 + (2) ½"xli以" STEEL FLITCH PLATES - D	(4)1¾"×16" - D	5¼"xi6" - D	(3)134"x16" - D	012
WI2xI4 - D	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - D	(3)1¾"×11%" - D	3½"xII%" - D	(2)134"×111%" - D	013
WI2xI4 - D	(3)2xl2 + (2) %"xllk" STEEL FLITCH PLATES - D	(3)1¾"x16" - D	3½"x16" - D	(2)134"x16" - D	014
WI2xI4 - D	(3)2xi2 + (2) ¼"xil¼" STEEL FLITCH PLATES - D	(3)13/4"x117/6" - D	N/A	(3)1¾"x11½" - D	015***
WI2xI4 - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F	(3)1¾"x11%" - F	3½"xII%" - F	(2)1¾"x11½" - F	016

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM
- "FB" INDICATES FLUSH BOTTOM BEAM

- "FP" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER
REFER TO DETAIL D/5D2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL E/5D2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W (2) 3"X0.120" NAILS @ 8" O.C.

FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION w/ (2) 3"X0.120" NAILS @ 8" O.C.

\*\*\* - SEE PLAN FOR EXTENT OF 3-PLY BEAM

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BLAKE POND Lot 7 - Stonefield 1 Raleigh, nc OR

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES

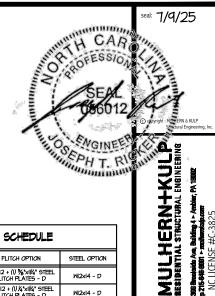
SD2, REFERS TO SD2, IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LEGEND

- IIIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

& SCHEDULES

IST FLOOR FRAMING PLAN



ENGINEERED BEAM MATERIAL SCHEDULE LVL OPTION PSL OPTION LSL OPTION FLITCH OPTION STEEL OPTION 2)2x12 + (I) %"xIIV" STEEL FLITCH PLATES - D OOL (2)13/4"×16" - D WI2xI4 - D (2)2xl2 + (l) ¼"xll¼" STEE FLITCH PLATES - D 002 (2)134"x1134" - D (3)13/4"x117/4" - D 35"xII%" - D WI2xI4 - D (2)2xl2 + (l) 从"xll以" STEEL FLITCH PLATES - F 003 (2)13/4"×14" - F 36"xl4" - F (2)13/4"×14" - F WI2xI4 - F (2)2xl2 + (l) %"xl以" STEE FLITCH PLATES - F 004 (2)134"x14" - F (3)13/4"×14" - F WI2xI4 - F 3½"xl4" - F (3)2xl2 + (2) %"xll4" STEEL FLITCH PLATES 005 (3)13/4"x18" - FT 5¼"xl8" - FT N/A WI2x26 - F 006 (3)13/4"x18" - FT 5¼"xi8" - FT WI2x26 - F (2)2xi2 + (i) 从"xil以" STEE FLITCH PLATES - F 001 (2)134"x14" - F (2)13/4"×14" - F WI2xI4 - F 3/5"xl4" - F 008 3½"xl4" - F WI2xI4 - F (2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - D (2)134"x94" - D 3½"x9¼" - D (2)13/4"x9/4" - E MIOxI2 - D 3)2xi2 + (2) ½"xii¼" Stee Flitch Plates - H 010 (2)13/4"x16" - H 31/5"x16" - H (3)13/4"x16" - H N/A (2)2xl2 + (l) ¼"xll¼" STEEL FLITCH PLATES - D (2)134"x1136" - D 3%"xII%" - D (3)134"x1136" - D WI2xI4 - D 3)2xi2 + (2) ½"xil¼" STE FLITCH PLATES - D 012 (3)134"x16" - D (4)134"x16" - D W12x19 - D 5¼"x16" - D (2)2xl2 + (1) ¼"xll¼" STEE FLITCH PLATES - D 013 (2)134"x1136" - D 3½"x11%" - D (3)134"x1136" - D WI2xI4 - D 3)2xI2 + (2) %"xIK" STEEL FLITCH PLATES - D (2)|3/4"x|6" - D (3)13/4"x16" - D WI2xI4 - D 36"x16" - D (3)2x12 + (2) ¼"x11¼" STEE FLITCH PLATES - D (3)134"x1136" - D (3)134"x1136" - D WI2xI4 - D NΑ (2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F 016 (3)13/4"x117/6" - F (2)134"x1136" - F 3½"x11%" - F WI2xI4 - F

BEAM NOTATION: - "F" INDICATES FLUSH BEAM - "FT" INDICATES FLUSH TOP BEAM

"FB" INDICATES FLUSH BOTTOM BEAM

- "FP" INDICATES FLUSH BOTTOM BEAM
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REFER TO DETAIL D/5D2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
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FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'ID. FASTEN PLATES IN SUCCESSION W (2) 3"X0.120" NAILS @ 8" O.C.

FOR FLUSH BOTTOM BEAMS PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION w/ (2) 3"XO.120" NAILS • 8" O.C.

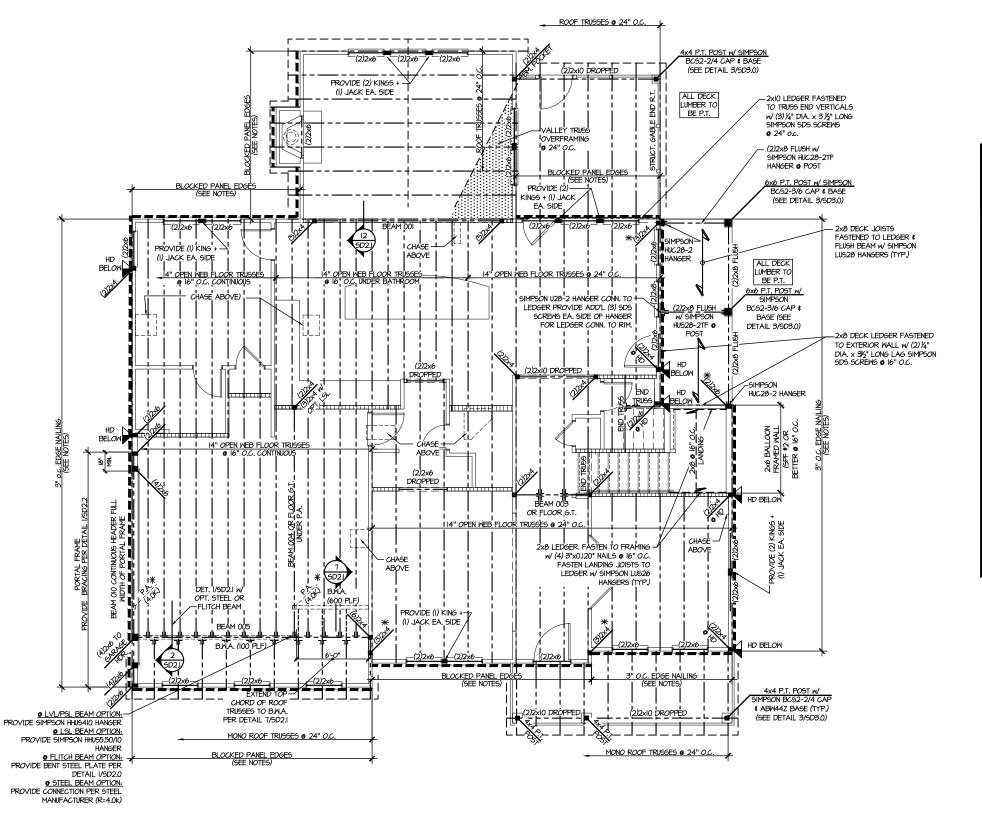
\*\*\* - SEE PLAN FOR EXTENT OF 3-PLY BEAM

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP.
  REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



2ND FLOOR FRAMING PLAN

I&K project number

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POND LOT 7 - STONEFIELD RALEIGH, NC AKE BL

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RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number: 126-23061

JTR drawn by: issue date: 07-09-2

REVISIONS:

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FRAMING

LEGEND

ullet = ullet INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

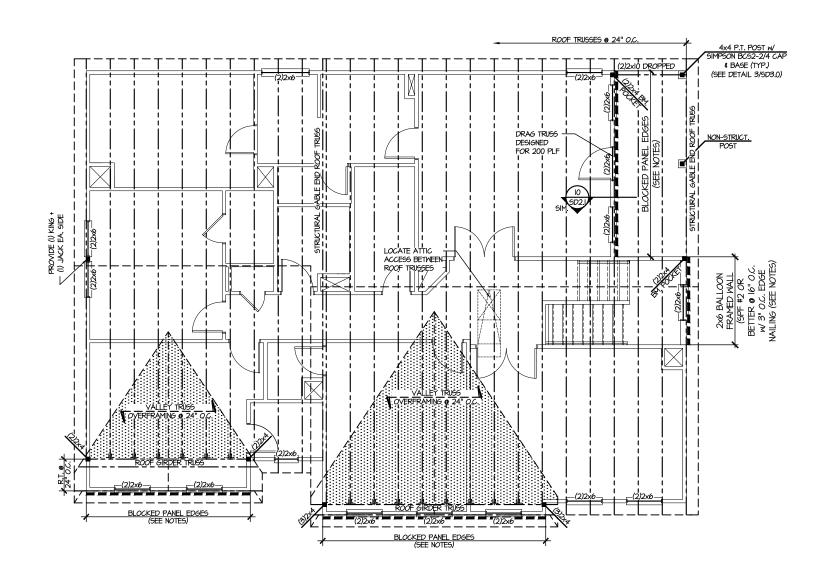
INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

INTERIOR BEARING WALL

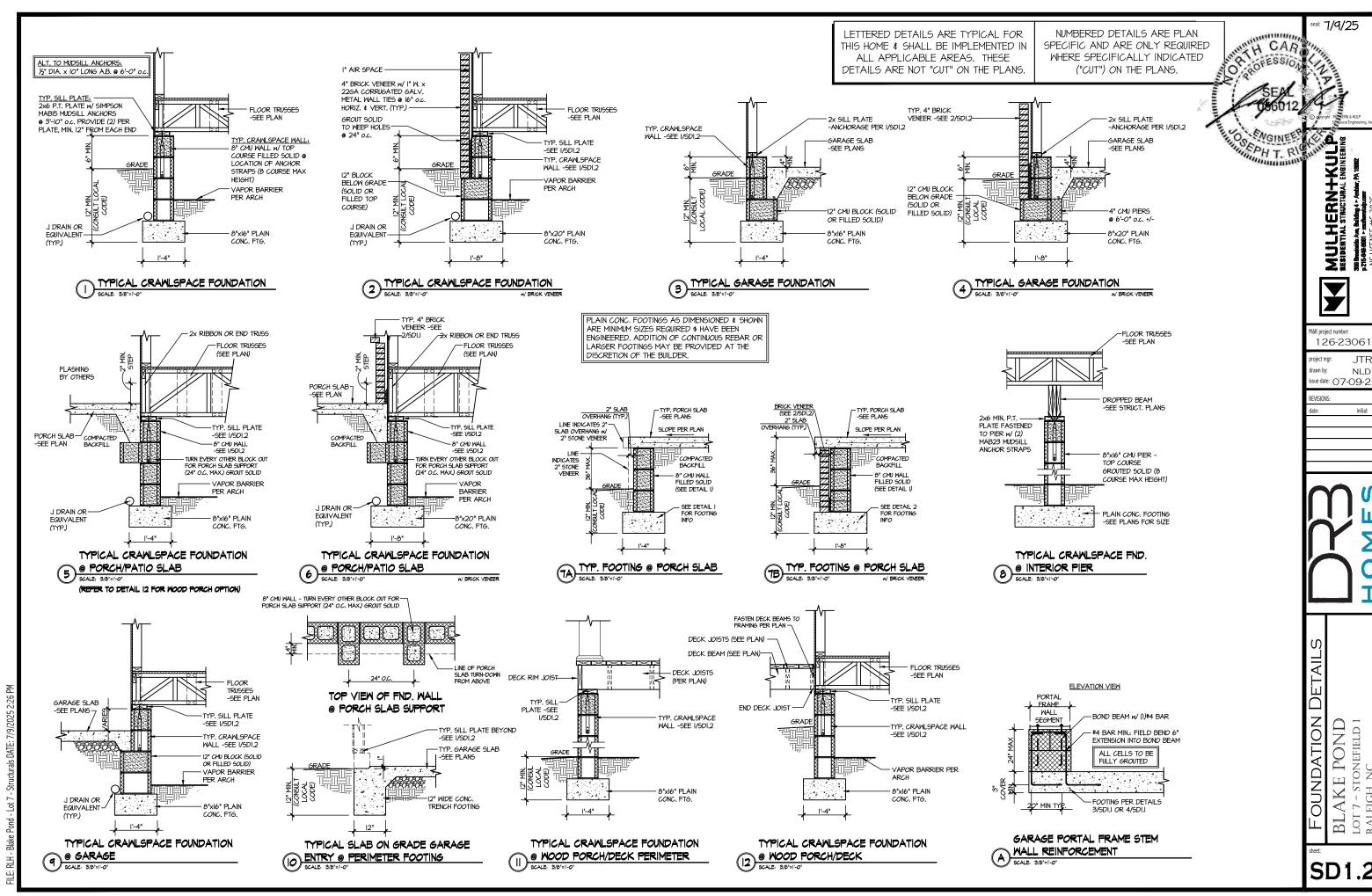
● □===□ BEARING WALL ABOVE BEAM / HEADER

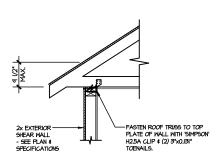
JL METAL HANGER

BLAKE POND Lot 7 - Stonefield 1 Raleigh, nc ROOF



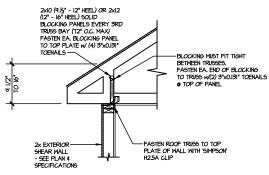






### TYPICAL SHEAR

TRANSFER DETAIL @ ROOF HEEL HEIGHT LESS THAN 9½" NO BLOCKING REQ'D



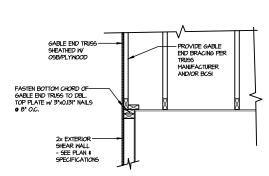
#### TYPICAL SHEAR

TRANSFER DETAIL @ ROOF SCALE: 3/8"=1"-0" HEEL HEIGHT BETWEEN 9½" - 16" BLOCKING REQ'D



2x BLK'G TO FIT TIGHT IN EA.

TRUSS BAY FASTENED TO TRUSS w/(2) 3"x0.131" TOENAILS @ EA. END.



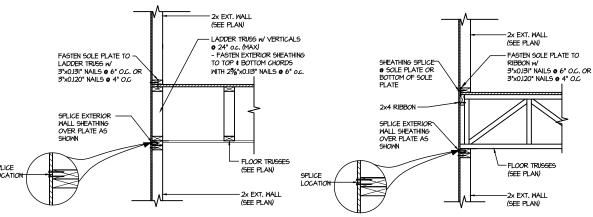
TYPICAL GABLE END DETAIL SCALE: 3/8"=1"-0"

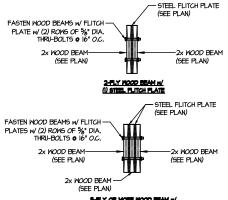
OSB/PLYWOOD SHEATHING UP VERTICALS OF ROOF TRUSSES, & FASTEN W

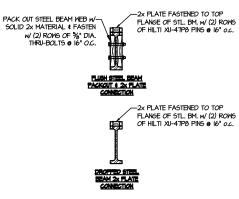
2%"x0.120" NAILS @ 6" O.C.

2x BLK'G IN EA. TRUSS BAY FASTENED TO DBL TOP PLATE w/ (3) 3"x0.131" NAILS.

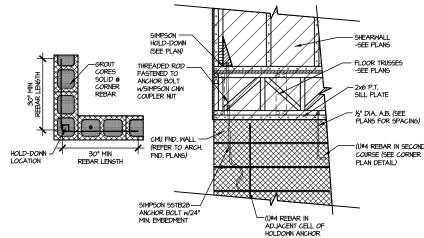
— FASTEN ROOF TRUSS TO TOP PLATE OF WALL WITH SIMPSON' H2.5A CLIP & (2) 3"X0.131" TOENAILS.







TYPICAL STEEL BEAM CONNECTION DETAIL



TYPICAL CORNER FOUNDATION HOLD-DOWN INSTALLATION SCALE: N.T.S.

THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

LETTERED DETAILS ARE TYPICAL FOR

LOT 7 - STONEFIELD 1 RALEIGH, NC

Ŋ

eal: 7/9/25

MULHERN+KULP HESIDENTIAL STRUCTURAL ENGINEERING

M&K project number:

drawn by:

REVISIONS

126-2306

ssue date: 07-09-2

**JTR** 

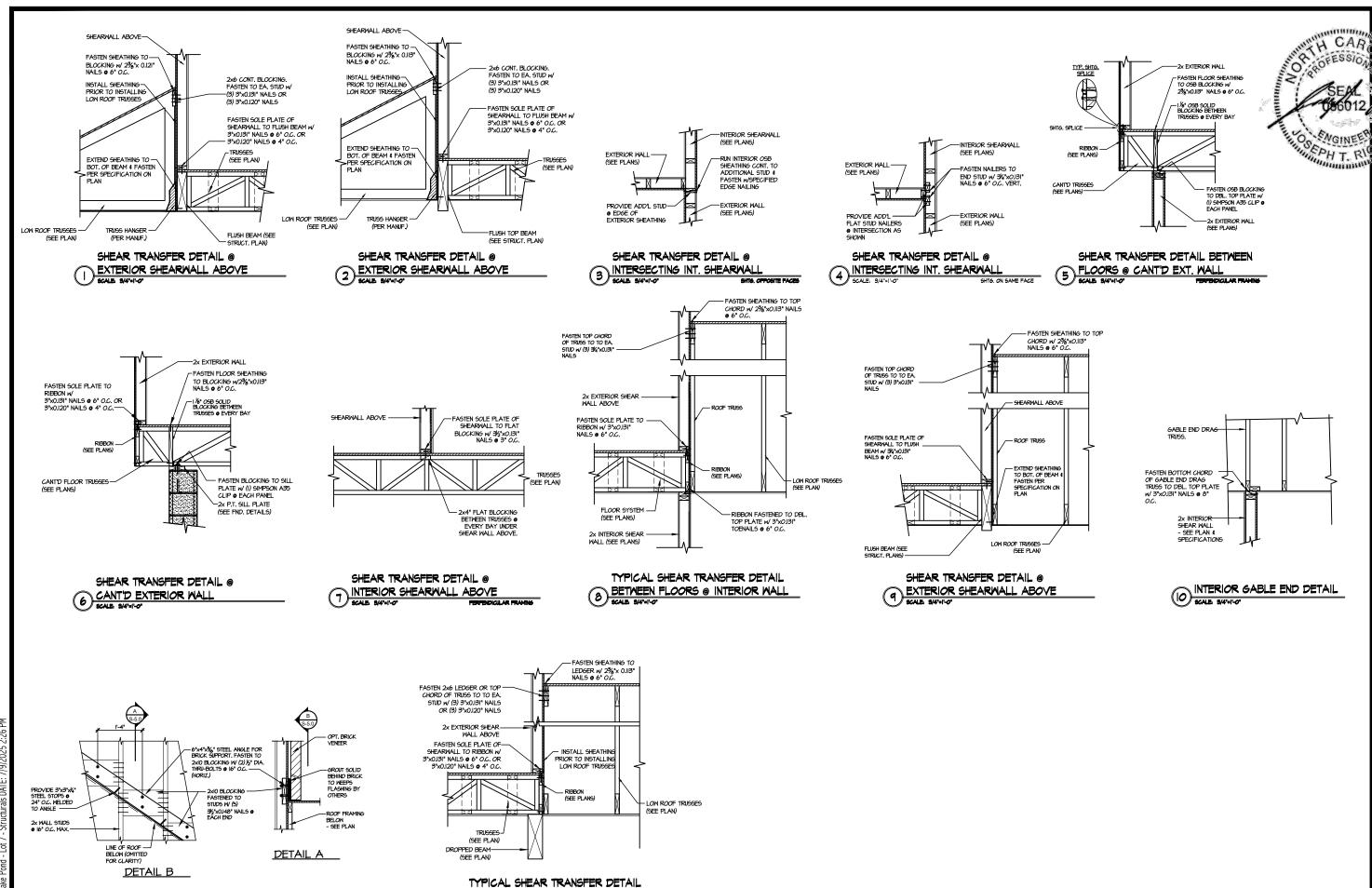
initial:

TH CAR P ROFESSION

OSEPH T. RI

O.

S-FLY OR MORE WOOD BEAM N/ (2 OR MORE) STEEL FLITCH FLATES TYPICAL SHEAR TRANSFER DETAIL TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL BETWEEN FLOORS @ EXTERIOR WALL SCALE 5/0'-1-0' FENEROIGIAN FROME TYPICAL FLITCH BEAM CONNECTION DETAIL SCALE SIA\*-IF-OF



BETWEEN FLOORS @ INTERIOR WALL

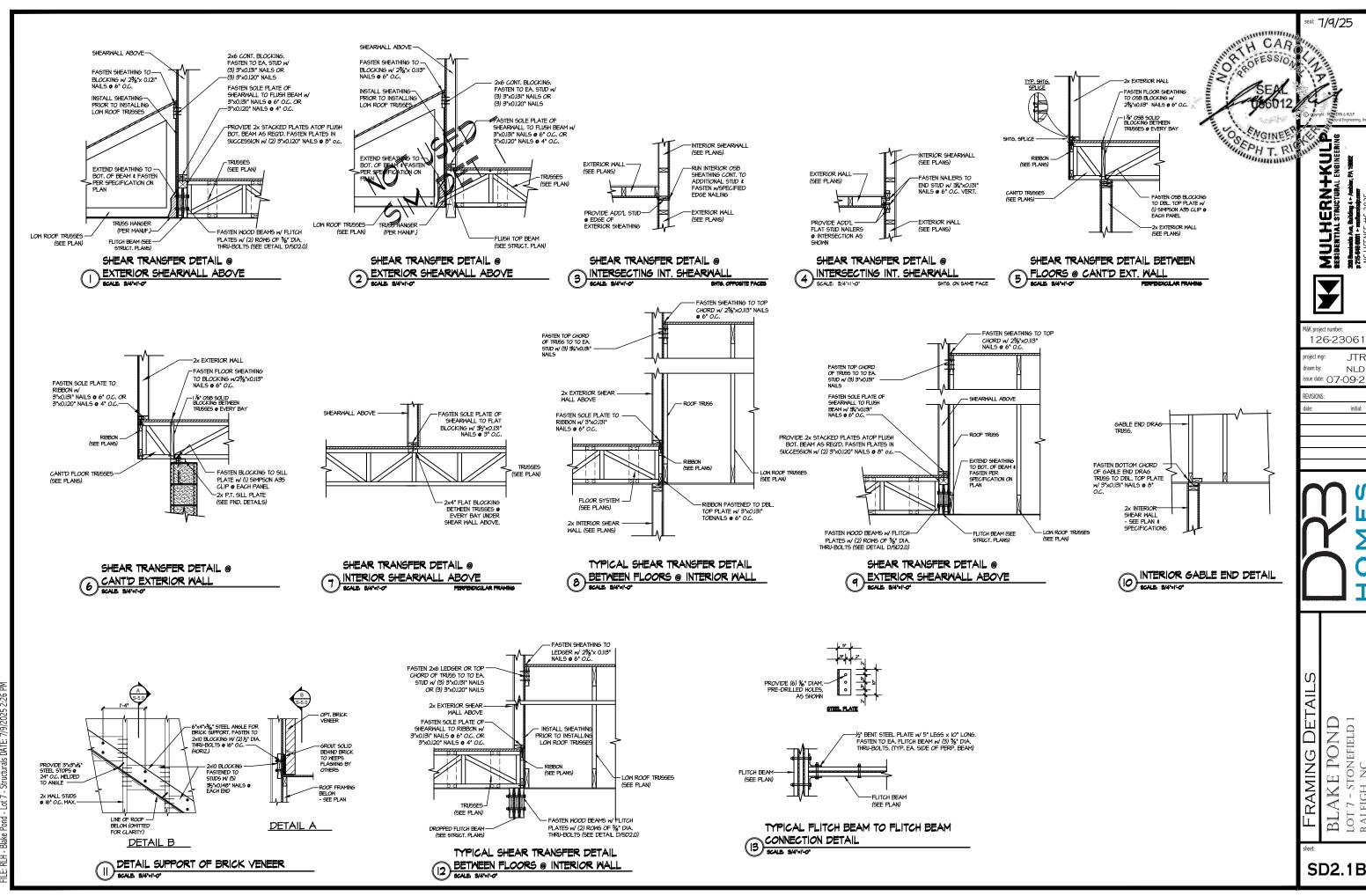
DETAIL SUPPORT OF BRICK VENEER

MULHERN+KULP RESIDENTIAL STRUCTURAL ENGINEERING M&K project number 126-2306 **JTR** frawn by: ssue date: 07-09-2 REVISIONS initial: POND Ü AKE

eal: 7/9/25

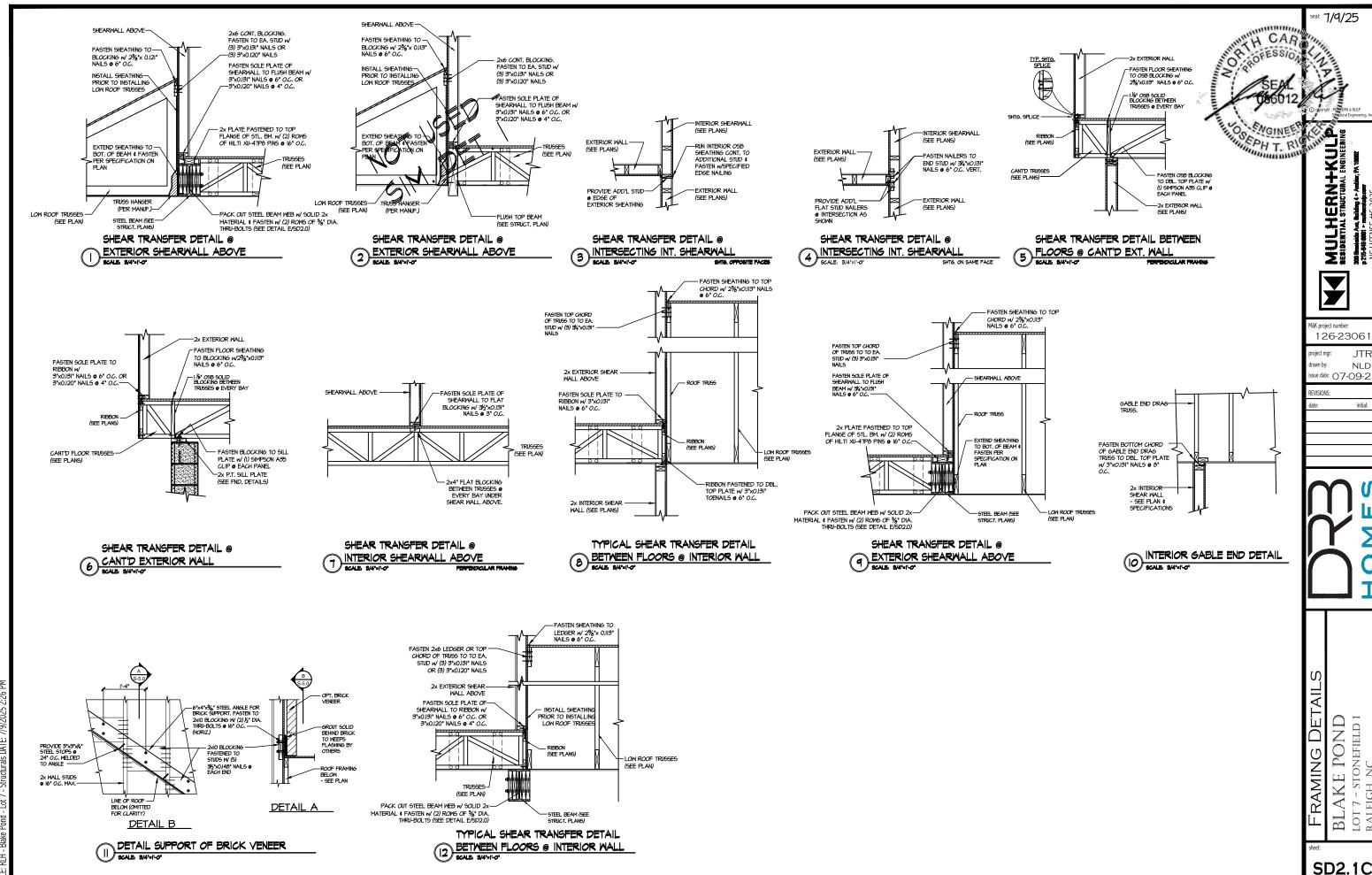
SD2.1A

BL



**JTR** 

initial:



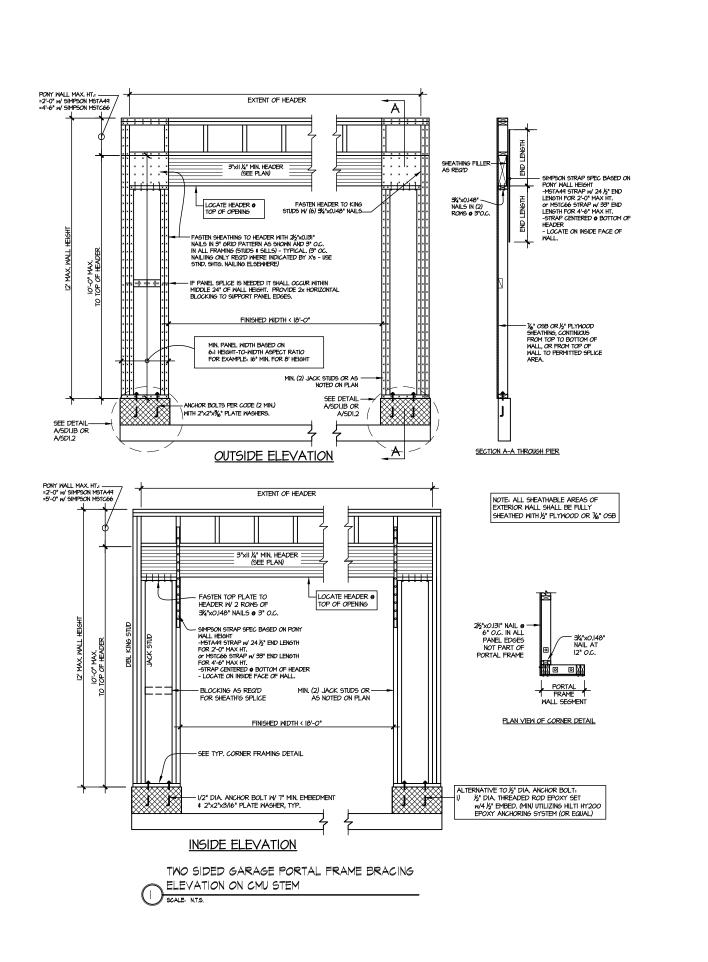
**JTR** 

initial:

POND

AKE

BL



eal: 7/9/25 OR AOFES OS ENGINEE MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING

M&K project number: 126-23061

roject mgr: **JTR** drawn by: issue date: 07-09-2

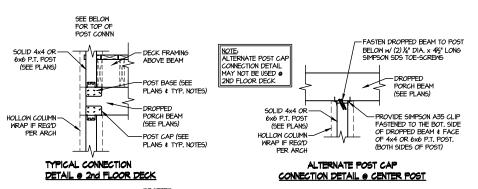
REVISIONS:

initial:

RAMING DETAILS

BLAKE POND LOT 7 - STONEFIELD 1 RALEIGH, NC





-CONC. TRENCH FOOTING

HOLLOW COLUMN-WRAP IF REQ'D PER ARCH — POST CAP (SEE PLANS & TYP. NOTES) -POST BASE (SEE PLANS & TYP. NOTES) W ½" DIA.

TYP. NOTES) W ½" DIA.

ANCHOR BOLT OR SIMPSON

TITEN HD W MIN. 6" EMBED.

SLOPE

TYPER PLAN

(Si SOLID 4x4 OR-6x6 P.T. POST (SEE PLANS) / TYP. PORCH SLAB (SEE FND DETAILS)

-FASTEN DROPPED BEAM TO POST

BELOW w/ (2) ¼" DIA. x 4½" LONG SIMPSON SDS TOE-SCREWS

— DROPPED PORCH BEAM (SEE PLANS)

-PROVIDE SIMPSON A35 CLIP FASTENED TO THE BOT. SIDE OF DROPPED BEAM & FACE OF 4x4 OR 6x6 P.T. POST.

ALTERNATE POST CAP CONNECTION DETAIL @ END CONDITION

NOTE: ALTERNATE POST CAP CONNECTION DETAIL MAY NOT BE USED @ 2ND FLOOR DECK

SOLID 4x4 OR -6x6 P.T. POST (SEE PLANS)

HOLLOW COLUMN-WRAP IF REQ'D PER ARCH

TYPICAL PORCH 3 POST CONNECTION DETAIL SUPPLY SUPPL

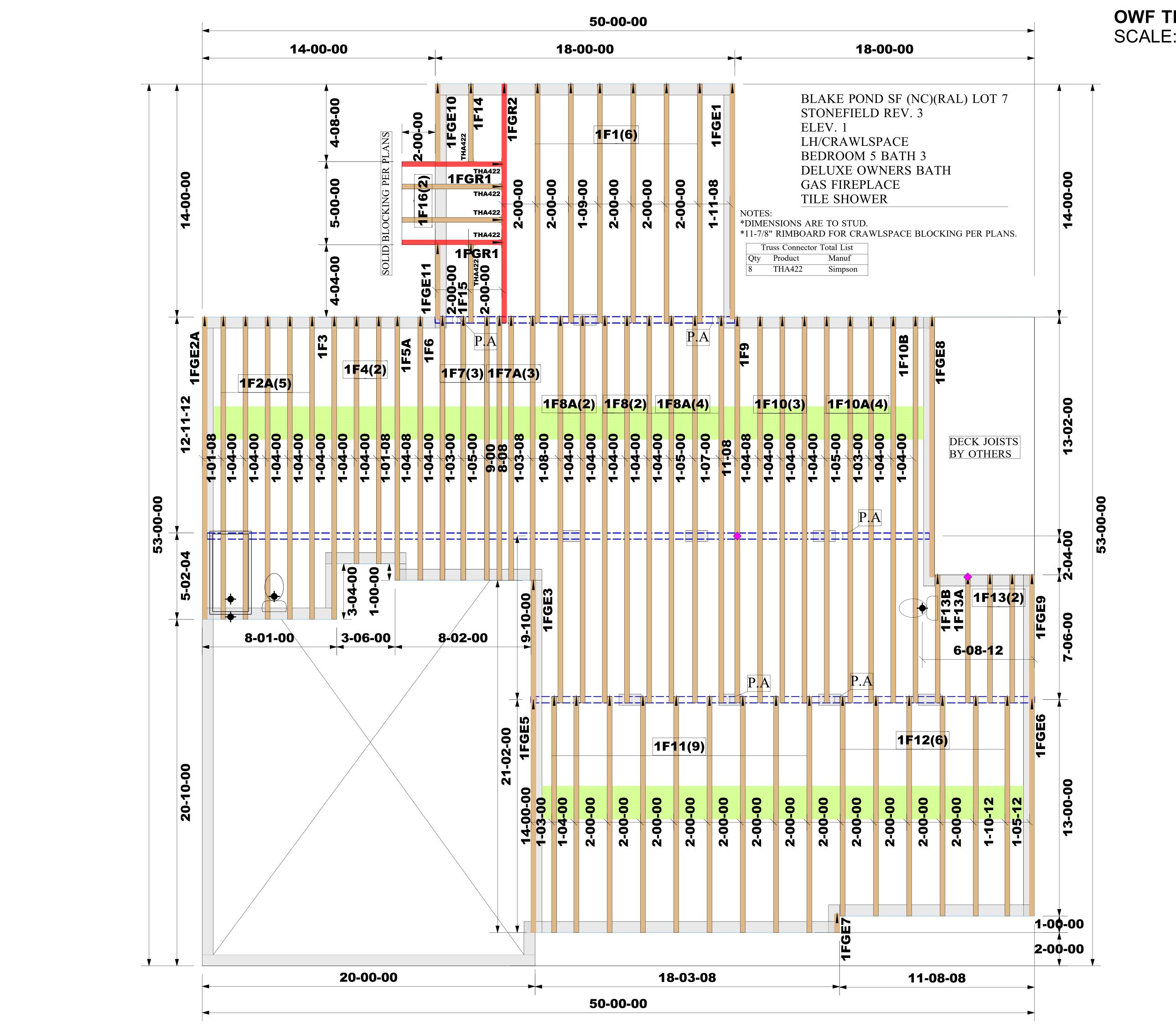
FRAMING DETAILS BLAKE POND LOT 7 - STONEFIELD 1 RALEIGH, NC

JTR

NLD

initial:

**SD3.0** 



**OWF TRUSS LAYOUT** SCALE: NTS



Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

OWF 00.0007 **Pond Lot** Raleigh Blake

Stonefield

Model Name: TPI Plant W974

lame:

00.000

ot

NOTE:

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR TH CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES

**WARNING:** 

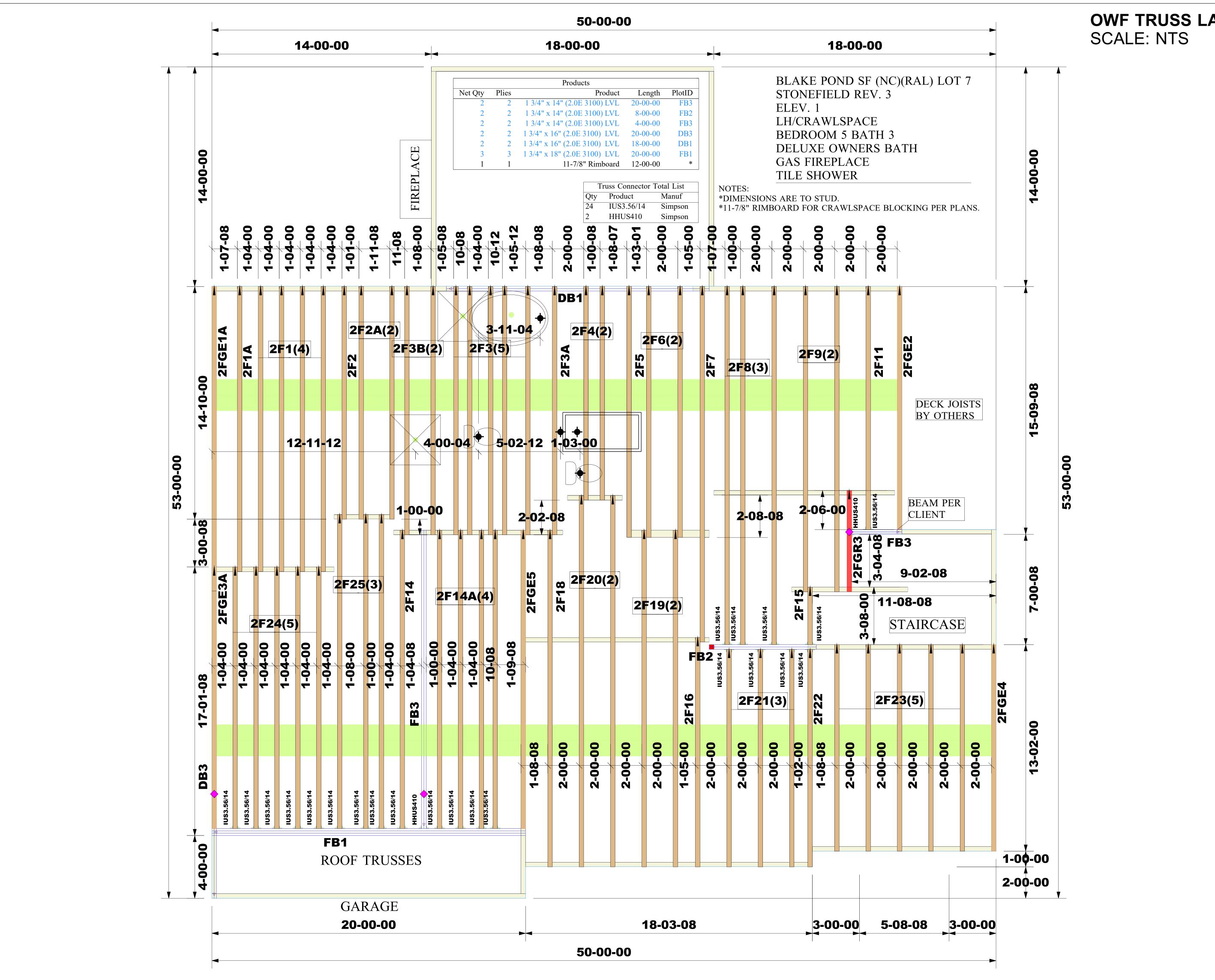
CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING TH **ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN** TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTAR AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY

2506-1217

Roger Espinoza

Robbie Zarobinski



**OWF TRUSS LAYOUT** 



Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

OWF 00.0007 Lot Raleigh Pond Blake

Stonefield

Model Name:

00.000

ot

TPI Plant W974

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**WARNING:** 

BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING TH ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN

POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TO PREVENT DAMAGE OR PERSONAL INJURY

2506-1217

Roger Espinoza

Robbie Zarobinski