

Trenco 818 Soundside Rd Edenton, NC 27932

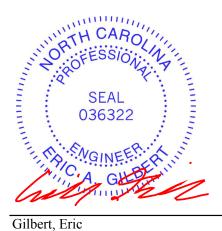
Re: J0225-1026 Lot 20 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I74893527 thru I74893556

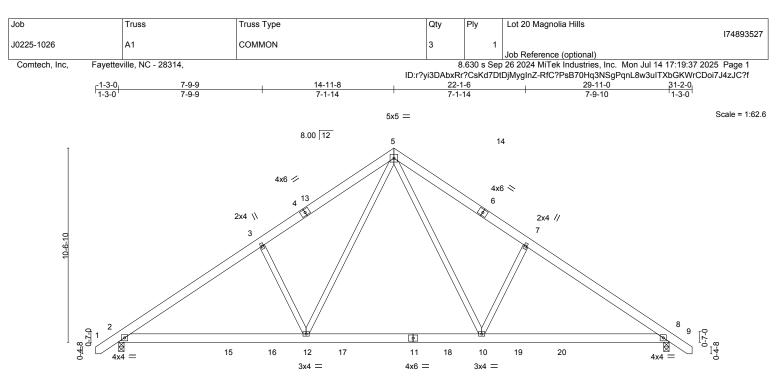
My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



July 15,2025

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	<u> </u>		<u>19-8-12</u> 9-6-9	29-11- 10-2	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.24 BC 0.51 WB 0.35 Matrix-S	Vert(LL) -0.11 1	8-10 >999 240 8 n/a n/a	PLATES GRIP MT20 244/190 Weight: 210 lb FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-257(LC 10) Max Uplift 2=-80(LC 12), 8=-80(LC 13)

Max Uplift 2=-80(LC 12), 8=-80(LC 13) Max Grav 2=1553(LC 19), 8=1553(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2075/347, 3-5=-1949/436, 5-7=-1950/436, 7-8=-2075/347

BOT CHORD 2-12=-152/1820, 10-12=0/1188, 8-10=-150/1652

WEBS 5-10=-163/1025, 7-10=-448/294, 5-12=-163/1024, 3-12=-447/294

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-7 to 3-3-6, Interior(1) 3-3-6 to 14-11-8, Exterior(2R) 14-11-8 to 19-4-5, Interior(1) 19-4-5 to 31-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

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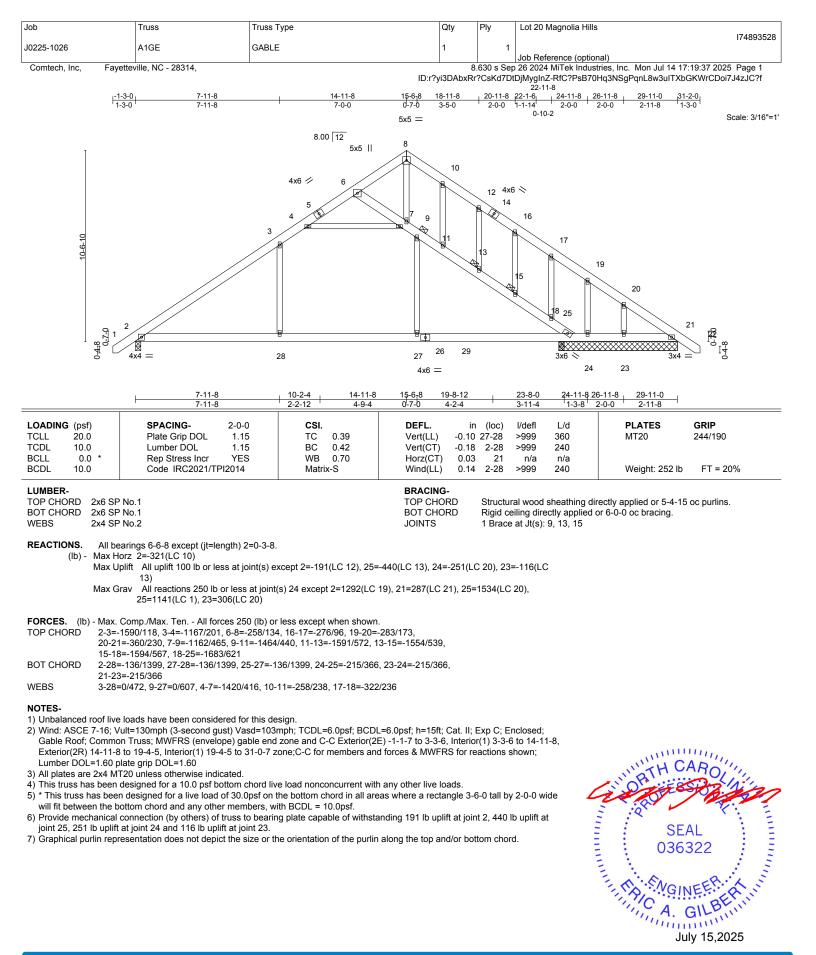
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

A MILLER ATT 818 Soundside Road Edenton, NC 27932



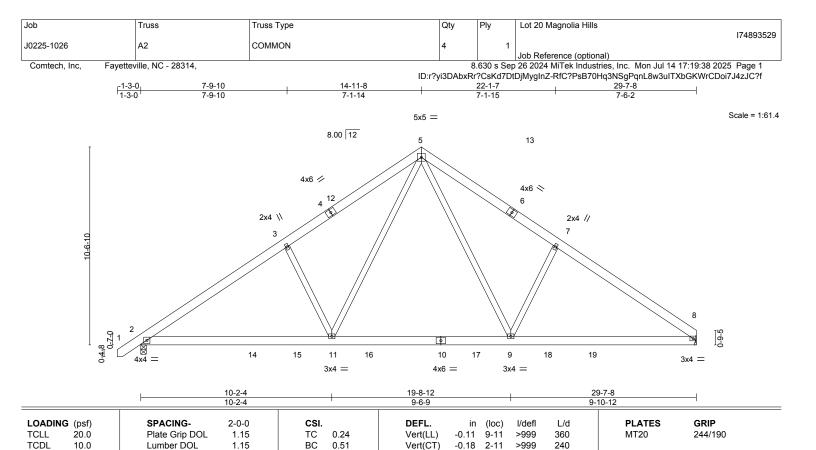
BOT CHORD

Structural wood sheathing directly applied or 5-2-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



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A MiTek Af 818 Soundside Road



Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.04

0.03 2-11

8

n/a

>999

n/a

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 5-2-12 oc purlins.

Weight: 205 lb

FT = 20%

LUMBER-

BCLL

BCDI

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

0.0

10.0

2x4 SP No.2

REACTIONS. 2=0-3-8, 8=Mechanical (size) Max Horz 2=252(LC 9) Max Uplift 2=-80(LC 12), 8=-61(LC 13) Max Grav 2=1544(LC 19), 8=1471(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2060/345, 3-5=-1934/435, 5-7=-1917/439, 7-8=-2043/347

BOT CHORD 2-11=-192/1799. 9-11=0/1166. 8-9=-177/1604

WEBS 5-9=-162/994, 7-9=-420/294, 5-11=-163/1025, 3-11=-448/295

Rep Stress Incr

Code IRC2021/TPI2014

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-7 to 3-3-6, Interior(1) 3-3-6 to 14-11-8, Exterior(2R) 14-11-8 to 19-4-5, Interior(1) 19-4-5 to 29-6-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-S

0.35

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

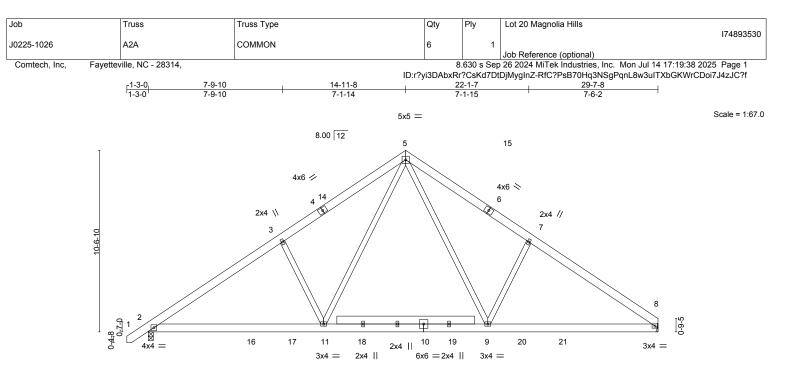
* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 2 and 61 lb uplift at joint 8.



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	10-2-4 10-2-4		<u>19-8-12</u> 9-6-9	29-7-8 9-10-12		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.24 BC 0.50 WB 0.35 Matrix-S	DEFL. in (loc Vert(LL) -0.10 9-11 Vert(CT) -0.18 2-11 Horz(CT) 0.04 4 Wind(LL) 0.03 2-11	1 >999 360 1 >999 240 8 n/a n/a	PLATES GRIP MT20 244/19 Weight: 223 lb FT =	0 20%

LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2 *Except*

 12-13: 2x6 SP No.1

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-2-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 8=Mechanical Max Horz 2=252(LC 9) Max Uplift 2=-80(LC 12), 8=-61(LC 13)

Max Grav 2=1537(LC 19), 8=1464(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2046/345, 3-5=-1921/435, 5-7=-1904/439, 7-8=-2030/347

BOT CHORD 2-11=-192/1789, 9-11=0/1159, 8-9=-177/1594

WEBS 5-9=-162/986, 7-9=-420/294, 5-11=-163/1017, 3-11=-448/295

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-7 to 3-3-6, Interior(1) 3-3-6 to 14-11-8, Exterior(2R) 14-11-8 to 19-4-5, Interior(1) 19-4-5 to 29-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

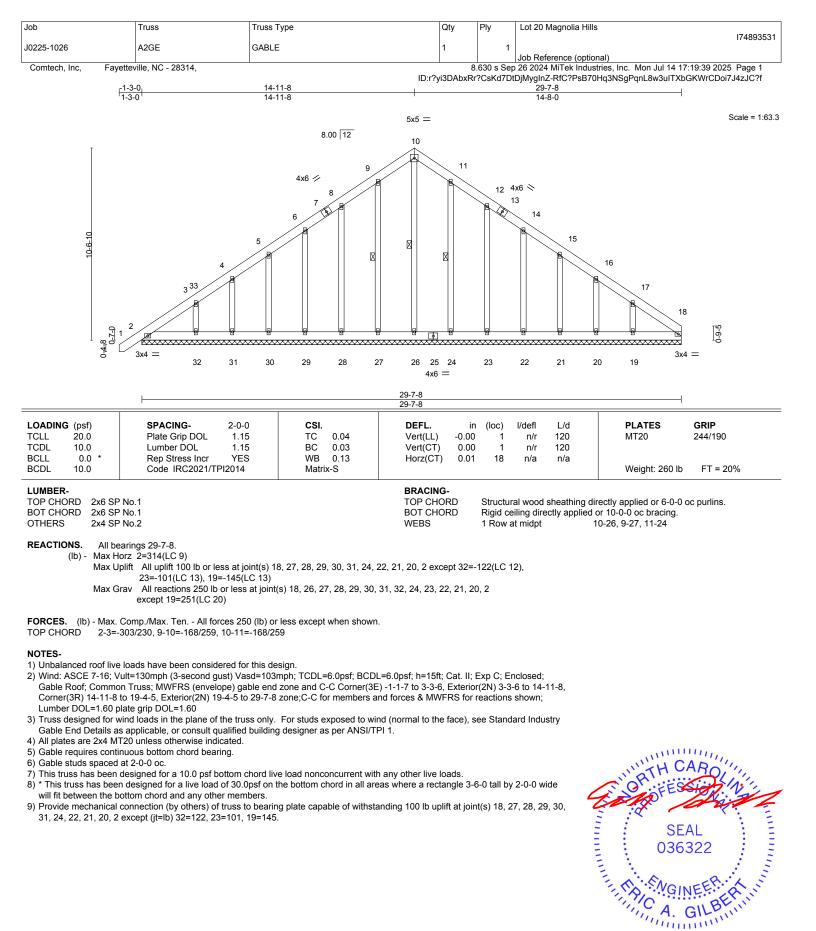
5) Refer to girder(s) for truss to truss connections

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 2 and 61 lb uplift at joint 8.

SEAL 036322 July 15,2025

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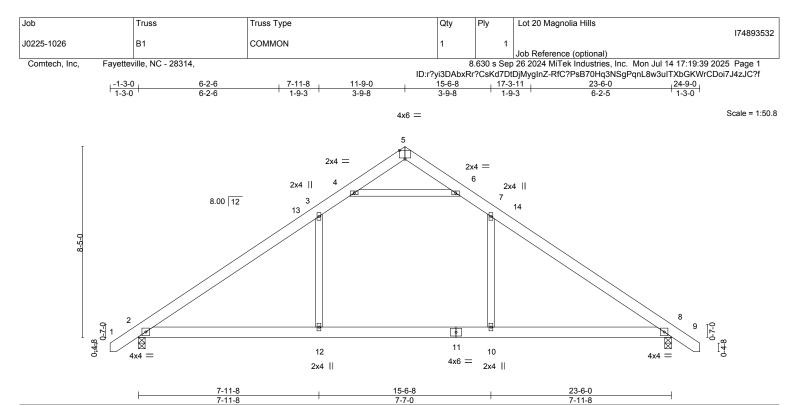


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_OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (le	oc) I/defl L/d	PLATES GRIP
· · · ·		-	(,	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.49	Vert(LL) -0.15 8-	10 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.41	Vert(CT) -0.20 8-	10 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.60	Horz(CT) 0.02	8 n/a n/a	
BCDL 10.0	Code IRC2021/TPI2014	Matrix-S	Wind(LL) 0.13 2-	12 >999 240	Weight: 151 lb FT = 20%

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=206(LC 11) Max Uplift 2=-67(LC 12), 8=-67(LC 13) Max Grav 2=1222(LC 19), 8=1222(LC 20)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- 2-3=-1593/246, 3-4=-1101/298, 4-5=-68/312, 5-6=-68/313, 6-7=-1101/298, TOP CHORD
- 7-8=-1593/246
- BOT CHORD 2-12=-39/1238, 10-12=-39/1238, 8-10=-39/1238
- WEBS 7-10=0/506, 3-12=0/506, 4-6=-1524/420

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-7 to 3-3-6, Interior(1) 3-3-6 to 11-9-0, Exterior(2R) 11-9-0 to 16-1-13, Interior(1) 16-1-13 to 24-7-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

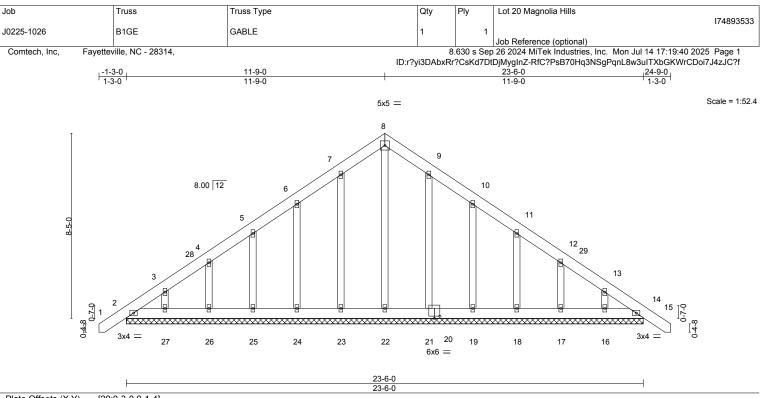


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818 Soundside Road Edenton, NC 27932



Structural wood sheathing directly applied or 5-7-9 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	-0.00	14	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	-0.00	14	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.00	14	n/a	n/a		
BCDL	10.0	Code IRC2021/T	PI2014	Matri	x-S						Weight: 192 lb	FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 23-6-0.

(Ib) - Max Horz 2=-257(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 23, 24, 25, 26, 27, 21, 19, 18, 17, 16

Max Grav All reactions 250 lb or less at joint(s) 2, 14, 22, 23, 24, 25, 26, 27, 21, 19, 18, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-1-7 to 3-3-6, Exterior(2N) 3-3-6 to 11-9-0, Corner(3R) 11-9-0 to 16-1-13, Exterior(2N) 16-1-13 to 24-7-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

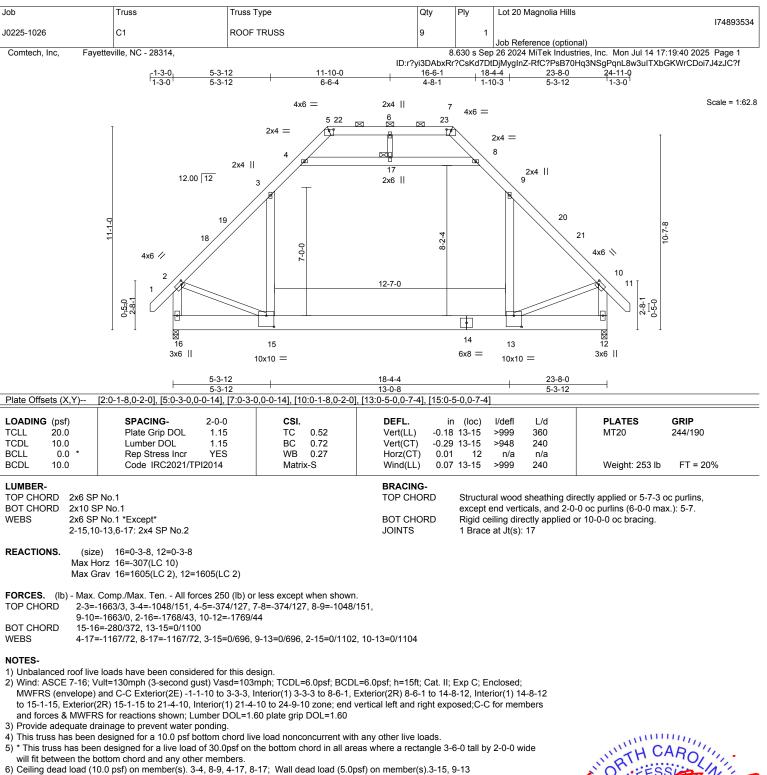
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 23, 24, 25,

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 23, 24, 25, 26, 27, 21, 19, 18, 17, 16.



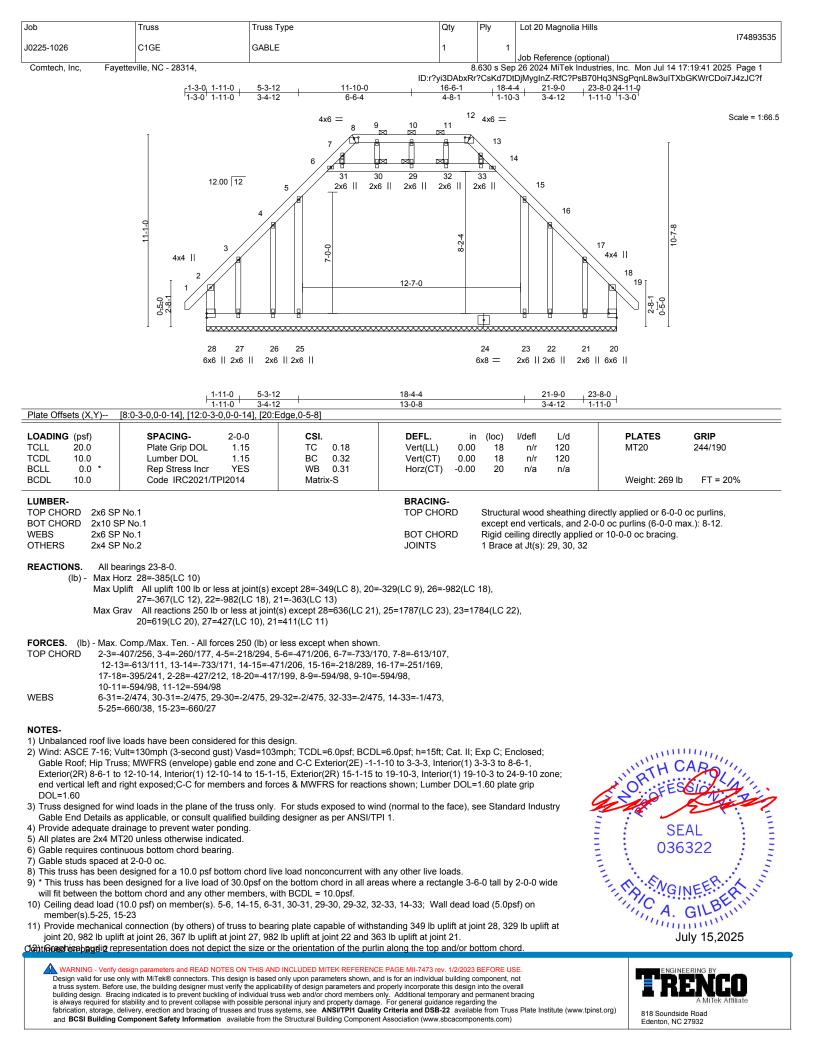
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- 6) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-17, 8-17; Wall dead load (5.0psf) on member(s).3-15, 9-13
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 9) Attic room checked for L/360 deflection.



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Job	Truss	Truss Type	Qty	Ply	Lot 20 Magnolia Hills
					174893535
J0225-1026	C1GE	GABLE	1	1	
					Job Reference (optional)
Comtech, Inc,	ayetteville, NC - 28314,		8	3.630 s Sep	26 2024 MiTek Industries, Inc. Mon Jul 14 17:19:41 2025 Page 2

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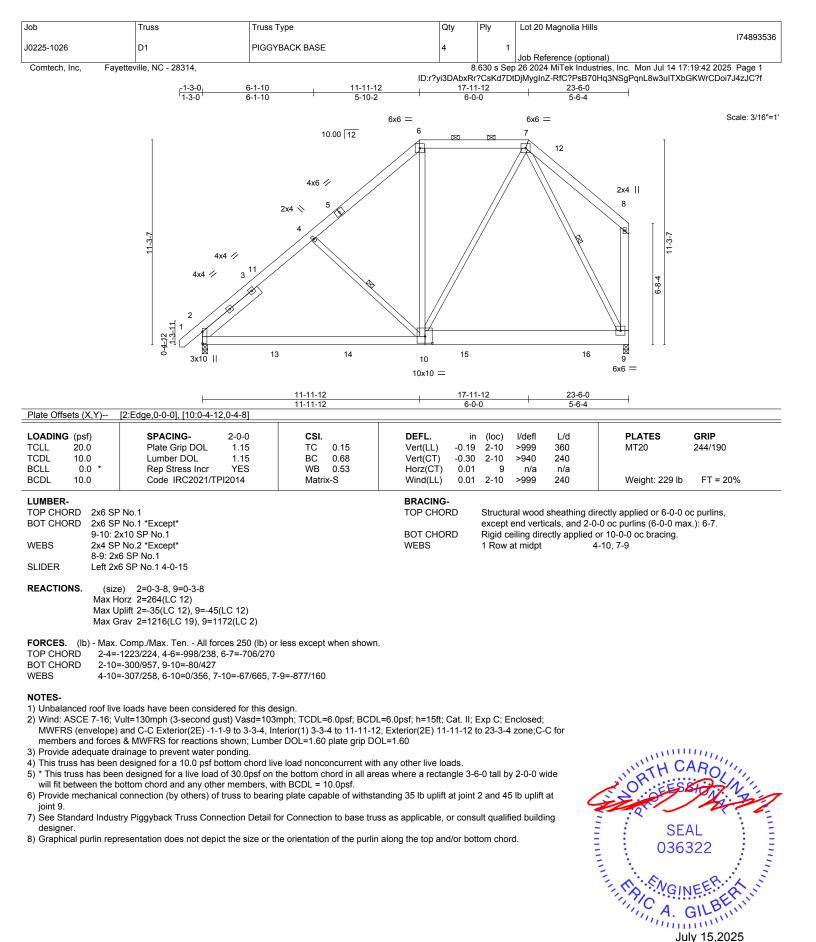
NOTES-

13) Attic room checked for L/360 deflection.



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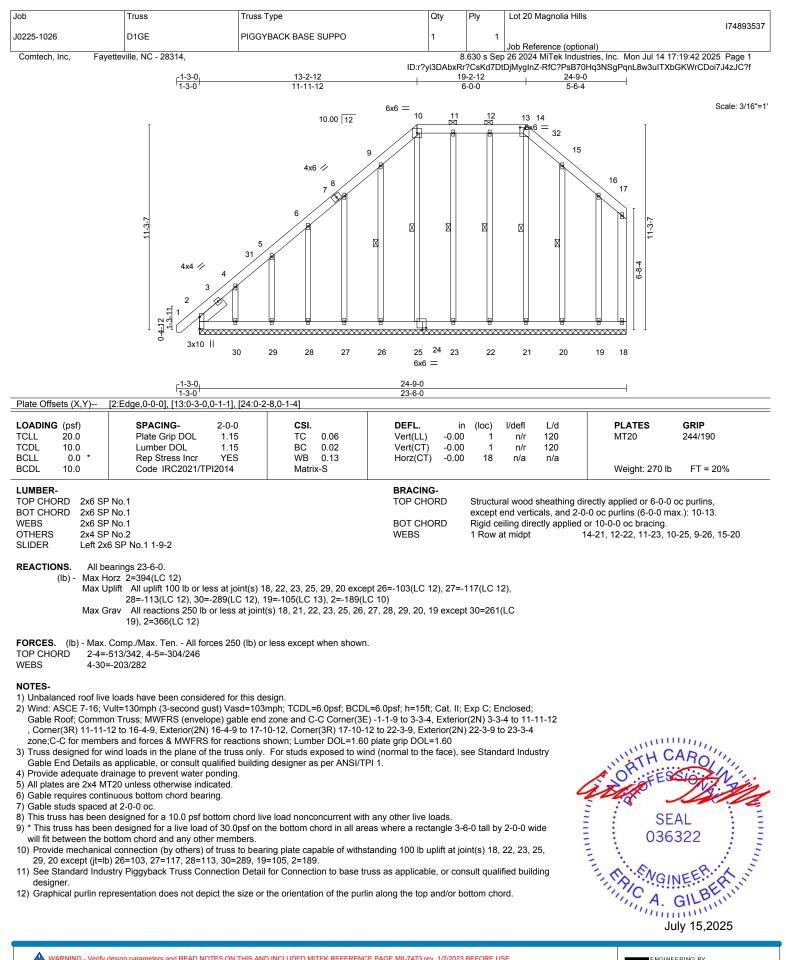
A Mi lek A 818 Soundside Road Edenton, NC 27932



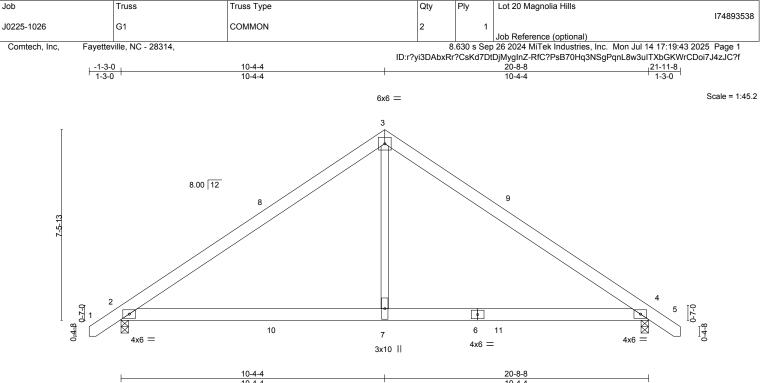
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818 Soundside Road



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	10		10-4-4	l		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.53 BC 0.57 WB 0.18	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.11 4-7 >999 360 MT20 244/190 Vert(CT) -0.19 4-7 >999 240 MT20 244/190 Horz(CT) 0.02 4 n/a n/a A A			
BCDL 10.0	Code IRC2021/TPI2014	Matrix-S	Wind(LL) 0.05 2-7 >999 240 Weight: 124 lb FT = 20%			

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 4=0-3-8, 2=0-3-8 Max Horz 2=183(LC 11) Max Uplift 4=-61(LC 13), 2=-61(LC 12) Max Grav 4=1118(LC 20), 2=1118(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-1279/212, 3-4=-1279/212 TOP CHORD

BOT CHORD 2-7=0/1006, 4-7=0/1006 3-7=0/818

WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-7 to 3-3-6, Interior(1) 3-3-6 to 10-4-4, Exterior(2R) 10-4-4 to 14-9-1, Interior(1) 14-9-1 to 21-9-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

Structural wood sheathing directly applied or 5-7-7 oc purlins.

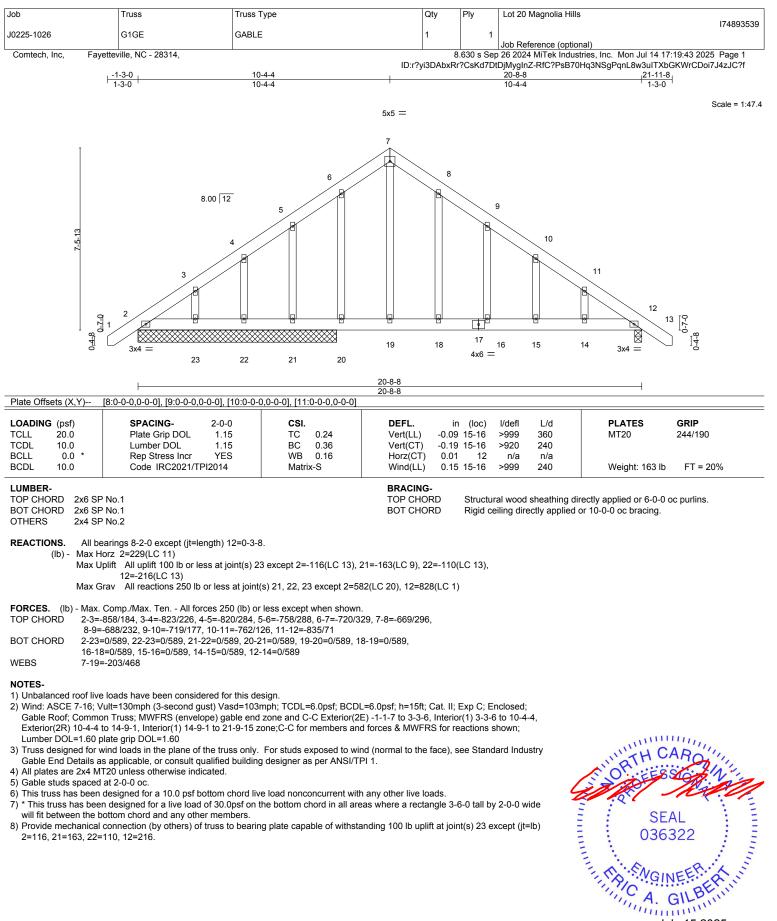
Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road

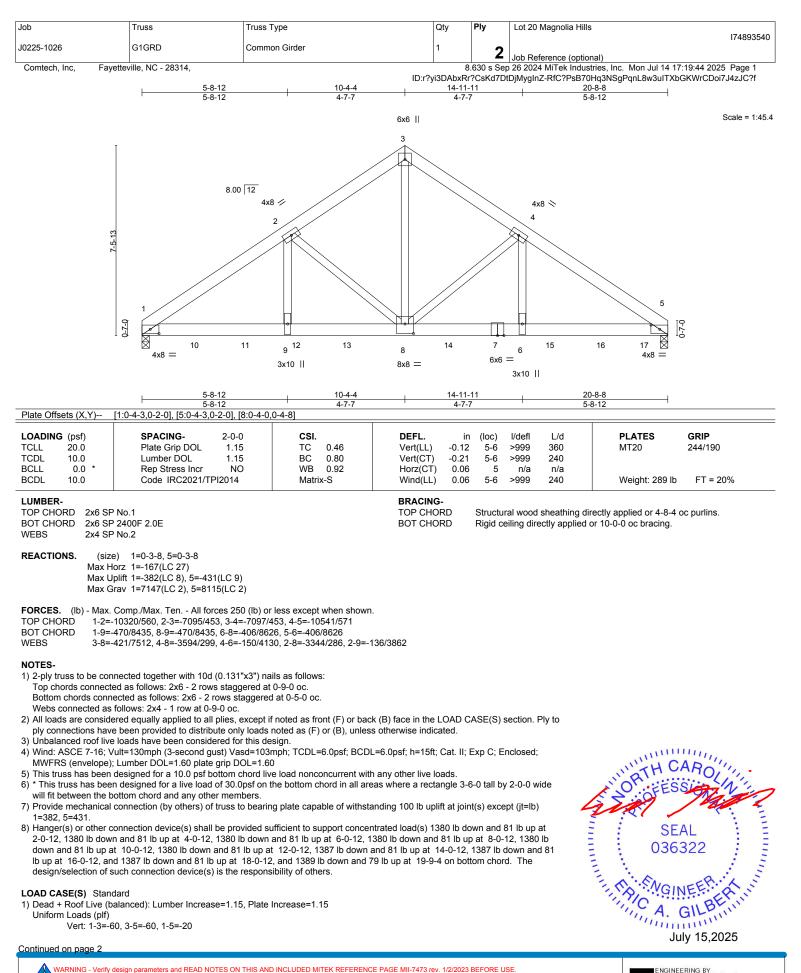
MILLING CONTRACT OF A STATE OF A CARO \cap VIIIIIIIIIIII SEAL 036322 G minn July 15,2025



July 15,2025



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-4/3 rev. 1/2/2/2/3 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPTI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

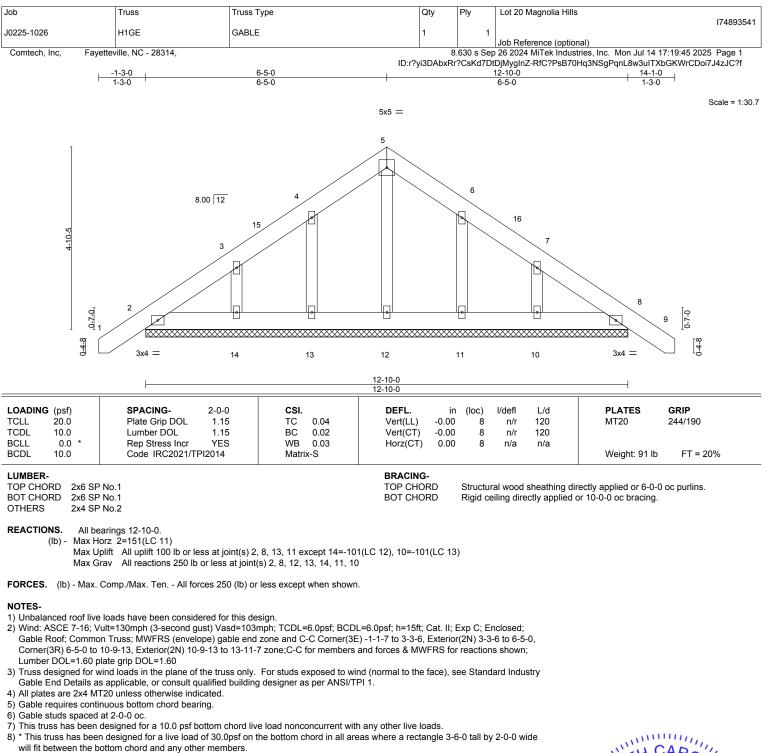
ſ	Job	Truss	Truss Type	Qty	Ply	Lot 20 Magnolia Hills
						174893540
	J0225-1026	G1GRD	Common Girder	1	2	
					_	Job Reference (optional)
	Comtech, Inc, Fayettev	ville, NC - 28314,		8	.630 s Sep	26 2024 MiTek Industries, Inc. Mon Jul 14 17:19:44 2025 Page 2
			ID:r?j	yi3DAbxRr	?CsKd7Dt	DjMygInZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 7=-1155(B) 8=-1155(B) 10=-1155(B) 11=-1155(B) 12=-1155(B) 13=-1155(B) 14=-1155(B) 15=-1155(B) 16=-1155(B) 17=-1157(B) 16=-1155(B) 16



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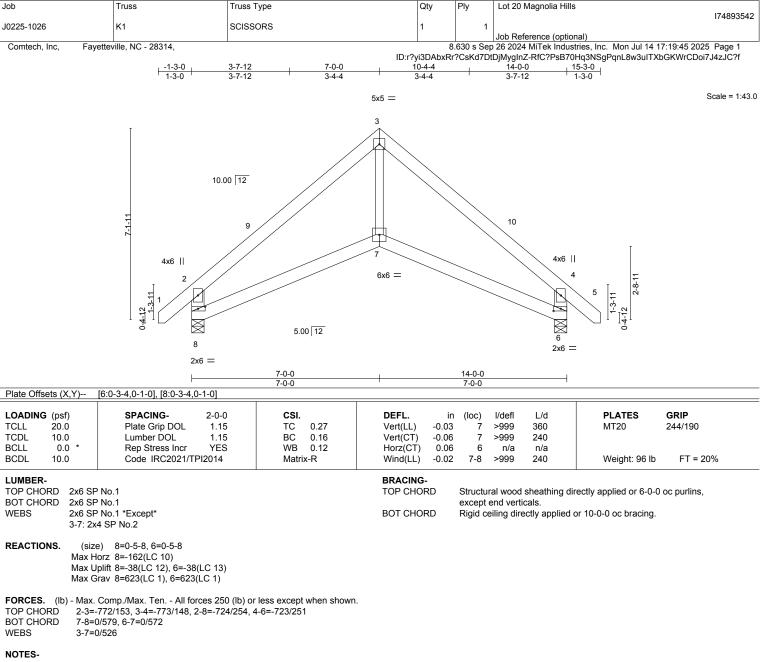


 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11 except (jt=lb) 14=101, 10=101.



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A MiTek : 818 Soundside Road Edenton, NC 27932



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-9 to 3-3-4, Interior(1) 3-3-4 to 7-0-0, Exterior(2R) 7-0-0 to 11-4-13, Interior(1) 11-4-13 to 15-1-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

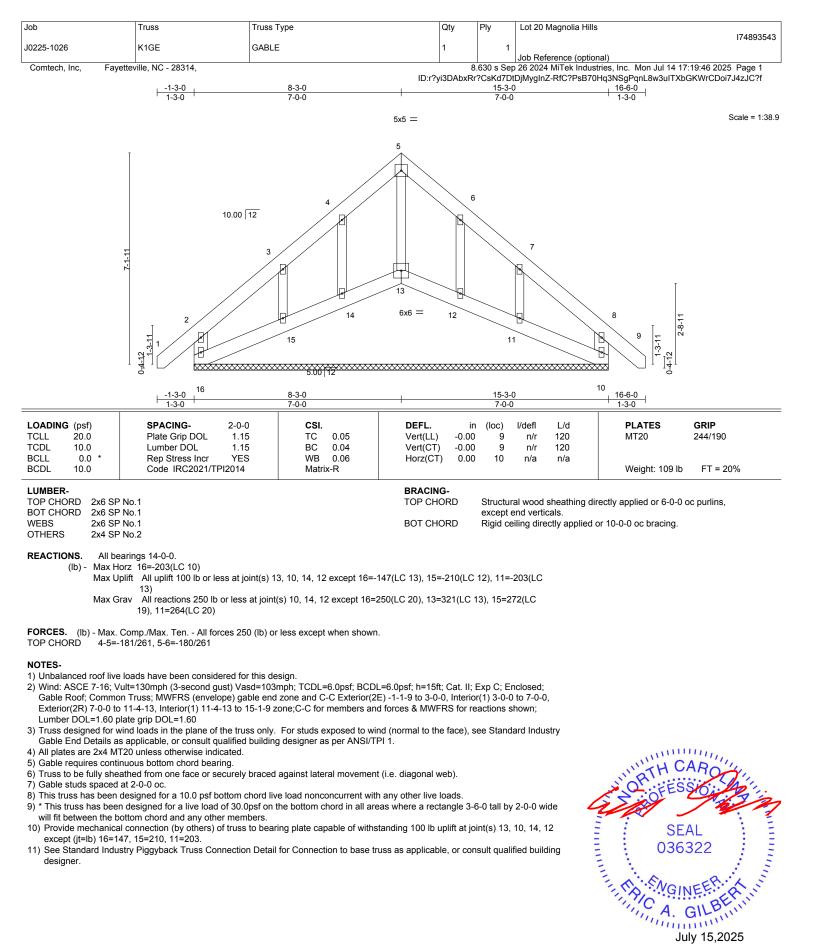
5) Bearing at joint(s) 8, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

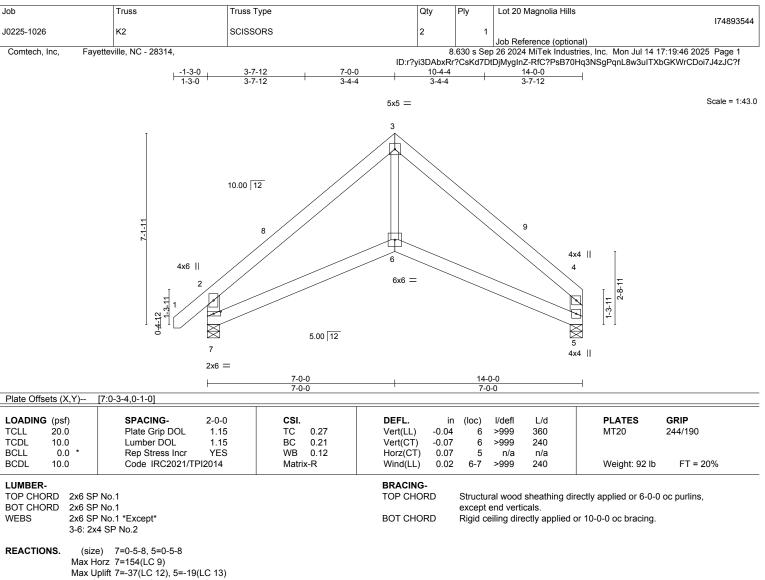


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818 Soundside Road



Max Grav 7=627(LC 1), 5=538(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-777/189, 3-4=-765/211, 2-7=-727/279, 4-5=-628/216

- BOT CHORD 6-7=-52/557. 5-6=-48/547
- WEBS 3-6=0/518

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-9 to 3-3-4, Interior(1) 3-3-4 to 7-0-0, Exterior(2R) 7-0-0 to 11-4-13, Interior(1) 11-4-13 to 13-9-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

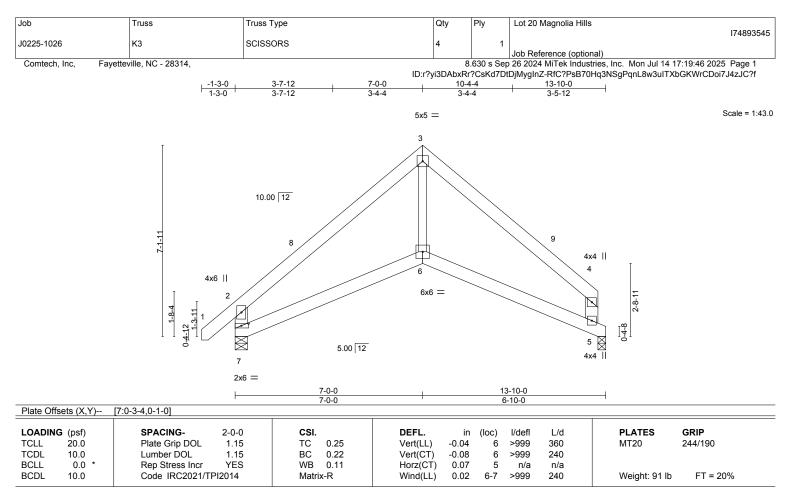
5) Bearing at joint(s) 7, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LUMBER- BRACING-	
TOP CHORD 2x6 SP No.1 TOP CHORD Structural wood sheathing	directly applied or 6-0-0 oc purlins,
BOT CHORD 2x6 SP No.1 except end verticals.	
WEBS 2x6 SP No.1 *Except* BOT CHORD Rigid ceiling directly applie	ed or 10-0-0 oc bracing.
3-6: 2x4 SP No.2	Ū.

REACTIONS. (size) 7=0-5-8, 5=0-3-8 Max Horz 7=153(LC 9) Max Uplift 7=-36(LC 12), 5=-20(LC 12) Max Grav 7=609(LC 1), 5=519(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-726/189, 3-4=-710/218, 2-7=-694/282, 4-5=-589/216 TOP CHORD

BOT CHORD 6-7=-67/515, 5-6=-61/501

WEBS 3-6=0/478

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-1-9 to 3-3-4, Interior(1) 3-3-4 to 7-0-0, Exterior(2R) 7-0-0 to 11-4-13, Interior(1) 11-4-13 to 13-3-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 7, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

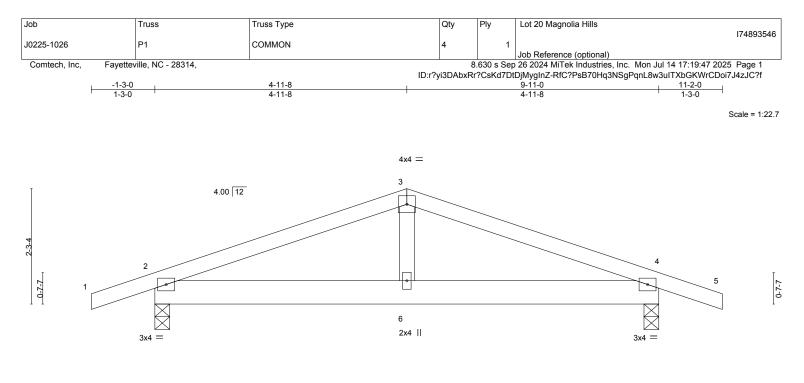
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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	<u>4-11-8</u> 4-11-8					9-11-0 4-11-8					
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.21	DEFL. Vert(LL)	in 0.01	(loc) 2-6	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL Rep Stress Incr Code IRC2021/TPI	1.15 YES 2014	BC WB Matri:	0.22 0.05 x-S	Vert(CT) Horz(CT)	-0.02 0.00	6 4	>999 n/a	240 n/a	Weight: 45 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=42(LC 12) Max Uplift 2=-271(LC 8), 4=-271(LC 9) Max Grav 2=469(LC 1), 4=469(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-609/283, 3-4=-609/283

BOT CHORD 2-6=-218/514, 4-6=-218/514

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

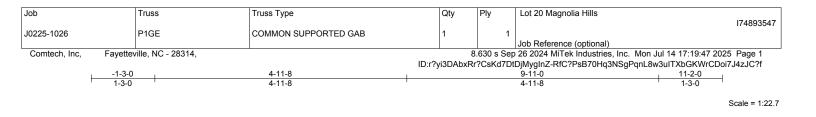
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=271, 4=271.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

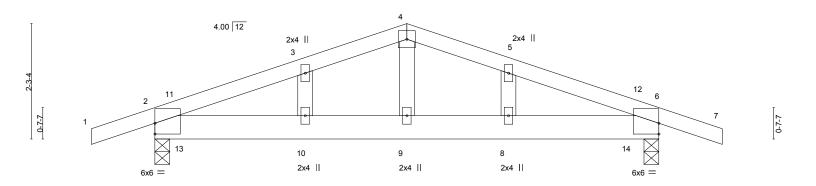


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4x4 =



			<u>9-11-0</u> 9-11-0				
Plate Offsets (X,Y)	[2:0-0-0,0-2-9], [6:Edge,0-2-9]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.37 BC 0.61 WB 0.07 Matrix-S	DEFL. in Vert(LL) 0.04 Vert(CT) -0.02 Horz(CT) -0.01	10 >999	L/d 240 240 n/a	PLATES MT20 Weight: 48 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP 3OT CHORD 2x6 SP DTHERS 2x4 SP REACTIONS. (size	2 No.1 2 No.2		BRACING- TOP CHORD BOT CHORD		0	rectly applied or 6-0-0 or 6-9-8 oc bracing.	oc purlins.

EACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=42(LC 16) Max Uplift 2=-271(LC 8), 6=-271(LC 9) Max Grav 2=469(LC 1), 6=469(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-599/1432, 3-4=-558/1457, 4-5=-558/1457, 5-6=-599/1432
- BOT CHORD 2-10=-1222/514, 9-10=-1222/514, 8-9=-1222/514, 6-8=-1222/514

WEBS 4-9=-690/210

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-3-0 to 2-11-8, Exterior(2N) 2-11-8 to 4-11-8, Corner(3R) 4-11-8 to 9-4-5, Exterior(2N) 9-4-5 to 11-2-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

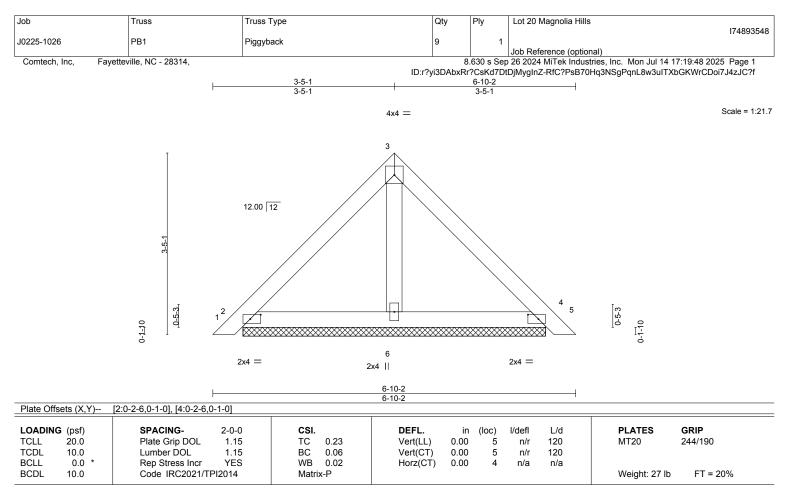
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=271, 6=271.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 2=5-8-8, 4=5-8-8, 6=5-8-8

Max Horz 2=-77(LC 10) Max Uplift 2=-27(LC 13), 4=-31(LC 13)

Max Grav 2=162(LC 1), 4=162(LC 1), 6=177(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

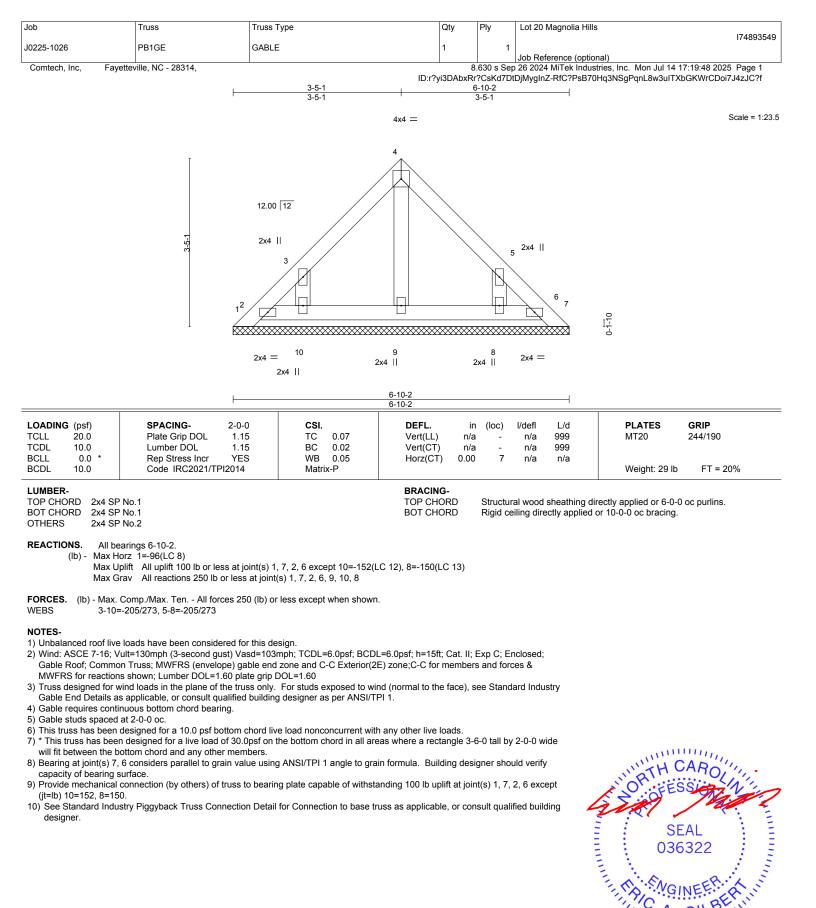
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

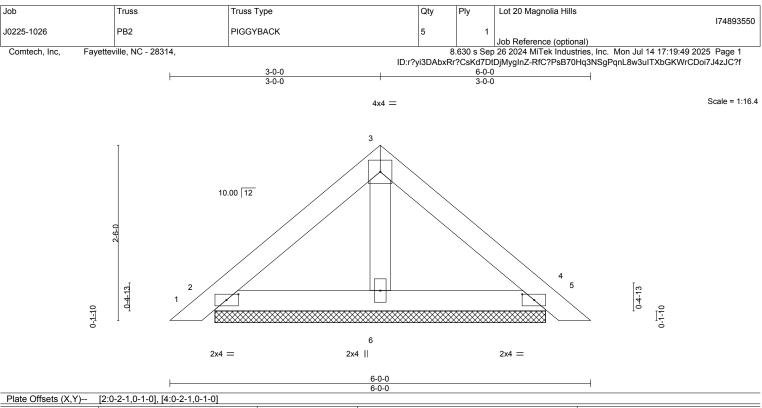
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)







July 15,2025



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	5	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2021/T	PI2014	Matri	x-P						Weight: 21 lb	FT = 20%
LUMBER	2-					BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD OTHERS 2x4 SP No.2

REACTIONS. (size) 2=4-8-9, 4=4-8-9, 6=4-8-9

Max Horz 2=-69(LC 10) Max Uplift 2=-47(LC 12), 4=-56(LC 13)

Max Grav 2=136(LC 1), 4=136(LC 1), 6=155(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

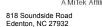
7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



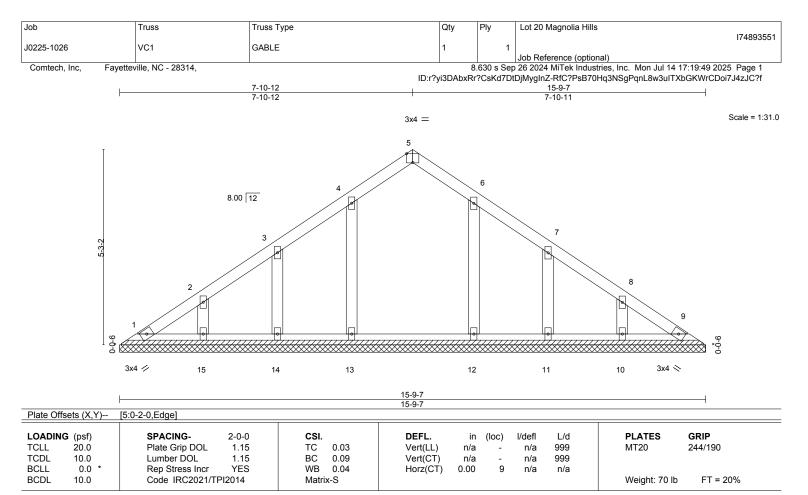
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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¹⁾ Unbalanced roof live loads have been considered for this design.



LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-9-7.

(lb) - Max Horz 1=-148(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 14, 15, 12, 10 except 11=-100(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 9, 14, 15, 11, 10 except 13=307(LC 19), 12=300(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

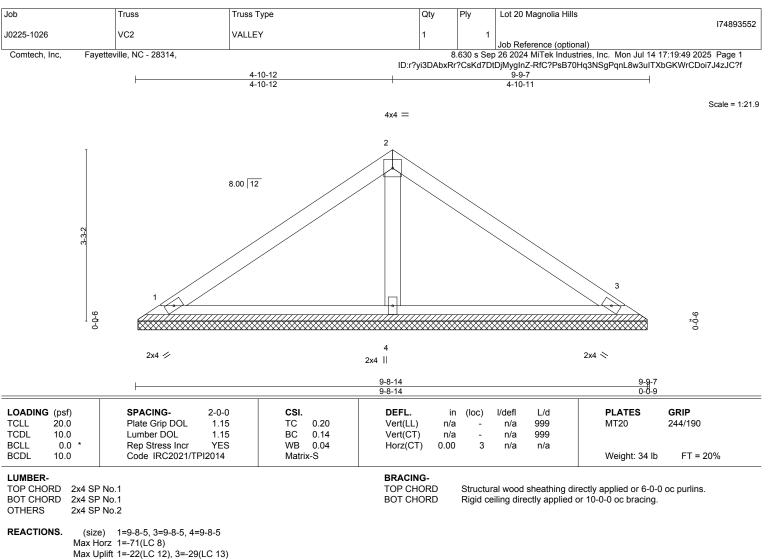
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 14, 15, 12, 10 except (it=lb) 11=100.



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¹⁾ Unbalanced roof live loads have been considered for this design.



Max Grav 1=175(LC 1), 3=175(LC 1), 4=354(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

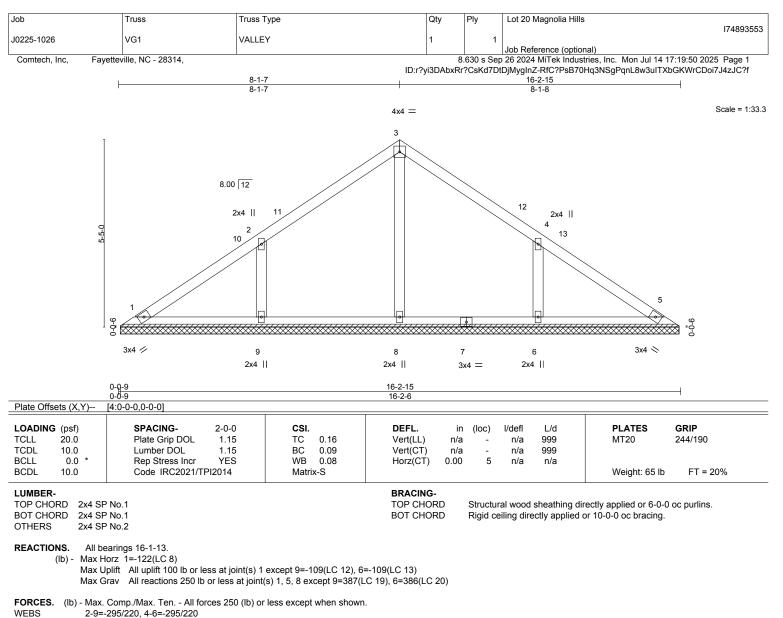
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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WEBS 2-9--295/220, 4-0--295/220

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 8-1-7, Exterior(2R) 8-1-7 to 12-6-4, Interior(1) 12-6-4 to 15-9-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

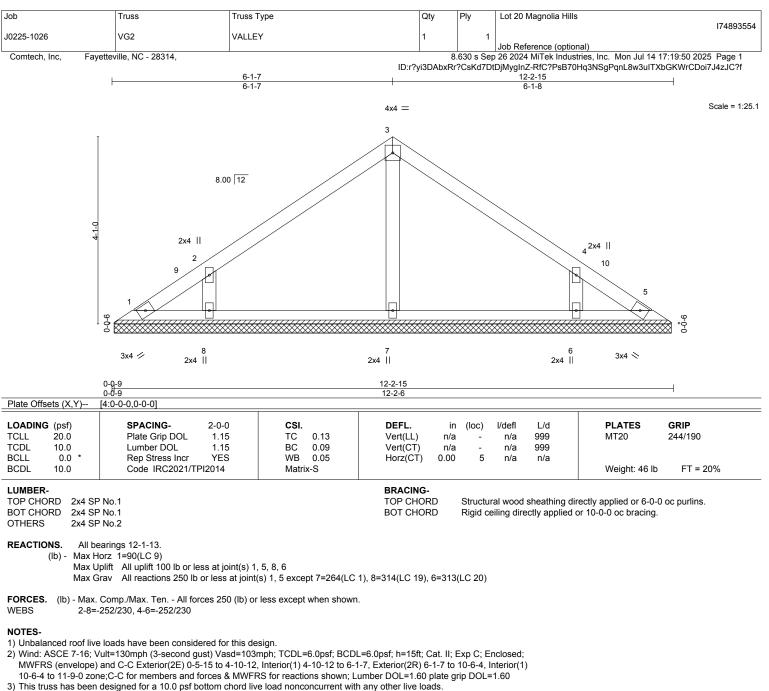
will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=109, 6=109.

6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



4) * This truss has been designed for a focus ps bottom chord invertible inforcement with any other invertibles.
 4) * This truss has been designed for a five load of 30.0ps for the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

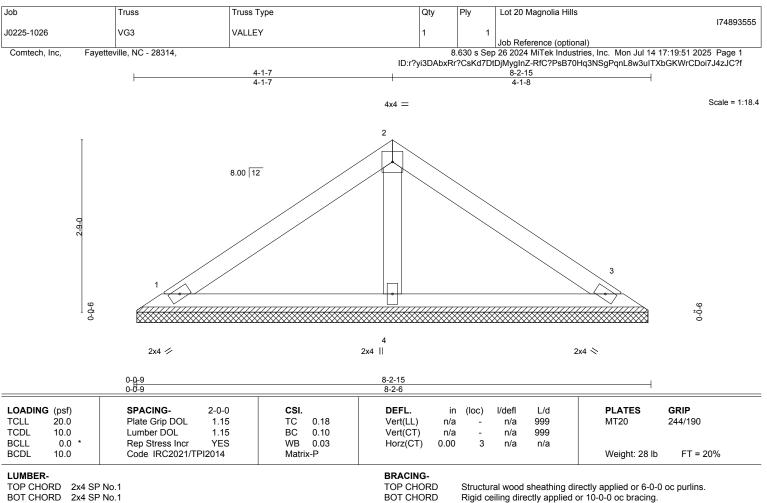
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.

6) Non Standard bearing condition. Review required.



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BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=8-1-13, 3=8-1-13, 4=8-1-13 (size) Max Horz 1=-58(LC 10) Max Uplift 1=-25(LC 12), 3=-31(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=265(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

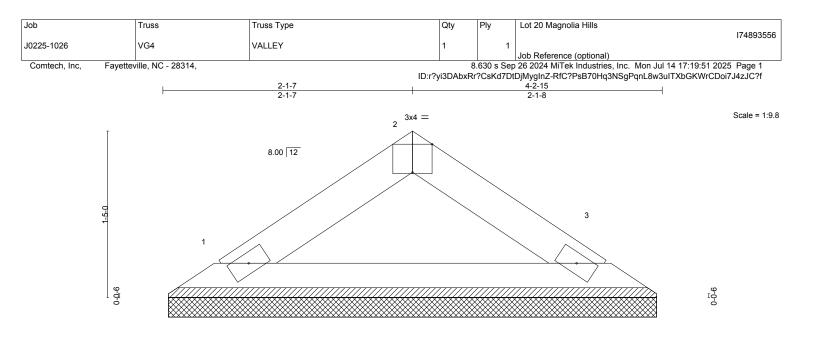
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buck rown the oppression of design had needed and the operation of the opera and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



2x4 🥢

2x4 📎

Structural wood sheathing directly applied or 4-2-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y)	0-0-9 0-0-9 [2:0-2-0,Edge]	<u>4-2-15</u> 4-2-6		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.05 BC 0.09 WB 0.00	DEFL. in (loc) l/defl Vert(LL) n/a - n/a Vert(CT) n/a - n/a Horz(CT) 0.00 3 n/a	L/d PLATES GRIP 999 MT20 244/190 999 n/a
BCDL 10.0	Code IRC2021/TPI2014	Matrix-P		Weight: 12 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=4-1-13, 3=4-1-13 Max Horz 1=-26(LC 8) Max Uplift 1=-7(LC 12), 3=-7(LC 13) Max Grav 1=130(LC 1), 3=130(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

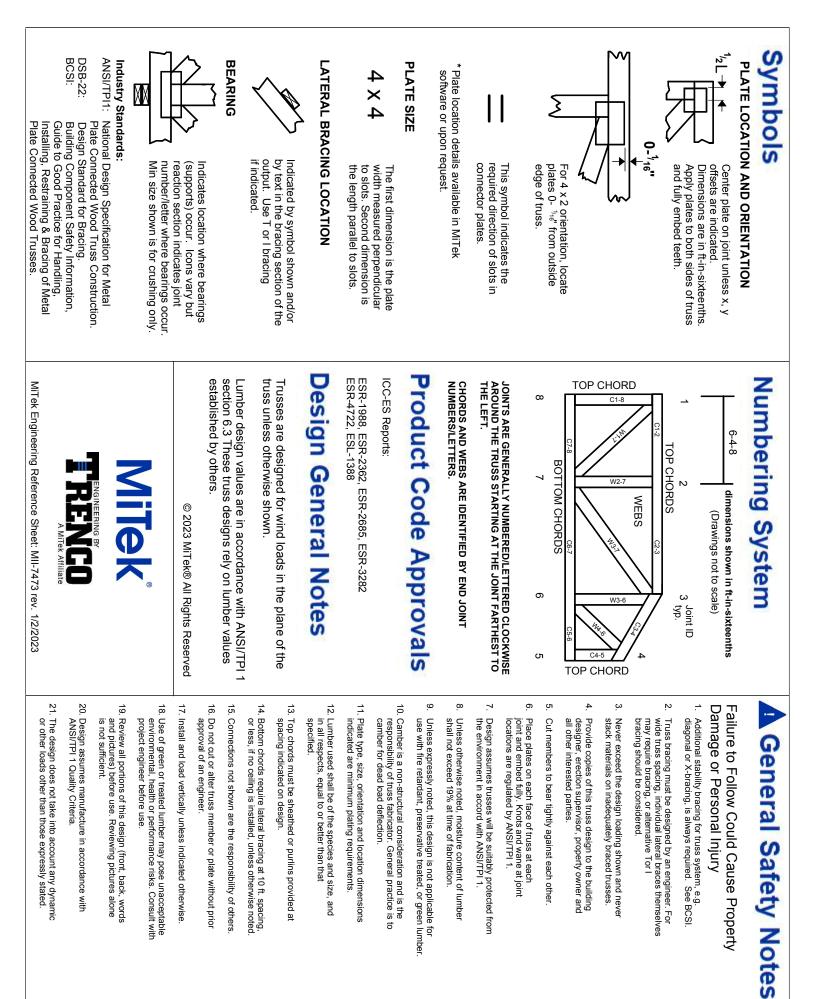
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)







Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0225-1027 Lot 20 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I74893557 thru I74893568

My license renewal date for the state of North Carolina is December 31, 2025.

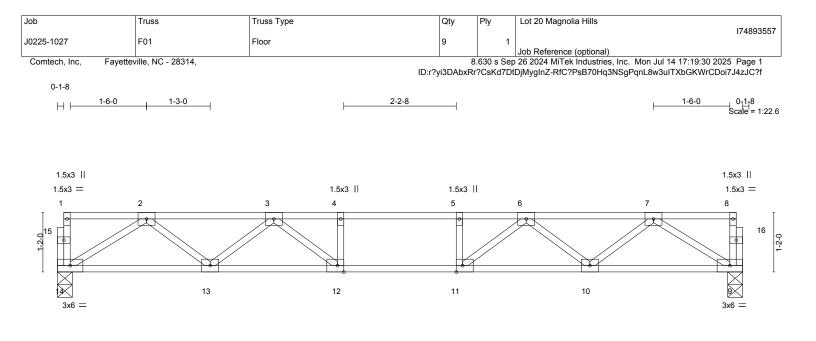
North Carolina COA: C-0844



July 15,2025

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



<u> </u>			<u>13-5-8</u> 13-5-8			
Plate Offsets (X,Y)	[11:0-1-8,Edge], [12:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.42 BC 0.51 WB 0.31 Matrix-S	Vert(LL) -0.12	n (loc) l/defl L/d 2 12-13 >999 480 5 12-13 >999 360 3 9 n/a n/a	PLATES MT20 Weight: 66 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) REACTIONS. (size) 14=0-3-8, 9=0-3-8			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.		

Max Grav 14=720(LC 1), 9=720(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1533/0, 3-4=-2255/0, 4-5=-2255/0, 5-6=-2255/0, 6-7=-1533/0

BOT CHORD 13-14=0/1028, 12-13=0/2002, 11-12=0/2255, 10-11=0/2002, 9-10=0/1028

WEBS 2-14=-1217/0, 2-13=0/658, 3-13=-610/0, 3-12=0/546, 4-12=-260/0, 7-9=-1217/0,

7-10=0/658, 6-10=-610/0, 6-11=0/546, 5-11=-260/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

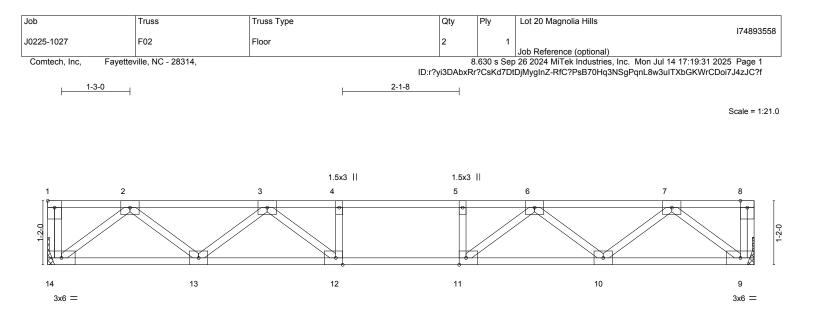
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





			12-10-8 12-10-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [11:0-1-8,Edge], [12:0-1	-8,Edge]				
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.38 BC 0.46 WB 0.31 Matrix-S	Vert(LL) -0.10	n (loc) l/defl L/d 0 12-13 >999 480 3 12-13 >999 360 3 9 n/a n/a	PLATES MT20 Weight: 65 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	,	oc purlins,

REACTIONS. (size) 14=Mechanical, 9=Mechanical Max Grav 14=694(LC 1), 9=694(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1351/0, 3-4=-2060/0, 4-5=-2060/0, 5-6=-2060/0, 6-7=-1351/0

BOT CHORD 13-14=0/851, 12-13=0/1815, 11-12=0/2060, 10-11=0/1815, 9-10=0/851

WEBS 2-14=-1068/0, 2-13=0/650, 3-13=-605/0, 3-12=0/519, 7-9=-1068/0, 7-10=0/650,

6-10=-605/0, 6-11=0/519

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

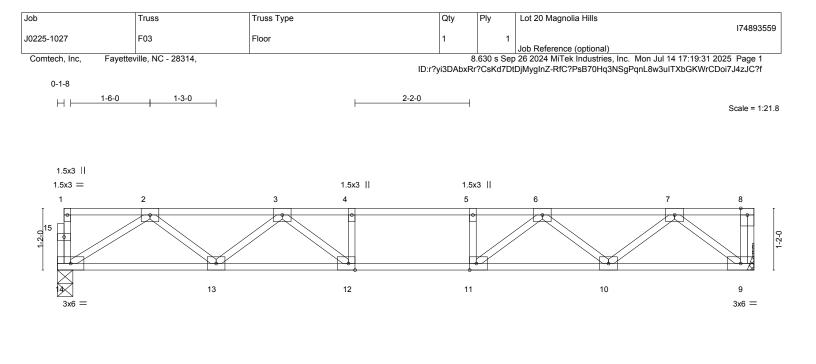
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





			13-2-0			
1			13-2-0			1
Plate Offsets (X,Y)	[11:0-1-8,Edge], [12:0-1-8,Edge]					
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.43 BC 0.51 WB 0.32 Matrix-S	Vert(LL) -0.12	n (loc) l/defl L/d 2 12-13 >999 480 5 12-13 >999 360 3 9 n/a n/a	PLATES MT20 Weight: 66 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or		oc purlins,

13 2 0

REACTIONS. (size) 14=0-3-8, 9=Mechanical Max Grav 14=704(LC 1), 9=710(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-1491/0, 3-4=-2156/0, 4-5=-2156/0, 5-6=-2156/0, 6-7=-1389/0

BOT CHORD

13-14=0/1003, 12-13=0/1938, 11-12=0/2156, 10-11=0/1875, 9-10=0/872

WEBS 2-14=-1188/0, 2-13=0/635, 3-13=-582/0, 3-12=0/504, 7-9=-1094/0, 7-10=0/672,

6-10=-633/0, 6-11=0/562, 5-11=-265/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

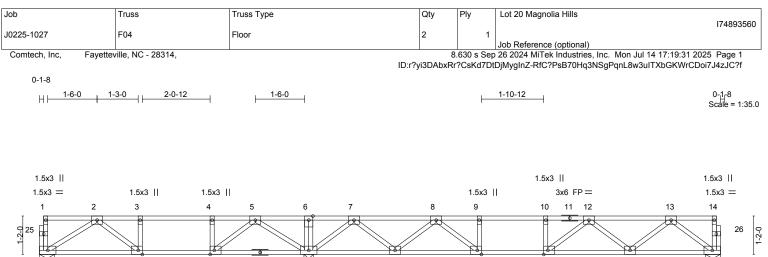
Strongbacks to be attached to walls at their outer ends or restrained by other means.

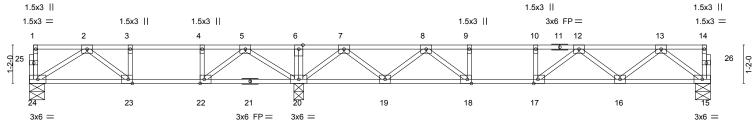
6) CAUTION, Do not erect truss backwards.



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L	8-2-4			20-8-8		1
I	8-2-4			12-6-4		1
Plate Offsets (X,Y)	[17:0-1-8,Edge], [18:0-1-8,Edge], [22:0	-1-8,Edge], [23:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 PCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.42 BC 0.46 WB 0.34 Matrix S	Vert(LL) -0.0	n (loc) l/defi L/d 9 16-17 >999 480 2 16-17 >999 360 2 15 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2021/1P12014	Matrix-S			Weight: 103 lb	FT = 20%F, 11%E
	² No.1(flat) ² No.1(flat)		BRACING- TOP CHORD	Structural wood sheathing dire except end verticals.	ectly applied or 6-0-0 o	oc purlins,
	P No.3(flat)		BOT CHORD	Rigid ceiling directly applied of	6-0-0 oc bracing.	
REACTIONS. (size Max G	e) 24=0-5-8, 20=0-3-8, 15=0-5-8 srav 24=403(LC 3), 20=1256(LC 1), 15=	=640(LC 7)				

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-694/30, 3-4=-694/30, 4-5=-694/30, 5-6=0/704, 6-7=0/703, 7-8=-977/0, 8-9=-1777/0, 9-10=-1777/0, 10-12=-1777/0, 12-13=-1233/0 TOP CHORD 23-24=0/505, 22-23=-30/694, 20-22=-263/350, 19-20=-11/451, 18-19=0/1479, BOT CHORD 17-18=0/1777, 16-17=0/1635, 15-16=0/787 2-24=-595/0, 5-20=-788/0, 5-22=0/589, 4-22=-297/0, 7-20=-1110/0, 7-19=0/717, WFBS 8-19=-700/0, 13-15=-985/0, 13-16=0/581, 12-16=-522/0, 12-17=-32/331, 8-18=0/554, 9-18=-254/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

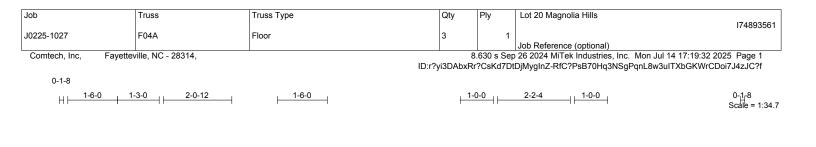
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

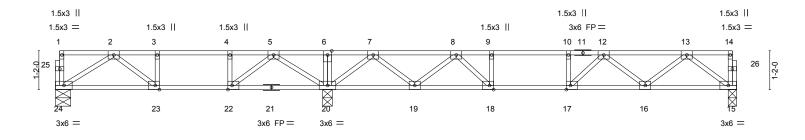
5) CAUTION, Do not erect truss backwards.



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	<u>8-2-4</u> 8-2-4				0-6-0 		
Plate Offsets (X,Y)	[17:0-1-8,Edge], [18:0-1-8,Edge], [22:0-	1-8,Edge], [23:0-1-8,Edge]		. 0 12		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYES	CSI. TC 0.43 BC 0.46 WB 0.33	Vert(CT) -0.	in (loc) l/de 09 16-17 >99 12 16-17 >99 02 15 n	9 480	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2021/TPI2014	Matrix-S				Weight: 101 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end v	verticals.	rectly applied or 6-0-0 o or 6-0-0 oc bracing.	oc purlins,
REACTIONS. (siz Max C	ze) 24=0-5-8, 20=0-3-8, 15=0-3-0 Grav 24=405(LC 3), 20=1235(LC 1), 15=	631(LC 7)					
TOP CHORD 2-3=	. Comp./Max. Ten All forces 250 (lb) oi -703/9, 3-4=-703/9, 4-5=-703/9, 5-6=0/6 =-1721/0, 10-12=-1721/0, 12-13=-1210//	52, 6-7=0/651, 7-8=-980/0					

BOT CHORD 23-24=0/509, 22-23=-9/703, 20-22=-229/363, 19-20=-0/473, 18-19=0/1467,

17-18=0/1721, 16-17=0/1601, 15-16=0/776

2-24=-599/0, 5-20=-785/0, 5-22=0/583, 4-22=-295/0, 7-20=-1086/0, 7-19=0/691, WEBS 8-19=-682/0, 13-15=-971/0, 13-16=0/566, 12-16=-508/0, 12-17=-27/324, 8-18=0/541, 9-18=-291/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

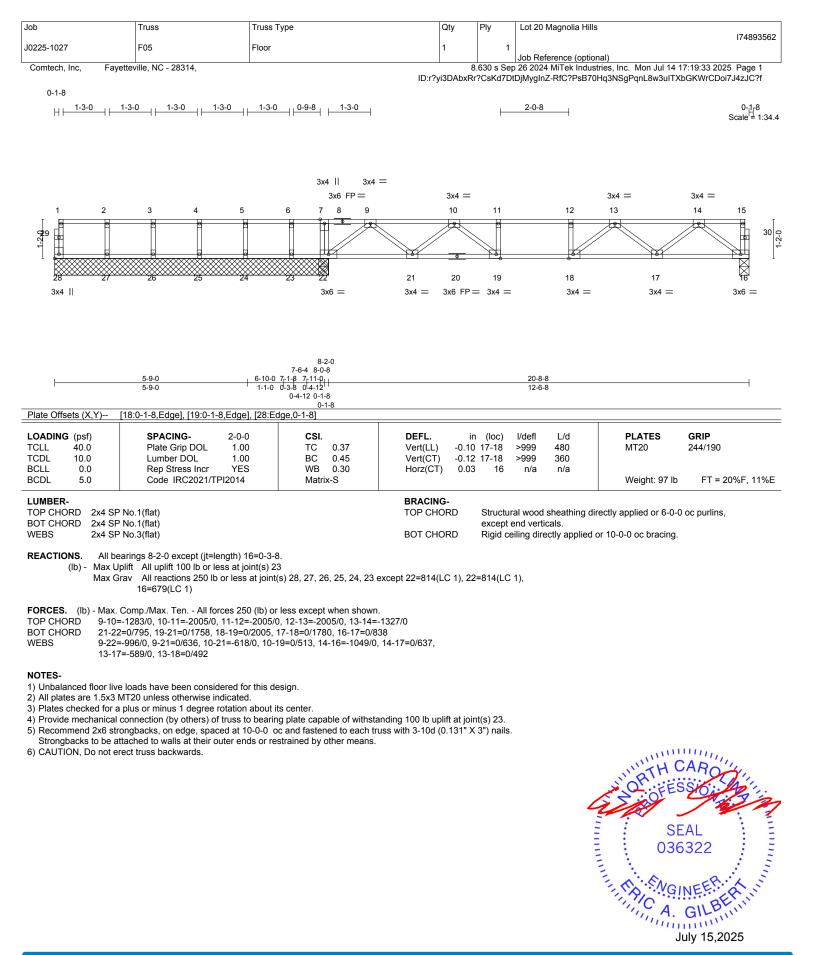
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



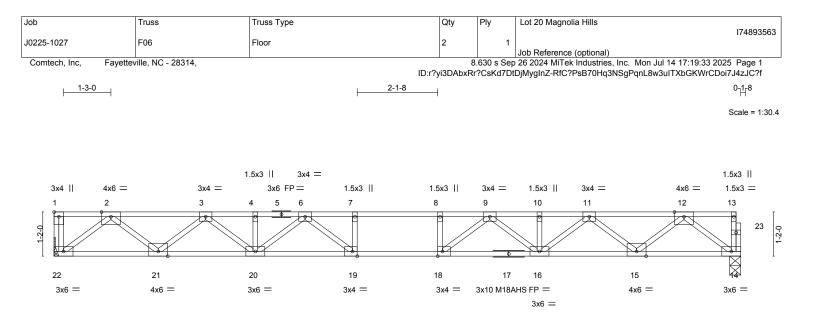
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of frusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scitut Information**. Building from the Structure Building Component Advance interpreted and the property damage. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





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818 Soundside Road



			<u>18-1-8</u> 18-1-8					I
Plate Offsets (X,Y)	[1:Edge,0-1-8], [18:0-1-8,Edge], [19:0-1	-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYES	CSI. TC 0.38 BC 0.48 WB 0.53		in (loc) -0.24 18-19 -0.33 18-19 0.06 14	l/defl >881 >641 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS	GRIP 244/190 186/179
BCDL 5.0	Code IRC2021/TPI2014	Matrix-S					Weight: 92 lb	FT = 20%F, 11%E
BOT CHORD 2x WEBS 2x REACTIONS .	4 SP 2400F 2.0E(flat) 4 SP 2400F 2.0E(flat) 4 SP No.3(flat) (size) 22=Mechanical, 14=0-3-8 ax Grav 22=983(LC 1), 14=977(LC 1)		BRACING- TOP CHORI BOT CHORI	excep	t end verf	icals.	rectly applied or 6-0-0 or 10-0-0 oc bracing.) oc purlins,
TOP CHORD 2 BOT CHORD 2 WEBS 2	Iax. Comp./Max. Ten All forces 250 (lb) or -3=-2075/0, 3-4=-3469/0, 4-6=-3469/0, 6-7= -10=-3469/0, 10-11=-3469/0, 11-12=-2075/ 1-22=0/1229, 20-21=0/2888, 19-20=0/3892 14-15=0/1228 -22=-1541/0, 2-21=0/1102, 3-21=-1058/0, 3 1-15=-1059/0, 11-16=0/741, 9-16=-539/0, 9 -19=-319/0, 8-18=-319/0	=-4167/0, 7 ⁻ 8=-4167/0, 8-{ 0 , 18-19=0/4167, 16-18=0/ ;-20=0/742, 12-14=-1538/	9=-4167/0, 3892, 15-16=0/288 0, 12-15=0/1103,	9,				

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



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Job	Truss Tr	uss Type	Qty	Ply	Lot 20 Magnolia Hills	174893564
J0225-1027	F06A FI	noc	1	1	Job Reference (optiona	D
Comtech, Inc, Fay	etteville, NC - 28314,				26 2024 MiTek Industrie	es, Inc. Mon Jul 14 17:19:34 2025 Page 1 g3NSgPgnL8w3uITXbGKWrCDoi7J4zJC?f
1-2-8		—	1-7-8		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0- <u>1</u> -8
						Scale = 1:30.7
3x6 = 6x8 =	= 6x8 =	3x6 FP = 6x6 =	6x6	3x6 F	P = 6x8 =	3x4 6x8 = 1.5x3 =
1 2	3 4 5	6 7 8	9 10	- 11		14 15 16
				-		
1-2-0						
				<u> </u>	•	
33 32	31 30 29 28	27 26	25 24	23	22 21 20	
4x8 = 1.5x3	4x8 = 1.5x3 6x8	= 4x6	4x6 3x10	M18AHS	FP = 1.5x3 6x8 =	4x8 = 1.5x3 4x8 =
		1	8-1-8 8-1-8			
	[2:0-3-8,Edge], [4:0-2-12,Edge], [8: [26:0-3-0,Edge], [28:0-3-12,Edge],		3:0-3-12,EdgeJ, [15:0-3-	8,Edge],	17:Edge,0-1-8], [22:0-2	2-12,Edgej, [25:0-3-0,Edge],
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl L/d	PLATES GRIP
TCLL 40.0 TCDL 10.0	Plate Grip DOL 1.00 Lumber DOL 1.00	TC 0.31 BC 0.71	()		>999 480 >475 360	MT20 244/190 M18AHS 186/179
BCLL 0.0	Rep Stress Incr NO	WB 0.94 Matrix-S	Horz(CT) 0.09	17	n/a n/a	
	Code IRC2021/TPI2014	Wallix-S				Weight: 141 lb FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP			BRACING- TOP CHORD		0	ctly applied or 6-0-0 oc purlins,
	? 2400F 2.0E(flat) ? No.3(flat)		BOT CHORD		nd verticals. iling directly applied or	10-0-0 oc bracing.
REACTIONS. (size	e) 33=Mechanical, 17=0-3-8			Ū		-
,	irav 33=1877(LC 1), 17=1864(LC 1)				
	Comp./Max. Ten All forces 250 (I					
9-10=	4395/0, 3-4=-4417/0, 4-5=-7551/0, =-8958/0, 10-12=-7551/0, 12-13=-7	551/0, 13-14=-4419/0, 14-15=-	4395/0			
	3=0/2488, 31-32=0/2490, 30-31=0/5 6=0/8958, 24-25=0/8493, 22-24=0/	,				
18-19	9=0/2482, 17-18=0/2481 =-3000/0, 2-31=0/2443, 3-31=-392/0					
7-28=	-1143/0, 7-26=0/928, 8-26=-390/0,	15-17=-2979/0, 15-19=0/2453	8, 14-19=-400/0,			
	9=-1846/0, 13-22=0/1970, 12-22=-2	77/0, 10-22=-1144/0, 10-25=0	/928, 9-25=-390/0			
NOTES- 1) Unbalanced floor live	e loads have been considered for th	is design.				
	plates unless otherwise indicated. T20 unless otherwise indicated.	·				
4) Plates checked for a	a plus or minus 1 degree rotation ab	out its center.				MUMULUU
6) Load case(s) 1, 2, 3	truss to truss connections. , 4, 5, 6 has/have been modified. B	uilding designer must review lo	ads to verify that they ar	e correct	for the	TH CARO
intended use of this 7) Recommend 2x6 str	truss. ongbacks, on edge, spaced at 10-0	-0 oc and fastened to each tru	uss with 3-10d (0.131" X	3") nails.		NO SESSION VE
Strongbacks to be a 8) CAUTION, Do not e	ttached to walls at their outer ends	or restrained by other means.			J.	
, · ·					Ē	SEAL
, ,	balanced): Lumber Increase=1.00, F	Plate Increase=1.00			E	036322
	=-10, 1-16=-200					N. M.E.
 Dead: Lumber Increa Uniform Loads (plf) 	ase=1.00, Plate Increase=1.00					MGINEEP A
Vert: 17-33	=-10, 1-16=-200 oor Live (unbalanced): Lumber Incr	ease=1.00 Plate Incrosco-1.0	00			A GILBE
J ISCONASE DEdu + FI	undalanceu). Lumbel MCI					A. GILMAN
Continued on page 2						July 15,2025

- 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 17-33=-10, 1-16=-200
- 2) Dead: Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf) Vert: 17-33=-10, 1-16=-200
- 3) 1st chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Lot 20 Magnolia Hills
10005 4007	F00A	F lass			174893564
J0225-1027	F06A	Floor	1	1	Job Reference (optional)
	III. NO. 00044				

Comtech, Inc, Fayetteville, NC - 28314,

8.630 s Sep 26 2024 MiTek Industries, Inc. Mon Jul 14 17:19:34 2025 Page 2 ID:r?yi3DAbxRr?CsKd7DtDjMygInZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

Uniform Loads (plf)

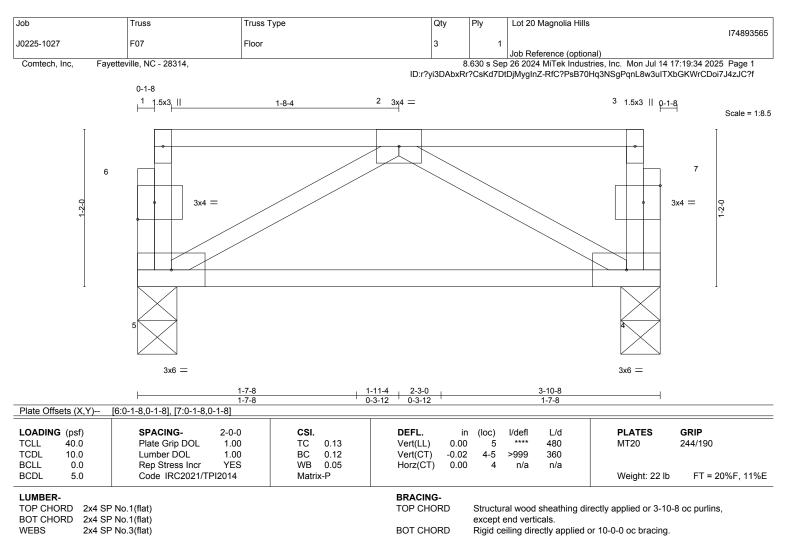
Vert: 17-33=-10, 1-9=-200, 9-16=-120

- 2nd chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 17-33=-10, 1-8=-120, 8-16=-200 5) 3rd chase Dead: Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf)
 - Vert: 17-33=-10, 1-9=-200, 9-16=-120
- 6) 4th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
 - Vert: 17-33=-10, 1-8=-120, 8-16=-200



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REACTIONS. (size) 5=0-3-8, 4=0-3-8

Max Grav 5=193(LC 1), 4=193(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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818 Soundside Road Edenton, NC 27932

	Truss	Truss	Туре	Qt	y P	Ply	Lot 20 Magnolia Hills	3	17 40005
225-1027	FKW1	GABL	E	1		1			1748935
			_				Job Reference (option		
Comtech, Inc, Fay	vetteville, NC - 28314,							tries, Inc. Mon Jul 14 1	
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	.1 20	19	18	17	16		15	14 13	
	.1 20	19	18	17	16		15	14 13	
	.1 20	19	18	17	16		15	14 13	
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	.1 20	19	18	17	16		15	14 13	
3x4					16				3x4 =
3x4    1-4-0	, 2-8-0 ,	4-0-0	5-4-0 6-8-0	, 8-0-0	16	9-4-0	, 10-8-0	, <u>12-0-0</u> ,	3x4 = 13-5-8
3x4      <u>1-4-0</u> 1-4-0	+ 2-8-0 1-4-0				16	<u>9-4-0</u> 1-4-0			3x4 =
3x4      <u>1-4-0</u> 1-4-0	, 2-8-0 ,	4-0-0	5-4-0 6-8-0	, 8-0-0	16	<u>9-4-0</u> 1-4-0	, 10-8-0	, <u>12-0-0</u> ,	3x4 = 13-5-8
3x4      <u>1-4-0</u> 1-4-0 ate Offsets (X,Y)	<u>- 2-8-0</u> 1-4-0 [22:Edge,0-1-8]	4-0-0 1-4-0	5-4-0 6-8-0 1-4-0 1-4-0	<u>8-0-0</u> 1-4-0		1-4-0	<u>− 10-8-0</u> − 1-4-0	<u>  12-0-0</u>   1-4-0	3x4 =
3x4    <u>1-4-0</u> <u>1-4-0</u> <u>ate Offsets (X,Y)</u> DADING (psf)	2-8-0 1-4-0 [22:Edge,0-1-8] SPACING-	<u>4-0-0</u> <u>1-4-0</u> 2-0-0	5-4-0 6-8-0 1-4-0 1-4-0 CSI.	8-0-0 1-4-0 DEFL.		1-4-0 (loc)	<u>+ 10-8-0</u> 1-4-0 //defl L/d	12-0-0 1-4-0 PLATES	3x4 =
3x4    <u>1-4-0</u> <u>1-4-0</u> <u>ate Offsets (X,Y)</u> DADING (psf) CLL 40.0	2-8-0 1-4-0 [22:Edge,0-1-8] SPACING- Plate Grip DOL	<u>4-0-0</u> <u>1-4-0</u> 2-0-0 _ 1.00	5-4-0 6-8-0 1-4-0 1-4-0 CSI. TC 0.07	B-0-0 1-4-0 DEFL. Vert(LL)	in n/a	1-4-0 (loc)	10-8-0 1-4-0 Vdefl L/d n/a 999	<u>  12-0-0</u>   1-4-0	3x4 =
3x4    <u>1-4-0</u> <u>1-4-0</u> <u>1-4-0</u> ate Offsets (X,Y) DADING (psf) CL 40.0 CDL 10.0	2-8-0 1-4-0 [22:Edge,0-1-8] SPACING- Plate Grip DOL Lumber DOL	4-0-0 1-4-0 2-0-0 - 1.00 1.00	5-4-0 6-8-0 1-4-0 1-4-0 CSI. TC 0.07 BC 0.01	8-0-0 1-4-0 DEFL. Vert(LL) Vert(CT)	in n/a n/a	1-4-0 (loc)	10-8-0 1-4-0 1/defl L/d n/a 999 n/a 999	12-0-0 1-4-0 PLATES	3x4 =
3x4    <u>1-4-0</u> <u>1-4-0</u> <u>ate Offsets (X,Y)</u> DADING (psf) CLL 40.0	2-8-0 1-4-0 [22:Edge,0-1-8] SPACING- Plate Grip DOL	4-0-0 1-4-0 2-0-0 - 1.00 1.00 cr YES	5-4-0 6-8-0 1-4-0 1-4-0 CSI. TC 0.07	B-0-0 1-4-0 DEFL. Vert(LL)	in n/a	1-4-0 (loc)	10-8-0 1-4-0 Vdefl L/d n/a 999	12-0-0 1-4-0 PLATES	3x4 =

TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-5-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



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Job	Truss	Truss Ty	pe		Qty	Ply	Lot 20 Magnolia	a Hills		
J0225-1027	FKW3	GABLE			1		1			174893567
						0000	Job Reference (			7 40 05 0005 D
Comtech, Inc, Faye	etteville, NC - 28314,			ID						17:19:35 2025 Page 1 bGKWrCDoi7J4zJC?f
0 ₁₁ 8							, ,,,			0 ₁ 18
										Scale = 1:21.
1 2	3	4	5	6		7	8	9		10 11
	• •	•	•	0 		0	•			24
					*****					
22 21 3x4 =	20	19	18	17		16	15	14		13 12 3x4 =
, 1-4-0	, <del>2-8-</del> 0 ,	4-0-0 ,	5-4-0	6-8-0 ,	8-0-0		9-4-0	10-8-0	12-0-0	12 10 0
1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	8-0-0 1-4-0		9-4-0 1-4-0	1-4-0	12-0-0	12-10-0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TF	2-0-0 1.00 1.00 YES Pl2014	<b>CSI.</b> TC 0.06 BC 0.01 WB 0.03 Matrix-R	DEFL Vert(I Vert( Horz(	_L) n/a CT) n/a	-	l/defl L/d n/a 999 n/a 999 n/a n/a	M	<b>ATES</b> T20 eight: 55 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
	No.1(flat) No.1(flat)			BRAC TOP (	CING- CHORD		ural wood sheathi t end verticals.	ng directly app	lied or 6-0-0	) oc purlins,

C 2x4 SP No.3(flat) WEBS OTHERS 2x4 SP No.3(flat) BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

# REACTIONS. All bearings 12-10-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

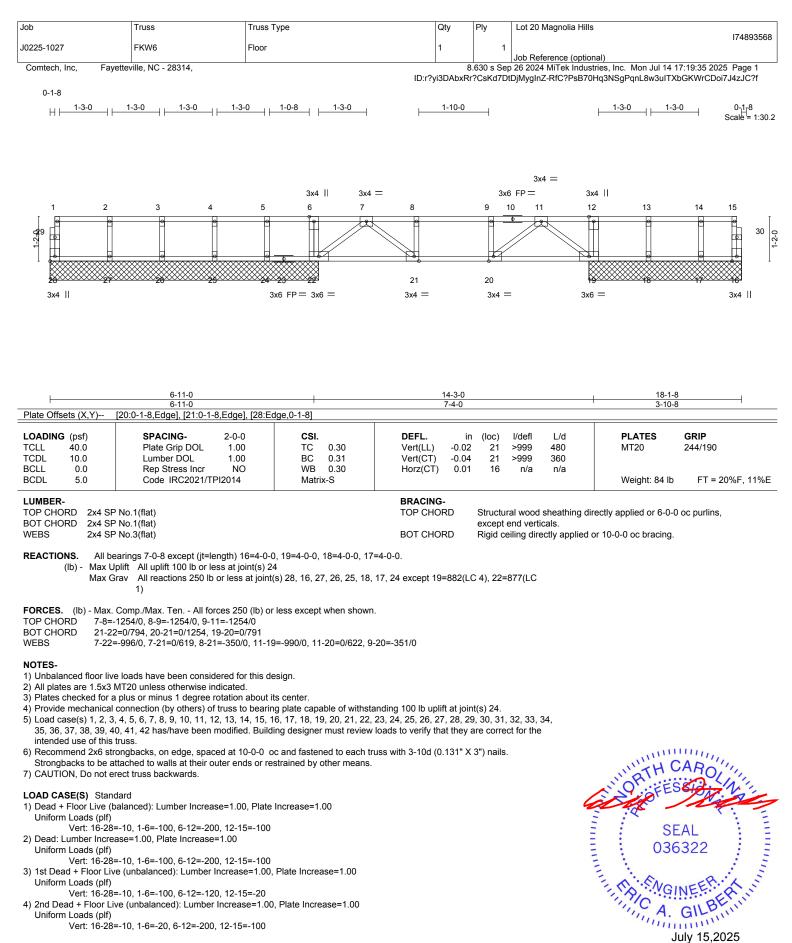
5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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Continued on page 2



818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 20 Magnolia Hills
				-	174893568
J0225-1027	FKW6	Floor	4	1	11 1000000
JUZZ5-10Z7	FRVO	Floor	1		
					Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8	.630 s Sec	26 2024 MiTek Industries, Inc. Mon Jul 14 17:19:35 2025 Page 2

ID:r?yi3DAbxRr?CsKd7DtDjMygInZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Comtech, Inc, Fayetteville, NC - 28314 LOAD CASE(S) Standard 5) 3rd unbalanced Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-120, 12-15=-20 6) 4th unbalanced Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-20, 6-12=-200, 12-15=-100 7) 1st chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 8) 2nd chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 9) 3rd chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 10) 4th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 11) 5th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 12) 6th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 13) 7th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 14) 8th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 15) 9th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 16) 10th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 17) 11th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-9=-200, 9-12=-120, 12-15=-100 18) 12th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-8=-120, 8-12=-200, 12-15=-100 19) 13th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 20) 14th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 21) 15th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 22) 16th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 23) 17th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 24) 18th chase Dead + Floor Live (unbalanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 25) 19th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 26) 20th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 27) 21st chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 28) 22nd chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 29) 23rd chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100

### Continued on page 3

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Job		Truss	Truss Type	Qty	Ply	Lot 20 Magnolia Hills
J0225-1027		FKW6	Floor	1	1	174893568
				-	-	Job Reference (optional)
Comtech, Inc,	Fayettev	/ille, NC - 28314,		8	.630 s Sep	26 2024 MiTek Industries, Inc. Mon Jul 14 17:19:35 2025 Page 3

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LOAD CASE(S) Standard

- 30) 24th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 31) 25th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 32) 26th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 33) 27th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 34) 28th chase Dead: Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100
- 35) 29th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-9=-200, 9-12=-120, 12-15=-100 36) 30th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-8=-120, 8-12=-200, 12-15=-100 37) 31st chase Dead: Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 38) 32nd chase Dead: Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100
- 39) 33rd chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100
- 40) 34th chase Dead: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)
- Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 41) 35th chase Dead: Lumber Increase=1.00, Plate Increase=1.00
- Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100 42) 36th chase Dead: Lumber Increase=1.00, Plate Increase=1.00
- 42) Son chase Dead. Lumber increase=1.00, Plate increase=1.00 Uniform Loads (plf) Vert: 16-28=-10, 1-6=-100, 6-12=-200, 12-15=-100

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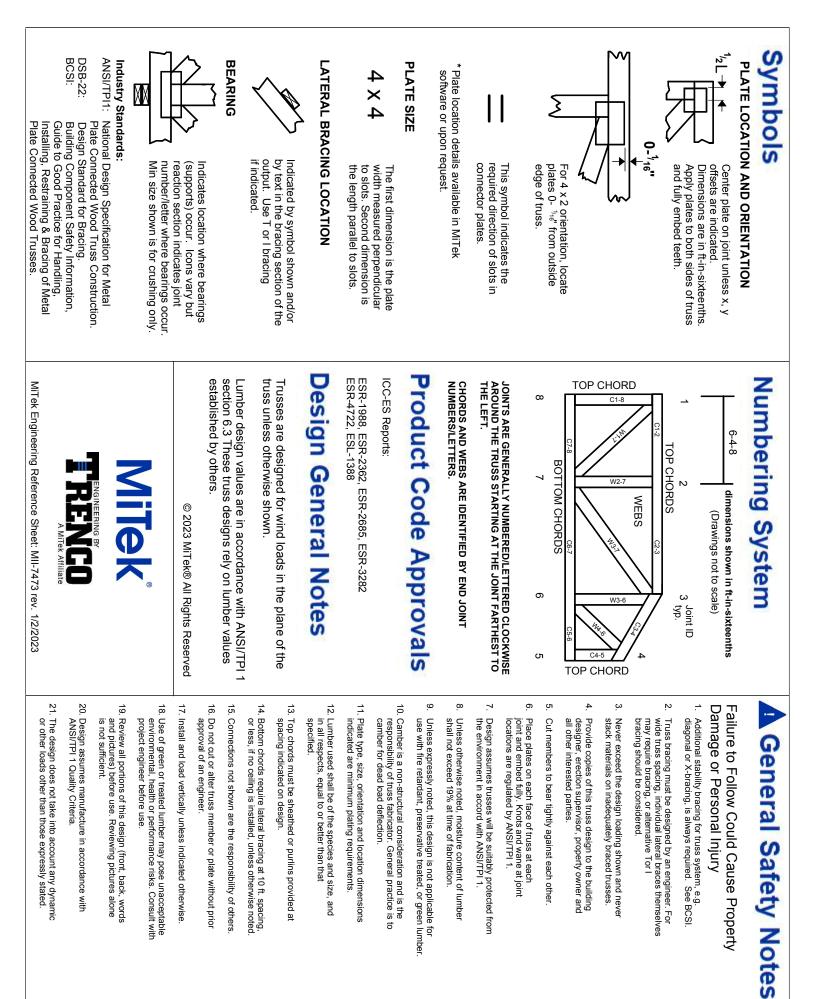
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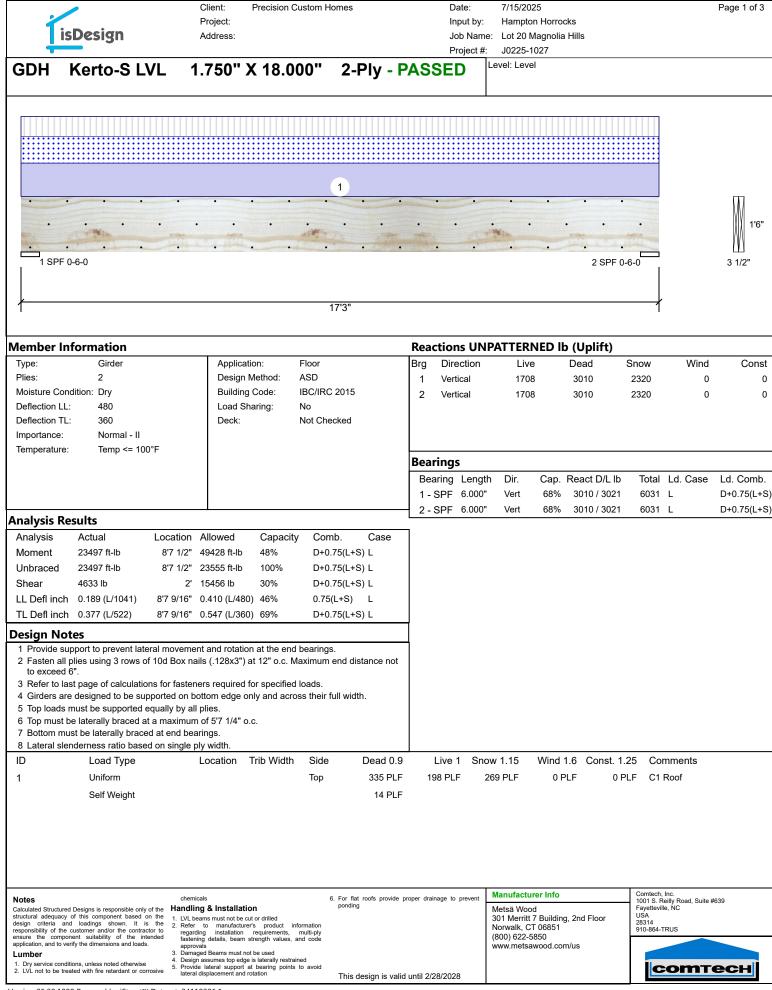


Contraction

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Version 25.20.1080 Powered by iStruct[™] Dataset: 24110201.1

		(	Client: F	Precision C	ustom Homes		Da	ate:	7/15/202	5				Page 2 of
· ·		I	Project:				In	put by:	Hampton	Horrocks				-
ÍS	Design	,	Address:						e: Lot 20 M					
								roject #:						
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1 SPF 0-	-3-8		2 SPF 0-3											⊥⊥ 3 1/2"
			2011 0-0											
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Member In	formation						Reaction	ns UN	PATTERN	IED lb (U	plift)			
Туре:	Girder		Applicatio		Floor		l v	ection	Live	De		Snow	Wind	Cor
Plies: Moisture Con	2 dition: Dry		Design M Building (		ASD IBC/IRC 2015		1 Vert		650		55	0	0	
Deflection LL:			Load Sha		No		2 Vert	ical	650	/	55	0	0	
Deflection TL:	360		Deck:	-	Not Checked									
Importance:	Normal - II													
Temperature:	Temp <= 100°	F												
							Bearings							
							Bearing	-		Cap. Rea			Ld. Case	Ld. Com
							1 - SPF				755 / 650	1405		D+L
Analysis Re	sults		•				2 - SPF	3.500"	Vert	27%	755 / 650	1405	L	D+L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	7							
Moment	1449 ft-lb	2'6" 2	26999 ft-lb	5%	D+L	L								
Unbraced	1449 ft-lb	2'6" 2	20546 ft-lb	7%	D+L	L								
Shear	983 lb	3'6 1/2"	10453 lb	9%	D+L	L								
LL Defl inch	0.003 (L/17407)	2'6" (	0.114 (L/480)	3%	L	L								
TL Defl inch	0.007 (L/8054)	2'6" (	0.151 (L/360)	4%	D+L	L								
Design Not	tes													
	pport to prevent latera plies using 3 rows of				0	stance not								
to exceed 6	5".					Stance not								
	st page of calculations designed to be supp		•	•		th								
	nust be supported eq			y and acro.		u1.								
	e laterally braced at e													
	st be laterally braced nderness ratio based													
ID	Load Type			rib Width	Side	Dead 0.9	Live ²	1 Sno	ow 1.15	Wind 1.6	Const. 1.	25 Cor	nments	
1	Uniform	•			Far Face	81 PLF	260 PLF		0 PLF	0 PLF	0 F			
2	Uniform				Тор	210 PLF	0 PLI		0 PLF	0 PLF	0 F			
-	Self Weight					11 PLF								
	een rreigin													
												1		
Notes	Designs is responsible only of	chemica			6. For fla ponding		roper drainage to	prevent	Manufacture Metsä Wood			Loomtech, I 1001 S. Re Fayetteville	eilly Road, Suite #	#639
structural adequacy	of this component based on loadings shown. It is	the 1. LVL bea	to manufacturer's	or drilled					301 Merritt 7	Building, 2nd	l Floor	USA 28314		
responsibility of the ensure the comport	customer and/or the contracto nent suitability of the inten	r to regardin	to manufacturer's ng installation r g details, beam str	equirements,	multi-ply				Norwalk, CT (800) 622-58	50		910-864-T	RUS	
application, and to ver Lumber	ify the dimensions and loads.	approva 3. Damage	ils ed Beams must not b	e used					www.metsaw					
1. Dry service condit	ions, unless noted otherwise ated with fire retardant or corros	. 5. Provide	assumes top edge is lateral support at lisplacement and rota	bearing points	to avoid								от	есн
		iateral 0	)201.1		This	design is valid	until 2/28/2028	8						

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~			Client: F Project:	Precision Cu	ustom Homes		Date Inpu	e: ut by:	7/15/202 Hamptor	25 n Horrock	S			Page 3 of 3
i	Design	1	Address:					Name		-	lills			
				44.00				ject #:	J0225-10 Level: Level					
BM3	Kerto-S LV	/L 1.	/50" X	14.00	0  2-I	PIY - P	ASSED	ľ						
	2													
		1												
•	•	·	•	•	•									M
·	a sitter	1	•	•	and the first									1'2"
• 	• •	بد والمطل	- A STATE	•										3 1/2"
	-3-6			2 SPF 0	-3-0									5 1/2
·		6'												
Member In Type:	formation Girder		Applicatio	n:	Floor		Reactions Brg Direc		PATTERN Live		<b>(Uplift)</b> Dead	Snow	Wind	Const
Plies:	2		Design M		ASD		1 Vertic		1900		667	0 0	0	0
Moisture Cor			Building C		IBC/IRC 2015		2 Vertic	al	1874		658	0	0	0
Deflection LL			Load Sha	-	No									
Deflection TL			Deck:		Not Checked									
Importance: Temperature:	Normal - II Temp <= 100	°F												
remperature.	Temp <= 100						Bearings							
							Bearing L	enath	n Dir.	Cap R	eact D/L	b Tota	Ld. Case	Ld. Comb.
							1 - SPF 3	-	Vert	49%	667 / 190			D+L
							2-SPF 3		Vert	57%	658 / 187			D+L
Analysis Re	esults						ـــــــــــــــــــــــــــــــــــــ							
Analysis	Actual	Location A		Capacity		Case								
Moment	3312 ft-lb		26999 ft-lb	12%	D+L	L								
Unbraced	3312 ft-lb		7594 ft-lb	19%	D+L	L								
Shear	1733 lb	1'5 1/2" 1		17%	D+L	L								
	0.014 (L/4667)		).140 (L/480)		L	L								
	0.019 (L/3454)	3' 1/4" (	).186 (L/360)	10%	D+L	L	-							
Design No 1 Provide su	pport to prevent late	ral movement	and rotation	at the end	bearings.		4							
2 Fasten all to exceed	plies using 3 rows of 6"	10d Box nail	s (.128x3") at	12" o.c. Ma	aximum end dis	stance not								
	st page of calculation	ns for fastene	rs required fo	r specified	oads.									
	e designed to be sup		-	y and acros	s their full widt	th.								
	must be supported e be laterally braced at													
	ist be laterally brace	-												
	nderness ratio based	• • •												
ID	Load Type	L	ocation T	ib Width	Side	Dead 0.9	Live 1	Sno			6 Const.		omments	
1	Uniform				Near Face	87 PLF	260 PLF		0 PLF	0 PL	F (	PLF FC	2	
2	Uniform				Тор	123 PLF	369 PLF		0 PLF	0 PL	F (	PLF FC	6	
	Self Weight					11 PLF								
Notes	ponding											Comtec 1001 S.	Reilly Road, Suite	#639
Calculated Structured Designs is responsible only of the Handling & Installation ponding structural adequacy of this component based on the 1. LVL beams must not be cut or drilled									Metsä Wood 301 Merritt 7 Building, 2nd Floor			Fayetter USA 28314	ille, NC	
design criteria and loadings shown. It is the 2. Refer to manufacturer's product information responsibility of the customer and/or the contractor to ensure the component suitability of the intended fastening details, beam strength values, and code									Norwalk, CT (800) 622-58	06851		28314 910-864	TRUS	
application, and to ve	rify the dimensions and loads.	approva	g details, beam stre s d Beams must not b	-	iu coue				www.metsav		us			
1. Dry service cond	tions, unless noted otherwise	<ol> <li>Design a</li> <li>Provide</li> </ol>	ssumes top edge is lateral support at	laterally restrain bearing points f	ed o avoid								comt	есн
<ol> <li>LVL NOT to be tre</li> </ol>	ated with fire retardant or corr	lateral di	splacement and rota	ition		design is valid	until 2/28/2028							
	0 Powered by iStruct™	D	201 1									CCE		