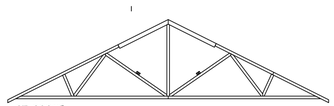
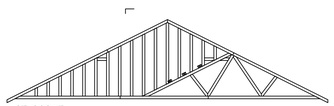
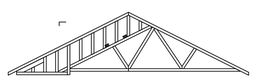


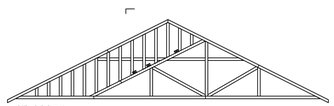
SE-22203



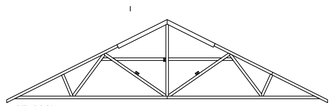
SE-22205



SE-22206



SE-22209



SE-22210

TRUSS BRACING DETAILS

1 / S-3

SCALE: 1/4" = 1'-0"

TRUSS BRACING NOTES:

1. IF TRUSS DOES NOT APPEAR ON THIS TRUSS BRACING SHEET, NO ADDITIONAL LATERAL BRACING IS REQUIRED.
2. 2X4 SPF#2 LATERAL BRACES SHALL BE NAILED TO MINIMUM (3) TRUSS MEMBERS WITH MINIMUM (2) 10D NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING.
3. WEB "T" BRACE, DETAIL **3/RF-1c**, IS REQUIRED WHERE LATERAL BRACING IS NOT CONTINUOUS ACROSS THREE (3) OR MORE TRUSSES AND MAY BE USED IN LIEU OF 2X4 LATERAL BRACING.
4. DIAGONAL BRACING REQUIRED WHEN LATERAL BRACING IS REQUIRED **(4/RF-1c)**
5. STUDDED GABLE BRACING DETAIL **(1/RF-1c)** TO BE UTILIZED FOR TRUSSES 6'-9" IN HEIGHT OR GREATER.
6. PARTIALLY SHEATHED GABLES, SEE **(5/RF-1c)** FOR "L" BRACING WHEN REQUIRED.
7. LATERAL BRACING CAN BE APPLIED TO EITHER SIDE OF THE WEB MEMBER IDENTIFIED IN THE DRAWING.
8. SHEATHING (OSB OR GYPSUM) REPLACES LATERAL AND DIAGONAL TRUSS BRACING.

DIV-COMM-LOT-UNIT: RLH-VK-0115

COMM-LOT: KIPLING VILLAGE - 0115

STREET ADDRESS: 84 SAINTSBURY DRIVE

APT. NO.: ----

CITY: FUQUAY VARINA

STATE: NC

ZIP: 27526

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NVR
NVR, Inc.
5285 Westview Drive, Suite 100
Frederick, MD 21703

SET NO. ABY00
VERSION 02
RELEASE NO. ----
DRAWN BY
DATE:
OPTION

MODEL: ARUBA BAY

DRAWING TITLE: TRUSS BRACING DETAILS

OPTION DESCRIPTION

SHEET NO.: S-3

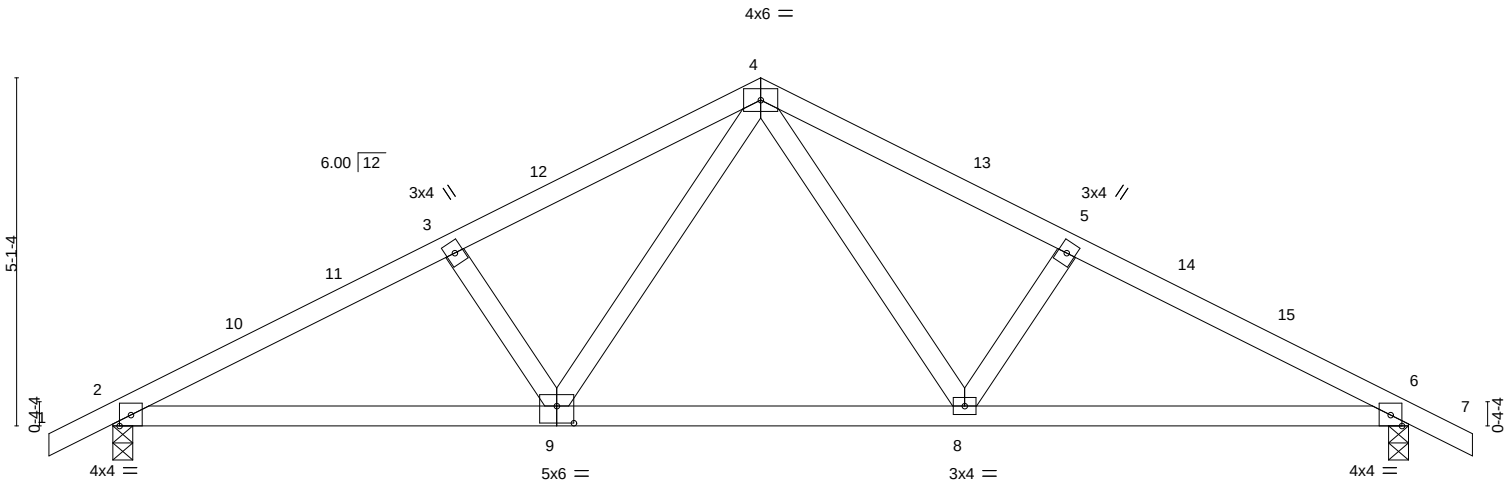
C:\NVR\Solvea\ARUBA BAY_ABY00_02\RLH-VK-0115\340198\Sheets\Lot Specific\S-3 TRS_LS.dwg 06/12/25 - 2:20 am

Job	Truss	Truss Type	Qty	Ply	10_Southeast
ORDERS	SE-22198	COMN	1	1	I58967546
					Job Reference (optional)

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:34 2023 Page 1
ID:6y08fdIBKOlJI8iZXMnHX1zFZU1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

-0-11-4 | 5-0-3 | 9-6-0 | 13-11-13 | 19-0-0 | 19-11-4
0-11-4 | 5-0-3 | 4-5-13 | 4-5-13 | 5-0-3 | 0-11-4

Scale = 1:33.8



6-6-2 | 12-5-14 | 19-0-0
6-6-2 | 5-11-13 | 6-6-2

Plate Offsets (X,Y)-- [9:0-3-0,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.52	Vert(LL)	-0.06	9	>999	360	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.55	Vert(CT)	-0.12	2-9	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.04	6	n/a	n/a		
BCLL	0.0 *	Code	IBC2021/TPI2014	Matrix-S		Wind(LL)	0.03	9	>999	240	Weight: 87 lb	FT = 20%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
WEBS 2x4 SP No.3 or 2x4 SPF Stud

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-0-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 6=0-3-8
Max Horz 2=90(LC 16)
Max Uplift 2=-121(LC 12), 6=-121(LC 13)
Max Grav 2=1094(LC 19), 6=1094(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1781/312, 3-4=-1530/322, 4-5=-1530/322, 5-6=-1781/312
BOT CHORD 2-9=-186/1510, 8-9=-54/913, 6-8=-186/1510
WEBS 3-9=-503/174, 4-9=-85/658, 4-8=-86/658, 5-8=-503/174

NOTES- (8-11)
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 6-6-0, Exterior(2R) 6-6-0 to 12-6-0, Interior(1) 12-6-0 to 16-11-4, Exterior(2E) 16-11-4 to 19-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 2 and 121 lb uplift at joint 6.
8) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
9) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
10) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

June 16,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

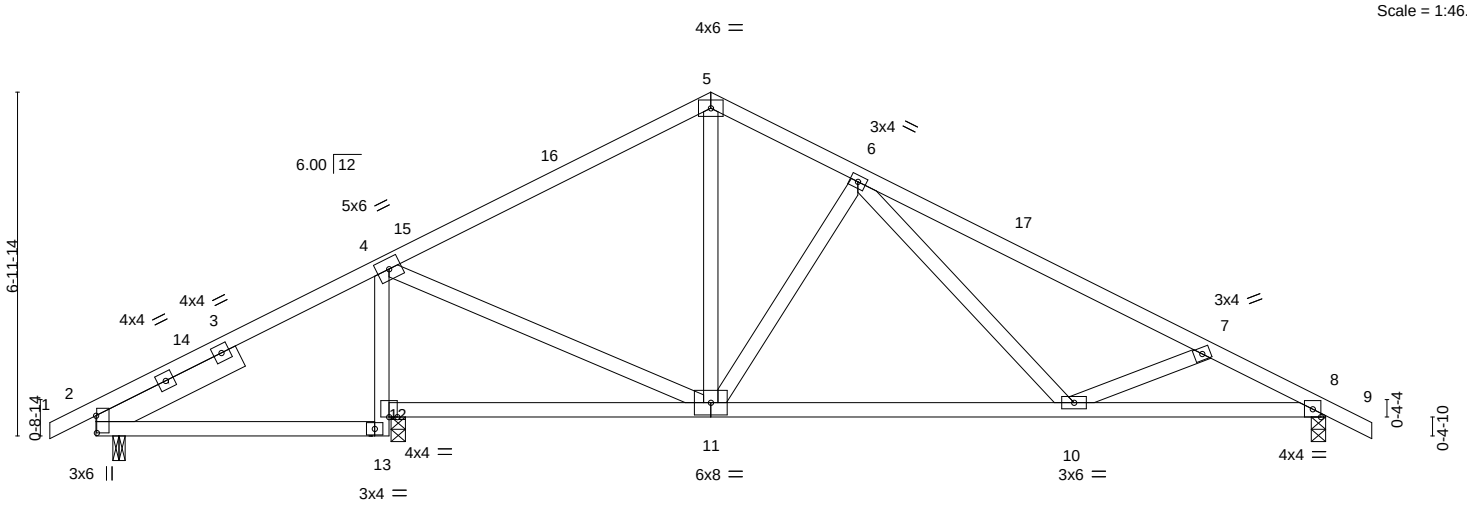
ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	10_Southeast	I58967550
ORDERS	SE-22202	SPEC	1	1	Job Reference (optional)	

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:37 2023 Page 1
ID:6y08fdIBKOlJI8iZXMnHX1zFZU1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-11-4 5-11-8 12-6-0 15-5-14 22-5-14 25-0-0 25-11-4
0-11-4 5-11-8 6-6-8 2-11-14 7-0-0 2-6-2 0-11-4

Scale = 1:46.8



0-4-2 5-11-8 6-0-0 9-6-0 12-6-0 25-0-0
0-4-2 5-7-6 0-0-8 3-6-0 3-0-0 12-6-0

Plate Offsets (X,Y)-- [2:0-4-3,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.82	Vert(LL)	-0.06	10-11	>999	360	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.51	Vert(CT)	-0.14	10-11	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(CT)	0.03	8	n/a	n/a		
BCLL	0.0 *	Code IBC2021/TPI2014		Matrix-S		Wind(LL)	0.03	10-11	>999	240	Weight: 135 lb	FT = 20%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
WEBS 2x4 SP No.3 or 2x4 SPF Stud
SLIDER Left 2x6 SP No.2 3-2-11

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-7-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 12=0-3-8, 8=0-3-8
Max Horz 12=-117(LC 17)
Max Uplift 2=-74(LC 12), 12=-100(LC 12), 8=-139(LC 13)
Max Grav 2=314(LC 19), 12=1349(LC 19), 8=1093(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 4-5=-856/195, 5-6=-863/227, 6-7=-1644/219, 7-8=-2035/310
BOT CHORD 4-12=-1229/219, 10-11=-51/1066, 8-10=-251/1805
WEBS 5-11=-89/476, 6-11=-777/214, 6-10=-21/423, 7-10=-495/255, 4-11=-6/834

NOTES- (8-11)
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 9-6-0, Exterior(2R) 9-6-0 to 15-5-14, Interior(1) 15-5-14 to 22-8-2, Exterior(2E) 22-8-2 to 25-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 2, 100 lb uplift at joint 12 and 139 lb uplift at joint 8.
8) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
9) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
10) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

June 16,2023

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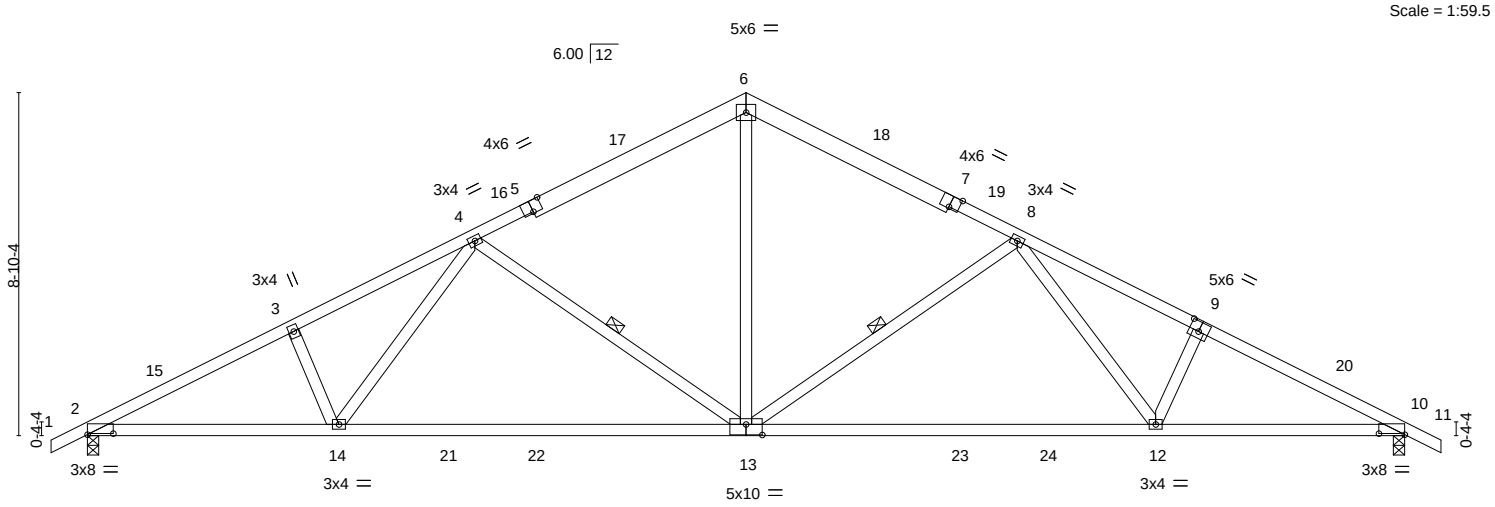
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	10_Southeast
ORDERS	SE-22203	COMN	1	1	I58967551
					Job Reference (optional)

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:38 2023 Page 1
ID:ohdzKV9G?y7BSGCelYVzPJyGyXY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-11-4	6-0-3	10-0-0	11-6-2	12-5-14	17-0-0	24-0-0	28-8-2	29-1-7	34-0-0	34-11-4
0-11-4	6-0-3	3-11-13	1-6-2	0-11-13	4-6-2	7-0-0	4-8-2	0-5-5	4-10-9	0-11-4

Scale = 1:59.5



8-9-3	12-5-14	17-0-0	27-6-14	34-0-0
8-9-3	3-8-12	4-6-2	10-6-14	6-5-2

Plate Offsets (X,Y)-- [2:0-8-0,0-0-5], [5:0-3-0,Edge], [7:0-3-0,Edge], [9:0-3-0,0-3-0], [10:0-8-0,0-0-5], [13:0-5-0,0-3-4]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.99	Vert(LL)	-0.23	13-14	>999	360	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.49	13-14	>826	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.48	Horz(CT)	0.11	10	n/a	n/a		
BCLL	0.0 *	Code	IBC2021/TPI2014	Matrix-S		Wind(LL)	0.09	13-14	>999	240	Weight: 183 lb	FT = 20%
BCDL	10.0											

LUMBER-

TOP CHORD	2x6 SP No.2 *Except*
	9-11,1-5: 2x4 SP No.2 or 2x4 SPF No.2, 7-9: 2x4 SP No.2D
BOT CHORD	2x4 SP 2250F 1.9E or 2x4 SPF 2100F 1.8E
WEBS	2x4 SP No.3 or 2x4 SPF Stud

BRACING-

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 4-13, 8-13

REACTIONS. (size) 2=0-3-8, 10=0-3-8
Max Horz 2=152(LC 16)
Max Uplift 2=-199(LC 12), 10=-199(LC 13)
Max Grav 2=1772(LC 1), 10=1772(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-3228/312, 3-4=-3061/348, 4-6=-2043/311, 6-8=-2043/311, 8-9=-3057/347, 9-10=-3228/313
BOT CHORD	2-14=-349/2780, 13-14=-253/2335, 12-13=-149/2337, 10-12=-198/2780
WEBS	6-13=-55/1107, 4-13=-881/254, 4-14=-56/567, 3-14=-298/164, 8-13=-885/256, 8-12=-54/561, 9-12=-297/163

NOTES- (8-11)

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 14-0-0, Exterior(2R) 14-0-0 to 20-0-0, Interior(1) 20-0-0 to 31-11-4, Exterior(2E) 31-11-4 to 34-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 2 and 199 lb uplift at joint 10.
8) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
9) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
10) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

June 16,2023

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TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	10_Southeast
ORDERS	SE-22204	COMN	1	1	I58967552
					Job Reference (optional)

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:40 2023 Page 1
ID:5lb56nez?NGp7jN5pS3vTuyrxGa-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

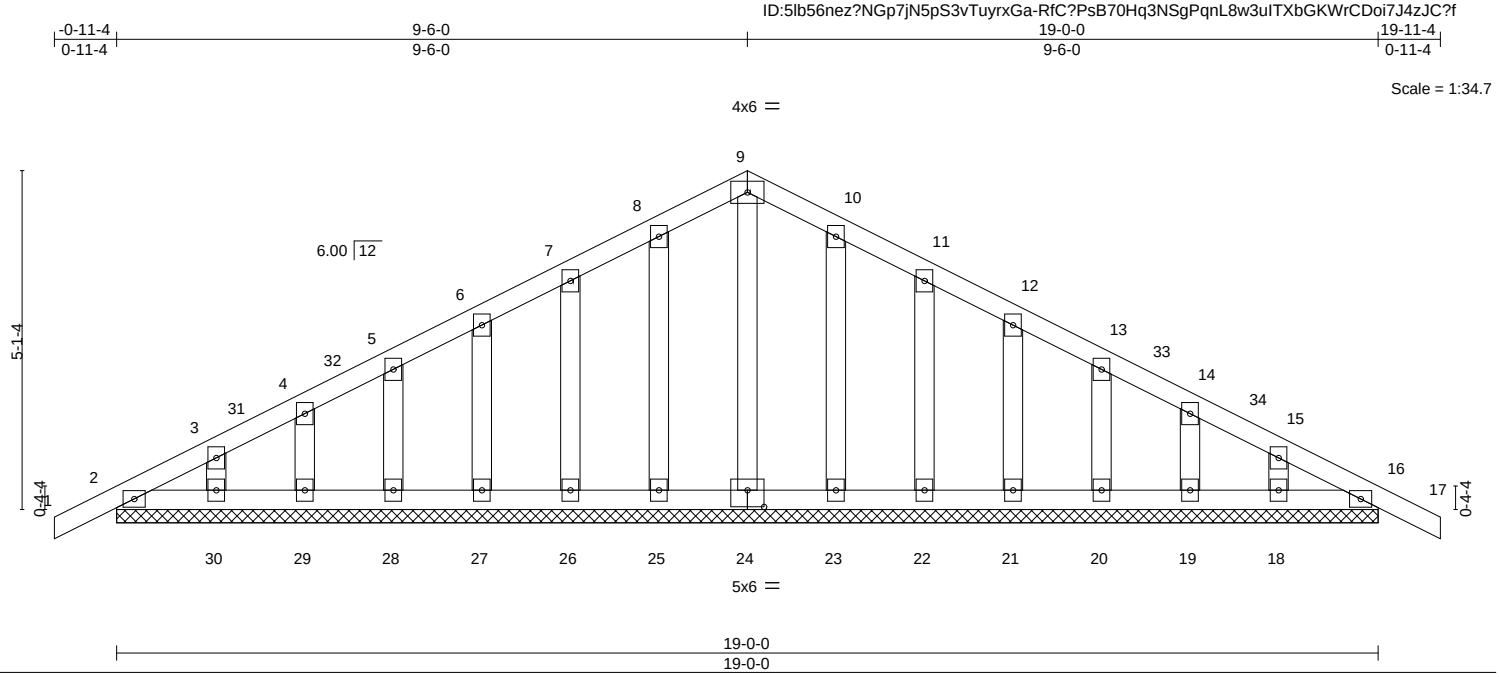

Plate Offsets (X,Y)-- [24:0-3-0,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	0.00	17	n/r	120	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	16	n/r	120		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	16	n/a	n/a		
BCLL	0.0 *	Code	IBC2021/TPI2014	Matrix-S							Weight: 110 lb	FT = 20%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
OTHERS 2x4 SP No.3 or 2x4 SPF Stud

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-0-0.
(lb) - Max Horz 2=90(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 23, 22, 21, 20, 19, 18, 16
Max Grav All reactions 250 lb or less at joint(s) 2, 24, 25, 26, 27, 28, 29, 30, 23, 22, 21, 20, 19, 18, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (12-15)
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-11-4 to 2-0-12, Exterior(2N) 2-0-12 to 6-6-0, Corner(3R) 6-6-0 to 12-6-0, Exterior(2N) 12-6-0 to 16-11-4, Corner(3E) 16-11-4 to 19-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
6) All plates are 3x4 MT20 unless otherwise indicated.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 1-4-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 25, 26, 27, 28, 29, 30, 23, 22, 21, 20, 19, 18, 16.
12) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
13) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
14) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
15) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

June 16,2023

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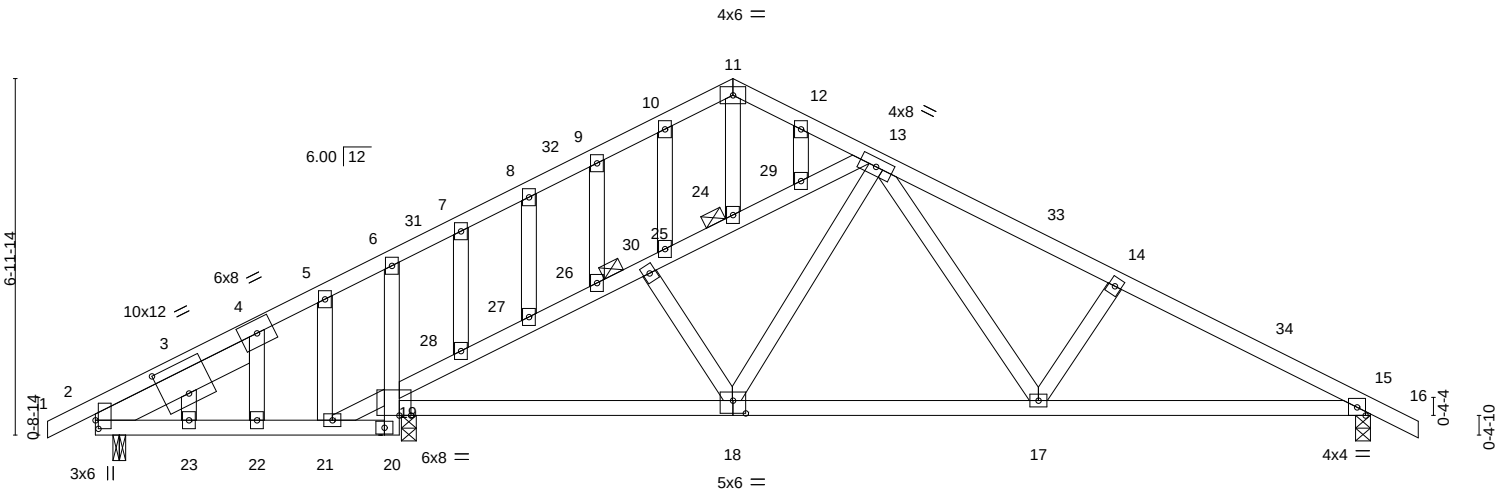
Job	Truss	Truss Type	Qty	Ply	10_Southeast
ORDERS	SE-22206	SPEC	1	1	I58967554
					Job Reference (optional)

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:43 2023 Page 1
ID:6y08fdIBKOlJI8iZXMnHX1zFZU1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-11-4 5-11-8 12-6-0 20-1-13 25-0-0 25-11-4
0-11-4 5-11-8 6-6-8 7-7-13 4-10-3 0-11-4

Scale = 1:45.2



0-4-2 5-11-8 6-0-0 12-5-14 12-6-0 18-5-14 25-0-0
0-4-2 5-7-6 0-0-8 6-5-14 0-0-2 5-11-14 6-6-2

Plate Offsets (X,Y)-- [2:0-2-0,0-0-11], [3:0-6-0,0-7-8], [18:0-3-0,0-3-0], [19:0-2-12,0-0-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.44	Vert(LL)	-0.07	17-18	>999	360	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.12	15-17	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.04	15	n/a	n/a		
BCLL	0.0 *	Code	IBC2021/TPI2014	Matrix-S		Wind(LL)	0.03	17	>999	240	Weight: 164 lb	FT = 20%
BCDL	10.0											

LUMBER-

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.3 or 2x4 SPF Stud
OTHERS	2x4 SP No.3 or 2x4 SPF Stud
SLIDER	Left 2x6 SP No.2 3-5-3

BRACING-

TOP CHORD	Structural wood sheathing directly applied or 3-11-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 20-21.
JOINTS	1 Brace at Jt(s): 24, 26

REACTIONS. (size) 2=0-3-0, 15=0-3-8, 19=0-3-8
Max Horz 19=-117(LC 17)
Max Uplift 15=-129(LC 13), 19=-202(LC 12)
Max Grav 2=427(LC 1), 15=1145(LC 20), 19=1237(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-341/0, 6-7=-253/5, 12-13=-289/77, 13-14=-1679/217, 14-15=-1891/204
BOT CHORD 20-21=-261/116, 6-19=-374/182, 18-19=-39/1153, 17-18=-0/1114, 15-17=-97/1608
WEBS 19-28=-1174/236, 27-28=-1123/226, 26-27=-1096/217, 26-30=-1076/200, 25-30=-1079/192, 24-25=-1076/159, 24-29=-1091/174, 13-29=-1050/160, 19-21=0/472, 18-30=-277/97, 13-18=-1/418, 13-17=-83/542, 14-17=-379/170

NOTES- (11-14)
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 1-9-5, Interior(1) 1-9-5 to 9-6-0, Exterior(2R) 9-6-0 to 15-4-9, Interior(1) 15-4-9 to 22-11-4, Exterior(2E) 22-11-4 to 25-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
6) All plates are 3x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 1-4-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=129, 19=202.

June 16,2023

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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11) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
12) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
13) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
14) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

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Job	Truss	Truss Type	Qty	Ply	10_Southeast
ORDERS	SE-22209	SPEC	1	1	I58967557
					Job Reference (optional)

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:46 2023 Page 1
ID:GUrlGSRQHYj5mxg0ZaxonnzqBL5-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

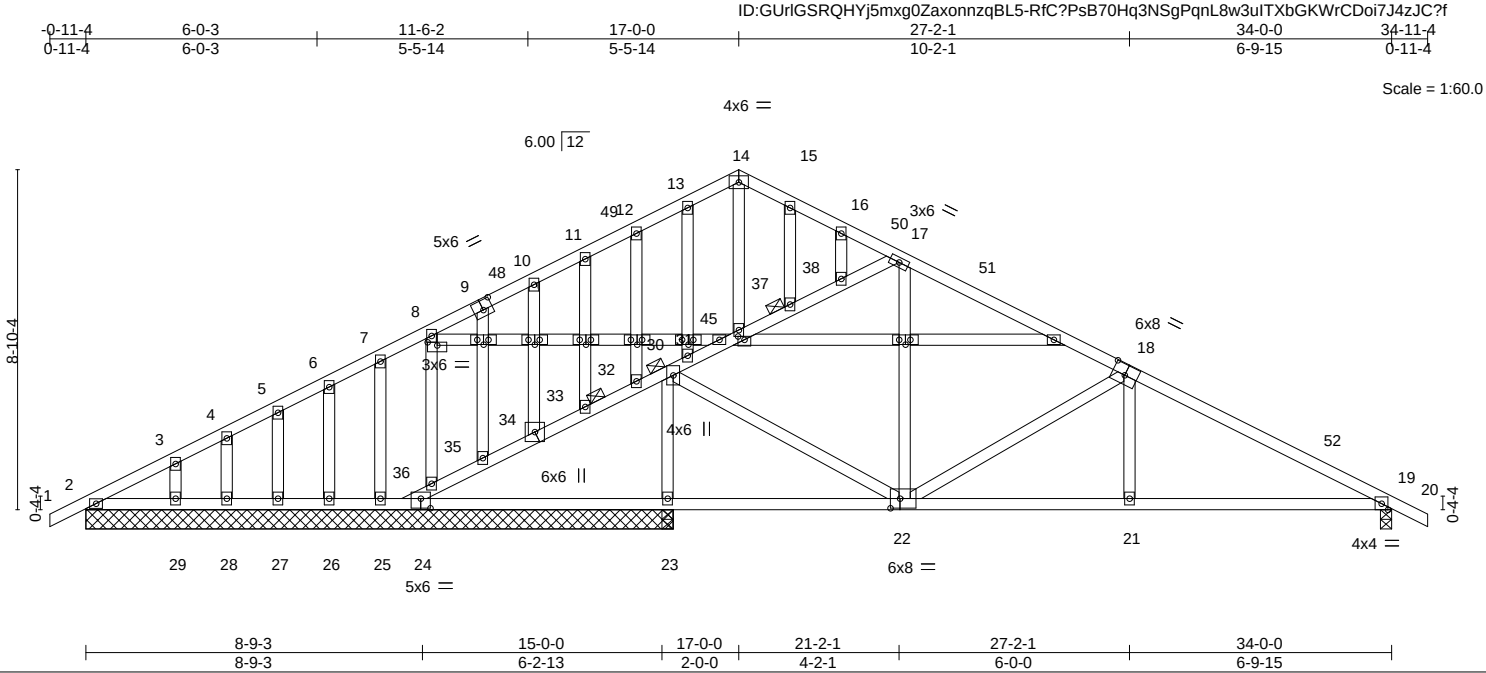

Plate Offsets (X,Y)-- [9:0-3-0,0-3-0], [18:0-4-0,0-3-4], [22:0-3-0,0-3-0], [24:0-3-0,0-3-0], [39:0-3-0,0-1-0], [40:0-1-8,0-1-8], [41:0-1-8,0-1-8], [42:0-1-8,0-1-8], [43:0-1-8,0-1-8], [44:0-1-8,0-1-8], [45:0-2-0,0-1-1], [46:0-1-8,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.74	Vert(LL)	-0.07	19-21	>999	360	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.15	19-21	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(CT)	0.03	19	n/a	n/a		
BCLL	0.0 *	Code	IBC2021/TPI2014	Matrix-S		Wind(LL)	0.05	19-21	>999	240		
BCDL	10.0										Weight: 256 lb	FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.2 or 2x4 SPF No.2 *Except* 23-30,17-22,22-30,18-21,18-22,16-38,24-34: 2x4 SP No.3 or 2x4 SPF Stud
OTHERS	2x4 SP No.3 or 2x4 SPF Stud

BRACING-

TOP CHORD	Structural wood sheathing directly applied or 3-0-14 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 21-22,19-21.
JOINTS	1 Brace at Jt(s): 30, 33, 37

REACTIONS. All bearings 15-3-8 except (jt=length) 19=0-3-8.
(lb) - Max Horz 2=154(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 23, 25, 26, 27, 28, 29 except 19=-150(LC 13), 24=-186(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 2, 25, 26, 27, 28, 29 except 23=1310(LC 20), 23=1307(LC 1), 19=1077(LC 20), 24=679(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-329/155, 3-4=-280/127, 4-5=-254/134, 17-18=-931/162, 18-19=-1650/204
BOT CHORD	2-29=-131/275, 28-29=-131/275, 27-28=-131/275, 26-27=-131/275, 25-26=-131/275, 24-25=-131/275, 21-22=-87/1365, 19-21=-86/1367
WEBS	24-36=-588/333, 35-36=-283/220, 34-35=-318/227, 33-34=-271/212, 30-31=-841/226, 31-45=-761/209, 37-45=-758/223, 37-38=-709/206, 17-38=-716/208, 23-30=-1174/150, 22-30=-62/899, 18-21=0/283, 18-22=-739/201, 13-31=-353/98, 12-32=-260/69, 8-36=-424/166

NOTES- (11-14)
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 2-4-0, Interior(1) 2-4-0 to 14-0-0, Exterior(2R) 14-0-0 to 20-0-0, Interior(1) 20-0-0 to 31-11-4, Exterior(2E) 31-11-4 to 34-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
6) All plates are 3x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 1-4-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

June 16,2023

Continued on page 2

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NOTES- (11-14)

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 25, 26, 27, 28, 29 except (jt=lb) 19=150, 24=186.
11) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
12) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
13) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
14) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	10_Southeast
ORDERS	SE-22210	COMN	1	1	I58967558
					Job Reference (optional)

NVR, Frederick, MD - 21703, 8.630 s Nov 19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:47 2023 Page 1
ID:ohdzKV9G?y7BSGCelYVzPJyGyXY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

-0-11-4 | 6-0-3 | 10-0-0 | 11-6-2 12-5-14 | 17-0-0 | 24-0-0 | 28-8-2 | 29-1-7 | 34-0-0 | 34-11-4
0-11-4 | 6-0-3 | 3-11-13 | 1-6-2 0-11-13 | 4-6-2 | 7-0-0 | 4-8-2 | 0-5-5 | 4-10-9 | 0-11-4

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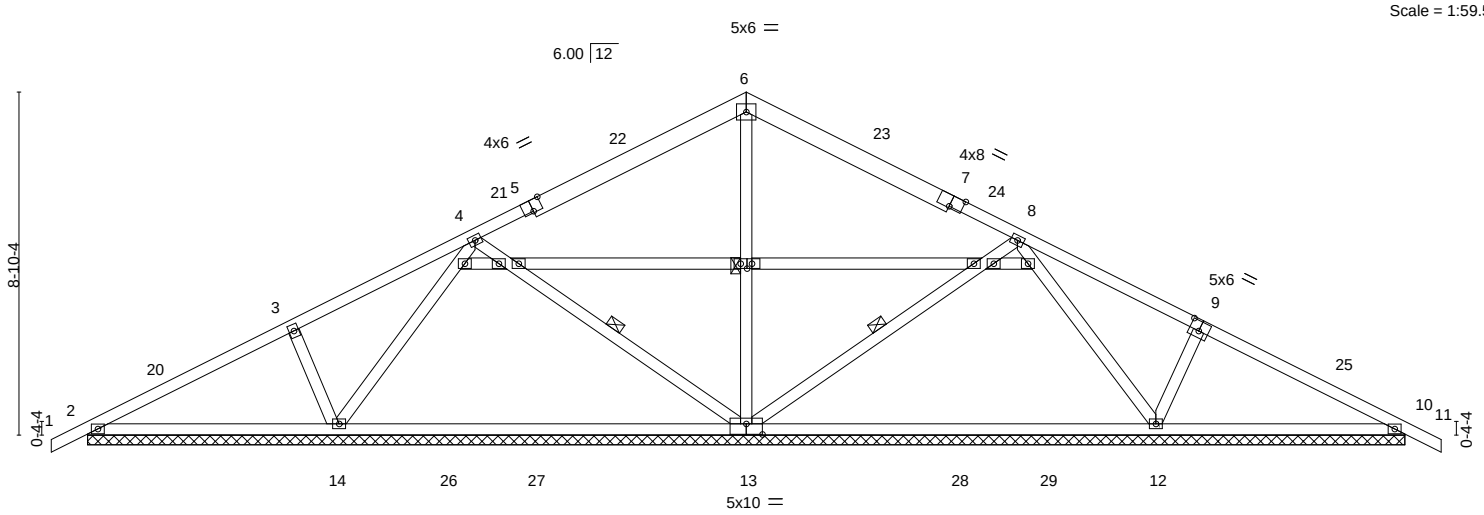

8-9-3 | 12-5-14 | 17-0-0 | 27-6-14 | 34-0-0
8-9-3 | 3-8-12 | 4-6-2 | 10-6-14 | 6-5-2

Plate Offsets (X,Y)-- [5:0-3-0,Edge], [7:0-4-0,Edge], [9:0-3-0,0-3-0], [13:0-5-0,0-3-4], [15:0-1-8,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.78	Vert(LL)	-0.01	11	n/r	120	MT20	197/144
(Roof Snow=30.0)		Lumber DOL	1.15	BC	0.87	Vert(CT)	0.00	11	n/r	120		
TCDL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(CT)	0.01	10	n/a	n/a		
BCLL	0.0 *	Code IBC2021/TPI2014		Matrix-S							Weight: 205 lb	FT = 20%
BCDL	10.0											

LUMBER-
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 5-6,6-7: 2x6 SP No.2
BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2
WEBS 2x4 SP No.3 or 2x4 SPF Stud

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-3 oc bracing.
WEBS 1 Row at midpt 6-13, 4-13, 8-13

REACTIONS. All bearings 34-0-0.
(lb) - Max Horz 2=152(LC 16)
Max Uplift All uplift 100 lb or less at joint(s) 2, 13, 10 except 14=-160(LC 12), 12=-153(LC 13)
Max Grav All reactions 250 lb or less at joint(s) except 2=353(LC 19), 13=1049(LC 1), 14=982(LC 19), 10=355(LC 20), 12=986(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
BOT CHORD 13-14=-49/290, 12-13=0/293
WEBS 6-13=-555/88, 4-13=-311/157, 4-14=-560/127, 3-14=-377/175, 8-13=-308/161, 8-12=-570/121, 9-12=-375/173

NOTES- (10-13)
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 14-0-0, Exterior(2R) 14-0-0 to 20-0-0, Interior(1) 20-0-0 to 31-11-4, Exterior(2E) 31-11-4 to 34-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
5) All plates are 3x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13, 10 except (jt=lb) 14=160, 12=153.
10) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
11) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
12) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
13) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

June 16,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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