

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0425-2358 Weaver/Graves Residence/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I75037578 thru I75037607

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



July 21,2025

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	⊢	6-0-12	13-10-12		25-10-12			3:	3-8-12 '-10-0	39-9-8	
Plate Offsets (X	(,Y)	[5:0-5-4,0-2-12], [7:0-5-4,0	)-2-12], [13:0-2	2-4,0-2-4], [16:0-2-4,0-2-4]				-			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	) ) ) * )	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TP	2-0-0 1.15 1.15 YES I2014	<b>CSI.</b> TC 0.20 BC 0.53 WB 0.66 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.15 -0.23 0.07 0.08	(loc) 13-16 13-16 10 16-17	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 357 lb	<b>GRIP</b> 244/190 FT = 25%
LUMBER- TOP CHORD         2x6 SP No.1           BOT CHORD         2x6 SP No.1           WEBS         2x4 SP No.2 *Except*           13-16: 2x6 SP No.1           REACTIONS.         (size)           2=0-3-8, 10=0-3-8           Max Horz           Max Uplift           2=72(LC 12), 10=-72(LC 13)           Max Grav           Max Grav           2=1930(LC 19), 10=1930(LC 20)						D	Structu 2-0-0 c Rigid c 1 Row 1 Brace	aral wood oc purlins eiling dire at midpt e at Jt(s):	sheathing dir (6-0-0 max.): ectly applied. 3 18	rectly applied, except : 5-7. 3-16, 5-16, 7-13, 9-13, 3	-19, 9-20
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-2940/342, 3-5=-1721/258, 5-6=-1582/423, 6-7=-1582/423, 7-9=-1712/254, 9-10=-2941/341         BOT CHORD       2-17=-230/2544, 16-17=-231/2541, 13-16=-71/2085, 12-13=-209/2407, 10-12=-208/2409											

WEBS 3-17=0/255, 3-16=-558/214, 16-19=0/806, 5-19=-191/539, 13-20=0/805, 7-20=-194/536, 9-13=-559/215, 9-12=0/255, 18-19=-1081/372, 18-20=-1092/374, 6-18=-298/118, 3-19=-1211/358, 9-20=-1222/360, 5-18=-637/1000, 7-18=-626/996

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 13-10-12, Exterior(2R) 13-10-12 to 20-0-12, Interior(1) 20-0-12 to 25-10-12, Exterior(2R) 25-10-12 to 32-1-7, Interior(1) 32-1-7 to 40-10-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 2 and 72 lb uplift at joint 10.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 24, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 34, 33, 31, 30, 29, 28, 27, 26.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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Edenton, NC 27932

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minn July 21,2025



		6-0-12		13-10-12			25-10-12				33-8-12	1	39-9-	B	
	1	6-0-12		7-10-0	1		12-0-0				7-10-0	1	6-0-1	2	
Plate Offsets (X,	,Y)	[5:0-5-4,0-2-12	], [7:0-5-4	,0-2-12], [12:0-:	2-4,0-2-4], [1	15:0-2-4,0-2-4	]								
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	*	SPACING-         2-0-0         CSI.           Plate Grip DOL         1.15         TC         0.20           Lumber DOL         1.15         BC         0.53           Rep Stress Incr         YES         WB         0.67           Code         IRC2021/TPI2014         Matrix-AS					DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.15 -0.23 0.07 0.08	(loc) 12-15 12-15 10 15-16	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight:	<b>3</b> 354 lb	<b>GRIP</b> 244/190 FT = 25%	
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 *Except* 12-15: 2x6 SP No.1 REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=235(LC 11) Max Uplift 2=-72(LC 12), 10=-57(LC 13) Max Grav 2=1931(LC 19), 10=1869(LC 20)						BRACING- TOP CHOR BOT CHOR WEBS JOINTS	D	Structu 2-0-0 o Rigid c 1 Row 1 Brace	ral wood c purlins eiling dire at midpt e at Jt(s):	sheathing dii (6-0-0 max.): ectly applied. 3 : 17	rectly applied, e : 5-7. 3-15, 5-15, 7-12,	xcept , 9-12, 3	i-18, 9-19		
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	<ul> <li>Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.</li> <li>2-3=-2942/345, 3-5=-1720/258, 5-6=-1583/426, 6-7=-1583/426, 7-9=-1716/ 9-10=-2949/343</li> <li>2-16=-270/2536, 15-16=-271/2533, 12-15=-113/2086, 11-12=-226/2414, 10 3-16=0/254, 3-15=-557/214, 15-18=0/806, 5-18=-192/538, 12-19=0/808, 7- 9-12=-567/218, 9-11=0/257, 17-18=-1084/372, 17-19=-1091/373, 6-17=-29</li> </ul>							7							

3-18=-1213/359, 9-19=-1221/360, 5-17=-637/1003, 7-17=-626/996

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 13-10-12, Exterior(2R) 13-10-12 to 20-0-12, Interior(1) 20-0-12 to 25-10-12, Exterior(2R) 25-10-12 to 32-1-7, Interior(1) 32-1-7 to 39-9-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	⊢	6-0-12	13-10-12		25-10-12				33-11-12	40-0-8	
Plate Offcets (X	V)	6-0-12 [5:0-5-4 0-2-12] [7:0	7-10-0	2-14 Edgel [12:0-2-4	12-0-0	1	-		8-1-0	6-0-12	
	, 1 )	[5.0-5-4,0-2-12], [7.0	5-5-4,0-2-12],[10.0-	z-14,Lugej, [12.0-2-4,	0-2-4], [13.0-2-4,0-2-2	'I					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	*	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir Code IRC202	2-0-0 DL 1.15 1.15 https://two.orgical	<b>CSI.</b> TC 0.21 BC 0.54 WB 0.71 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.15 -0.23 0.08 0.08	(loc) 12-15 12-15 10 11-12	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 355 lb	<b>GRIP</b> 244/190 FT = 25%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP 2x6 SP 2x4 SP 12-15:	No.1 No.1 No.2 *Except* 2x6 SP No.1			BRACING- TOP CHOR BOT CHOR WEBS JOINTS	D D	Structu 2-0-0 c Rigid c 1 Row 1 Brac	ural wood oc purlins ceiling dire at midpt re at Jt(s):	sheathing di (6-0-0 max.) ectly applied.	irectly applied, except ): 5-7. - 3-15, 5-15, 7-12, 9-12, 3	3-18, 9-19
REACTIONS.											
FORCES. (lb) TOP CHORD	- Max. 2-3=- 9-10=	Comp./Max. Ten A 2956/347, 3-5=-1709 =-3059/353	All forces 250 (lb) or 5/265, 5-6=-1593/42	less except when sho 27, 6-7=-1593/427, 7-9	own. 9=-1767/259,	_					

 
 BOT CHORD
 2-16=-267/2548, 15-16=-268/2545, 12-15=-111/2106, 11-12=-237/2523, 10-11=-236/2527

 WEBS
 3-16=0/253, 3-15=-550/215, 15-18=0/806, 5-18=-207/524, 12-19=0/831, 7-19=-170/567, 9-12=-662/224, 9-11=0/287, 17-18=-1123/374, 17-19=-1070/374, 6-17=-299/119, 3-18=-1253/360, 9-19=-1204/361, 5-17=-634/1023, 7-17=-634/978

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 13-10-12, Exterior(2R) 13-10-12 to 20-0-12, Interior(1) 20-0-12 to 25-10-12, Exterior(2R) 25-10-12 to 32-1-7, Interior(1) 32-1-7 to 39-11-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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A MiTek Affili 818 Soundside Road



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RENC



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a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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Job		Iruss	Iruss Type	Q	ty	РІУ	Weaver/Graves Residence/Harnett	
					•	-		175027504
								1/503/564
10/25-2358		B2	POOF TRUSS	2		-		
10425-2550		02		2		2		
						<b>_</b>	Job Reference (optional)	
Comtech Inc	Favette	ille NC - 28314			8	630 s Sa	n 26 2024 MiTek Industries, Inc., Mon. Jul 21 07:27:05 2025	5 Page 2
Contractin, inc,	1 ayonev	1110, 140 - 20014,			0.	000 3 00		1 age 2
				ID:CGkTRi2h	h5a8R?	MIFumG	Wnz.IVvM-RfC?PsB70Ha3NSaPanL8w3uITXbGKWrCDoi7	.147.1C?f
					n5q8K /1	MIEUMG	WhzJVWH-RIC?PSB/UHq3NSqPqnL8w3u11XbGKWrCD01/	J4ZJC ?I

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-195, 2-4=-195, 4-5=-260, 5-6=-195, 7-8=-195, 8-9=-260, 9-11=-195, 11-12=-195, 16-17=-260(F=-195), 14-16=-130, 13-14=-260(F=-195), 5-8=-65, 6-7=-195

Drag: 9-14=-33, 4-16=-33



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L	5-11-0	11-11-0	17-11-0	23-10-0						
	5-11-0	6-0-0	6-0-0	5-11-0						
Plate Offsets (X,Y)	[5:0-5-4,0-2-8]			1						
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	<b>CSI.</b> TC 0.25 BC 0.40 WB 0.50 Matrix-MS	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.12         13-14         >999         360           Vert(CT)         -0.25         13-14         >999         240           Horz(CT)         0.05         9         n/a         n/a           Wind(LL)         0.10         11-13         >999         240	PLATES         GRIP           MT20         244/190           Weight: 125 lb         FT = 25%						
LUMBER-       BRACING-         TOP CHORD 2x4 SP No.1       TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.         BOT CHORD 2x6 SP No.1       BOT CHORD Rigid ceiling directly applied or 9-11-10 oc bracing.         WEBS 2x4 SP No.2       Structural wood sheathing directly applied or 9-11-10 oc bracing.										
REACTIONS. (Ib/size) 2=1026/0-3-8, 9=1026/0-3-8 Max Horz 2=46(LC 12) Max Uplift 2=-107(LC 8), 9=-107(LC 9)										
FORCES.         (lb) - Max           TOP CHORD         2-2           7-8           BOT CHORD         2-1           WEBS         8-1	k. Comp./Max. Ten All forces 250 (lb) o 1=-2719/695, 3-21=-2683/703, 3-4=-202 =-2030/567, 8-22=-2683/706, 9-22=-2718 4=-618/2603, 13-14=-618/2603, 12-13=-6 3=-767/205, 3-13=-768/205, 5-13=-48/58	r less except when shown. 3/559, 4-5=-2257/798, 5-7=-22 3/698, 4-6=-254/364, 6-7=-248, 518/2603, 11-12=-618/2603, 9- 1, 5-6=-514/354	252/794, 3/358 -11=-618/2603							
<ul> <li>NOTES- <ol> <li>Unbalanced roof live loads have been considered for this design.</li> <li>Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 11-11-0, Exterior(2E) 11-11-0 to 12-7-6, Interior(1) 12-7-6 to 25-0-8 zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.</li> </ol> </li> <li>Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 2 and 107 lb uplift at joint 9.</li> <li>See Standard Industry Pingwback Truss Connection Detail for Connection to hase truss as applicable, or consult gualified building</li> </ul>										

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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5-11-0	11-11-0		17-11-0	23-10-0	
Plate Offsets (X,Y) [5:0-5-4,0-2-8]	0-0-0		0-0-0	5-11-0	
LOADING (psf)         SPACING-         2-0-0           TCLL         20.0         Plate Grip DOL         1.15           TCDL         10.0         Lumber DOL         1.15           BCLL         0.0 *         Rep Stress Incr         YES           BCDL         10.0         Code IRC2021/TPI2014	CSI. TC 0.25 BC 0.40 WB 0.50 Matrix-MS	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0. Wind(LL) 0.	in (loc) l/defl l 12 13-14 >999 3 .25 13-14 >999 2 .05 9 n/a r .12 13-14 >999 2	L/d <b>PLATES</b> 660 MT20 140 n/a 140 Weight: 136 lb	<b>GRIP</b> 244/190 FT = 25%
LUMBER-           TOP CHORD         2x4 SP No.1           BOT CHORD         2x6 SP No.1           WEBS         2x4 SP No.2           OTHERS         2x4 SP No.2		BRACING- TOP CHORD BOT CHORD	Structural wood she Rigid ceiling directly	eathing directly applied or 3-7-11 y applied or 9-11-10 oc bracing.	oc purlins.
REACTIONS. (lb/size) 2=1026/0-3-8, 9=1026/0-3-8 Max Horz 2=77(LC 12) Max Uplift 2=-265(LC 8), 9=-265(LC 9)					
<ul> <li>FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) c</li> <li>TOP CHORD 2-35=-2719/695, 3-35=-2683/703, 3-4=-202 7-8=-2030/567, 8-36=-2683/706, 9-36=-2713</li> <li>BOT CHORD 2-14=-618/2603, 13-14=-618/2603, 12-13=-</li> <li>WEBS 8-13=-767/269, 3-13=-768/267, 5-13=-48/56</li> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been considered for this d</li> <li>2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103 MWFRS (envelope) gable end zone and C-C Exterior(2E) - Interior(1) 12-7-6 to 25-0-8 zone;C-C for members and forc</li> <li>3) Truss designed for wind loads in the plane of the truss only Gable End Details as applicable, or consult qualified buildir</li> <li>4) All plates are 2x4 MT20 unless otherwise indicated.</li> <li>5) Gable studs spaced at 2-0-0 cc.</li> <li>6) This truss has been designed for a 10.0 psf bottom chord li</li> <li>7) * This truss has been designed for a dive load of 30.0psf on will fit between the bottom chord and any other members.</li> <li>8) Provide mechanical connection (by others) of truss to beari joint 9.</li> <li>9) See Standard Industry Piggyback Truss Connection Detail designer.</li> <li>10) Graphical purlin representation does not depict the size of</li> </ul>	r less except when shown. 9/559, 4-5=-2257/798, 5-7= 3/698, 4-6=-254/364, 6-7=- 518/2603, 11-12=-618/2600 1, 5-6=-514/354 esign. Imph; TCDL=6.0psf; BCDL 1-2-8 to 3-2-5, Interior(1) 3 es & MWFRS for reactions . For studs exposed to wi g designer as per ANSI/TF ve load nonconcurrent with the bottom chord in all are ing plate capable of withsta for Connection to base trus the orientation of the purli	2252/794, 248/358 3, 9-11=-618/2603 2-5 to 11-11-0, Exteri shown; Lumber DOL= nd (normal to the face) Pl 1. any other live loads. as where a rectangle 3 inding 265 lb uplift at jo ss as applicable, or con n along the top and/or	I; Exp C; Enclosed; or(2E) 11-11-0 to 12-7-( =1.60 plate grip DOL=1. ), see Standard Industry 3-6-0 tall by 2-0-0 wide bint 2 and 265 lb uplift a nsult qualified building bottom chord.	6, 60 t t t SEA 0363	L 22



G un unin Scale = 1:43.1

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

July 21,2025



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 2-3=-1153/858, 3-4=-1151/869, 2-9=-568/574, 4-7=-504/489 TOP CHORD
- BOT CHORD 8-9=-327/282.7-8=-304/344

WEBS 3-8=-132/431, 2-8=-493/773, 4-8=-474/711

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-2-8 to 3-2-5, Exterior(2N) 3-2-5 to 6-4-0, Corner(3R) 6-4-0 to 10-8-13, Exterior(2N) 10-8-13 to 12-7-8 zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

- 7) Bearing at joint(s) 9, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=188.7=125
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 21,2025

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-10=-286/155, 4-6=-286/155

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-2-6 to 3-2-7, Exterior(2N) 3-2-7 to 4-6-0, Corner(3R) 4-6-0 to 8-9-4, Exterior(2N) 8-9-4 to 10-2-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 6, 8.

![](_page_13_Picture_12.jpeg)

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![](_page_13_Picture_14.jpeg)

![](_page_14_Figure_0.jpeg)

- BOT CHORD 1-6=-92/1688. 5-6=-92/1688
- WEBS 3-6=-202/4363

# NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-5-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=170, 5=166.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1853 lb down and 78 lb up at 1-5-4, 1853 lb down and 78 lb up at 3-5-4, and 1853 lb down and 78 lb up at 3-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 7-11=-20

# Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_14_Picture_17.jpeg)

![](_page_14_Picture_18.jpeg)

lah		Truco	Truce Ture	014	Div	Waayar/Crayaa Baaidanaa/Harnatt	
JOD		Truss	Truss Type	QUY	FIY	weaver/Graves Residence/Harriell	
							175037590
10425-2358		E2CDP		1	-		
30423-2330		LZODI	COMMON GINDER	1	2		
					-	Job Reference (optional)	
Comtech, Inc,	Fayette	ville, NC - 28314,			8.630 s Sei	26 2024 MiTek Industries, Inc. Mon Jul 21 07:27:08 202	25 Page 2
							7 14-1004
				ID.CGKIRJ/IIDQ0R	riviteumo	/viizJvyivi-Ric?PSD/URq3N3gPqiiLow3uIIADGRWICD0	734236 ?1

# LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 15=-1578(B) 16=-1578(B) 17=-1578(B) 18=-1578(B)

![](_page_15_Picture_3.jpeg)

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![](_page_15_Picture_5.jpeg)

![](_page_16_Figure_0.jpeg)

3x6 =

						8-0-0						
		' '		1		8-0-0						•
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	0.21	4-7	>455	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.48	Vert(CT)	-0.11	4-7	>853	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.03	2	n/a	n/a		
BCDL	10.0	Code IRC2021/TF	PI2014	Matri	x-AS						Weight: 37 lb	FT = 25%
I UMBER-						BRACING						

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1

REACTIONS. 2=0-3-8, 4=0-1-8 (size)

Max Horz 2=78(LC 8) Max Uplift 2=-157(LC 8), 4=-127(LC 8) Max Grav 2=389(LC 1), 4=305(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 7-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=157. 4=127.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

![](_page_16_Picture_16.jpeg)

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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![](_page_16_Picture_18.jpeg)

![](_page_17_Figure_0.jpeg)

		1				8-0-0						1
		Γ				8-0-0						
Plate Of	fsets (X,Y)	[2:0-0-12,0-0-15]										
LOADIN	IG (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	-0.05	4-11	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.11	4-11	>853	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.02	2	n/a	n/a		
BCDL	10.0	Code IRC2021/TPI2	2014	Matri	x-AS	Wind(LL)	0.09	4-11	>992	240	Weight: 40 lb	FT = 25%

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x6 SP No.1

 OTHERS
 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=110(LC 8) Max Uplift 2=-139(LC 8), 4=-101(LC 12) Max Grav 2=389(LC 1), 4=305(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-4=-188/292

#### NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-2-8 to 3-2-5, Exterior(2N) 3-2-5 to 7-9-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=139, 4=101.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

![](_page_17_Picture_16.jpeg)

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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![](_page_17_Picture_18.jpeg)

![](_page_18_Figure_0.jpeg)

	G (psf)	SPACING- 2-0-0		DEFL. in (loc) //defl L/d PLATES GRIP	
	20.0	Plate Grip DOL 1.15	BC 0.23	Vert(LL) 0.04 4-7 >999 240 M120 244/190	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.02 $+7$ >333 240 Horz(CT) -0.01 2 n/a n/a	
BCDL	10.0	Code IRC2021/TPI2014	Matrix-AS	Weight: 26 lb FT = 25%	

# LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=57(LC 8) Max Uplift 2=-122(LC 8), 4=-84(LC 8) Max Grav 2=292(LC 1), 4=203(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-3-4 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=122.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

![](_page_18_Picture_16.jpeg)

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![](_page_18_Picture_18.jpeg)

![](_page_19_Figure_0.jpeg)

		L	5-0-0						11-0-0							
		1	5-0-0							6-0-0		I				
Plate Offse	ets (X,Y)	[2:0-3-5,0-0-2]														
		-														
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP				
TCLL	20.0	Plate Grip DOL	1.15	тс	0.22	Vert(LL)	-0.02	6	>999	360	MT20	244/190				
TCDL	10.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	-0.04	6	>999	240						
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.49	Horz(CT)	-0.01	5	n/a	n/a						
BCDL	10.0	Code IRC2021/TF	912014	Matrix	(-AS	Wind(LL)	0.06	5-6	>999	240	Weight: 59 lb	FT = 25%				

# LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except*
	4-5: 2x6 SP No.1

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (size) 2=0-3-8, 5=0-1-8 Max Horz 2=102(LC 8) Max Uplift 2=-200(LC 8), 5=-176(LC 8) Max Grav 2=507(LC 1), 5=427(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-941/1263

BOT CHORD 2-6=-1341/891, 5-6=-1341/891

WEBS 3-6=-340/205, 3-5=-840/1270

# NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 10-9-4 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=200, 5=176.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

![](_page_19_Picture_19.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_20_Figure_0.jpeg)

ł	3-0-0	5-0-0			11-0-0		
	3-0-0	2-0-0	•		6-0-0		
Plate Offsets (X,Y)	[2:0-3-5,0-0-2]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	<b>CSI.</b> TC 0.25 BC 0.15 WB 0.49 Matrix-AS	DEFL.         in           Vert(LL)         -0.02           Vert(CT)         -0.04           Horz(CT)         0.01           Wind(LL)         0.03	(loc) l/de 6 >99 6 >99 5 n/ 6 >99	fl L/d 9 360 9 240 /a n/a 9 240	<b>PLATES</b> MT20 Weight: 64 lb	<b>GRIP</b> 244/190 FT = 25%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF 4-5: 2x OTHERS 2x4 SF REACTIONS. (siz)	P No.1 P No.1 P No.2 *Except* 6 SP No.1 P No.2 e) 2=0-3-8, 5=0-1-8		BRACING- TOP CHORD BOT CHORD	Structural we Rigid ceiling	ood sheathing dir directly applied.	ectly applied, except	end verticals.
Max H Max U Max G	lorz 2=145(LC 8) lplift 2=-170(LC 8), 5=-138(LC 12) irav 2=507(LC 1), 5=427(LC 1)						
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-6=-           WEBS         3-5=-	Comp./Max. Ten All forces 250 (lb) or -941/613 -766/891, 5-6=-766/891 -840/715	less except when shown.					
NOTES- 1) Wind: ASCE 7-16; M MWFRS (envelope) forces & MWFRS fo 2) Truss designed for N Gable End Detais a 3) Gable studs spaced 4) This truss has been 5) * This truss has been 6) Bearing at joint(s) 5 capacity of bearing at 7) Provide mechanical 8) Provide mechanical 2=170, 5=138. 9) This truss design re- sheetrock be applied	/ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Corner(3E) -1- r reactions shown; Lumber DOL=1.60 pla wind loads in the plane of the truss only. Is applicable, or consult qualified building at 2-0-0 oc. designed for a 10.0 psf bottom chord live n designed for a live load of 30.0psf on t bottom chord and any other members. considers parallel to grain value using A surface. connection (by others) of truss to bearin connection (by others) of truss to bearin quires that a minimum of 7/16" structural d directly to the bottom chord.	nph; TCDL=6.0psf; BCDL: 2-8 to 3-2-5, Exterior(2N) : ate grip DOL=1.60 For studs exposed to win g designer as per ANSI/TP e load nonconcurrent with he bottom chord in all area NSI/TPI 1 angle to grain for g plate at joint(s) 5. g plate capable of withstar wood sheathing be applie	=6.0psf; h=15ft; Cat. II; I 3-2-5 to 10-9-4 zone;C-0 d (normal to the face), s I 1. any other live loads. as where a rectangle 3-6 ormula. Building design nding 100 lb uplift at join ed directly to the top cho	Exp C; Enclos C for member ee Standard I 5-0 tall by 2-0- er should veri it(s) except (jt rd and 1/2" g)	eed; s and ndustry 0 wide fy =Ib) ypsum	SE 036	AROLUNI AL 322 NEFERENTING

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

818 Soundside Road Edenton, NC 27932

A. GILD

![](_page_21_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com) **TRENCO** 

![](_page_22_Figure_0.jpeg)

![](_page_22_Figure_1.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_23_Figure_0.jpeg)

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 2=2-10-2, 4=2-10-2 Max Horz 2=-43(LC 10) Max Uplift 2=-8(LC 12), 4=-8(LC 13) Max Grav 2=135(LC 1), 4=135(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_23_Picture_13.jpeg)

Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_23_Picture_15.jpeg)

![](_page_24_Figure_0.jpeg)

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.06 BC 0.02 WB 0.01 Matrix-P	DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         0.00         4         n/r         120         MT20         244/190           Vert(CT)         0.00         4         n/r         120         Weight: 14 lb         FT = 25%	
LUMBER- TOP CHORD 2x4 SP	No.1		BRACING- TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.	

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. (size) 2=2-10-2, 4=2-10-2, 6=2-10-2

Max Horz 2=-53(LC 10) Max Uplift 2=-31(LC 12), 4=-37(LC 13)

Max Grav 2=93(LC 1), 4=93(LC 1), 6=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_24_Picture_16.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design and the second design much reacting of design and the second design much reacting and and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

![](_page_24_Picture_18.jpeg)

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

![](_page_25_Figure_0.jpeg)

![](_page_25_Figure_1.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_26_Figure_0.jpeg)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7, 8, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-283/239, 4-6=-283/239

### NOTES-

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=217, 6=217.

![](_page_26_Picture_10.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

![](_page_27_Figure_0.jpeg)

LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	<b>SPACING-</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.08 0.03 0.01	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2021/TF	912014	Matri	x-P						Weight: 19 lb	FT = 25%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. 1=4-2-10, 3=4-2-10, 4=4-2-10 (size) Max Horz 1=-64(LC 8) Max Uplift 1=-22(LC 13), 3=-18(LC 12) Max Grav 1=97(LC 20), 3=96(LC 1), 4=111(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_27_Picture_15.jpeg)

Structural wood sheathing directly applied or 4-3-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

![](_page_28_Figure_0.jpeg)

Max Uplift 1=-4(LC 13), 3=-4(LC 12)

Max Grav 1=67(LC 1), 3=67(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_28_Picture_12.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_29_Figure_0.jpeg)

REACTIONS. (size) 1=8-8-10, 3=8-8-10, 4=8-8-10 Max Horz 1=-142(LC 8) Max Uplift 1=-48(LC 13), 3=-40(LC 12)

Max Grav 1=215(LC 20), 3=214(LC 1), 4=246(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_29_Picture_13.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

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![](_page_30_Figure_0.jpeg)

Max Horz 1=-107(LC 8) Max Uplift 1=-37(LC 13), 3=-31(LC 12)

Max Grav 1=163(LC 20), 3=161(LC 1), 4=186(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_30_Picture_12.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

![](_page_30_Picture_14.jpeg)

![](_page_31_Figure_0.jpeg)

BCLL         0.0 *         Rep Stress Incr         YES         WB         0.01         Horz(CT)         0.00         3         n/a         Weight: 21 lb         FT = 25%	LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.10 BC 0.03 WB 0.01 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) a - a - ) 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES         GRIP           MT20         244/190           Weight: 21 lb         FT = 25%
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BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2

OTHERS

REACTIONS. 1=4-8-10, 3=4-8-10, 4=4-8-10 (size) Max Horz 1=72(LC 9) Max Uplift 1=-25(LC 13), 3=-21(LC 12) Max Grav 1=110(LC 20), 3=109(LC 1), 4=126(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_31_Picture_16.jpeg)

Structural wood sheathing directly applied or 4-9-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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![](_page_32_Figure_0.jpeg)

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_32_Picture_9.jpeg)

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![](_page_33_Figure_0.jpeg)