# STONEFIELD-RALE

RALEIGH - LOT 00.0067 BLAKE POND SF (MODEL# 1635)

ELEVATION 7 - GR

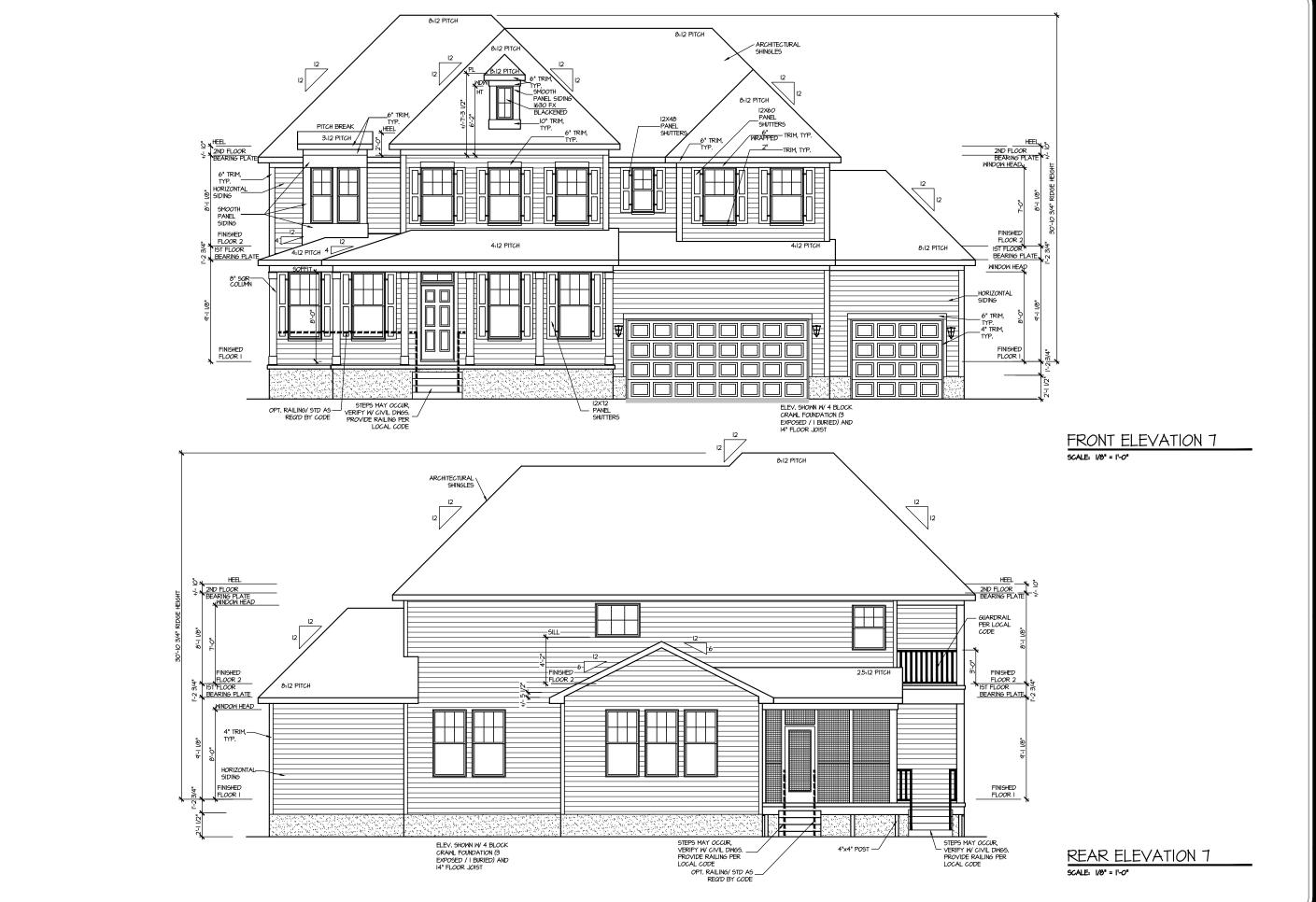


AREA CALCULATIONS ELEVATION 7		HEATED	COVERED / UNHEATED	UNCOVERED
FIRST FLOOR		1542 SF		
GARAGE		1012 31	496 SF	
FRONT PORCH - ELEVATION 7			214 SF	
THORT FORCET LEE TATION 7			211 31	
SECOND FLOOR		1622 SF		
OPTIONS				
REAR SCREENED PORCH			+144 SF	
3RD CAR GARAGE			+264 SF	
BED 5 W/ BATH 3		+55 SF	-55 SF	
BEB 6 My BAIN 6		100 01	00 01	
	TOTAL	3219 SF	1063 SF	
	-			
	-			
			1	
			+	+

# 19 Biscayne Court

LOT SPECIFIC  1 LOT 00.0067 BLAKE POND SF STONEFIELD REV. RALE—3 ELEVATION 7  2 ADDRESS 19 BISCAYNE COURT LILLINGTON, NC 27546	
1 LOT 00.0067 BLAKE POND SF STONEFIELD REV. RALE—3 ELEVATION 7	
STONEFIELD REV. RALE—3 ELEVATION 7	
2 ADDRESS 19 BISCAYNE COURT LILLINGTON, NC 27546	
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 MASTER PLAN INFORMATION
 UPDATED DATE

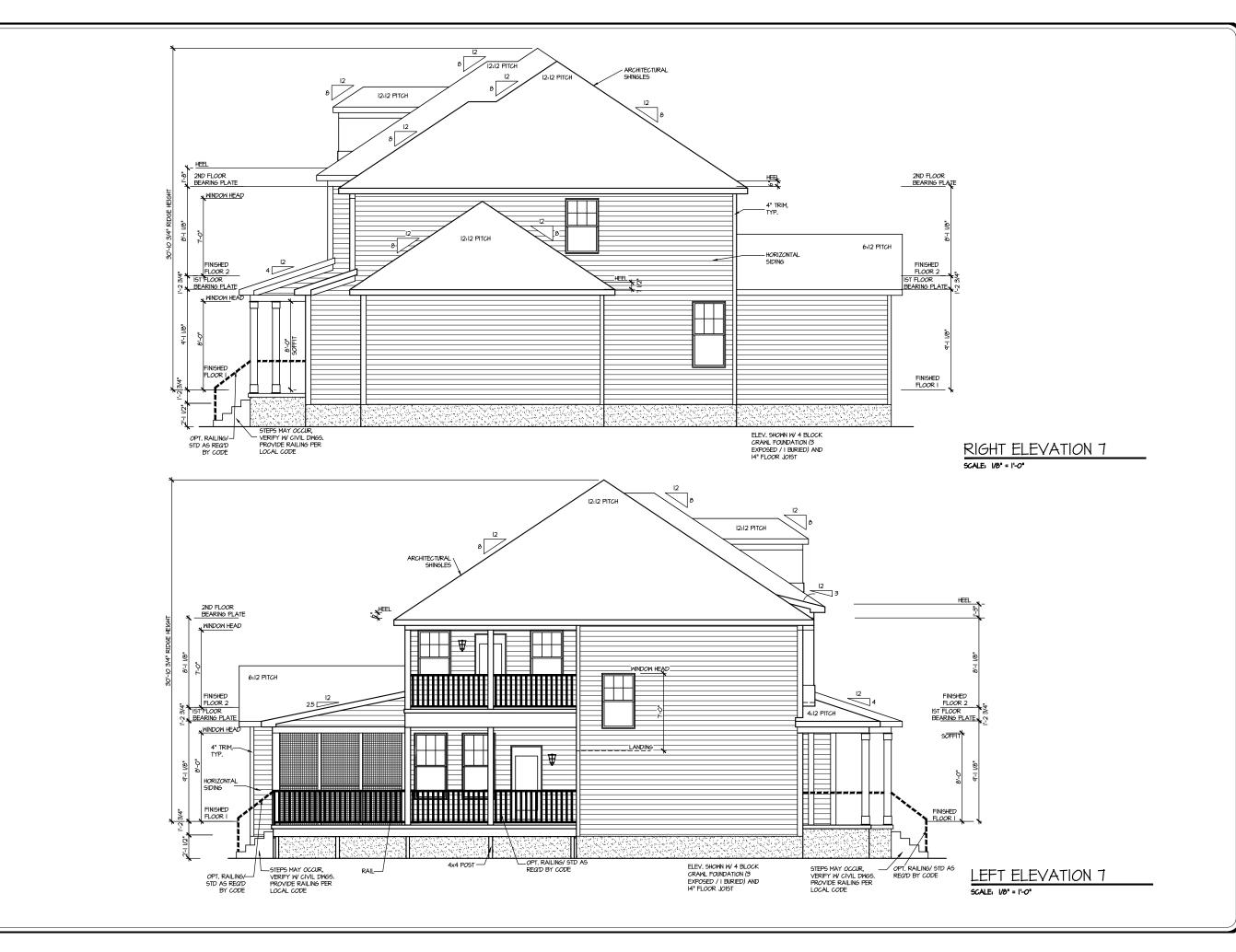
 3-RALE
 07-31-2024
 04-30-2025

DRAWN BY: ITS DATE: 05/30/2025 PLAN NO. 1635



HOUSE NAME:
STONEFIELD
DRAWING TITLE
FRONT & REAR ELEVATIONS

SHEET No.



MASTEN REVISION 3-RALE DRAWN BY:

DATE: 05/30/2025 PLAN NO. 1635



000 ᇳ HOUSE NAME: STONEFIELD DRAWING TITLE 祌 ₩ 0 ⊢±

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SHEET No.

UPPER ROOF VENTILATION CALCULATIONS: 

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOWER VENTING. (BOTTON 2/3 RDS)

20 LINEAR FEET OF SOFFIT X 5,1 50. IN. = 3,245 50. FT.

IPPER VENTING. (1/0 PJ 80.)

24 LINEAR FEET OF RIDGE X IO 50. IN = 3 50. FT.

3 50. FT. AT 50/8

(1 10 300 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS:

OPPER ROUP VENTILATION

ROOF AREA 3 = 45 50, FT.

OVERALL REQUIRED VENTILATION:

1 TO 150 = 0.63 50, FT.

1 TO 300 = 0.311 50, FT.

50% IN TOP THIRD = 0.158 50, FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOWER VENTING: (BOTTOM 2/3 RDS) 23 LINEAR FEET OF SOFFIT X 5.7 SQ. IN. = 0.41 SQ. FT. 23 Linear Peet of Soffit x 5.1 5q. in. = 0.41 5q IPPER VERTINE. (10P JZ RD) 9 Linear Peet of Ridge x 16 5q. in = 15q. ft. 15q. ft. Between 5q. (1 to 300 Allowed)

UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA 5 = 144 50, FT.
OVERALL REQUIRED VENTILATION:
1 TO 150 = 0.46 50, FT.
1 07 300 = 0.46 50, FT.
50% IN TOP THIRD = 0.24 50, FT. (1 TO 300) NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOWER VENTINS, (BOTTOM 2/3 RDS)
10 LINEAR FIET OF SOFFIT X 5.1 SQ. N. = 0.39 SQ. FT.
1PPER VENTINS, (100 - 10 RD)
0 LINEAR FIET OF RIDGE X 10 SQ. N. = 1 SQ. FT.
1SQ. FT, AT SQ.
(1 TO 300 ALLOYED)

UPPER ROOF VENTILATION CALCULATIONS: 

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOMER VENTING. (BOTTOM 2/3 BDG)

II LIMEAR FIET OF SOFFIT X 5.1 50. IN. = 0.495 50. FT.

IPPER VENTING. (TOP 1/3 BD)

4 LIMEAR FIET OF RIDGE X IB 50. IN = 0.5 50. FT.

05 50. FT. AT 50%

(1 TO 300 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS;
ROOF AREA 4 - 205 20; FT,
OVERAL SEQUENCE VENTILATION;
10 50 - 10 50; FT,
10 300 - 10 50; FT,
508 IN TOP THIRD = 0.341 50. FT, (1 TO 300) NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOWER VENTING: (BOTTOM 2/3 RDS)

20 LINEAR FEET OF SOFFIT X 5.7 SQ. IN. = 0.74 SQ. FT. 20 LINEAR FEET OF SOFFIT X 5.7 SQ. IN. = 0.74 SQ. FT. IPPER YEATHNS. (100 JB 20) 5 LINEAR FEET OF RIDGE X IS SQ. IN = 1 SQ. FT. 1 SQ. FT. AT 50.8 (1 TO 300 ALLOWED) TOTAL ROOF VENTILATION. 1.74 SQ. FT. > 0.32 SQ. FT. (RQTD)

UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA 6 = 234 S.G. FT.

OVERALL REQUIRED VEHTILATION:

1 TO 150 = 156 S.G. FT.

1 TO 300 = 0.716 S.G. FT.

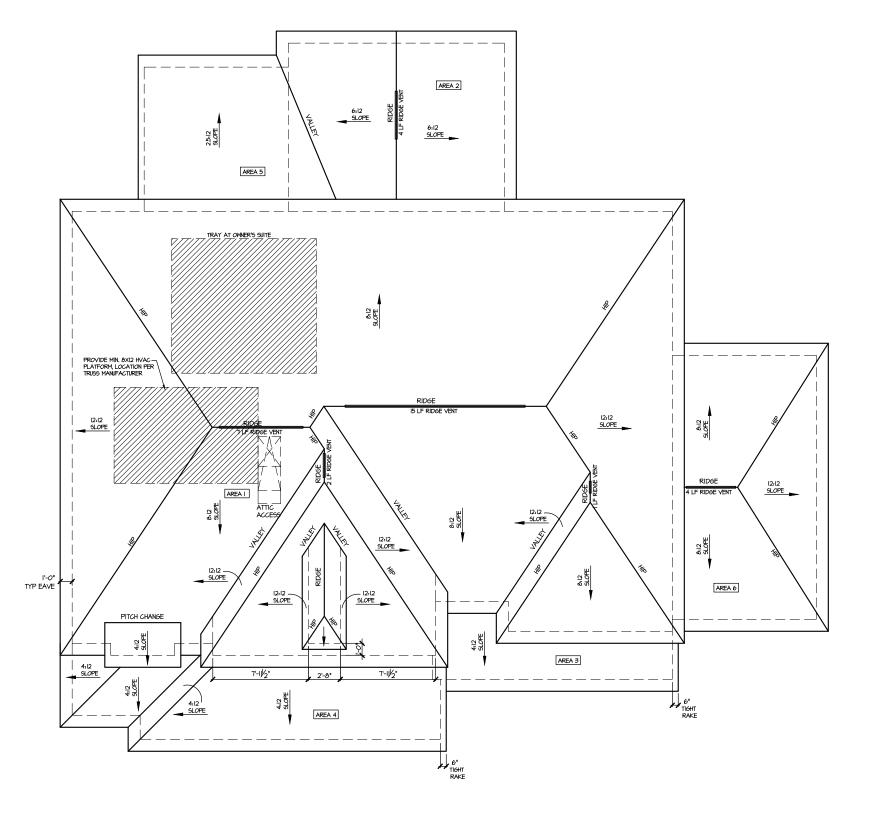
50% IN TOP THIRD = 0.34 S.G. FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOMER VENTING. (BOTTOM 2/3 RDS)
10 LINEAR FEET OF SOFFIT X 5.1 Sq. IN. = 0.346 Sq. Ft.

VETER VENTING. (TOP 1/3 RD)
4 LINEAR FEET OF RINGE X 16 Sq. IN = 0.5 Sq. Ft.
1 Sq. Ft. A1 50%
(1 To 300 ALLOWED)

NOTE: ROOF PLANS SHOWN W/ MIN. REQ'D RIDGE VENT LOCATIONS. ACTUAL RIDGE VENT LOCATIONS AND QUANTITY PER BUILDER



ROOF PLAN ELEV. 7

SCALE: 1/8" = 1'-0"

SHEET No. Al.3

HOUSE NAME: STONEFIEL DRAWING TITLE

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DRAWN BY:

PLAN NO. 1635

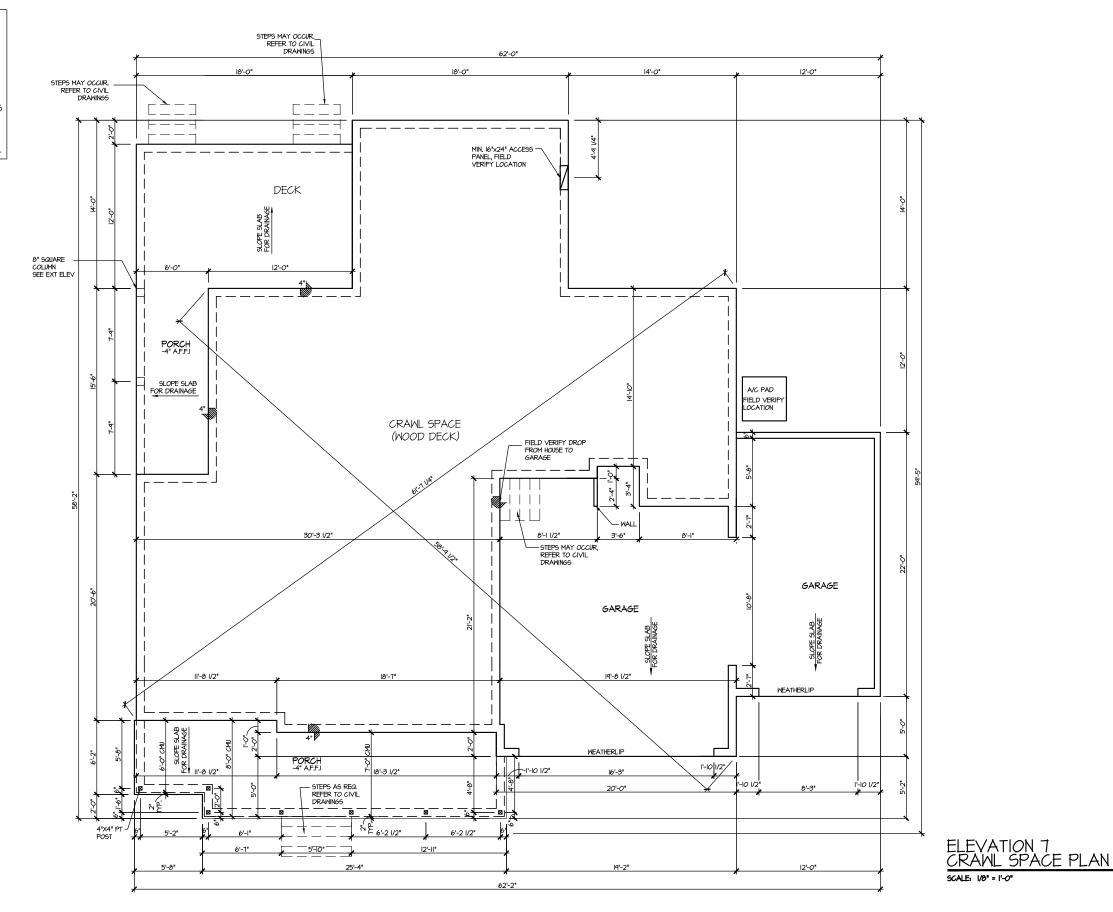
DATE: 05/30/2025

CRAWL SPACE VENT CALCULATIONS: ELEV T CRAWL AREA = 1542 SQ. FT. OVERALL REQUIRED VENTILATION: 1 SQ. IN. PER I SQ. FT. = 1542 SQ. IN.

NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

<u>VENTING REQUIREMENT:</u> 1542 SQ. IN. / 72 SQ. IN. = 21.4 VENTS = 22 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED. VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1



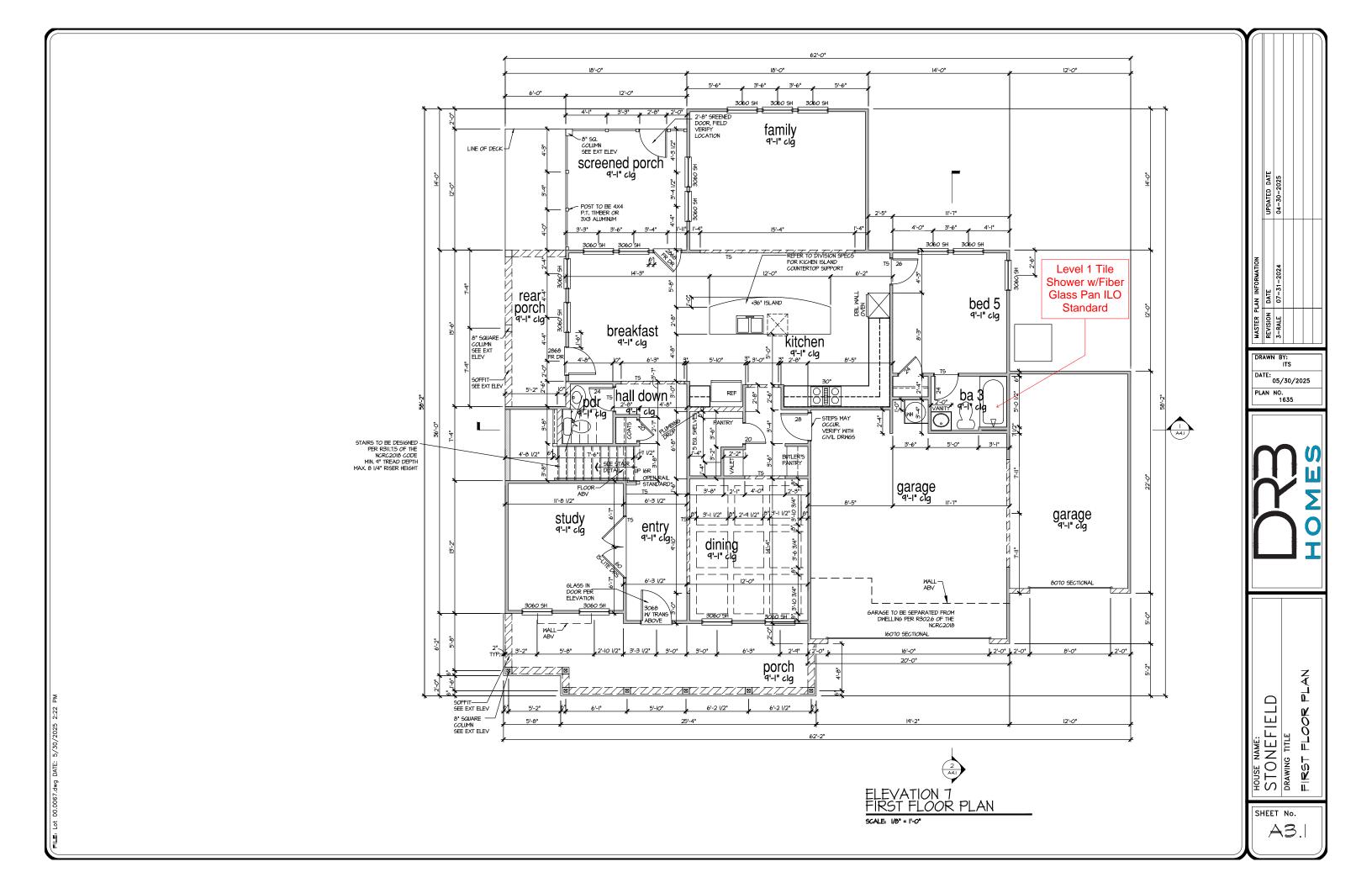
DRAWN BY: DATE: 05/30/2025 PLAN NO. 1635

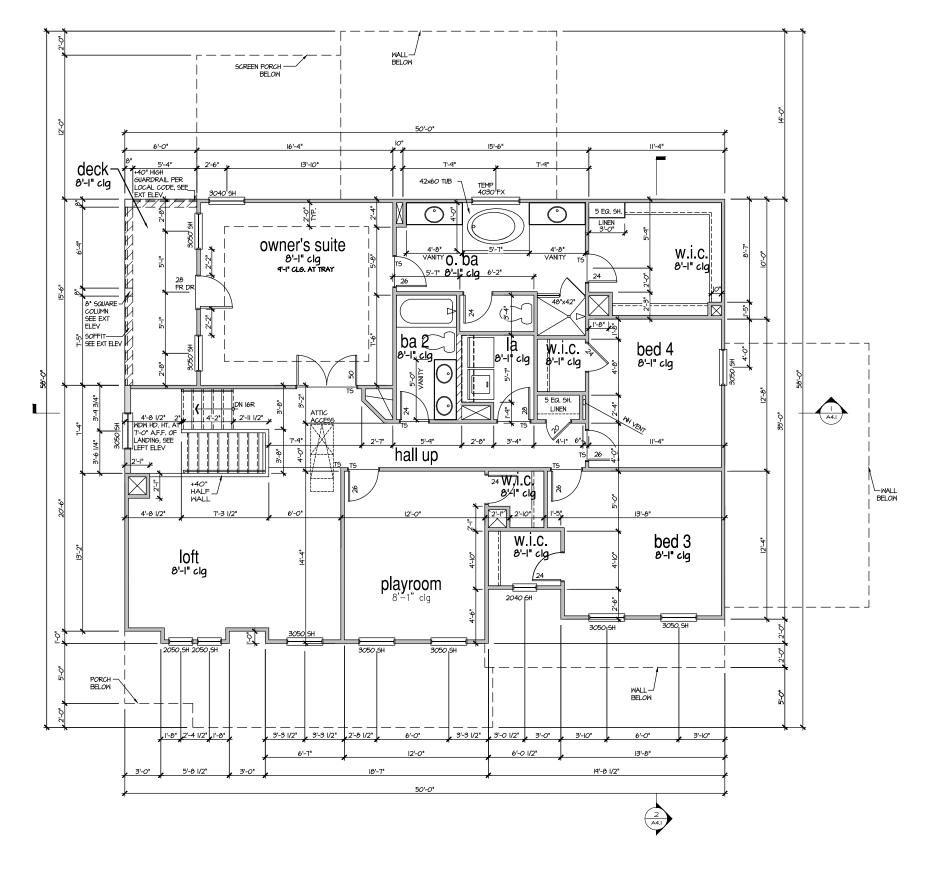
HOUSE NAME:
STONEFIELD
DRAWING TITLE SHEET No.

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SCALE: 1/8" = 1'-0"

ELEVATION 7 SECOND FLOOR PLAN

HOUSE NAME:
STONEFIELD
DRAWING TITLE
SECOND FLOOR

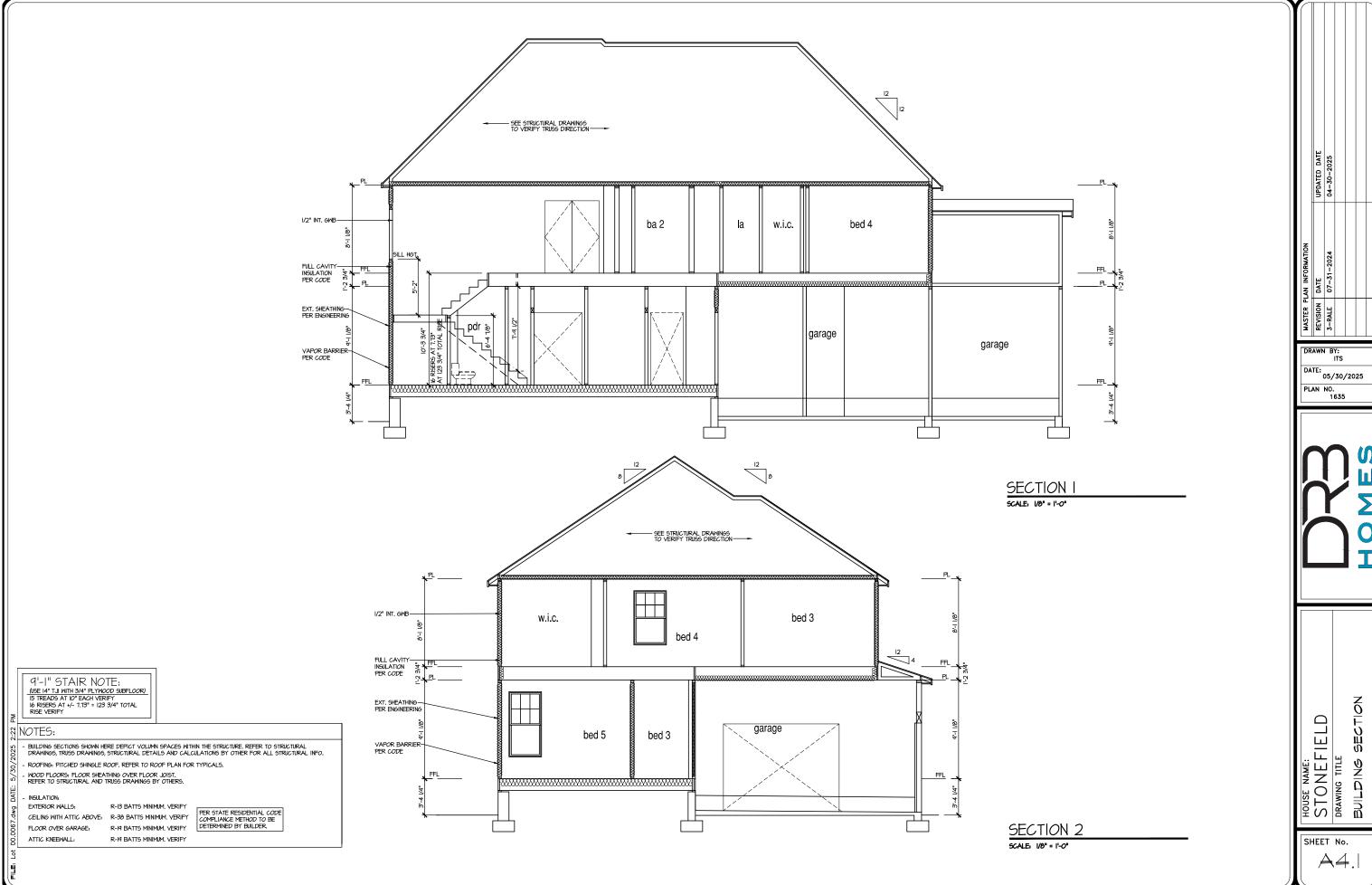
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PLAN NO. 1635

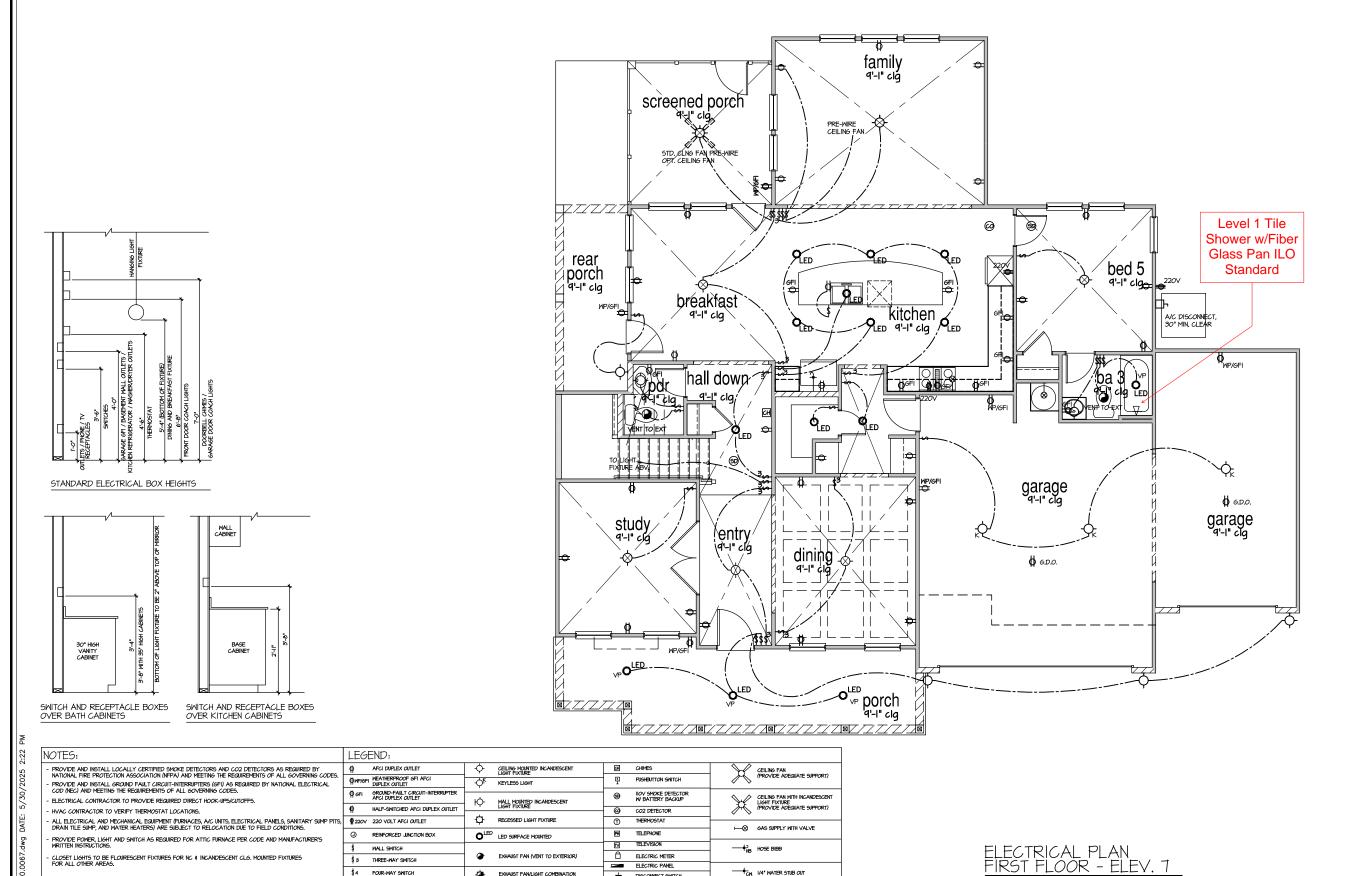
DATE: 05/30/2025

SHEET No.

A3.2







ELECTRIC PANEL

DISCONNECT SMITCH

EXHAUST FAN/LIGHT COMBINATION (VENT TO EXTERIOR)

TECH HUB SYSTEM

-+CM I/4" WATER STUB OUT

SCALE: 1/8" = 1'-0"

\$4 FOUR-WAY SWITCH

□ ≣ ⊒ LED STRIP FIXTURE

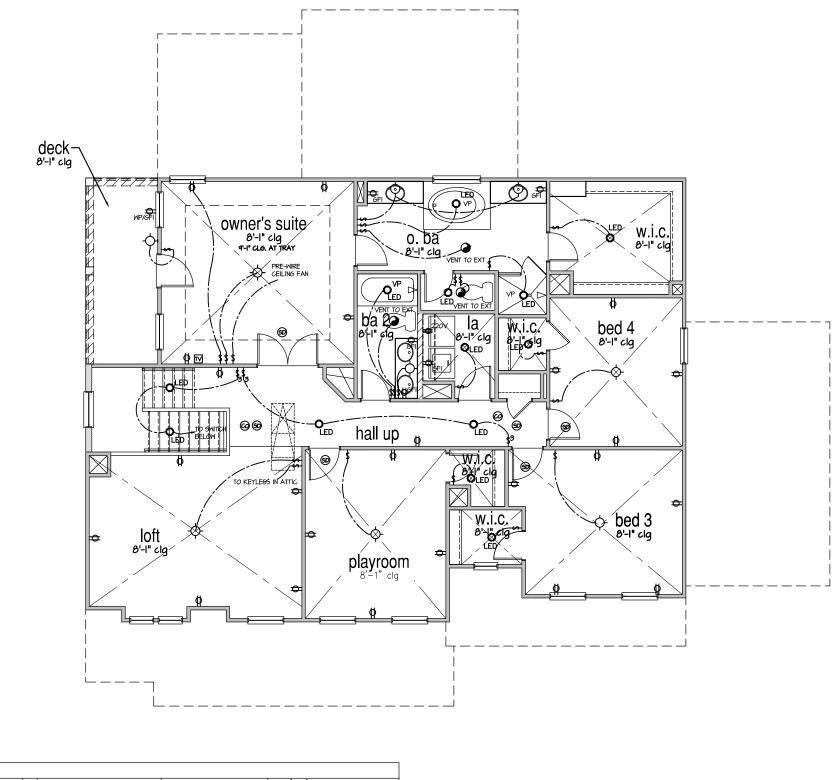
ᇳ HOUSE NAME: STONEFIEL DRAWING TITLE **ο**Κ

DRAWN BY:

PLAN NO. 1635

DATE: 05/30/2025

SHEET No.



NOTES:

- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY MATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL COVERNING CODES.
- PROVIDE AND INSTALL GROUND FAULT (GROUT-IMPERPURIES (FIF) AS REQUIRED BY NATIONAL ELECTRICAL COD (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FIRMACES, A/C UNTS, ELECTRICAL PANELS, SANITARY SIMP PITS, PARAIN TILE SIMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.

- PROVIDE POWER LIGHT AND SMITCH AS REQUIRED FOR ATTIC PURNACE PER CODE AND MANUFACTURERS
- REVISION INSTRUCTIONS.

CLOSET LIGHTS TO BE FLOURESCENT FIXTURES FOR NC & INCANDESCENT CLG. MOUNTED FIXTURES FOR ALL OTHER AREAS.

SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

WALL CABINET

STANDARD ELECTRICAL BOX HEIGHTS

SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

LEGEND: CEILING FAN (PROVIDE ADEQUATE SUPPORT) AFCI DUPLEX OUTLET CHIMES MP/GFI MEATHERPROOF GFI AFCI DUPLEX OUTLET PUSHBUTTON SWITCH -ÒK KEYLESS LIGHT GFI GROUND-FAULT CIRCUIT-INTERRUPTER
 AFCI DUPLEX OUTLET IIOV SMOKE DETECTOR W BATTERY BACKUP CEILING FAN WITH INCANDESCENT LIGHT FIXTURE (PROVIDE ADEQUATE SUPPORT) WALL MOUNTED INCANDESCENT LIGHT FIXTURE HALF-SWITCHED AFCI DUPLEX OUTLET ① 220V 220 VOLT AFCI OUTLET ----- GAS SUPPLY WITH VALVE REINFORCED JUNCTION BOX PH TELEPHONE O<sup>LED</sup> LED SURFACE MOUNTED TELEVISION WALL SMITCH → HB HOSE BIBB EXHAUST FAN (VENT TO EXTERIOR) ELECTRIC METER \$ 3 THREE-WAY SMITCH ELECTRIC PANEL \$4 FOUR-WAY SWITCH CM 1/4" WATER STUB OUT EXHAUST FANLIGHT COMBINATION (VENT TO EXTERIOR) DISCONNECT SWITCH □ ≣ ⊒ LED STRIP FIXTURE TECH HUB SYSTEM

ELECTRICAL PLAN SECOND FLOOR - ELEV. 7 SCALE: 1/8" = 1'-0"

HOUSE NAME:
STONEFIEL

DRAWING TITLE

SHEET No.

DRAWN BY:

DATE: 05/30/2025

PLAN NO. 1635

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DESCRIPTION OF BLDG, ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS 🛭 4" O.C.	(3) NAILS • 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS @ 8" o.c.	TOENAILS @ 6" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS @ 24" o.c.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" o.c.	NAILS @ 16" O.C.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP @ CORNERS & INTERSECTING WALLS	(2) NAILS	(2) NAILS
INTEROLOTINO TO CLES		

\* 2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE \* ARE SHOWN)

#### MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSIRE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENT IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO: FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES

## ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS OTED OTHERWISE ON PLAN

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN AD IACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS:
  - LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

#### GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

#### DESIGN LOADS

ROOF DEAD = 7 PSF T.C., IO PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

#### GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS, ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION. ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(I) FOR ALL CONNECTIONS, TYP, U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
   I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. • WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPF) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY, CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) • HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (1)2x4/6 FLAT @ OPENINGS UP TO 4'. (2)2x4/6 FLAT UP TO 8'
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15). ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- 'LSL' Fb=2325 psi: Fv=3I0 psi: E=L55xI0^6 psi
  - 'LVL' Fb=2600 psi; Fv=285 psi; E=2.0xI0^6 psi
- 'PSL' FB=2900 PSI; FV=290 PSI; E=2.0XI0^6 PSI M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR
- INSTALLATION. FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x3/5" SIMPSON SDS SCREMS (OR 3½" TRUSSLOK SCREMS) @ 16" O/C. USE A MINIMUM OF 3 ROMS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF  $\frac{1}{4}$ "x6" SIMPSON SDS SCREWS (OR 6  $\frac{3}{4}$ " TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE, A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND,/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

#### FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT M&K FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131" NAILS @ 6"o.c. @ PANEL EDGES & @ 12"o.c. FIELD.
- 2 3 × 0.120" NAILS 4" O.C. PANEL EDGES € 8" O.C. FIELD. - 2 3" x 0,113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN. SCREWS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

#### ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) @ ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H2 5T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO
  - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS W/ (3) 3"x0.131" NAILS @ 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12" O.C. FIELD. - W/ 2 (8 × 0.120" NAILS @ 4"0.c. @ PANEL EDGES \$ @ 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"O.C. @ PANEL EDGES & @ 6" O.C. FIELD.

## VENEER LINTEL SCHEDULE

1-1				
SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size		
3'-0"	20 FT. MAX	L3"x3"x/4"		
6'-0"	3 FT. MAX	L3"x3"x/4"		
	I2 FT. MAX	L4"x3"x/4"		
	20 FT. MAX	L5"x31½"x7%"		
&'-O"	3 FT. MAX	L4"x4"x4" *		
	I2 FT. MAX	L5"x31½"x7%"		
	I6 FT. MAX	L6"x31/2"x3%"		
q-6"	I2 FT. MAX	L6"x3½"x%;"		
16'-0"	2 FT. MAX	L7"x4"x/2" **		
	3 FT. MAX	L8"x4"x½" **		

## SHALL SUPPORT 2 %" - 3 ½" VENEER w/ 40 pef MAXIMUM WEIGHT.

- 16' SHALL HAVE 8" MIN. BEARING
- 16' SHALL NOT BE FASTENED BACK TO HEADER
- 16' SHALL BE EASTENED BACK TO WOOD HEADER IN WALL 448" W/½" DIA. x 3 ½" LONG LAG SCREMG IN 2" LONG VERTICALLY SLOTTED HOLES.
- MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
  WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE  $3\,\text{\ensuremath{\mathbb{Z}}}^*$  WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT
- FINISHING. SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL FASTENED BACK TO BEAM, FASTENERS SHALL MAINTAIN A 2/2" (MINIMUM) CLEAR DISTANCE FROM BOTTOM OF BEAM. FOR QUEEN VENEER USE L4x3x1/a
- FOR 3/5" VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM:

20 MPH WIND IN 2018 NCSBC:RO (120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301.2.1.1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NCSBC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY

ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

#### EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES & ● 12" O.C. IN THE PANEL FIELD. TYP, U.N.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1½" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6" O.C IN FIELD.

#### BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 36" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 34" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C. IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2X HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

#### 3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING w/8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- PRE-MANUFACTURED PANELIZED WALLS FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

OR 3" O.C. OSB SHEARWALL.

INDICATES HOLDOWN BELOW

INDICATES EXTENT OF INT. OSB SHEARWAI

#### GENERAL STRUCTURAL NOTES

#### **FOUNDATION**

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE 12" MAX FROM PLATE ENDS - LITTLIZING
- I/2" DIA. ANCHOR BOLTS € 6'-0" O.C, 7" MIN. EMBEDMENT
- (CONC.) 15" MIN EMBEDMENT (CMU)
- SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS 2'-8" O.C. (CMU) (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER. CONCRETE DESIGN BASED ON ACL 318 CONCRETE SHALL ATTAIN
- THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: ...... FOUNDATION WALLS 2,500 psi: ...... FOOTINGS € INTERIOR SLABS ON GRADE 3,000 psi: ...... GARAGE & EXTERIOR SLABS ON GRADE 60,000 psi
- BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
- TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (91/5" FOR 10" THICK WALL).
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC, ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKELLING BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS
- FOR OPENINGS UP TO 36", PROVIDE MINIMUM IO" CONCRETE DEPTH OVER OPENING OR (3)2x10 W/ (2)2x6 JACK STUDS, U.N.O.
- LARGER OPENINGS SHALL BE PER PLAN. • ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS
- THAN 5% OR MORE THAN 7% AIR ENTRAINMENT. • ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB. EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY
- TO DEVELOP. JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" OC (MAXIMUM)
- JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I.I RATIO), WITH A MAXIMUM OF I.I.5 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi), MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 \$ 530.1.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x6 (MIN.) x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE FASTENED PER ANCHORAGE SPECIFICATIONS NOTED ABOVE. TOP 2 COURSES (MIN.) OF PIER TO BE GROUTED SOLID (8 COURSE MAX. PIER HEIGHT).
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE. TOP 2 COURSES (MIN.) OF WALL TO BE GROUTED SOLID (8 COURSE MAX, WALL HEIGHT)
- DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES, METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

#### HOLD-DOWN SCHEDULE

	<u> </u>
SYMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM U.N.O.) -OR- MSTC66B3 ALTERNATE
<b>▶</b> HD-3	SIMPSON STHDI4/STHDI4RJ

\* UTILIZE THE SSTB24 ANCHOR BOLT ● ALL MONOSLAB & INTERIOR RAISED 5LAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 15" (FOR 7/8" DIA.) MIN. EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D.

<u>DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE</u>

PROFESSION,

O.

CAR ENGINE SEPHT. R

6/25/25



I&K project numbe 126-2306

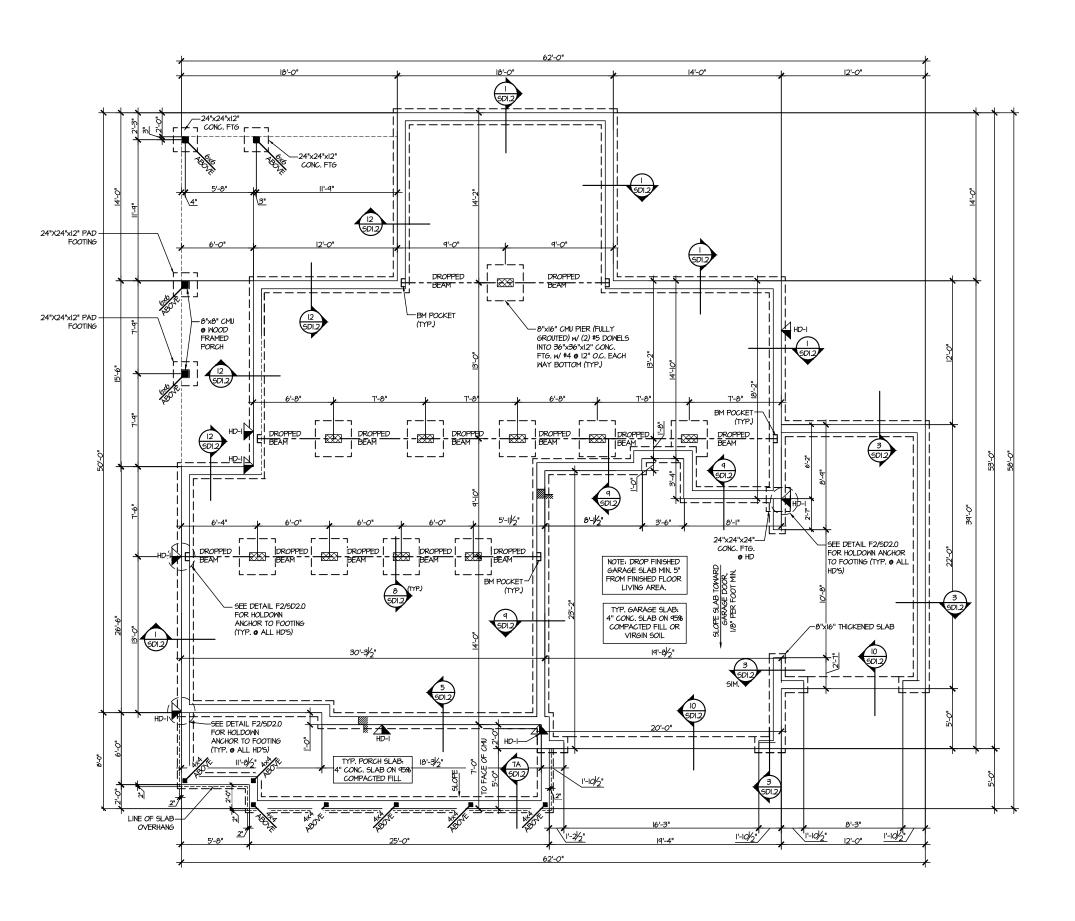
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SEAL OF PRINCIPLE OF THE PLANT REPRESENTATION OF THE PLANT

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINERNI
SENDENISME AM BAIRDS 1- AMAIN, PA 1800

M&K project number:

126-23061

project mgr: JTR drawn by: XJG issue date: 06-10-25

REVISIONS:

te: initial:

**──** "

LEGEND

- IIIIII INTERIOR BEARING WALL
- ==== BEARING WALL ABOVE
- BEAM / HEADER

  INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP.
  REFER TO SCHEDULE.

REFER TO SO.O FOR
TYPICAL STRUCTURAL NOTES
\$ SCHEDULES

SD2.IJ/SD2.IT REFERS TO
SD2.IJA/SD2.ITA FOR LVL/PSL/LSL
BEAMS OR SD2.IJB/SD2.ITB FOR
FLITCH BEAMS OR
SD2.IJC/SD2.ITC FOR STEEL
BEAMS

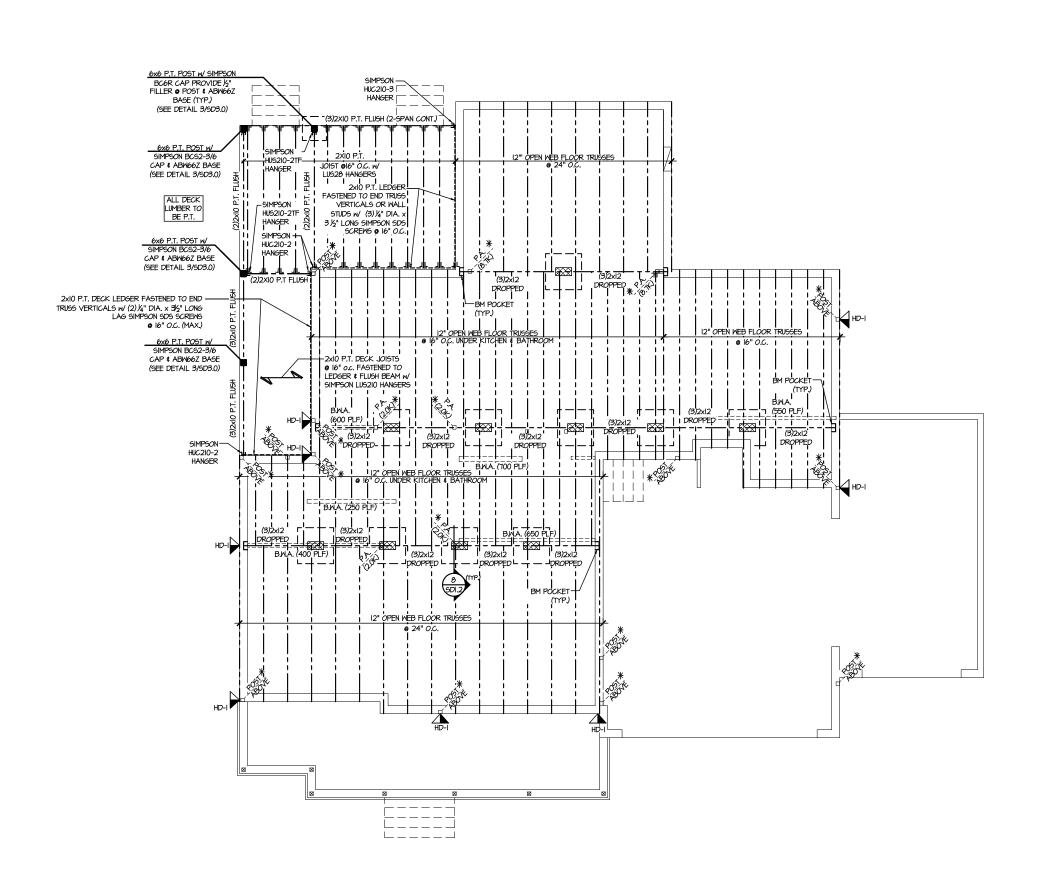
CRAWL SPACE FOUNDATION PLAN
SCALE: 1/8"=1"-0"

- DIANE I OIIU - 0/ - JUUCUUI AIS DATE, 0/23/2023 12.1/

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BLAKE POND COMMUNIT Lot 67 - Stonefield 7 raleigh, nc

OUNDATION PL



IST FLOOR FRAMING PLAN

SEPH T. R

#### LEGEND

- IIIIIII INTERIOR BEARING WAI I
- □□□□□ BEARING WALL ABOVE
- ---- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

#### ALTERNATE F.J MANUFACTURERS

• FLOOR JOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED I-JOISTS DESIGN AND CONSTRUCTION GUIDE. MINIMUM JOIST PROPERTIES INCLUDING, BUT NOT LIMITED TO, ALLOWABLE SHEAR, ALLOWABLE MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I-JOISTS. ALL ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST CONNECTIONS ARE PER THE JOIST MANUFACTURER.

### ENGINEERED BEAM MATERIAL SCHEDULE

BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)1¾"x16" - D	3½"x16" - D	N/A	(2)2xl2 + (l) %"xl以" STEEL FLITCH PLATES - D	WI2xI4 - D
002	(2)13/4"x117/6" - D	3½"xll%" - D	(3)13/4"x117/6" - D	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - D	WI2xI4 - D
003	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2x12 + (1) ¼"x1l¼" STEEL FLITCH PLATES - F	WI2xI4 - F
004	(2)13/4"×14" - F	3½"x 4" - F	(3)1¾"×14" - F	(2)2xl2 + (l) %"xll以" STEEL FLITCH PLATES - F	WI2xI4 - F
005	(3)13/4"x18" - FT	5¼"xl8" - FT	N/A	(3)2x12 + (2) %"xII¼" STEEL FLITCH PLATES - F	WI2×26 - F
006	(3)13/4"x18" - FT	5¼"xl8" - FT	N/A	(3)2xi2 + (2) %"xil4" STEEL FLITCH PLATES - F	WI2×26 - F
007	(2)134"×14" - F	3½"xl4" - F	(2)13/4"×14" - F	(2)2x12 + (1) ¼"x11¼" STEEL FLITCH PLATES - F	WI2xI4 - F
000	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xi2 + (I) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F
P00	(2)134"×94" - D	3½"×9¼" - D	(2)13/4"×91/4" - D	(2)2xi0 + (i) ¼"xq¼" Steel Flitch Plates - D	WI0xI2 - D
010	(2)194"×16" - H	3½"x16" - H	(3)19¼"×16" - H	(3)2xl2 + (2) 火"xll以" STEEL FLITCH PLATES - H	N/A
OII	(2)1¾"x11%" - D	3½"x11%" - D	(3)1¾"x11%" - D	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - D	WI2xI4 - D
012	(3)1¾"x16" - D	5¼"xl6" - D	(4)1¾"x16" - D	(3)2xi2 + (2) ½"xil½" Steel Flitch Plates - D	WI2xI9 - D
013	(2)1¾"x11%" - D	3½"x11%" - D	(3)1¾"x11%" - D	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - D	WI2xI4 - D
014	(2)1¾"x16" - D	3½"x16" - D	(3)1¾"x16" - D	(3)2xl2 + (2) %"xl以" STEEL FLITCH PLATES - D	WI2xI4 - D
015***	(3)13/4"x117/6" - D	N/A	(3)13/4"x117/6" - D	(3)2xl2 + (2) ¼"xll¼" STEEL FLITCH PLATES - D	WI2xI4 - D
016	(2)134"×1136" - F	3½"xll⅓" - F	(3)194"×11%" - F	(2)2x12 + (1) ¼"x11¼" STEEL FLITCH PLATES - F	WI2xI4 - F

- BEAM NOTATION:

   "F" INDICATES FLUSH BEAM

   "FT" INDICATES FLUSH TOP BEAM

   "FB" INDICATES FLUSH BOTTOM BEAM

PLATES IN SUCCESSION W/ (2) 3"x0.120" NAILS @ 8" O.C.

- "FB" INDICATES FLUSH BOTTOM BEAM
   "1" INDICATES DROPPED BEAM
   "H" INDICATES DROPPED OPENING HEADER
  REFER TO DETAIL D/5D2.0 FOR TYPICAL FILTCH BEAM CONNECTIONS
  REFER TO DETAIL E/5D2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
  FOR FLUSH TO'P BEAM'S PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN
  PLATES IN SUCCESSION W (2) 3"X01.20" NAILS 0 8" O.C.
  FOR FLUSH BOTTOM BEAM'S PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D, FASTEN
  PLATES IN SUCCESSION W (2) 3"X01.20" NAILS 0 8" O.C.
- \*\*\* SEE PLAN FOR EXTENT OF 3-PLY BEAM

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6/25/25

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I&K project number 126-2306

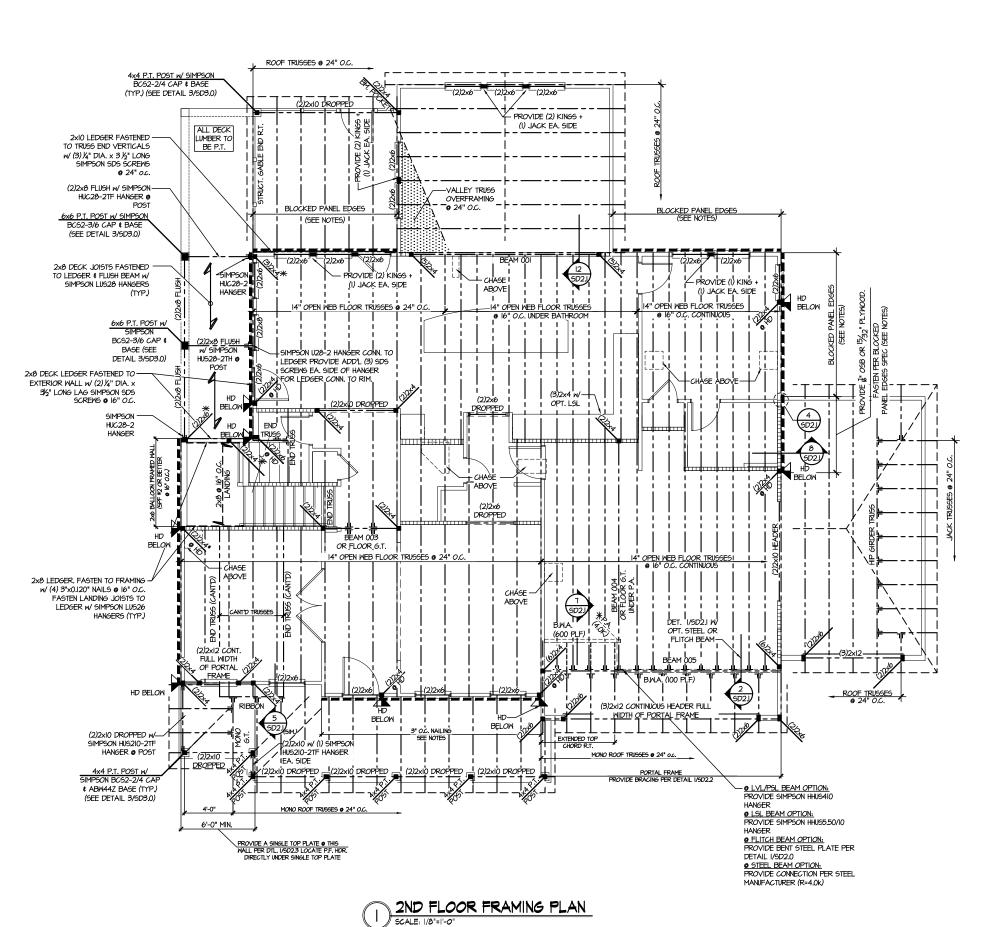
**JTR** roject mgr: rawn by: XJG sue date: 06-10-2

REVISIONS:

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OR

BLAKE POND COMMUNIT Lot 67 - Stonefield 7 Raleigh, nc



SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LEGEND

- INTERIOR BEARING WALL
- □==== BEARING WALL ABOVE
- ---- BEAM / HEADER
- == INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP.

REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

#### ENGINEERED BEAM MATERIAL SCHEDULE

BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)1 <sup>3</sup> / <sub>4</sub> "×16" - D	3½"x16" - D	N/A	(2)2xl2 + (l) %"xl以" STEEL FLITCH PLATES - D	WI2xI4 - D
002	(2)13/4"x111/6" - D	3½"xII%" - D	(3)1¾"x11%" - D	(2)2xl2 + (l) ¼"xll¼" STEEL FLITCH PLATES - D	WI2xI4 - D
003	(2)13/4"×14" - F	3½"xl4" - F	(2)194"×14" - F	(2)2xi2 + (I) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F
004	(2)13/4"×14" - F	3½"x 4" - F	(3)13/4"×14" - F	(2)2xi2 + (i) %"xik" Steel Flitch Plates - F	WI2xI4 - F
005	(3)194"x18" - FT	5¼"xl8" - FT	N/A	(3)2x12 + (2) %"xIK" STEEL FLITCH PLATES - F	WI2×26 - F
006	(3)1¾"x18" - FT	5¼"xl8" - FT	N/A	(3)2x12 + (2) %"xII¼" STEEL FLITCH PLATES - F	WI2x26 - F
001	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xi2 + (I) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F
800	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"x14" - F	(2)2xi2 + (I) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F
009	(2)194"×94" - D	3½"×9¼" - D	(2)134"×944" - D	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - D	WI0xI2 - D
010	(2)1¾"x16" - H	3½"x16" - H	(3)19/4"×16" - H	(3)2xi2 + (2) 片"xi以" STEEL FLITCH PLATES - H	N/A
OII	(2)1¾"x11%" - D	3½"xII%" - D	(3)1¾"x11%" - D	(2)2xi2 + (l) ¼"xil¼" STEEL FLITCH PLATES - D	WI2xI4 - D
012	(3)1¾"×16" - D	5¼"x16" - D	(4)15/4"×16" - D	(3)2xl2 + (2) 片"xli片" STEEL FLITCH PLATES - D	WI2xI9 - D
013	(2)1¾"x11½" - D	3½"×11%" - D	(3)1¾"x11%" - D	(2)2xi2 + (l) ¼"xil¼" STEEL FLITCH PLATES - D	WI2xI4 - D
014	(2)13/4"×16" - D	3½"x16" - D	(3)13/4"x16" - D	(3)2xl2 + (2) %"xll"," STEEL FLITCH PLATES - D	WI2xI4 - D
015***	(3)1¾"x11½" - D	N/A	(3)134"×11%" - D	(3)2xi2 + (2) ¼"xil¼" STEEL FLITCH PLATES - D	WI2xI4 - D
016	(2)1¾"x11¾" - F	3½"×II%" - F	(3)1¾"x11½" - F	(2)2xl2 + (l) 从"xl以" STEEL FLITCH PLATES - F	WI2xI4 - F

- BEAM NOTATION: "F" INDICATES FLUSH BEAM "FT" INDICATES FLUSH TOP BEAM "FB" INDICATES FLUSH BOTTOM BEAM

- TD' INDICATES PLUGH BOTTOM BEAM'
   "D' INDICATES PROPPED BEAM
   "H' INDICATES DROPPED DEAM
   "H' INDICATES DROPPED OPENING HEADER
  REFER TO DETAIL D/SD2.0 FOR TYPICAL FILITCH BEAM CONNECTIONS
  REFER TO DETAIL E/SD2.0 FOR TYPICAL SITEL BEAM CONNECTIONS
  FOR FLUGH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN
  PLATES IN SUCCESSION W (2) 3"XOJ.20" NAILS @ 8" O.C. FOR FLUSH BOTTOM BEAMS PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION w/ (2) 3"XO.120" NAILS • 8" O.C.
- \*\*\* SEE PLAN FOR EXTENT OF 3-PLY BEAM

6/25/25 CAR SEPH T. R

MULHERN+KUL

MESIDENTIAL STRUCTURAL ENGINEERI

I&K project number

126-2306

**JTR** rawn by: XJG sue date: 06-10-2!

FVISIONS

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BLAKE POND COMMUNIT Lot 67 - Stonefield 7 raleigh, nc OOR

MULHERN+KUL

RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number: 126-23061

**JTR** drawn by: issue date: 06-10-2

REVISIONS:

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LEGEND

• == INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

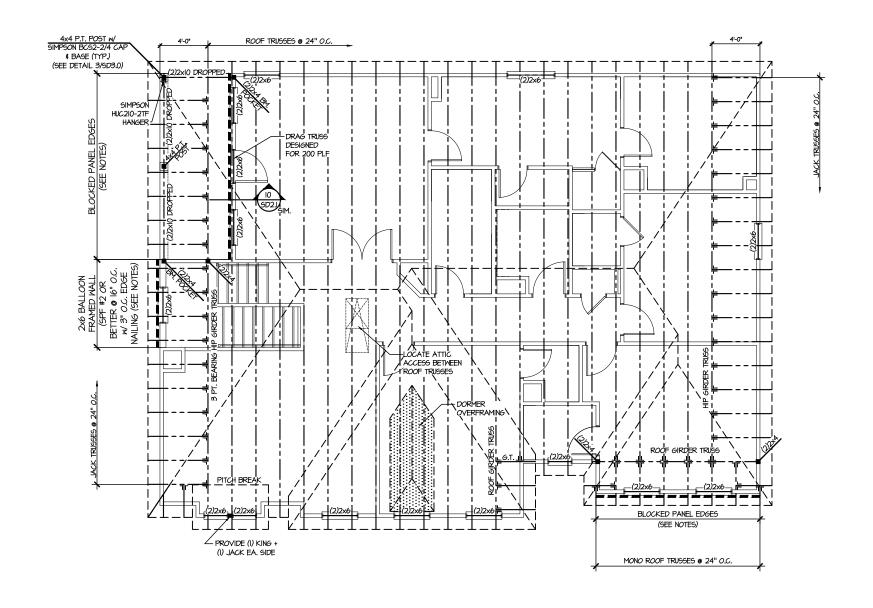
REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

 INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE BEAM / HEADER

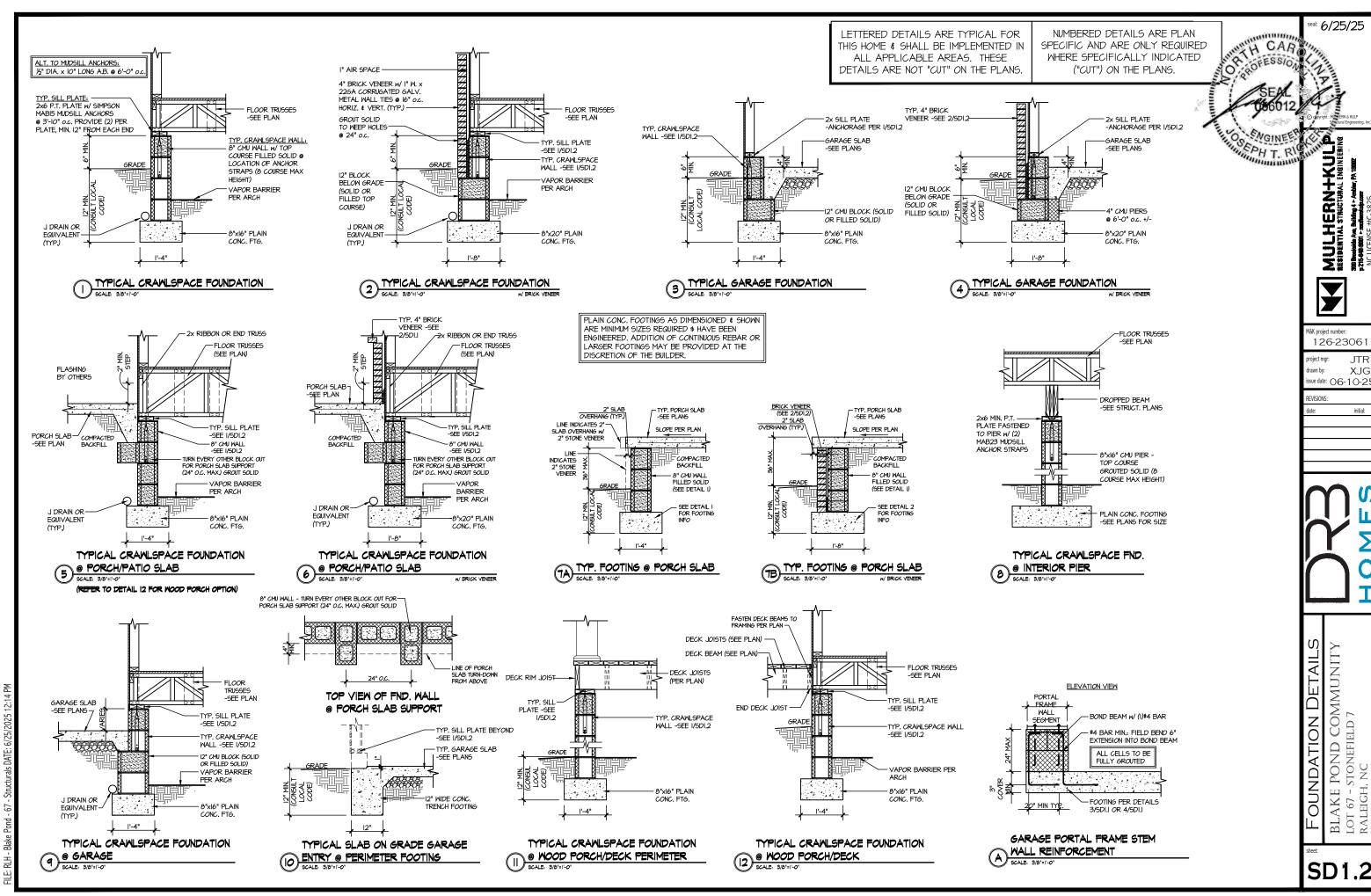
JL METAL HANGER

BLAKE POND COMMUNITY Lot 67 - Stonefield 7 Raleigh, nc ROOF

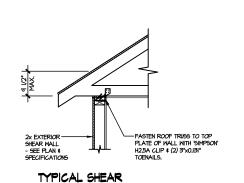




ROOF FRAMING PLAN



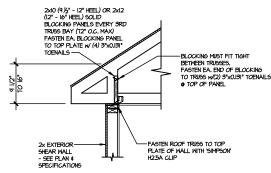
initial:



HEEL HEIGHT LESS THAN 9½" NO BLOCKING REQD

TRANSFER DETAIL @ ROOF

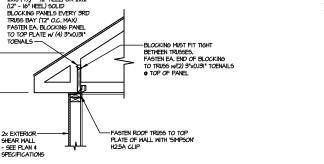
SCALE: 3/8'=1'-0' HEEL HEIGHT LESS THAN



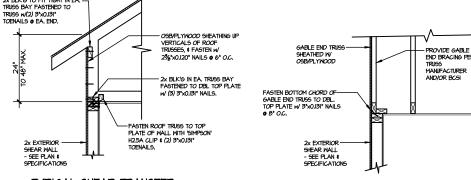
TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

SCALE: 3/8'=1-0' HEEL HEIGHT BETWEEN 4.)

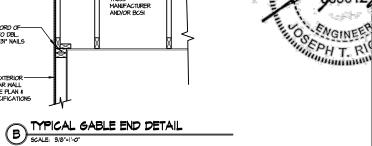


HEEL HEIGHT BETWEEN 9½" - 16" BLOCKING REQ'D





2x BLK'G TO FIT TIGHT IN EA.





a: 6/25/25

TH CAR

M&K project number:

126-2306

**JTR** drawn by: XJG issue date: 06-10-2!

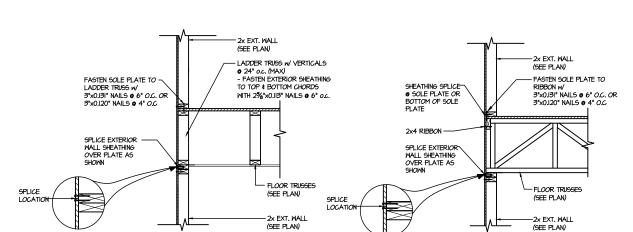
REVISIONS:

initial:



BLAKE POND COMMUNIT Lot 67 - Stonefield 7 raleigh, nc RAMING DETAILS

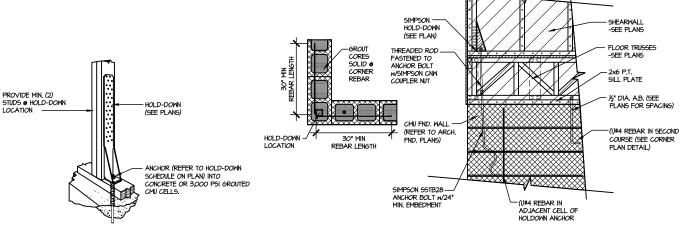
NUMBERED DETAILS ARE PLAN **SD2.0** 



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL
SCALE 580\*\*10" PARALLE FROM

TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR MALL

SCALE SOUTHOUT

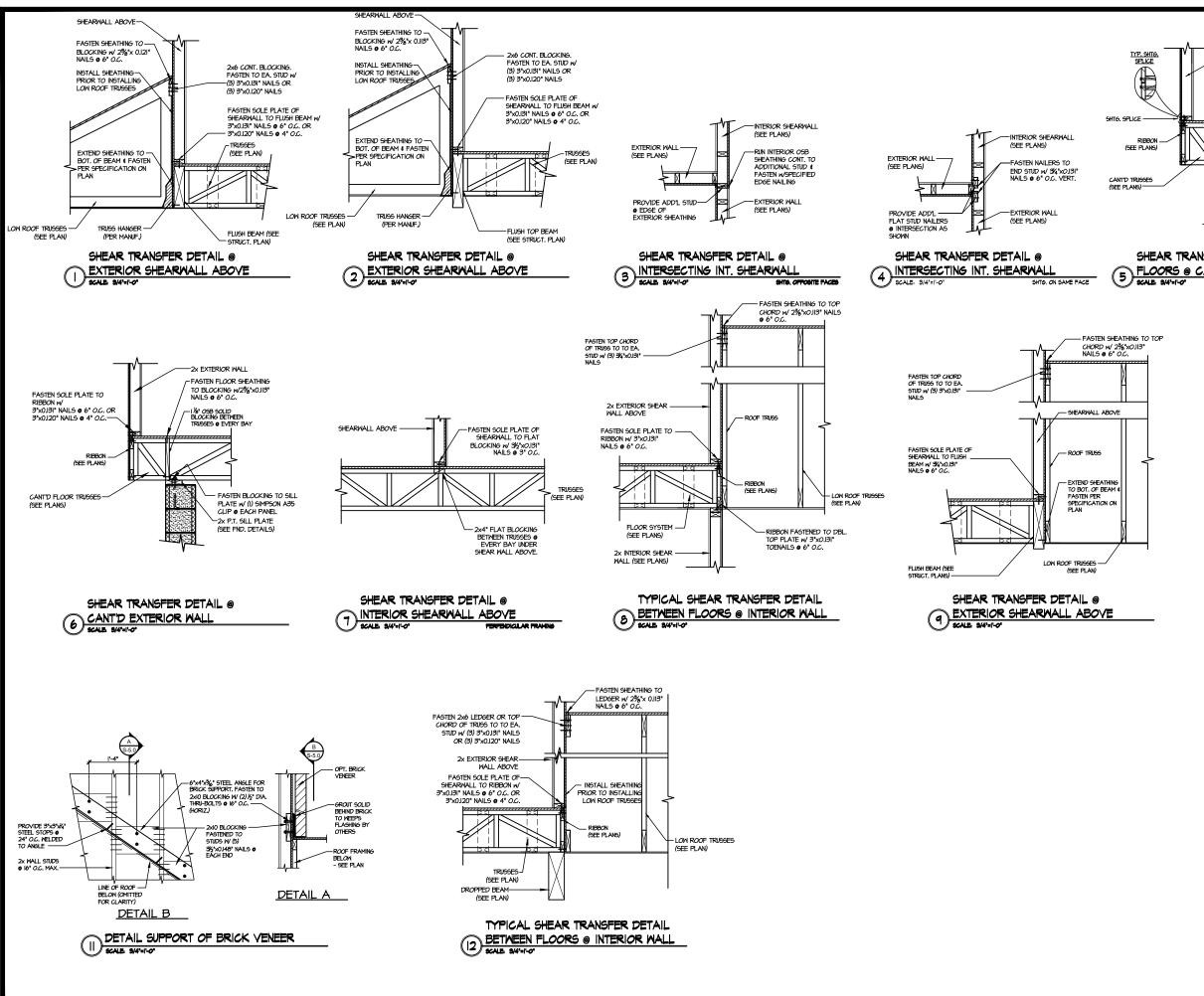


TYPICAL HOLD DOWN INSTALLATION FI SCALE: N.T.S.

TYPICAL CORNER FOUNDATION HOLD-DOWN INSTALLATION SCALE: NTS.

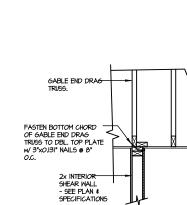
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE

SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED DETAILS ARE NOT "CUT" ON THE PLANS. ("CUT") ON THE PLANS.



TH CAR PROFESSION. OR FASTEN EL COR SHEATHING TO OSB BLOCKING W 1 % OSB SOLID BLOCKING BETWEEN SEPH T. R - FASTEN OSB BLOCKING TO DBL. TOP PLATE W (I) SIMPSON A35 CLIP © EACH PANEL -2x EXTERIOR WALL

SHEAR TRANSFER DETAIL BETWEEN 5 FLOORS @ CANT'D EXT. WALL



INTERIOR GABLE END DETAIL

6/25/25

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING

M&K project number:

roject mgr: drawn by:

REVISIONS

126-2306

ssue date: 06-10-2!

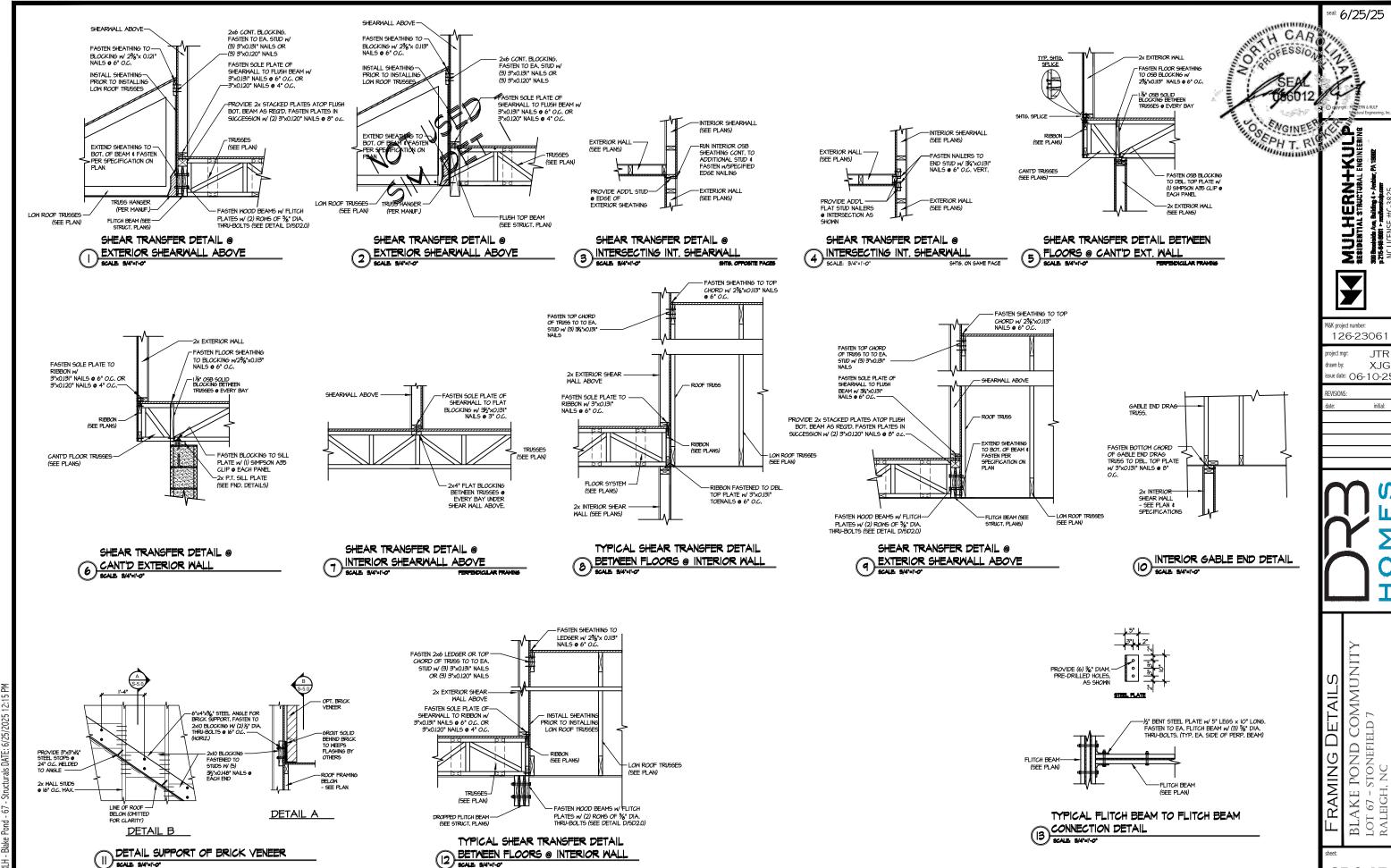
**JTR** 

XJG

initial:

BLAKE POND COMMUNIT Lot 67 - Stonefield 7 Raleigh, nc DETAIL RAMIN

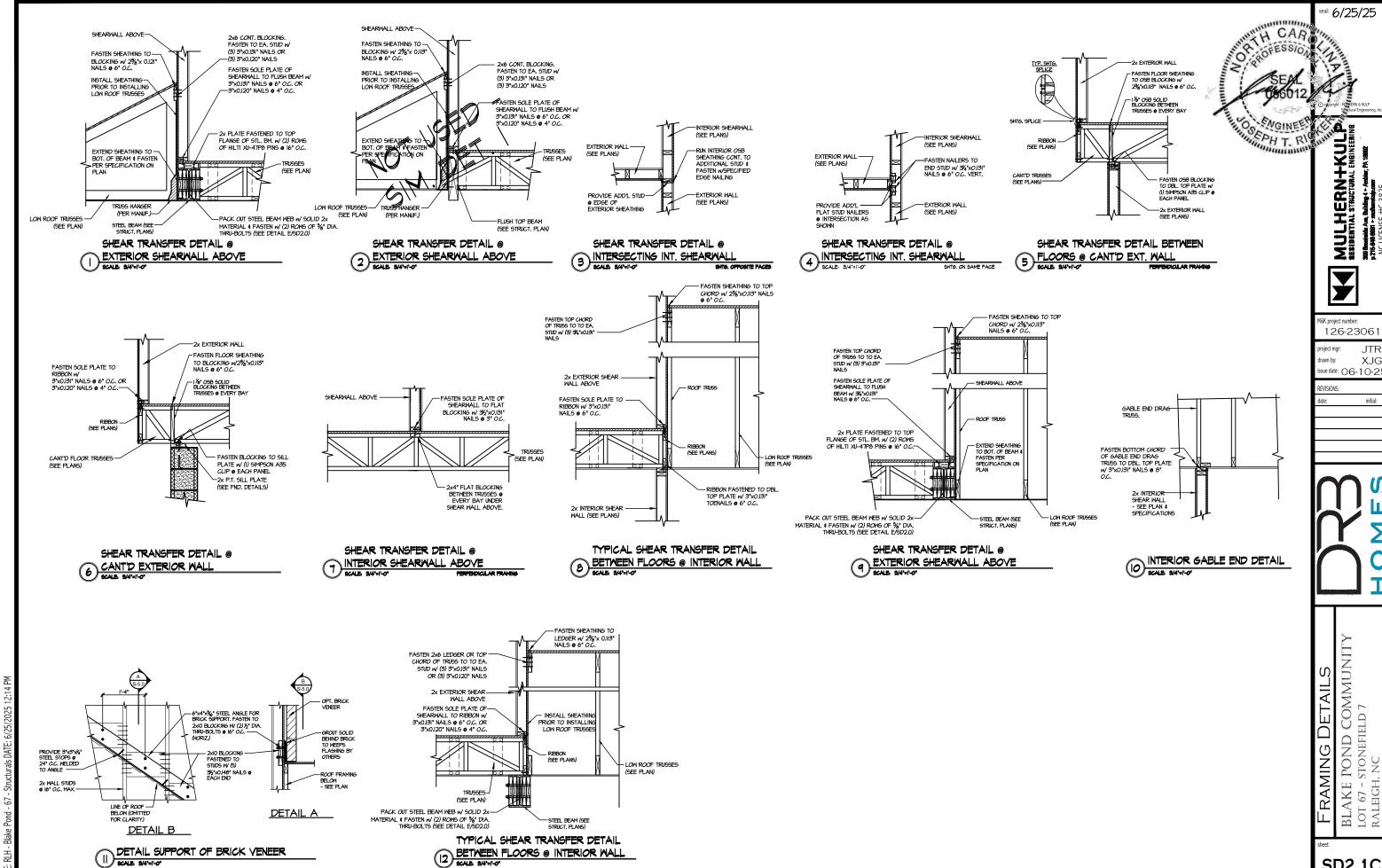
**SD2.1A** 



SD2.1B

**JTR** 

initial:

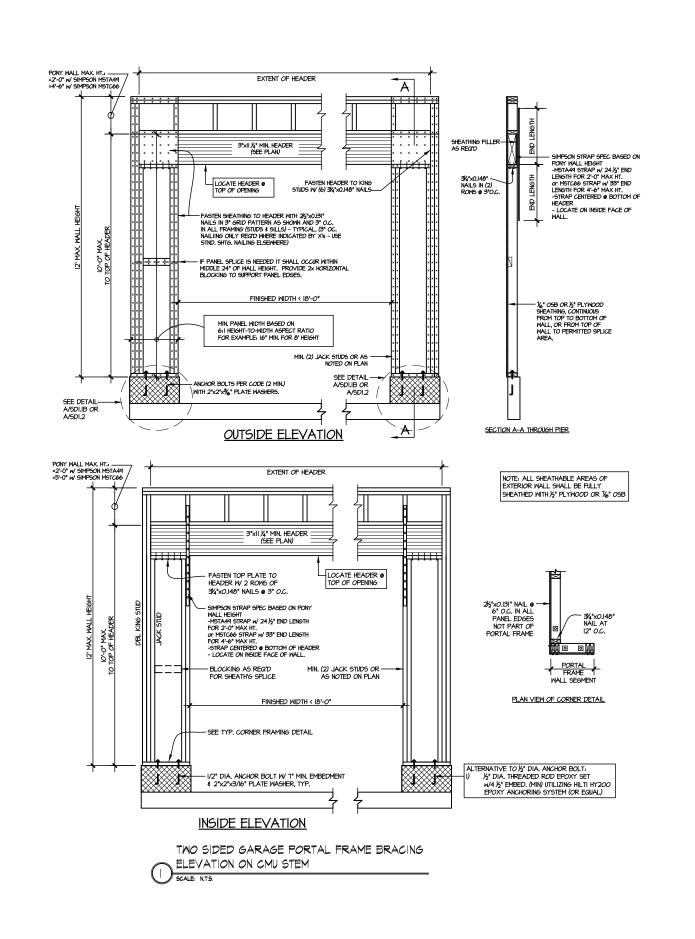


SD2.1C

BLAKE POND COMMUNIT Lot 67 - Stonefield 7 Raleigh, nc

**JTR** 

initial:



a: 6/25/25 MORA OFES OSEPH T. RI MULHERN+KULP RESIDENTIAL STRUCTURAL ENGINEERING

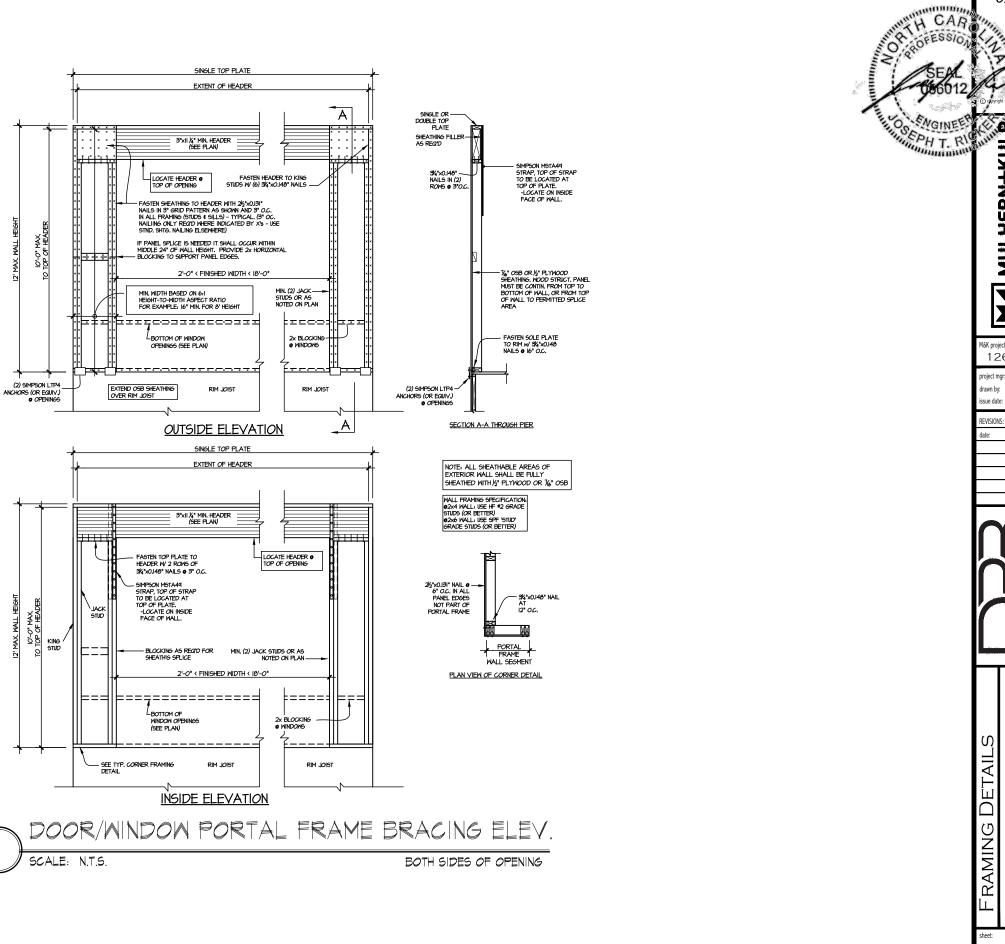
M&K project number: 126-23061

roject mgr: **JTR** drawn by: XJG issue date: 06-10-2

REVISIONS:

initial:

BLAKE POND COMMUNITY Lot 67 - Stonefield 7 Raleigh, nc FRAMING DETAILS

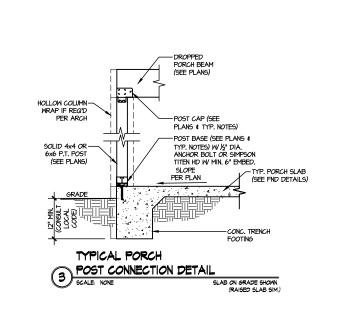


MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEERII M&K project number 126-23061 **JTR** drawn by: XJG issue date: 06-10-2 initial:

a: 6/25/25

BLAKE POND COMMUNITY Lot 67 - Stonefield 7 Raleigh, nc

**SD2.3** 



al: 6/25/25 O ENGINEE MULHERN+KULP

RESIDENTIAL STRUCTURAL ENGINEERING



M&K project number: 126-23061

project mgr: JTR

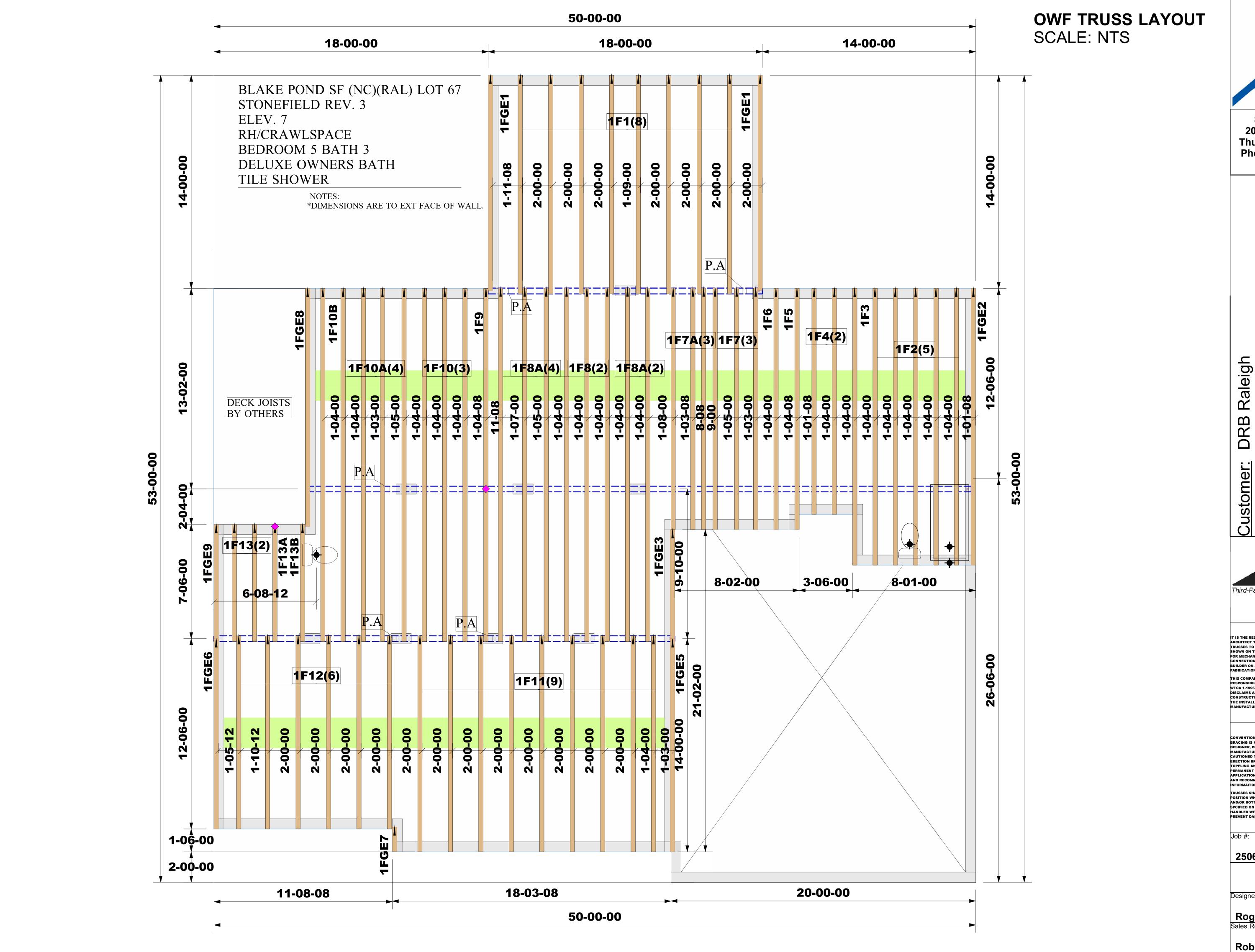
drawn by: XJG issue date: 06-10-25 REVISIONS:

initial:



BLAKE POND COMMUNITY Lot 67 - Stonefield 7 Raleigh, nc FRAMING DETAILS

SD3.0





Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

> OWF 00.0067 **Pond Lot**

STONEFIELD Blake 00.0067

Model Name:

TPI Plant W974

ot

NOTE:

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR TH CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES

**WARNING:** CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT

BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING TH **ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN** TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.

2506-0079

Roger Espinoza

Robbie Zarobinski

OWF TRUSS LAYOUT SCALE: NTS



Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

mer: DRB Raleigh

ame: Blake Pond Lot 00.0067 OWF
00.0067

STONEFIELD

Model Name:



ot

NOTE:

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOI TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATION FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THE CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THI CONSTRUCTIION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.

# WARNING:

BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS
DESIGNER, PLATE MANUFACTURER, OR THE TRUSS
MANUFACTURER. PERSONS ERECTING TRUSSES ARE
CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THI
ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN
TOPPLING AND DOMINOING DURING ERECTION; AND
PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC
APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTAR
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POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TO AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.

Job #:

2506-0079

signer:

Roger Espinoza

Robbie Zarobinski

