

Dimension Notes All exterior wall to wall dimensions are to face of stud unless noted otherwise
 All interior wall dimensions are to face of stud unless noted otherwise
 All exterior wall to truss dimensions are to face of stud unless noted otherwise

 = 3108.93 sq.ft.
 Roof Area

 = 67.18 ft.
 Ridge Line

 = 0 ft.
 Hip Line

 = 140.43 ft.
 Horiz. OH

 = 177.6 ft.
 Raked OH

 = 107 sheets
 Decking

All Walls Shown Are Considered Load Bearing

= Indicates Left End of Truss 🔺 (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards



	A = Indicates Left End of Truss
(F	Reference Engineered Truss Drawing)
	Do NOT Erect Truss Backwards

co n her for ams, e kage	соттесн
e ables m eater	ROOF & FLOOR TRUSSES & BEAMS
e	Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Truss Placement Plan SCALE: NTS

LOAD	CHART FOR	R JACK STUD	s B	BUILDER	New Home Inc.	CITY / CO.	Lillington / Harnette	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
D) DS FOR			DS FOR		Lot 44 Duncan's Creek	ADDRESS	1047 Beacon Hill Road	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	сотте
END REAC (UP TH)	(2) PLY HI (2) PLY HI END REAC (UP TC	EREQ'D STU (3) PLY HI (3) PLY EREAC		LAN	The Clayton - Low Country - Face	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLO
1700 1 3400 2 5100 3	2550 5100 7650	1 3400 2 6800 3 10200	1 2 3	EAL DATE	Seal Date	DATE REV.	6/3/25	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BE
6800 4 8500 5 10200 6	10200 12750 15300	4 13600 5 17000 6	4 5 G	QUOTE #	Quote #	DRAWN BY	Johnnie Baggett	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28 Phone: (910) 864-8
11900 7 13600 8 15300 9			J	IOB #	J0525-2855	SALES REP.	Johnnie Baggett	Johnnie Baggett	Fax: (910) 864-444

	24' 6"	15' 6"	
1	FJ2		
	∑ 1' 7 3/16"		
	1' 7 3/16"		
	······································		12
	1' 7 3/16"		
	FJ5		
		<u> </u>	
	1' 1 1/4" FJ2	1' 7 3/16"	
	FJ1	1' 7 3/16"	
	FJ1	≅ <u>1' 7 3/16"</u>	
		∞ FJ1	
	FJ5		
	FJ1		
		1' 7 3/16"	
		1' 7 3/16"	
		1' 7 3/16"	
64		1' 7 3/16"	
		E 1' 7 3/16"	
		1' 7 3/16"	
		1' 7 3/16" 5	4
			51.
		1' 7 3/16" FJ2	
		2 1/2 FJ4 ↓ 1 4 1 1/16" FJ4 FJ2	
	FJ6	1' 3 1/2" FJ4	
		G F J3 1'7 3/16'' F J3	

Plumbing Drop Notes 1. Plumbing drop locations shown are NOT exact. 2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. 3. Adjust spacing as needed not to exceed 19.2°oc.

All Walls Shown Are Considered Load Bearing

= Indicates Left End of Truss 🔺 (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

WALL SCHEDULE

Non-Bearing Walls $\Box \equiv \equiv \equiv \equiv$

1st Floor Walls

Garage Walls Dropped

2nd Floor Walls

Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



		Products		
Net Qty	Plies	Product	Length	PlotID
18	1	11 7/8" NI-40x	40' 0"	FJ1
12	1	11 7/8" NI-40x	26' 0"	FJ2
10	1	11 7/8" NI-40x	20' 0"	FJ3
9	1	11 7/8" NI-40x	14' 0"	FJ4
2	1	11 7/8" NI-40x	12' 0"	FJ5
1	1	11 7/8" NI-40x	8' 0"	FJ6
18	1	1 1/8" x 11 7/8" Rim Board	12' 0"	RIM1
65	1	11 7/8" NI-40x	2' 0"	Bk1

Indicates Left End of Truss
(Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

LOAD CH	ART FOR JAC	CK STUDS	BUILDER	New Home Inc.	CITY / CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
O) O		NOLUNA NOS FOR RADER	JOB NAME	Lot 44 Duncan's Creek	ADDRESS	1047 Beacon Hill Road	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
UP T (UP T REQ'D STU H	END REAC (UP TT (UP TT (UP TT (3) PLY H	END REAC (UP T (UP T) (A) PLY H	PLAN	The Clayton - Low Country - Face	MODEL	Crawl	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1/00 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6800 2 10200 3	SEAL DATE	Seal Date	DATE REV.	6/20/25	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote#	DRAWN BY	Johnnie Baggett	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
13600 7 13600 8 15300 9			JOB #	J0625-3141	SALES REP.	Johnnie Baggett	Johnnie Baggett	Fax: (910) 864-4444

<u>Truss Placement Plan</u> SCALE: NTS



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0625-2855 Lot 44 Duncan's Creek

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I73927450 thru I73927475

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



June 4,2025

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Job	Truss	Truss Type	Qty	Ply	Lot 44 Duncan's Creek	
						73927450
J0625-2855	A01GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayette	/ille, NC - 28314,		8	.630 s Sep	26 2024 MiTek Industries, Inc. Tue Jun 3 10:48:48 2025 P	age 2

ID:?e2f6D20Mb87TBwFO5hPsSyEJ4d-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 60, 59, 58, 57, 56, 55, 54, 53, 52, 51, 50, 49, 48, 47, 46, 45, 44, 43, 41, 40, 39, 38, 37, 36, 35 except (jt=lb) 33=126, 34=329.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





L	7-8-0	19-4-0	1		31-0-0			41-0-4		51-4-0	
	7-8-0	11-8-0	1		11-8-0	1		10-0-4		10-3-12	
Plate Offsets (X,Y)	[3:0-5-8,0-3-0], [6:0-5	5-4,0-3-0], [18:0-2-	4,0-2-12]								
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress In Code IRC202	2-0-0 L 1.15 1.15 cr YES 1/TPI2014	CSI. TC BC WB Matrix	0.79 0.57 0.56 :-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.23 -0.38 0.09 0.08	(loc) 13-15 13-15 10 13-15	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 481 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2xi 7-1 BOT CHORD 2xi WEBS 2xi 2-1 SLIDER Rig	5 SP No.1 *Except* 0: 2x6 SP 2400F 2.0E 3 SP No.1 4 SP No.2 *Except* 9: 2x6 SP No.1 9ht 2x6 SP No.1 5-9-6				BRACING- TOP CHOR BOT CHOR WEBS	RD RD	Structu except Rigid c 1 Row	iral wood end verti eiling dire at midpt	sheathing dii icals, and 2-0 ectly applied o 3	rectly applied or 2-10-1 -0 oc purlins (3-8-15 m or 10-0-0 oc bracing. }-18, 3-15, 5-15, 6-15, 8	1 oc purlins, ax.): 3-6. 3-13, 2-19
REACTIONS. Ma Ma Ma	(size) 19=0-3-8, 10=0- ax Horz 19=-319(LC 10) ax Uplift 10=-44(LC 13) ax Grav 19=2780(LC 2),	3-8 10=2556(LC 2)									
FORCES. (Ib) - M TOP CHORD 2	lax. Comp./Max. Ten A -3=-1945/345, 3-5=-3020	ll forces 250 (lb) o /521, 5-6=-3020/5	r less except v 21, 6-8=-3446	when shown 6/565, 8-10≕	-4380/595,						
BOT CHORD 1 WEBS 3	8-19=-222/331, 15-18=0/ -18=-926/388, 3-15=-273 -13=-868/280, 8-11=0/61	1679, 13-15=-141, /1899, 5-15=-796/ 0, 2-18=-184/2268	/3022, 11-13= 344, 6-15=-11 3	362/3701, 1 19/307, 6-13:	10-11=-362/3701 =-23/1006,						
NOTES- 1) Unbalanced roo	f live loads have been co	nsidered for this de	esign.								

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-2 to 4-4-8, Interior(1) 4-4-8 to 7-8-0, Exterior(2R) 7-8-0 to 14-11-2, Interior(1) 14-11-2 to 31-0-0, Exterior(2R) 31-0-0 to 38-3-2, Interior(1) 38-3-2 to 51-4-0 zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) 200.0lb AC unit load placed on the bottom chord, 12-11-8 from left end, supported at two points, 5-0-0 apart.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

SEAL 036322 June 4,2025

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1	7-8-0	19-4-0	1	31-0-0	41-0-	4	45-10-0	51-4-0
	7-8-0	11-8-0	1	11-8-0	10-0-	4	4-9-12	5-6-0
Plate Offsets (X,Y)	[3:0-5-4,0-3-0],	[6:0-5-4,0-3-0], [13:0-3-8	3,0-5-8], [18:0-2-0,0-2-0]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING Plate Grip Lumber I Rep Strea Code IRG	3- 2-0-0 DOL 1.15 DOL 1.15 ss Incr YES C2021/TPI2014	CSI. TC 0.65 BC 0.55 WB 0.73 Matrix-S	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0. Wind(LL) 0.	in (loc) l/defl 19 11-13 >999 31 11-13 >999 05 11 n/a 06 13-15 >999	L/d 360 240 n/a 240	PLATES MT20 Weight:	GRIP 244/190 483 lb FT = 20%
LUMBER- TOP CHORD 2x6 BOT CHORD 2x8 WEBS 2x4 2-19 SLIDER Right	SP No.1 SP No.1 SP No.2 *Except* : 2x6 SP No.1 t 2x6 SP No.1 5-8-	1		BRACING- TOP CHORD BOT CHORD WEBS	Structural woo except end ve Rigid ceiling di 6-0-0 oc bracir 1 Row at midp	d sheathing ticals, and 2 rectly applie Ig: 10-11.	directly applied or 2-0-0 oc purlins (4- ed or 10-0-0 oc bra 3-15, 5-15, 3-18,	4-6-13 oc purlins, 3-8 max.): 3-6. cing, Except: .6-13, 2-19, 8-11
REACTIONS. (s Max Max Max	ize) 19=0-3-8, 1 Horz 19=-319(LC Uplift 11=-49(LC Grav 19=2470(LC	1=0-3-8 ; 10) 13) 2 2), 11=2791(LC 2)						
FORCES. (lb) - Ma TOP CHORD 2-3 2-1 2-1 BOT CHORD 18 WEBS 3-1 6-1 18	x. Comp./Max. Ter 3=-1705/293, 3-5=- 9=-2314/309 -19=-222/331, 15-1 5=-198/1443, 5-15 3=-33/490 2-18=-	n All forces 250 (lb) or 2475/410, 5-6=-2475/41 8=0/1490, 13-15=0/216 5=-802/346, 3-18=-763/3 129/1974 8-11=-2717/8	less except when shown 10, 6-8=-2545/346, 8-10= 39, 11-13=-6/1539, 10-11: 356, 6-15=-81/550, 8-13= 88	-560/503, =-308/585 0/798,				

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-2 to 4-4-8, Interior(1) 4-4-8 to 7-8-0, Exterior(2R) 7-8-0 to 14-11-2, Interior(1) 14-11-2 to 31-0-0, Exterior(2R) 31-0-0 to 38-3-2, Interior(1) 38-3-2 to 51-4-0 zone; cantilever right exposed ; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) 200.0lb AC unit load placed on the bottom chord, 12-11-8 from left end, supported at two points, 5-0-0 apart.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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A MiTek Affil 818 Soundside Road



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Job	Truss	Truss Type	Qty	Ply	Lot 44 Duncan's Creek	
						173927453
J0625-2855	A04GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayett	eville, NC - 28314,			.630 s Sep	26 2024 MiTek Industries, Inc. Tue Jun 3 10:48:51 202	5 Page 2
		ID:?e2f6	D20Mb87	BwFO5hP	sSyEJ4d-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7	/J4zJC?f

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

11) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26 except (jt=lb) 2=110, 23=267, 22=377, 27=157, 14=378.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Scale = 1:116.1

818 Soundside Road Edenton, NC 27932



L	10-3-12 20-4-0	32-0-0	43-8-0	51-0-2	58-4-4 58-6-0 64-0-0
Plate Offsets (X V)	10-3-12 10-0-4 [6:0-5-4 0-3-0] [9:0-5-4 0-3-0] [17:0-4-0	11-8-0 0.6-01 [19:0-1-12 0-2-0]	11-8-0	7-4-2	7-4-2 0-1-12 5-6-0
	[0.0-3-4,0-3-0], [9.0-3-4,0-3-0], [17.0-4-0	<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.85 BC 0.62 WB 0.96	DEFL. in Vert(LL) -0.24 Vert(CT) -0.41 Horz(CT) 0.11	(loc) l/defl L/d 16-17 >999 360 17-19 >999 240 14 n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2021/TPI2014	Matrix-S	Wind(LL) 0.10	17 >999 240	Weight: 563 lb FT = 20%
LUMBER- TOP CHORD 2x6 SI 6-7,9- BOT CHORD 2x8 SI 2-20,1 WEBS 2x4 SI SLIDER Left 22	P 2400F 2.0E *Except* 10,7-9: 2x6 SP No.1 P No.1 *Except* 3-15: 2x8 SP 2400F 2.0E P No.2 46 SP No.1 5-9-6, Right 2x6 SP No.1 5-8-	-1	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathi 2-0-0 oc purlins (2-2-0 r Rigid ceiling directly ap 6-0-0 oc bracing: 13-14 1 Row at midpt	ing directly applied or 3-6-8 oc purlins, except max.): 6-9. plied or 10-0-0 oc bracing, Except: 4-19, 6-17, 8-17, 9-16, 11-14
REACTIONS. (siz Max H Max L Max C	te) 2=0-3-8, 14=0-3-8 Horz 2=-153(LC 8) Jplift 14=-5(LC 13) Grav 2=2998(LC 2), 14=3456(LC 2)				
FORCES. (lb) - Max. TOP CHORD 2-4= 11-1 11-1 BOT CHORD 2-21 13-1 WEBS 4-21 9-17	Comp./Max. Ten All forces 250 (lb) or -5148/543, 4-6=-4280/500, 6-8=-4077/56 3=-559/484 =-341/4372, 19-21=-341/4372, 17-19=-8: 4=-291/587 =0/585, 4-19=-785/297, 6-19=0/1083, 6- =-139/1458, 9-16=-198/316, 11-16=0/119	less except when shown. 60, 8-9=-4077/559, 9-11=- 3/3773, 16-17=-40/3069, 17=-178/589, 8-17=-788/3 99, 11-14=-3548/970	3555/450, 14-16=-93/2091, 348,		
 NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-16; ' MWFRS (envelope) 28-9-13 to 43-80, E forces & MWFRS fc 3) 200.0lb AC unit loar 4) WARNING: This lo handling and erecti Trusses ("BCSI"), jc qualified registered permanent individus bracing. 5) Provide adequate d 6) All plates are 4x6 M 7) This truss has been will fit between the l 9) Provide mechanical 	e loads have been considered for this de Vult=130mph (3-second gust) Vasd=103r) and C-C Exterior(2E) -0-9-2 to 5-2-14, li Exterior(2R) 43-8-0 to 52-1-13, Interior(1) or reactions shown; Lumber DOL=1.60 pl d placed on the bottom chord, 25-7-8 fror ng span truss requires extreme care and on guidance, see Guide to Good Practice bintly produced by SBCA and TPI. The bu design professional for the design and in al truss member restraint/bracing. MiTek Irainage to prevent water ponding. IT20 unless otherwise indicated. In designed for a 10.0 psf bottom chord live an designed for a live load of 30.0psf on t pottom chord and any other members, wi I connection (by others) of truss to bearin	sign. nph; TCDL=6.0psf; BCDL 1terior(1) 5-2-14 to 20-4-0 52-1-13 to 64-0-0 zone; o tate grip DOL=1.60 n left end, supported at tw experience for proper and for Handling, Installing & iliding owner or the owner spection of the temporary assumes no responsibilit e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf. g plate capable of withsta	=6.0psf; h=15ft; Cat. II; f , Exterior(2R) 20-4-0 to 2 antilever right exposed ; vo points, 5-0-0 apart. d safe handling and erec Bracing of Metal Plate C 's authorized agent shal 'installation restraint/bra y for truss manufacture, any other live loads. as where a rectangle 3-6 nding 100 lb uplift at join	Exp C; Enclosed; 28-9-13, Interior(1) C-C for members and ttion. For general Connected Wood I contract with a cing and the handling, erection, or 6-0 tall by 2-0-0 wide tt(s) 14.	SEAL 036322

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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2-3-8	8-7-12 15	20-6-0	32-0-0	43-6-0		1 50-11-2	53-7-4	58-6-0	64-0-0 I
2-3-8	6-4-4 6-	-4-4 5-6-0	11-6-0	11-6-0		7-5-2	2-8-2	4-10-12	5-6-0 ¹
Plate Offsets (X,Y)-	[3:0-3-14,0-2-4], [5	:0-4-0,Edge], [7:0-5	-4,0-2-12], [10:0-5-4,0-3-0],	[24:0-4-8,0-4-8], [26:0-2	2-8,0-3-0]				
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL. in	(loc)	l/defl L/d		PLATES	GRIP
TCLL 20.0	Plate Grip D	DOL 1.15	TC 0.73	Vert(LL) -0.32	20-22	>999 360		MT20	244/190
TCDL 10.0	Lumber DOI	L 1.15	BC 0.67	Vert(CT) -0.53	20-22	>999 240			
BCLL 0.0 *	Rep Stress I	Incr YES	WB 0.96	Horz(CT) 0.26	15	n/a n/a			
BCDL 10.0	Code IRC20	021/TPI2014	Matrix-S	Wind(LL) 0.14	6	>999 240		Weight: 57	1 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1 *Except* 1-5: 2x8 SP No.1 BRACING- TOP CHORD 5: 2x8 SP No.1 TOP CHORD									

	1-5: 2x8 SP No.1		except	
BOT CHORD	2x6 SP No.1 *Except*		2-0-0 oc purlins (3-2-	14 max.): 7-10.
	3-24: 2x6 SP 2400F 2.0E, 21-23,17-19,19-21: 2x8 SP No.1	BOT CHORD	Rigid ceiling directly a	applied or 10-0-0 oc bracing, Except:
	14-17: 2x8 SP 2400F 2.0E		6-0-0 oc bracing: 14-	15.
WEBS	2x4 SP No.2	WEBS	1 Row at midpt	4-24, 7-22, 7-20, 9-20, 10-20, 10-18, 12-15
SLIDER	Left 2x6 SP No.1 1-9-4, Right 2x6 SP No.1 5-7-10			

REACTIONS. (size) 2=0-3-8, 15=0-3-8 Max Horz 2=-155(LC 8) Max Uplift 2=-100(LC 12), 15=-94(LC 13) Max Grav 2=2728(LC 2), 15=3305(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2742/525, 3-4=-5695/907, 4-6=-4730/829, 6-7=-4582/949, 7-9=-3773/769, 9-10=-3772/769, 10-12=-3354/599, 12-14=-411/464
- BOT CHORD
 2-27=-190/763, 3-26=-506/4311, 25-26=-684/5066, 24-25=-684/5066, 22-23=-112/744, 20-22=-293/3423, 18-20=-172/2888, 16-18=-101/2052, 15-16=-101/2052, 14-15=-275/463

 WEBS
 4-25=0/505, 4-24=-1140/269, 22-24=-185/2718, 7-24=-363/1435, 7-20=-167/644, 9-20=-786/349, 10-20=-243/1310, 10-18=-167/309, 12-18=-139/1010, 12-16=-26/286, 12-15=-3616/775

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-7-2 to 5-4-14, Interior(1) 5-4-14 to 20-6-0, Exterior(2R) 20-6-0 to 28-11-13, Interior(1) 28-11-13 to 43-6-0, Exterior(2R) 43-6-0 to 51-11-13, Interior(1) 51-11-13 to 64-0-0 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)







2-3-8	8-7-12 15-0-0 20-4-0	32-0-0	43-8-0	53-8-4	64-0-0				
Plate Offsets (X,Y)	[3:0-2-2,0-1-4], [7:0-5-4,0-2-12], [10:0-5	j-12,0-4-0], [18:0-3-12,0-5	-0], [22:0-4-12,0-3-0], [24:	:0-0-8,0-2-0]	10-3-12				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.64 BC 0.81 WB 0.97 Matrix-S	DEFL. in Vert(LL) -0.27 Vert(CT) -0.43 Horz(CT) 0.16 Wind(LL) 0.10	(loc) l/defl L/d 18-20 >999 360 18-20 >999 240 14 n/a n/a 23-24 >999 240	PLATES GRIP MT20 244/190 Weight: 513 lb FT = 20%				
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x6 S 16-18 WEBS 2x4 S 7-20,5 SLIDER Left 2	LUMBER- BRACING- TOP CHORD 2x6 SP No.1 *Except* TOP CHORD 2x6 SP No.1 *Except* 2-0-0 oc purlins (6-0-0 max.): 7-10. BOT CHORD 2x6 SP 2400F 2.0E BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. WEBS 2x4 SP No.2 *Except* WEBS 1 Row at midpt 7-20, 9-18, 10-18, 12-17 7-20,9-18,10-17: 2x4 SP No.1 2 Rows at 1/3 pts 7-18, 10-17 SLIDER Left 2x6 SP No.1 1-5-12, Right 2x6 SP No.1 5-10-2 WEBS 1 Row at midpt 7-20, 9-18, 10-17								
Max I Max I Max I Max I	$\begin{array}{l} \text{Max Horz } 2=-156(\text{LC 8}) \\ \text{Max Uplift } 2=-110(\text{LC 12}), 17=-1(\text{LC 9}), 14=-142(\text{LC 13}) \\ \text{Max Grav } 2=1816(\text{LC 27}), 17=3849(\text{LC 2}), 14=597(\text{LC 28}) \end{array}$								
FORCES. (lb) - Max TOP CHORD 2-3= 9-10 9-10 BOT CHORD 2-25 20-2 20-2 WEBS 4-23 7-18 12-1	$\begin{array}{l} \mbox{Max Grav } 2=1816(LC 27), 17=3849(LC 2), 14=597(LC 28) \\ \hline \mbox{ORCES.} (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. \\ \mbox{OP CHORD} & 2-3=-1871/417, 3-4=-3437/662, 4-6=-2616/613, 6-7=-2523/737, 7-9=-1103/487, \\ & 9-10=-1103/486, 10-12=0/1014, 12-14=-566/260 \\ \mbox{OT CHORD} & 2-25=-163/531, 3-24=-308/2540, 23-24=-463/3023, 22-23=-463/3023, 6-22=-262/227, \\ & 20-21=-68/268, 18-20=-116/1707, 17-18=-821/280, 15-17=-167/398, 14-15=-167/398 \\ \mbox{VEBS} & 4-23=0/433, 4-22=-946/233, 20-22=-50/1491, 7-22=-338/1199, 7-20=-39/409, \\ & 7-18=-901/99, 9-18=-807/354, 10-18=-392/2629, 10-17=-2678/505, 12-17=-1241/326, \\ & 12-15=0/675 \\ \hline \end{array}$								
 NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-16; MWFRS (envelope 28-9-13 to 43-8-0, I reactions shown; L 3) WARNING: This lo handling and erecti Trusses ("BCSI"), ji qualified registered permanent individu bracing. 4) Provide adequate of 5) This truss has beer 6) * This truss has beer 	e loads have been considered for this de Vult=130mph (3-second gust) Vasd=103) and C-C Exterior(2E) -0-9-2 to 5-2-14, Exterior(2R) 43-8-0 to 52-1-13, Interior(1 umber DOL=1.60 plate grip DOL=1.60 ing span truss requires extreme care and on guidance, see Guide to Good Practic bintly produced by SBCA and TPI. The b design professional for the design and i al truss member restraint/bracing. MiTel trainage to prevent water ponding. In designed for a 10.0 psf bottom chord lin en designed for a live load of 30.0psf on	esign. mph; TCDL=6.0psf; BCDI interior(1) 5-2-14 to 20-4-0) 52-1-13 to 64-0-0 zone;(d experience for proper an e for Handling, Installing & uilding owner or the owne hspection of the temporary c assumes no responsibility re load nonconcurrent with the bottom chord in all are	L=6.0psf; h=15ft; Cat. II; E 0, Exterior(2R) 20-4-0 to 2 C-C for members and force d safe handling and erect & Bracing of Metal Plate C wr's authorized agent shall y installation restraint/brac ty for truss manufacture, h n any other live loads. eas where a rectangle 3-6-	Exp C; Enclosed; 28-9-13, Interior(1) es & MWFRS for tion. For general connected Wood contract with a cing and the handling, erection, or -0 tall by 2-0-0 wide	SEAL 036322				

- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17 except (jt=lb) 2=110, 14=142.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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June 4,2025



2-3-8	8-7-12	15-0-0	20-4-0	1	32-0-0	43-	8-0		5	3-8-4	64-0-0	
2-3-8	6-4-4	6-4-4	5-4-0	1	11-8-0	11-	8-0	1	1	0-0-4	10-3-12	1
Plate Offsets (X,Y)	[3:0-2-2,0-1-4],	[7:0-5-4,0-2	2-12], [10:0-5-	·12,0-4-0], [[·]	19:0-3-12,0-5-0	0], [23:0-4-12,0-3	-0], [25:	:0-0-8,0-	-2-0]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACIN Plate Gri Lumber I Rep Stre Code IR	IG- rip DOL DOL ess Incr RC2021/TPI2	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matri	0.64 0.81 0.97 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.27 -0.43 0.16 0.11	(loc) 19-21 19-21 14 24-25	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 516 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 BOT CHORD 2x6 17-' WEBS 2x4 7-2' SLIDER Left	SP No.1 SP No.1 *Except* 9: 2x6 SP 2400F 2 SP No.2 *Except* ,9-19,10-18: 2x4 S 2x6 SP No.1 1-5-1	2.0E SP No.1 12, Right 2x6	6 SP No.1 5-	10-2		BRACING- TOP CHOR BOT CHOR WEBS	D D	Structur 2-0-0 or Rigid ce 1 Row a 2 Rows	ral wood c purlins eiling dire at midpt s at 1/3 pt	sheathing dir (6-0-0 max.): ctly applied o 7 s 7	rectly applied or 3-8-6 (: 7-10. or 6-0-0 oc bracing. /-21, 9-19, 10-19, 12-18 /-19, 10-18	oc purlins, except
REACTIONS. (Ma: Ma: Ma:	size) 2=0-3-8, 18 (Horz 2=148(LC ((Uplift 2=-108(LC (Grav 2=1816(LC	8=0-3-8, 14= 9) 5 12), 18=-7(l C 27), 18=384	⊧0-3-8 LC 9), 14=-14 48(LC 2), 14⊧	48(LC 13) =636(LC 28)							
FORCES. (lb) - M. TOP CHORD 2- 9- 9- BOT CHORD 2- 21 21 WEBS 4- 7- 12	ax. Comp./Max. Te 3=-1871/414, 3-4=: 10=-1105/463, 10- 26=-173/528, 3-25: -22=-73/268, 19-2: 24=0/433, 4-23=-9 19=-899/103, 9-19: -16=0/672	en All force 3438/655, 4 -12=0/1007, 5=-324/2541, 21=-140/1708 045/236, 21-2 9=-807/354, 1	es 250 (lb) or 4-6=-2617/60 12-14=-563/2 24-25=-497/ 8, 18-19=-820 23=-70/1491, 10-19=-398/2	less except 10, 6-7=-252 251 (3024, 23-24 0/275, 16-18 7-23=-345/ 2628, 10-18=	when shown. 4/724, 7-9=-11 4=-497/3024, 6 3=-168/395, 14 1198, 7-21=-3 2677/512, 12	105/463, 6-23=-262/227, 4-16=-168/395 8/409, 2-18=-1239/326,						
NOTES- 1) Unbalanced roof 2) Wind: ASCE 7-16 MWFRS (envelop 28-9-13 to 43-8-C reactions shown; 3) WARNING: This	live loads have bee ; Vult=130mph (3-)e) and C-C Exterior , Exterior(2R) 43-8 Lumber DOL=1.6C long span truss re	en considere -second gust ior(2E) -0-9-2 8-0 to 52-1-1 0 plate grip E equires extre	ed for this de t) Vasd=103r 2 to 5-2-14, Ir 3, Interior(1) DOL=1.60 me care and	sign. nph; TCDL= nterior(1) 5-2 52-1-13 to (experience	6.0psf; BCDL 2-14 to 20-4-0, 64-9-2 zone;C for proper and	=6.0psf; h=15ft; C , Exterior(2R) 20- -C for members a d safe handling ar	Cat. II; E 4-0 to 2 Ind force	Exp C; E 28-9-13, es & MV tion. For	nclosed; Interior(1 VFRS for		TH CA	RO

- 3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18 except (jt=lb) 2=108, 14=148.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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L2-3	-8 ₁ 8-7-12	15-0-0	20-4-0	32-0-0	43-8-	0	5	3-8-4	64-0-0	
2-3	-8 6-4-4	6-4-4	5-4-0	11-8-0	11-8-	0	1	0-0-4	10-3-12	1
Plate Offsets (X,Y	') [3:0-2-2,0- ⁻	1-4], [7:0-5-4,0 [,]	-2-12], [10:0-5	-12,0-4-0], [19:0-3-12,0-5	-0], [23:0-4-12,0-3-0)], [25:0-0-8,0	-2-0]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	* Rep Cod	CING- e Grip DOL ber DOL Stress Incr e IRC2021/TP	2-0-0 1.15 1.15 YES 12014	CSI. TC 0.64 BC 0.81 WB 0.97 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) 0.27 19-21 0.43 19-21 0.16 14 0.11 24-25	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 516 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 *Except* 7-21,9-19,10-18: 2x4 SP No.1 SLIDER Left 2x6 SP No.1 1-5-12, Right 2x6 SP No.1 5-10-2 REACTIONS. (size) 2=0-3-8, 18=0-3-8 (req. 0-4-8), 14=0-3-8 Max Horz 2=148(LC 9) Max Grav 2=1821(LC 27), 18=3835(LC 2), 14=642(LC 28)										
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1876/415, 3-4=-3448/658, 4-6=-2627/603, 6-7=-2534/726, 7-9=-1118/467, 9-10=-1118/467, 10-12=0/993, 12-14=-570/252 BOT CHORD 2-26=-173/529, 3-25=-326/2549, 24-25=-499/3033, 23-24=-499/3033, 6-23=-262/227, 21-22=-73/269, 19-21=-142/1716, 18-19=-806/270, 16-18=-155/401, 14-16=-155/401 WEBS 4-24=0/433, 4-23=-945/236, 21-23=-72/1498, 7-23=-346/1200, 7-21=-40/407, 7-19=-893/101, 9-19=-807/354, 10-19=-397/2624, 10-18=-2668/508, 12-18=-1239/325, 12-16=0/673										

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-2 to 5-2-14, Interior(1) 5-2-14 to 20-4-0, Exterior(2R) 20-4-0 to 28-9-13, Interior(1) 28-9-13 to 43-8-0, Exterior(2R) 43-8-0 to 52-1-13, Interior(1) 52-1-13 to 64-9-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 7) W(APMINC) Required hearing area of isot(a) 10, a restart the isot(b between the bottom chord and any other members.
- 7) WARNING: Required bearing size at joint(s) 18 greater than input bearing size.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18 except (jt=lb) 2=109, 14=149.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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818 Soundside Road



2-3-8	8-7-12	15-0-0	20-4-0	32-0-0	39-10-4	43-8-0	53-8-4	64-0-0	1
2-3-8	6-4-4	6-4-4	5-4-0	11-8-0	7-10-4	3-9-12	10-0-4	10-3-12	1
Plate Offsets (X,Y)	[2:0-7-6,0-1	1-8], [7:0-5-4,0-	2-12], [10:0-10	0-4,0-3-4], [14:0-8-10,0-1-1]	, [19:0-3-0,0-3-12],	21:0-5-0,0	0-6-0], [23:0-3-0,0-3-12],	[25:0-4-12,0-3-0]	

LOADING (psi	f)	SPACING- 2-7-12	CSI.	DEFL. i	n (loc) l/defl	L/d	PLATES	GRIP
TCLL 20.	0	Plate Grip DOL 1.15	TC 0.94	Vert(LL) -0.24	4 21-23 >999	360	MT20	244/190
TCDL 10.	0	Lumber DOL 1.15	BC 0.59	Vert(CT) -0.3	9 21-23 >999	240	M18AHS	186/179
BCLL 0.	0 *	Rep Stress Incr NO	WB 0.96	Horz(CT) 0.1	6 14 n/a	n/a		
BCDL 10.	0	Code IRC2021/TPI2014	Matrix-S	Wind(LL) 0.1	1 26-27 >999	240	Weight: 570 lb	FT = 20%
LUMBER-				BRACING-				
TOP CHORD	2x6 SP	? No.1		TOP CHORD	Structural wood	d sheathing di	irectly applied or 3-4-8 oc	purlins, except
BOT CHORD	2x8 SP	No.1 *Except*			2-0-0 oc purlins	s (6-0-0 max.)): 7-10.	
	2-28,17	7-21: 2x8 SP 2400F 2.0E, 27-28,6-24: 2	2x6 SP No.1	BOT CHORD	Rigid ceiling di	rectly applied	or 10-0-0 oc bracing, Ex	cept:
	3-25: 2	x6 SP 2400F 2.0E			6-0-0 oc bracin	g: 24-25,19-2	21.	
WEBS	2x4 SP	PNo.2 *Except*		WEBS	1 Row at midpt	-	4-25, 7-21, 9-21, 10-21	
	7-21,10	0-21,12-19,10-19: 2x4 SP 2400F 2.0E			2 Rows at 1/3	ots	12-19	
OTHERS	2x8 SP	2400F 2.0E			T-Brace:	:	2x6 SPF No.2 - 10-19	
LBR SCAB	18-20 2	2x8 SP 2400F 2.0E one side			Fasten (2X) T	and I braces	to narrow edge of web wit	h 10d
SLIDER	Left 2x	6 SP No.1 1-5-0, Right 2x6 SP No.1 5-	9-6		(0.131"x3") nai	ls, 6in o.c.,wit	th 3in minimum end distar	ice.
REACTIONS	(size	2=0-3-8 19=(0-3-8 + bearing block	() (reg 0-4-7) 14=0-3-8		Diace must co		eb leligili.	
	Max H	$r_{2} = 194(1 \text{ C} 9)$	() (log. 0 1 1), 1 = 0 0 0					
	Max U	plift $2=-138(C 22)$ 19=-68(C 9) 14=	-239(I C 13)					
	Max G	ray 2=2085(LC 27), 19=5351(LC 2), 1	4=1524(LC 28)					
FORCES. (lb) - Max.	Comp./Max. Ten All forces 250 (lb) of	r less except when shown.					
TOP CHORD	2-3=-	2214/519, 3-4=-3813/735, 4-6=-2737/6	65, 6-7=-2639/830, 7-9=-52	23/452,				
	9-10=	=-522/452, 10-12=0/1241, 12-14=-1826	/387					
BOT CHORD	2-28=	-237/734, 3-27=-300/2667, 26-27=-53	5/3339, 25-26=-535/3339, 6	-25=-352/301,				
	23-24	4=-89/367, 21-23=-106/1695, 19-21=-2	190/617, 16-19=-190/1426,	14-16=-190/1426				
WEBS	4-26=	=0/570, 4-25=-1230/310, 23-25=-38/13	39, 7-25=-434/1427, 7-23=0	/710,				
	7-21=	-1713/213, 9-21=-1058/466, 10-21=-6	60/3607, 12-19=-2391/549,	12-16=0/1077,				

1) Attached 4-0-0 scab 18 to 20, back face(s) 2x8 SP 2400F 2.0E with 2 row(s) of 10d (0.131"x3") nails spaced 7" o.c..

2) Scab(s) 18 to 20 to provide bearing enhancement at jt.19, a cluster of 16 evenly spaced - 10d (0.131"x3") nails are required within 12" of jt.19. Total nails to be divided equally between front and back if scabs are on both sides. Bearing is assumed to be SP 2400F 2.0E.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-2 to 5-2-14, Interior(1) 5-2-14 to 20-4-0, Exterior(2R) 20-4-0 to 28-9-13, Interior(1) 28-9-13 to 43-8-0, Exterior(2R) 43-8-0 to 52-1-13, Interior(1) 52-1-13 to 64-9-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

5) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

Confirmedenadequate drainage to prevent water pondin

10-19=-3936/850

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818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 44 Duncan's Creek	
						173927459
J0625-2855	A10	PIGGYBACK BASE	1	1		
					Job Reference (optional)	
Comtech, Inc, Faye	tteville, NC - 28314,		6	.630 s Sep	26 2024 MiTek Industries, Inc. Tue Jun 3 10:48:56 2025	Page 2
-		ID:?e2f6	D20Mb87	BwFO5hP	sSyEJ4d-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J	4zJC?f

7) All plates are MT20 plates unless otherwise indicated.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19 except (jt=lb) 2=138, 14=239.
 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-7=-79, 7-10=-79, 10-15=-119(F=-40), 2-28=-26, 25-27=-26, 14-24=-26



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Scale = 1:116.1



	10-3-12 20-4	.0	40-0-0	43-8-0	53-8-4	64-0-0	
	10-3-12 10-0	4 11-8-0	8-0-0	3-8-0	10-0-4	10-3-12	
Plate Offsets (X,Y)	[6:0-5-4.0-3-0], [9:0-5-4.0-3-0]	[17:0-3-0.0-3-12], [19:0-4-0.0-6-0	0]. [20:0-3-0.0-3-12]				
		, <u>[, ,</u>], <u>[</u>], [,]				
LOADING (psf)	SPACING- 2-0	-0 CSI.	DEFL. in	n (loc) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.	15 TC 0.89	Vert(LL) -0.33	3 19-20 >999	360	MT20	244/190
TCDI 10.0	Lumber DOI 1	15 BC 0.65	Vert(CT) -0.54	19-20 >892	240		
BCLL 0.0 *	Rep Stress Incr Y	-S WB 0.71	Horz(CT) 0.11	13 n/a	 n/a		
BCDI 10.0	Code IRC2021/TPI201	4 Matrix-S	Wind(LL) 0.13	3 19-20 >999	240	Weight: 532 lb	FT = 20%
DODE 10:0			Wind(EE) 0.10	715 20 2555	240	Weight: 002 lb	11 = 2070
LUMBER-			BRACING-				
TOP CHORD 2v6	SP No 1			Structural wood	sheathing di	irectly applied or 2-2-0 (oc nurlins excent
BOT CHORD 2x8	SP 2400E 2 0E				(3-11-6 may	10000 applied of 2 2 0 0	o putitito, except
	SP No 2			Pigid coiling dire	(3-11-0 max	0.0.0-3	
	276 SP No 1 5 0 6 Dight 276 SP	No 1 5 0 6	WERS	1 Pow at midat	cuy applied	4 20 6 10 9 10 0 10 10 10) 17 11 17
SLIDER Leit	2x0 SF N0.1 5-9-0, Right 2x0 SF	110.1 3-9-0	WEB3	T KOW at mupt		4-20, 0-19, 0-19, 9-19, 3	5-17, 11-17
Max Max Max	Horz 2=146(LC 9) Uplift 2=-116(LC 12), 13=-125(L Grav 2=2581(LC 2), 13=2183(L	C 13) C 2), 18=1623(LC 2)					
FORCES. (lb) - Ma	x. Comp./Max. Ten All forces 2	250 (lb) or less except when showr	າ.				
TOP CHORD 2-4	=-4321/753, 4-6=-3469/729, 6-8	=-2889/744, 8-9=-2889/744, 9-11=	=-2455/620,				
11-	-13=-3640/676						
BOT CHORD 2-2	22=-557/3657, 20-22=-557/3657,	19-20=-332/3023, 18-19=-219/20	98, 17-18=-219/2098,				
15	-1/=-464/3059, 13-15=-464/305	9	0/0/0				
WEBS 4-2	2=0/553, 4-20=-823/260, 6-20=-	24/1120, 6-19=-300/223, 8-19=-78	38/343,				
9-1	9=-175/1223, 9-17=-422/173, 11	-17=-1156/300, 11-15=0/810					
NOTES							
1) Unbalanced reef l	ive leads have been considered	for this design					
2) Mind: ASCE 7.16	Wilt 120mph (2 accord quat)	lond 102mph TCDL 6 0ppf BCD	- 6 Opof: b-1Eft: Cot II:	Eve C: Englaged			
2) WITU, ASCE 7-10	a) and C C Exterior(2E) 0.0.2 to	asu = 103 mpH, TCDL = 0.0pSI, BCD = 5.2.14 to 20.4	0 = 0.000 si, 11 = 1511, 0.001, 11, 0.000 si, 11 = 1511, 0.000	28.0.12 Interior(1	`		
29 0 12 to 42 9 0	Evtorior(2P) 42 9 0 to 52 1 12	5^{-2-14} , interior(1) 5-2-14 to 20-4-	C C for mombars and for	20-9-13, Interior()	annun (1111.
20-3-13 10 43-0-0,	$L_{1} = 160 \text{ (2R)} + 3^{-0} \cdot 0 \cdot$	1 - 1.60		000 0 IVIVERO 101		IN'TH CA	Rall
	Lumber DOL= 1.00 plate grip DO	L=1.00	nd aafa handling and ara	otion For general		N'all'	10/11/
S) WARINING: THIS	iong span iruss requires extreme	care and experience for proper a	nu sale nandling and ere	cuon. For general		201.00	T. J. A. C.

3) WARNING: This long span truss requires extreme care and experience for proper and state handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=116. 13=125.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







19

 $10 \times 10 =$

29

∎ 18

17

5x5 =

31

15

3x10 ||

16

10x10 =

32

30

1	10-3-12	20-4-0	32-1-12	39-10-4	43-8-0	53-8-4	64-0-0	1
	10-3-12	10-0-4	11-9-12	7-8-8	3-9-12	10-0-4	10-3-12	
Plate Offsets (X,	Y) [2:0-0-0,0-5-0	6], [6:0-5-4,0-3-8], [9:0-5-4,	0-4-0], [19:0-4-8,0-7-8], [2	0:0-3-8,0-4-12]				
LOADING (psf) TCLL 20.0 TCDL 10.0	SPAC Plate Lumbe	ING- 2-7-4 Grip DOL 1.15 er DOL 1.15	CSI. TC 0.91 BC 0.77	DEFL. in Vert(LL) -0.30 Vert(CT) -0.50	(loc) l/defl 19-20 >999 19-20 >958	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 10.0	* Rep S Code	tress Incr NO IRC2021/TPI2014	WB 0.94 Matrix-S	Horz(CT) 0.10 Wind(LL) 0.14	13 n/a 19-20 >999	n/a 240	Weight: 580 lb	FT = 20%
LUMBER- TOP CHORD 2 60T CHORD 2	2x6 SP 2400F 2.0E 3-7,7-9: 2x6 SP No.	*Except* 1 -		BRACING- TOP CHORD	Structural wood except	d sheathing dire	ctly applied or 3-11-1	3 oc purlins,
WEBS 2 SLIDER L	2x4 SP No.2 _eft 2x6 SP No.1 5-4	- 3-8, Right 2x4 SP No.2 5-8	-8	BOT CHORD	Rigid ceiling di 6-9-5 oc bracin 6-8-6 oc bracin	ectly applied or g: 18-19 g: 17-18.	10-0-0 oc bracing, I	Except:
				WEBS	1 Row at midpt T-Brace: Fasten (2X) T (0.131"x3") nai Brace must cov	4-2 2xi and I braces to ls, 6in o.c.,with 3 ver 90% of web	20, 6-19, 9-19, 8-19, 9 6 SPF No.2 - 11-17 narrow edge of web v 3in minimum end dist length	9-17 with 10d ance.

REACTIONS.	(size)	2=0-3-8, 13=0-3-8, 18=0-3-8
	Max Horz	2=188(LC 9)
	Max Uplift	2=-156(LC 12), 13=-222(LC 13), 18=-19(LC 8)
	Max Grav	2=3150(LC 2), 13=2947(LC 2), 18=2910(LC 2)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-4=-5252/932, 4-6=-4148/895, 6-8=-3157/873, 8-9=-3168/879, 9-11=-2691/712, 11-13=-4709/927

 BOT CHORD
 2-22=-680/4451, 20-22=-680/4451, 19-20=-382/3605, 18-19=-213/2190, 17-18=-213/2190, 15-17=-668/3943, 13-15=-668/3943

 WEBS
 4-22=0/674, 4-20=-1091/345, 6-20=-51/1540, 6-19=-716/83, 9-19=-210/1506,

ι¢

6x8 =

20

8x8 =

28 21

22

2x6 ||

27

8x12 =

11-17=-2089/544, 11-15=0/1097, 8-19=-1017/446, 9-17=-770/233

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-2 to 5-2-14, Interior(1) 5-2-14 to 20-4-0, Exterior(2R) 20-4-0 to 28-9-13, Interior(1) 28-9-13 to 43-8-0, Exterior(2R) 43-8-0 to 52-1-13, Interior(1) 52-1-13 to 64-9-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

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14 13

8x8 ||

-4-1 -4-1 -4-1 -4-1



Job	Truss	Truss Type	Qty	Ply	Lot 44 Duncan's Creek	
						173927461
J0625-2855	A12	PIGGYBACK BASE	2	1		
					Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			.630 s Sep	26 2024 MiTek Industries, Inc. Tue Jun 3 10:48:57 202	5 Page 2
		ID:?e2f6	D20Mb871	BwFO5hF	sSvEJ4d-RfC?PsB70Ha3NSaPanL8w3ulTXbGKWrCDoi	7J4zJC?f

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18 except (jt=lb) 2=156, 13=222.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-78, 6-9=-78, 9-14=-118, 2-13=-26



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)





Scale = 1:118.9



	64-0-0	
Plate Offsets (X,Y) [19:0-2-0,0-2-0], [50:0-4-0,0-4-8], [58:0-4-0,0-4-8], [66:0-4-0,0-4-8]	64-0-0 3]	
LOADING (psf) SPACING- 2-0-0 CSI. TCLL 20.0 Plate Grip DOL 1.15 TC 0.06 TCDL 10.0 Lumber DOL 1.15 BC 0.03 BCLL 0.0 * Rep Stress Incr YES WB 0.12 BCDL 10.0 Code IRC2021/TPI2014 Matrix-S Xet in the stress incr	DEFL. in (lt Vert(LL) -0.00 Vert(CT) -0.00 Vert(CT) -0.00 Horz(CT) 0.01	(loc) I/defi L/d PLATES GRIP 41 n/r 120 MT20 244/190 41 n/r 120 41 n/a n/a Weight: 674 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2 SLIDER Left 2x6 SP No.1 1-9-3, Right 2x6 SP No.1 1-9-3	BRACING- TOP CHORD Str 2-C BOT CHORD Rig WEBS T-E	tructural wood sheathing directly applied or 6-0-0 oc purlins, except -0-0 oc purlins (6-0-0 max.): 15-28. igid ceiling directly applied or 10-0-0 oc bracing. -Brace: 2x4 SPF No.2 - 12-66, 13-65, 14-64, 16-63 , 17-62, 18-61, 20-60, 21-59, 22-58, 23-57, 24-56, 25-55, 26-54, 27-53, 29-52, 30-51, 31-50
REACTIONS. All bearings 64-0-0. (lb) - Max Horz 2=-148(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 2, 72, 71, 70, 69, 68, 67, 60 63, 62, 61, 60, 59, 58, 57, 56, 55, 54, 53, 51, 50, 49, 48, 47, - except 73=-128(LC 12), 43=-102(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 73, 72, 71, 70, 69, 6 65, 64, 63, 62, 61, 60, 59, 58, 57, 56, 55, 54, 53, 52, 51, 50, 46, 45, 44, 41, 43	Fa (0. Bra 6, 65, 46, 45, 44 8, 67, 66, 49, 48, 47,	asten (2X) T and I braces to narrow edge of web with 10d 0.131"x3") nails, 6in o.c.,with 3in minimum end distance. race must cover 90% of web length.
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown TOP CHORD 2-4=-303/110, 11-12=-104/269, 12-13=-126/330, 13-14=-144/384, 14- 15-16=-134/375, 16-17=-134/375, 17-18=-134/375, 18-20=-134/375, 2 21-22=-134/375, 22-23=-134/375, 23-24=-134/375, 24-25=-134/375, 2 26-27=-134/375, 27-28=-134/375, 28-29=-139/359, 29-30=-144/384, 3 31-32=-104/269, 39-41=-271/71	15=-139/359, 0-21=-134/375, 5-26=-134/375, 0-31=-126/330,	TH CARO
 WEBS 4-73=-127/273, 39-43=-115/262 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDI MWFRS (envelope) and C-C Corner(3E) -0-9-2 to 5-2-14, Exterior(2N) 5-2-14 to 20-4 26-4-0 to 43-8-0, Corner(3R) 43-8-0 to 49-8-0, Exterior(2N) 49-8-0 to 64-9-2 zone;C-C reactions shown; Lumber DCL=1.60 plate grip DCL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to win Gable End Details as applicable, or consult qualified building designer as per ANSI/TI WARNING: This long span truss requires extreme care and experience for proper an handling and erection guidance, see Guide to Good Practice for Handling, Installing & Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owne qualified registered design professional for the design and inspection of the temporary permanent individual truss member restraint/bracing. MiTek assumes no responsibili 	L=6.0psf; h=15ft; Cat. II; Exp -0, Corner(3R) 20-4-0 to 26- C for members and forces & I nd (normal to the face), see S PI 1. d safe handling and erection Bracing of Metal Plate Conr r's authorized agent shall con i installation restraint/bracing ty for truss manufacture, han	b C; Enclosed; 44-0, Exterior(2N) MWFRS for Standard Industry n. For general nected Wood ontract with a g and the ndling, erection, or
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and a truss system. Before use, the building designer must verify the applicability of design parameters and pre building design. Bracing indicated is to prevent buckling of individual truss, we handre chord members on	PAGE MII-7473 rev. 1/2/2023 BEFOR is for an individual building compone operly incorporate this design into the v. Additional temporary and perman	neet, not he overall aneet bracing

a doss system: property incorporate and subscription dost with the approaching of design property incorporate and subscription design indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always to storage, delivery, rection and bracing of trusses safet truss systems, see **ANSI/TPI Quality Criteria and DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Lot 44 Duncan's Creek	
					173	3927462
J0625-2855	A13GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Jun 3 10:48:59 2025 Page 2				ge 2		

ID:?e2f6D20Mb87TBwFO5hPsSyEJ4d-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

5) Provide adequate drainage to prevent water ponding.

- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 72, 71, 70, 69, 68, 67, 66, 65, 63, 62, 61, 60, 59, 58, 57, 56, 55, 54, 53, 51, 50, 49, 48, 47, 46, 45, 44 except (jt=lb) 73=128, 43=102.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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BCDL	10.0	Code IRC2021/TPI2014	Matrix-R	10/2(01) 0.0	JO 10 11/A	n/a	Weight: 202 lb	FT = 20%
LUMBER- TOP CHOR	RD 2x6 SP	No.1		BRACING- TOP CHORD	Structural wood s	heathing dir	ectly applied or 6-0-0 o	c purlins,
BOT CHOR WEBS	RD 2x6 SP 2x6 SP	No.1 No.1		BOT CHORD	except end vertice Rigid ceiling direct	als. tly applied o	or 10-0-0 oc bracing.	
OTHERS	2x4 SP	No.2		WEBS	T-Brace: Fasten (2X) T an	2 d I braces to	x4 SPF No.2 - 7-23, 9-2 o narrow edge of web w	22 rith 10d

REACTIONS. All bearings 20-2-0.

Max Horz 28=-339(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 26, 18 except 28=-255(LC 10),

- 16=-198(LC 11), 24=-133(LC 12), 25=-122(LC 12), 27=-329(LC 12), 20=-134(LC 13), 19=-122(LC 13), 17=-314(LC 13)
- Max Grav All reactions 250 lb or less at joint(s) 23, 24, 25, 26, 22, 20, 19, 18 except 28=357(LC 9), 16=303(LC 8), 27=338(LC 10), 17=300(LC 11)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-271/238, 6-7=-186/361, 7-8=-140/257, 8-9=-140/257, 9-10=-186/362

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-9-9 to 3-5-4, Exterior(2N) 3-5-4 to 10-5-0, Corner(3R) 10-5-0 to 14-9-13, Exterior(2N) 14-9-13 to 21-7-9 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26, 18 except
- (jt=lb) 28=255, 16=198, 24=133, 25=122, 27=329, 20=134, 19=122, 17=314.
- 10) Non Standard bearing condition. Review required.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



(0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

June 4,2025

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818 Soundside Road Edenton, NC 27932

June 4,2025



			23-4-0			
	23-4-0					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.06 BC 0.04 WB 0.06	DEFL. in (loc) l/defl L/d PLAT Vert(LL) n/a - n/a 999 MT20 Vert(CT) n/a - n/a 999 MT20 Horz(CT) 0.00 12 n/a n/a 1/2	ES GRIP 244/190		
BCDL 10.0	Code IRC2021/TPI2014	Matrix-S	Weig	nt: 109 lb FT = 20%		
LUMBER-			BRACING-			

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 23-4-0.

Max Horz 1=114(LC 16) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 2, 20, 21, 22, 23, 18, 17, 15, 14, 12

Max Grav All reactions 250 lb or less at joint(s) 1, 13, 2, 19, 20, 21, 22, 23, 18, 17, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 0-4-5 to 4-9-1, Exterior(2N) 4-9-1 to 11-8-0, Corner(3R) 11-8-0 to 16-0-13, Exterior(2N) 16-0-13 to 22-11-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated. 5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 2, 20, 21, 22, 23, 18, 17, 15, 14, 12.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-4-5 to 4-9-1, Interior(1) 4-9-1 to 11-8-0, Exterior(2R) 11-8-0 to 16-0-13, Interior(1) 16-0-13 to 22-11-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 11=114. 8=113.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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818 Soundside Road





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Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=406(LC 22), 9=527(LC 19), 6=535(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-322/273, 4-6=-322/273

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 8-4-10, Exterior(2R) 8-4-10 to 12-9-7, Interior(1) 12-9-7 to 16-4-6 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 On the training and the problem of the

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=153, 6=153.



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Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=132, 6=132.



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3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.



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Max Grav 1=187(LC 1), 3=187(LC 1), 4=327(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=4-8-5, 3=4-8-5, 4=4-8-5

Max Horz 1=-40(LC 8)

Max Uplift 1=-14(LC 13), 3=-17(LC 13)

Max Grav 1=92(LC 1), 3=92(LC 1), 4=134(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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3x4 🥢

3x4 🚿

0 ₁ 0 ₁ 7	2-4-7	
0-0-7	2-4-0	

Plate Offsets (X,Y)	[2:0-2-0,Edge]	1			1
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.02 BC 0.02 WB 0.00 Matrix-P	DEFL. i Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	n (loc) l/defl L/d a - n/a 999 a - n/a 999 0 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 7 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF	P No.1 P No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di Rigid ceiling directly applied o	rectly applied or 2-4-7 oc purlins. or 10-0-0 oc bracing.

REACTIONS. (size) 1=2-3-8, 3=2-3-8

Max Horz 1=-16(LC 8) Max Uplift 1=-3(LC 12), 3=-3(LC 13)

Max Grav 1=63(LC 12), 3=63(LC 1)Max Grav 1=63(LC 1), 3=63(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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TREALERING OF A MITCH Affiliate 818 Soundside Road

