# TELFAIR

BRIARWOOD BLUFF LOT 27

PLAN ID 010225



# 110 VILLAGE TRAIL SUITE 215 WOODSTOCK, GA. 30188

	DRAWING INDEX
A0.0	COVER SHEET
A1.1	FRONT ELEVATIONS
A2.1	SIDE & REAR ELEVATIONS
A3.1	SLAB FOUNDATIONS
A5.1	FIRST FLOOR PLAN
A6.1	ROOF PLANS
A7.2	ELECTRICAL PLANS

AREA TABULATION			
FIRST FLOOR	1803		
TOTAL	1803		
GARAGE	403		
FRONT PORCH ELEVATION B E H (COVERED)	61		
REAR PATIO	120		

## **GOVERNMENTAL CODES & STANDARDS**

HOME TO BE BUILT TO CONFORM TO ALL APPLICABLE LOCAL CODES, PRACTICES AND STANDARDS

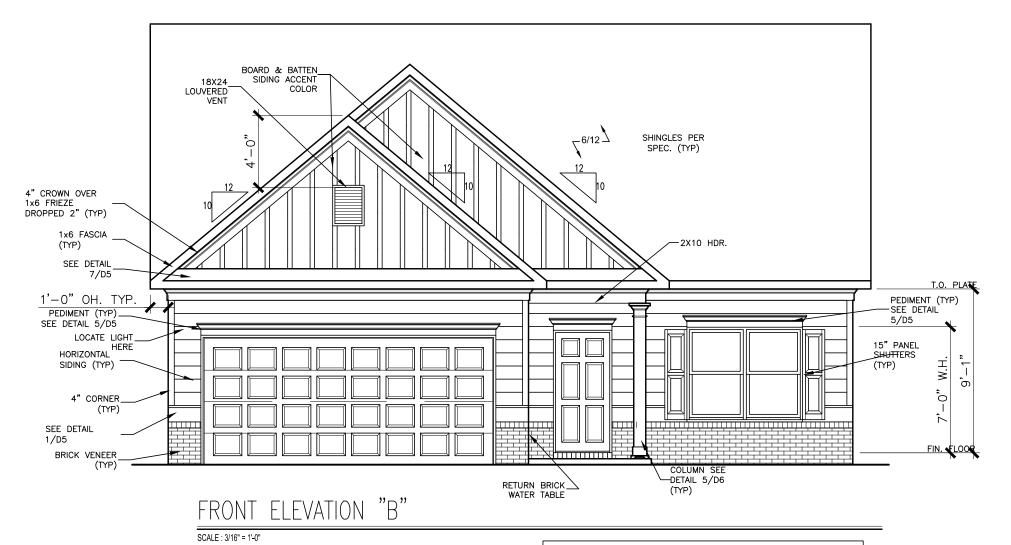
## BUILDING CODE ANALYSIS / DESIGN CRITERIA

HOME TO BE BUILT TO MEET OR EXCEED ALL LOCAL CODES AND DESIGN CRITERIA

		PLAN REVISIONS	
DATE	BY	REVISION	PAGE #
7/19/2019	AW	Added elevations K & L	A1.11-A1.12
7/25/2019	AW	Relocated water heater to increase sq. ft. from 1795 to 1803	A3.1, A5.1, A7.2, A7.2.1, A8.1
8/29/2019	AW	PROTOTYPE WALK CHANGES - SEE REVISION SHEET	ALL
11/18/2019	AW	For 2-story version made wall between stairs and Owner's 2x6 and coat closet wall 2x6	A5.1.1
11/18/2019	AW	Flipped location of HVAC platform for 2-story version to rear of house which relocated B-5 W.I.C. and window	A5.3.2, A6.1-A6.3, A7.3.2
2/12/2020	AW	Added note for LVL at rear 3050 twin for plans without 2nd floor per truss/engineering	A5.1
5/1/2020	AW	PCR #3744 Changed hall bath 3068 door to 2868 to clear cabinet knobs	A5.1
5/6/2020	AW	PCR #3777 Removed small piece of foyer chair rail/shadow box between living rm opng & kitchen	A8.1
8/11/2020	AW	Elevations K & L - changed front porch cedar columns to std. box column and removed decorative brackets on front porch of Elev L and added 1x10 at fin. flr. level where B&B shown	A1.11, A1.12
11/1/2021	AW	PCR #4579 Adjust location of opt. pendants & opt. LED Kitchen lights for better placement	A7.3
12/11/2025	SL	PCR # 6101 Adjusted optional second floor front wall to 8' in height.	A5.2, A5.4, A7.3

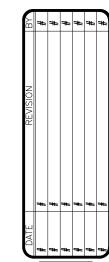
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## BRIARWOOD BLUFF LOT 27



ALL NON-MASONRY RETURNS TO BE HORIZONTAL SIDING

SEE SHEET D3 OF SDH TYPICAL DETAILS FOR SOFFIT DETAILS PER SOFFIT MATERIAL



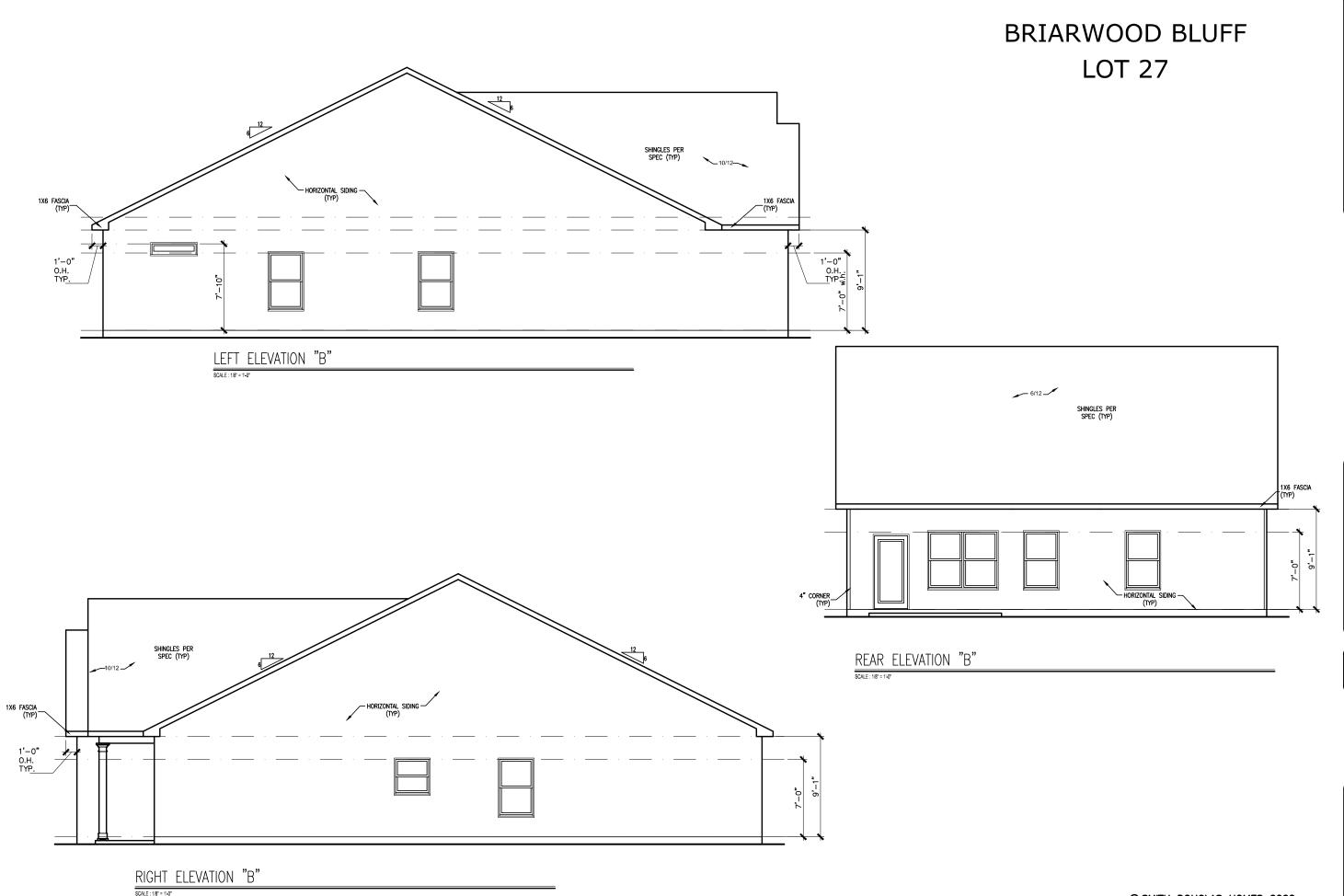
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FRONT ELEVATION
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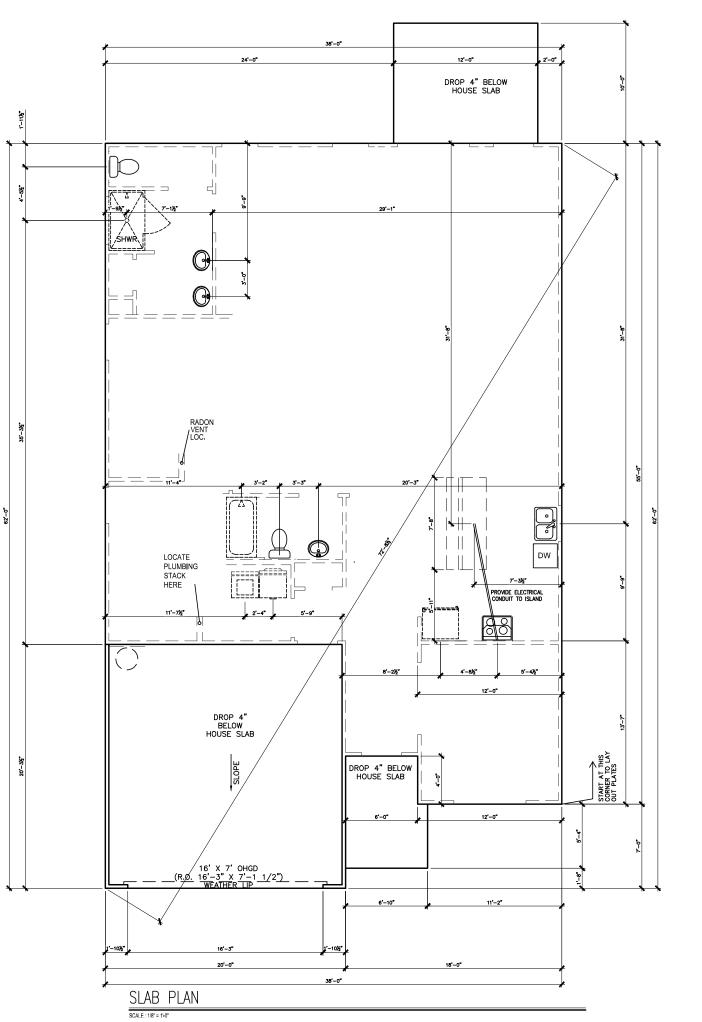




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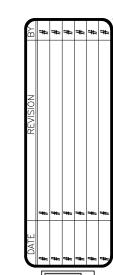




## BRIARWOOD BLUFF LOT 27

\*RADON VENT PROVIDED PER LOCAL CODE

REFER TO DETAIL 3/D1 FOR BRICK LEDGE DETAIL WHEN BRICK VENEER IS CHOSEN







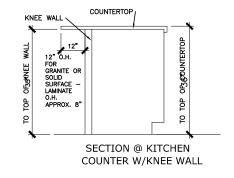
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## 9'-2½" 6'-6" PATIO 10'X12' 2-PLY 9.25 LVL 42'-0" 3068 F.L. OWNER'S OWNER'S SUITE 9' CLG. HGT. FAMILY ROOM 9' CLG. HGT. BEDROOM 3 🛒 LOC. TBD PER SITE CONDITIONS/ COMMUNITY EXCEPTIONS 4 SH **BREAKFAST** \*3" DIA. RADON -VENT LOC. 1'-3½" CABINET SKIN . 없음 BEDROOM 2 9' CLG. HGT. KITCHEN 4'-3½" 2'-6" 4'-1½" LIVING RM. 9' CLG. HGT. FOYER NO LIVING SPACE ABOVE GAAGE GARAGE 9' CLG. HGT. 3068 COVERED PORCH -8"X8" BOX COL. (TYP) 16' X 7' OHGD (R.O. 16'-3" X 7'-1 1/2") 6'-6" 20'-0" 18'-0" FIRST FLOOR PLAN SCALE : 1/8" = 1'-0"

## BRIARWOOD BLUFF LOT 27



SMITH DOUGLAS HOMES FLOOR TELFAIR FLOOR PLAN

REFER TO MANUFACTURER'S SPECS. FOR DRAIN LOCATIONS ON DETAIL SHEETS D12,D12.1,D12.2 & D12.3

\*RADON VENT PROVIDED PER LOCAL CODE

"AAP 6/6/25 В ^A5.1

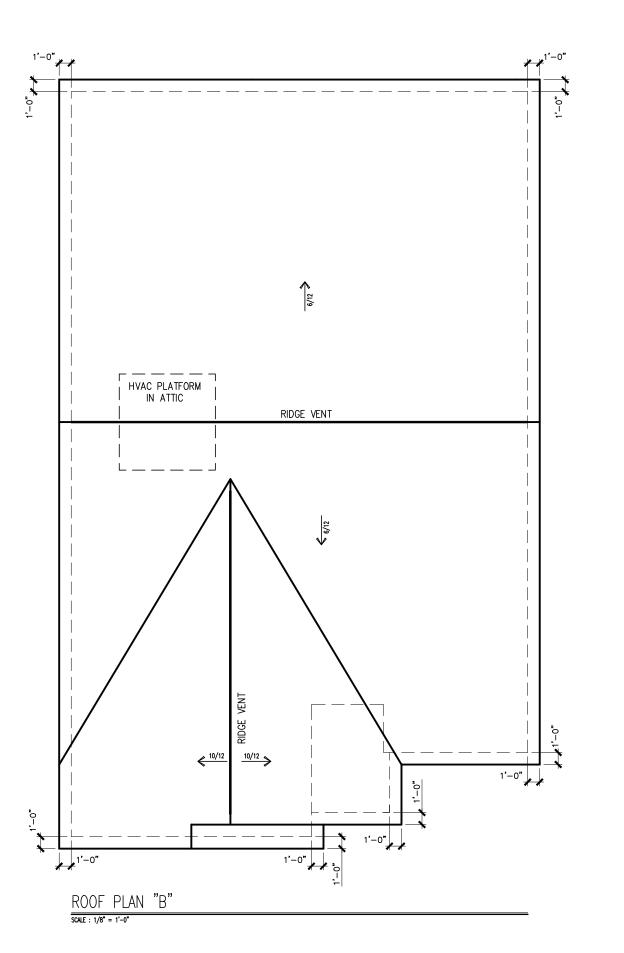
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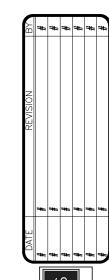
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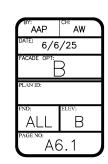






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## PATIO 0 OWNER'S SUITE \_OWNER'S FAMILY \_ <sub>(≡)</sub>BATH ROOM BEDROOM 3 W.I.C. BREAKFAST KITCHEN I DO NOT INSTALL DISPOSAL SWITCH AND OUTLET FOR SEPTIC COMMUNITIES BEDROOM 2 LNDRY LIVING ROOM FOYER ` GARAGE COVERED PORCH

## BRIARWOOD BLUFF LOT 27

FIE	ELECTRICAL LEGEND				
	_CINICAL L		ND		
\$	SWITCH	ŢV	TV		
\$3	3 WAY SWITCH	φ	120V RECEPTACLE		
\$4	4 WAY SWITCH	<b>P</b>	120V SWITCHED RECEPTACLE		
Ø	CEILING FIXTURE	Φ	220V RECEPTACLE		
- <b>∳</b> <sub>K</sub>	KEYLESS	P <sub>GFCI</sub>	GFCI OUTLET		
+XX	WALL MOUNT FIXTURE	PAFCI	ARCH FAULT CIRCUIT		
0	CEILING FIXTURE	† <sub>GL</sub>	GAS LINE		
•	FLEX CONDUIT	T <sub>wL</sub>	WATER LINE		
СН	CHIMES	¥	HOSE BIBB		
PH	TELEPHONE	8	FLOOD LIGHT		
SD/Co	SMOKE DETECTOR & CARBON MONOXIDE		1x4 LUMINOUS FIXTURE		
SO	SECURITY OUTLET				
	GARAGE DOOR OPENER		CEILING FAN		
■	EXHAUST FAN		ELECTRICAL WIRING		
9	FAN/LIGHT	-\$-	CEILING FIXTURE		
ELECTRICAL PLANS TO FOLLOW ALL LOCAL CODES					
APPROX. FIXTURE HGTS (MEASURED FROM BOTTOM OF FIXTURE)					
BREA	BREAKFAST/DINING ROOM 63" ABOVE FINISHED FLOOR				
KITCH	IEN PENDANT LIGHTS	33" ABO	VE COUNTER TOP		
TWO	STORY FOYER FIXTURE	96" ABO	VE FINISHED FLOOR		
CEILIN	NG FAN	96" ABO	VE FINISHED FLOOR		

NOTE: FINAL PLACEMENT OF PHONE/CABLE T.B.D. ON SITE BY THE BUILDER



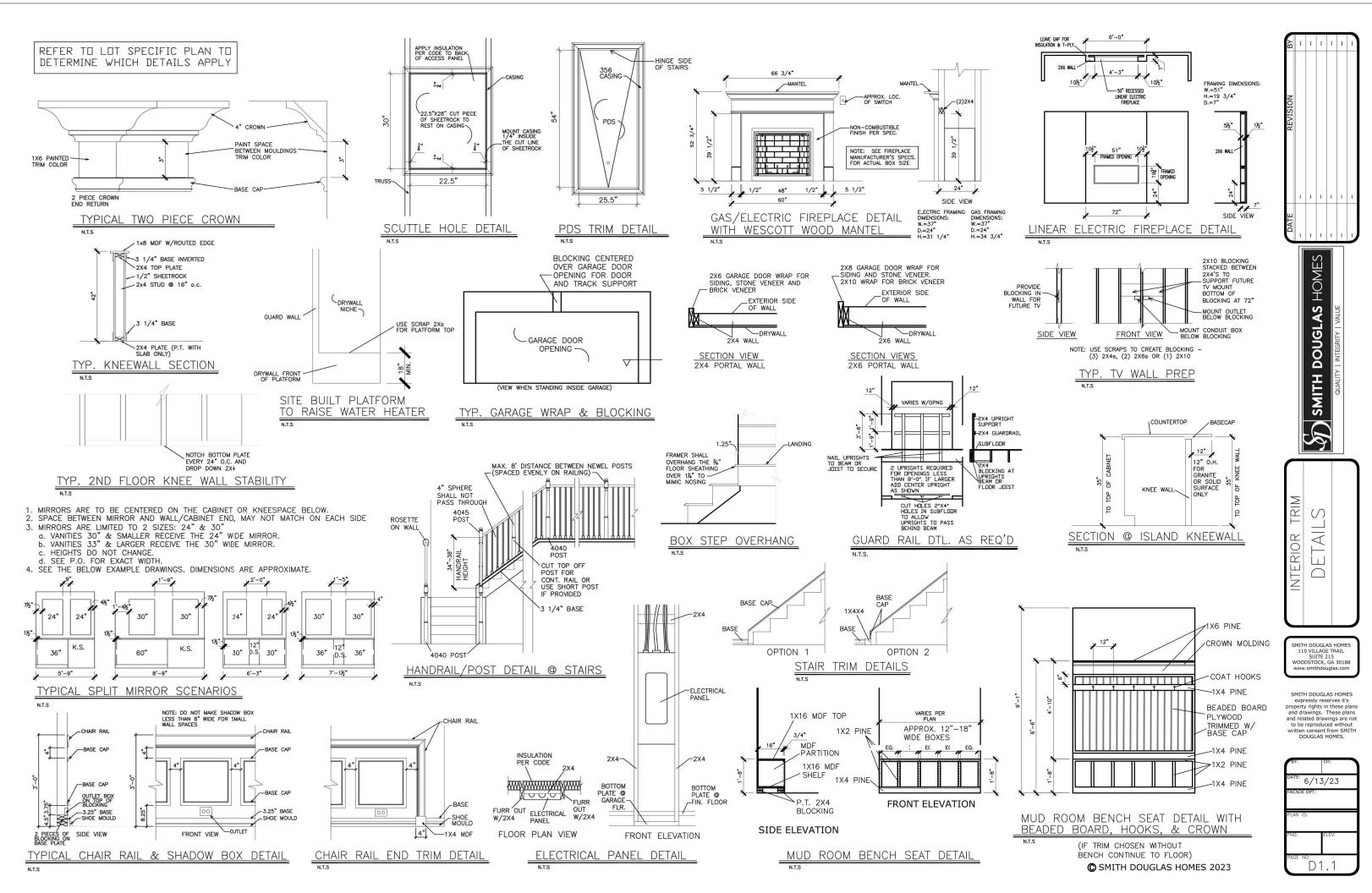
SMITH DOUGLAS HOMES QUALITY I INTEGRITY I VALUE

FIRST FLOOR
TELFAIR

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### DESIGN SPECIFICATIONS:

Construction Type: Commerical ☐ Residential ☒

## Applicable Building Codes:

- 2018 North Carolina Residential Building Code
- ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

## Design Loads:

sic	gn Loads:	
	1. Roof	
	Live	20 PSF
	1.2 Dead	10 PSF
	1.3 Snow	15 PSF
	1.3.1 Importance Factor	1.0
	2. Floor Live Loads	
	2.1 Typ. Dwelling	40 PSF
	2.2 Sleeping Areas	30 PSF
	2.3 Balconies (exterior) and Decks	40 PSF
	2.4 Garage Parking	50 PSF
	3. Floor Dead Loads	
	3.1 Conventional 2x	10 PSF
	3.2  -Joist	15 PSF
	3.3 Floor Truss	15 PSF
	4. Ultimate Wind Speed (3 sec. gust)	130 MPH
	4.1 Exposure	В
	4.2 Importance Factor	1.0
	4.3 Wind Base Shear	

4.3.2 Vy = 5. Component and Cladding (in PSF)

4.3.1 Vx =

MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

1	Sei		
0	20	5 m	10

6. Site Class	D
6.2 Design Category	C
6.3 Importance Factor	1.0
6.4 Seismic Use Group	1

6.5 Spectral Response Acceleration

6.5.1 Sms = %q

6.5.2 Sml = %g 6.6 Seismic Base Shear

6.6.1 Vx =

6.6.2 Vy =

6.7 Basic Structural System (check one)

■ Bearing Wall

☐ Building Frame
☐ Moment Frame

□ Dual w/ Special Moment Frame

□ Dual w/ Intermediate R/C or Special Steel

□ Inverted Pendulum

6.8 Arch/Mech Components Anchored?......No

6.9 Lateral Design Control: Seismic Wind W

Assumed Soil Bearing Capacity.......

\_\_\_\_\_2000psf



STRUCTURAL PLANS PREPARED FOR:

## TELFAIR

PROJECT ADDRESS: TBD OWNER:

Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 27539

## ARCHITECT/DESIGNER:

Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineer of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, INC. before construction begins.

### PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
oc	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, INC. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by SMITH DOUGLAS HOMES. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

## SHEET LIST:

Sheet No.	Description	
CSI	Cover Sheet, Specifications, Revisions	
C52	Specifications Continued	
SI.Øm	Monolithic Slab Foundation	
S1.Øs	Stem Wall Foundation	
51.0c	Crawl Space Foundation	
S1.0b	Basement Foundation	
52.0	Basement Framing Plan	
S3.Ø	First Floor Framing Plan	
54.0	Second Floor Framing Plan	
S5.Ø	Roof Framing Plan	
56.0	Basement Bracing Plan	
S7.Ø	First Floor Bracing Plan	
58.0	Second Floor Bracing Plan	

## REVISION LIST:

Revision No.	Date	Project No.	Description
0	9.24.19	3Ø3	Original Drawing
1	9.30.19	303R	Revised per brick loading
2	1.28.20	318	Added optional 2nd floor
3	1.31.20	318R	Added load-bearing wall in basement
4	2.7.20	318R2	Updated per new truss drawings
5	6.29.21	3832.TØØ34	Added LIB Bracing Options
6	8.23.22	TØØ34	Revised Garage Header Sizes
7	11.15.22	3832.70041	Updated Garage opening on Bracing Plans

BRIARWOOD LOT 27



SUMMIT
ENGINEERING • LABORATORY • TESTING
A Universal Engineering Sciences Company
2520 Whitehall Park Dr. Suite 250
Charlotte, NC 28273
Office: 704.504.1717
Fax: 704.504.1125



Coversheet CLIENT Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 21539

CURRENT DRAWING

elfair LH

DATE: 11/15/2022

SCALE: 1/8"=1'-0"

PROJECT \*: 3832.T0041

DRAWN BY: EO

CHECKED BY: HKM

ORIGINAL DRAWING

**DATE PROJECT •** 09/24/2019 3832303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SUFFT

CS<sub>1</sub>

### GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, INC. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents should any non-conformities occur.
- Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code
- This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

### FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones above 130mph)
- Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- 3. Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below arade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- 9. Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- 10. Crawl spaced to be graded level and clear of all debris
- Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

## CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned mixed and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings"
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
  - Footings: 5% 32. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (WWF.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of WWF.

## CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- 4. Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry standard
- Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- . Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

## WOOD FRAMING:

- 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
  - E = 1,900,000 psi Fb = 2600 psi
  - 2.3. Fv = 285 bsi
  - 24 Fc = 700 psi

with AWPA standard C-2

- 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other maisture exposed wood shall be treated in accordance
- Nails shall be common wire nails unless otherwise noted
- 5. Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted
- Exterior and load bearing stud walls are to be 2x4 SPF#2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one 10d nail \$6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- 9. Multi-ply beams shall have each ply attached wth (3)10d nails a 24" OC
- 10. Flitch beams and four and five bly beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

## WOOD TRUSSES

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design" Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be ber the manufacturer.

### WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards
- All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure I or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

## STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

## EXTERIOR WOOD FRAMED DECKS:

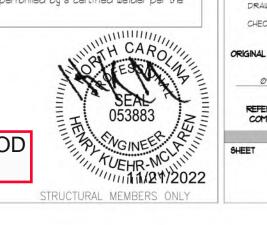
Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

### STRUCTURAL STEEL:

BRIARWOOD

LOT 27

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance" Factor Design" latest editions.
- All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shopt and field welding shall be class ETOXX. All welding shall be performed by a certified welder per the above standards.



A Universal Engineering Sciences Compa 2520 Whitehall Park Dr. Suite 250 Charlotte, NC 28273 Office: 704.504.1717 Fax: 704.504.1125 www.summit-companies.com

SUMMIT REngineering, Laboratory NO. F.1454

No. F.1454

OF AUTHURAN OF AUTHURA

8 <u>o</u> oz Oz Homes 0 o A o glas ance 2753 Coversheet Dougl Relian Smith 2520

CURRENT DRAWING

elfair

DATE: 11/15/2022

SCALE: 1/8"=1'-0"

PROJECT \*: 3832,70041

DRAWN BY: EO

CHECKED BY: HKM

ORIGINAL DRAWING

DATE PROJECT \*

09/24/2019 3832303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

CS2

### FOUNDATION NOTES

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- 2009 NORTH CARCLING RESIDENTIAL SUILDING BODG SITH RELEASED.
  STRUCTURAL CONCRETE TO BE F<sub>c</sub> = 3000 PSI, PREPARED AND PLACED IN
  ACCORDANCE UITH ACI STANDARD 318.
  FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12<sup>th</sup>
  BELOU ADJACENT FINISHED GRADE, OR AS OTHERUSE DIRECTED BY THE
  CODE ENFORCEMENT OFFICIAL.
  FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000
  PSF CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFTING THE SUITABILITY
  OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
- OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
  FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE
  ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF

- ELEMENTS PROVIDE 2" MINIMUM POOTING PROJECTION PROVI THE FACE OF MASONSY.

  6. MAXIMUM DEPTH OF UNBALLANCED FILL AGAINST MASONSY WALLS TO BE AS SPECIFIED IN SECTION REVOLD FILL AGAINST MASONSY WALLS TO BE AS SPECIFIED IN SECTION REVOLD FILE 20% NORTH CAROLINA RESIDENTIAL BUILDING CODE.

  7. PROVIDER FOUNDATION WATER-PROCRING, AND DRAIN WITH POSITIVE SLOPE TO CUTLET AS REQUIRED BY SITE CONDITIONS.

  9. PROVIDED PERIPETER NISULATION FOR ALL FOUNDATIONS FER 20% NORTH CAROLINA RESIDENTIAL BUILDING CODE.

  10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENERS.

  11. CRAWL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.

  12. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED FER THE 20% NORTH CAROLINA RESIDENTIAL CODE SECTION RASSIS, MINIMUM 12" DIA BOLTS SPACED AT 6-6" OR CHITTE WITH A "INMIMIM PEDED" FILE THE 20% NORTH CAROLINA RESIDENTIAL CODE SECTION RASSIS, MINIMUM 12" DIA BOLTS SPACED AT 6-6" OR CHITTE WITH A "I'MINIMIM PEDED" OF EACH PLATE SECTION MINIMIM 12" DIA BOLTS OR CONCRETE. ANCHOR BOLTS SHALL BE 10" FROM THE END OF EACH PLATE SECTION MINIMIM 12" DOLTS OF EA SECTION MINIMIM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- ABBREVIATIONS:

- DJ DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE SJ + SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = OR CENTER PL + PONT LOAD
- 14. ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x16"
- ALL PIERS TO DE IS "NIS" MASONEY AND ALL PILASTERS TO DE S"NIS" MASONEY, TYPICAL (UND)

  WALL PROTINGS TO DE CONTINUIS CONCRETE, SIZES PER STRUCTURAL PLAN.

  A FOUNDATION EXCAVATION DESERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENSINEER OR HIS GUAL FIED BY A PROFESSIONAL GEOTECHNICAL ENSINEER OR HIS GUAL FIED BY A POTENTIALLY EXPLANSIVE SOILS AREA OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUPPRIT ENGINEERING, LABORATORY 1 TESTINS, INC. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN FRIOR TO CONCRETE PLACEMENT.

  ALL FOOTINGS I SLABS ARE TO BEAR ON WORDSTRUCTION OF SSA COMPACTED FILL, VERTIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS. ADDITIONAL INFO. PER SECTION R602104 AND FIGURE R602103(4) OF THE 2018 NCRC.

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UND

THESE PLANS ARE DESIGNED IN ACCORD ANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED/REVISED ON 8/07/2022. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SMITH TENSIFIERING, LABORATORY (TESTING, INC., FANY CHANGES ARE HADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SUMMIT EXCHANGE THE APEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS WERD WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

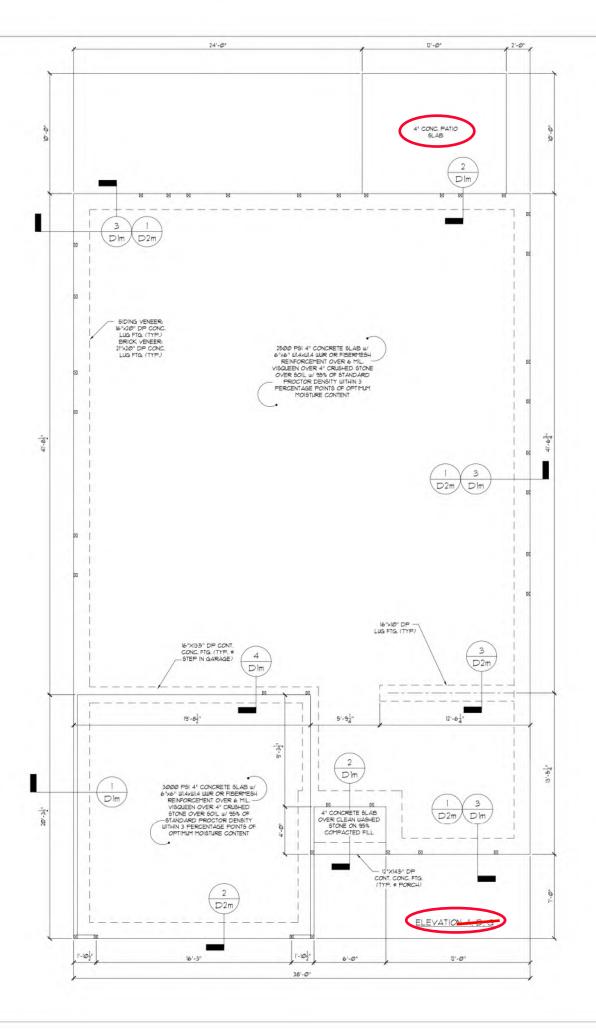
NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSFIED AS GROUP I PER TABLE R405.1

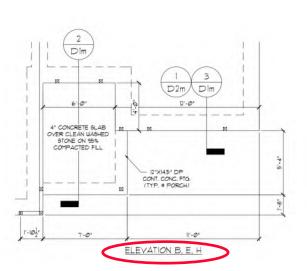
## STRUCTURAL MEMBERS ONLY

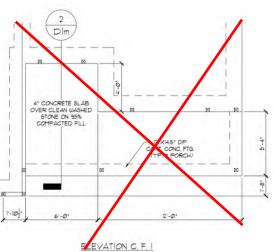
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

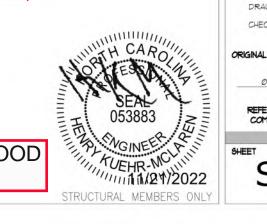
MONOLITHIC SLAB FOUNDATION







BRIARWOOD LOT 27





2520 Whitehall Park Dr. Suite 250 Charlotte, NC 28273 Office: 704.504.1717 Fax: 704.504.1125 www.summit-companies.com



8 <u>o</u> oz Oz Fnd Homes Ave Slab Douglas H Reliance Monolithic elfair Apex, Smith 1 2520

CURRENT DRAWING

DATE: 11/15/2022

SCALE: 1/8"=1'-0"

PROJECT \*: 3832.T0041

DRAWN BY: EO

CHECKED BY: HKM

ORIGINAL DRAWING

DATE PROJECT .

09/24/2019 3832.303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S1.0m

### GENERAL STRUCTURAL NOTES:

- GENERAL STRUCTURAL NOTES:

  1. CONSTRUCTION SHALL CONFORM TO 20% NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL APENDMENTS.

  2. CONTRACTOR SHALL NEWERY ALL DIMENSIONS CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. SHAINERS IS NOT RESPONSIBLE FOR ANY DEVIATIONS PROFILE PROGRAMS PROJECT SHAINERS IS NOT TRESPONSIBLE FOR PROVIDING TEPPORART BRACING REQUIRED TO RESIST ALL POWER SENDANTIESED DEVIALS EXPECTION.

  4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS. MICROLAM (LIVL.) S. 2600 PSI (F. v. 285 PSI (E. v. 1540<sup>6</sup> PSI PARALLAM (PSI), F. p. 2500 PSI (F. v. 285 PSI (E. v. 1540<sup>6</sup> PSI PARALLAM (PSI), F. p. 2500 PSI (F. v. 285 PSI (E. v. 1540<sup>6</sup> PSI PARALLAM (PSI), F. p. 2500 PSI (F. v. 285 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI ALL DEVIA PSI (E. V. 1550 PSI (E. v. 1540<sup>6</sup> PSI PSI (E. V. 15

- SJ = SNGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER PL = POINT LOAD DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST

CL : CENTER LINE

NOTE: NOTE:
DESIGNATES JOIST SUPPORTED LOAD
BEARING WALL ABOVE PROVIDE BLOCKING UNDER
JOIST SUPPORTED LOAD BEARING WALL.

NOTE: SHADED WALLS INDICATE LOAD BEARING WALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS,

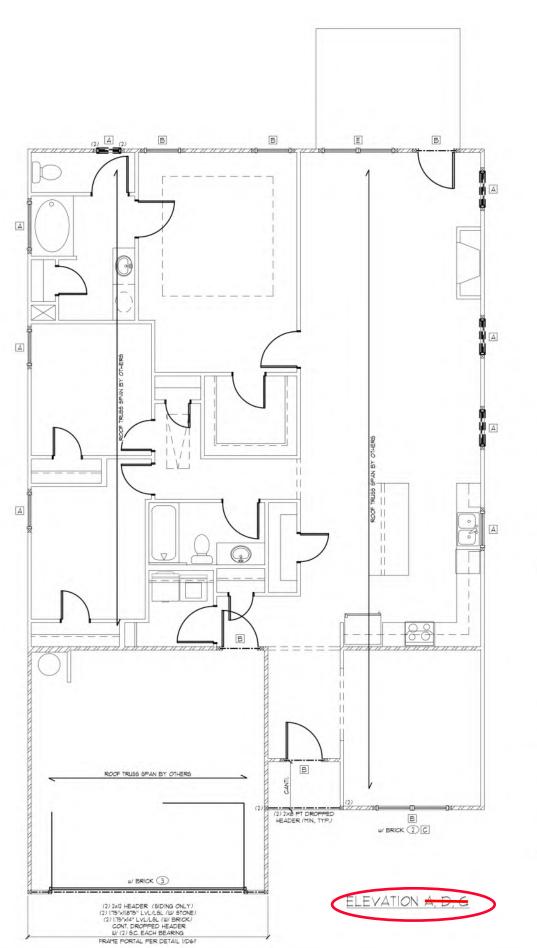
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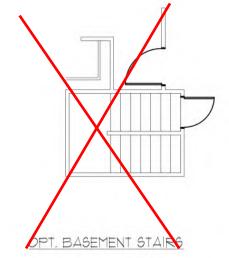
## STRUCTURAL MEMBERS ONLY

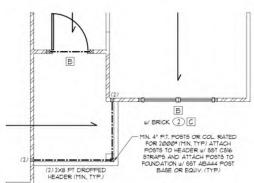
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

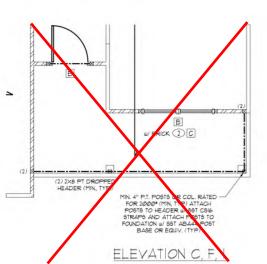
FIRST FLOOR FRAMING PLAN SCALE: 1/8"=1"







EVATION B, E, H



HEADER TAG	BEAM TAG	SIZE	JACKS (EACH END
	BI	(I) I4" FLOOR JOIST	(2)
	B2	(2) I4" FLOOR JOIST	(2)
A	B3	(2) 2x6	(1)
В	B4	(2) 2x8	(2)
С	B5	(2) 2xlØ	(2)
D	B6	(2) 2x12	(2)
E	B1	(2) 9-1/4" LVL	(3)
F	B8	(2) II-T/8" LVL	(3)
G	B9	(2) 14" LVL	(3)
H	BIØ	(2) 16" LVL	(3)
4	BII	(2) 18" LVL	(3)
J	BI2	(2) 24" LVL	(4)
K	B13	(3) 9-1/4" LVL	(3)
L	BI4	(3) II-7/8" LVL	(3)
М	Bl6	(3)14" LVL	(3)
N	вп	(3) 16" LVL	(3)
0	Bi8	(3) 18" LVL	(3)
P	B/9	(3) 24" LVL	(4)

ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. ALL BEAMS TO BE FLUSH UNLESS NOTED OTHERWISE.

LI	NTEL SCHEE	DULE
TAG	SIZE	OPENING SIZE
0	L3x3x1/4"	LESS THAN
2	L5x3x1/4"	6'-0" TO 10'-0"
3	L5×3-1/2"×5/16"	GREATER THAN
4	L5x3-l/2"x5/l6" ROLLED OR	ALL ARCHED OPENINGS

SECURE LINTEL TO HEADER #/(2) 1/2"
DIAMETER LAG SCREUS STAGGERED # 16"
OC. (TYP FOR 3)

ALL HEADERS WITH BRICK ABOVE: (UNO)

IIIALL STUD SCHEDULE | 15 1 200 FLOOR LOAD BEARING WALLS: 2x6 STUDS = 24" O.C. OR 2x4 STUDS = 16" O.C. | 15T FLOOR LOAD BEARING WALLS SUPPORTING 2X0 FLOOR LIGHTS = 16" O.C. OR 2x4 STUDS = 2" O.C. 2x6 STUDS = 16" O.C. OR 2x4 STUDS = 2" O.C. BASEMENT LOAD BEARING WALLS: 2x6 STUDS \$ 16" O.C. OR 2x4 STUDS \$ 12" O.C. NON-LOAD BEARING WALLS (ALL FLOORS): 2x4 STUDS # 24" O.C. TWO STORY WALLS: 2x4 STUDS = 12" O.C. OR 2x6 STUDS = 16" O.C. W/2x BRACING = 6"-0" O.C. VERTICALLY (AKA "BALLOON FRAMING")

KING STUE	REQUIR	REMENTS
OPENING WIDTH	KINGS (E	4CH END?
(FT)	16" O.C.	24" O.C.
LESS THAN 3'-Ø"	(D)	(1).
3'-0 TO 4'-0"	(2)	(1)
4'-0" TO 8'-0"	(3)	(2)
8'-0" TO 12'-0"	(5)	(3)
12'-0" TO 16'-0"	(6)	(4)



BRIARWOOD LOT 27





Charlotte, NC 28273 Office: 704.504.1717 Fax: 704.504.1125 www.summit-companies.com

SUMMIT

CE Engineering, Laboratory

& Testing, Inc.

No. F-1454

OF AUTHORITIAN

OF AUTHORITIA

8 <u>ø</u> <u>w</u> Homes Ave Framing Douglas H Reliance Floor 土 elfair Apex, Smith 1 2520 First

CURRENT DRAWING

DATE: 11/15/2022

SCALE: 1/8"=1'-0"

PROJECT \*: 3832.T0041

DRAWN BY: EO

CHECKED BY: HKM

ORIGINAL DRAWING

DATE PROJECT . 09/24/2019 3832.303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

S3.0

MAX, UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO PNE
535 LBS	H2.5A	PER WALL SHEATHIN	G 4 FASTENERS
1010 LB6	(2) H2.5A	CSI6 (END = 13*)	DTT2Z
1245 LB6	HT82Ø	CSI6 (END = 13*)	DTT2Z
112Ø LBS	(2) MTS2Ø	(2) CSI6 (END = 13")	DTT2Z
249Ø LB6	(2) HT92Ø	(2) C8(6 (END + 13")	HTT4
2365 LB6	LGT3-5D525	(2) C5I6 (END = I3")	HTT4

I. ALL PRODUCTS LISTED ARE SMIPSON STRONG-TIE, EQUIVALENT PRODUCTS MAY BE USED PER HAND-ACTURER'S SPECIFICATIONS.

2. UPILIT VALUES LISTED ARE FOR SPE? GAGADE MEMBERS.

3. REFER TO TRUSS LAYOUT PER HANUF, FOR UPILIT VALUES AND TRUSS TO TRUSS CONSICTIONS, OWNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.

4. CONTACT SIMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 19T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION RESULT. WALL SHEATHING AND FASTENES HAVE BEEN DEBIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION RESULTS OF THE 2018 NOCE, CREER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

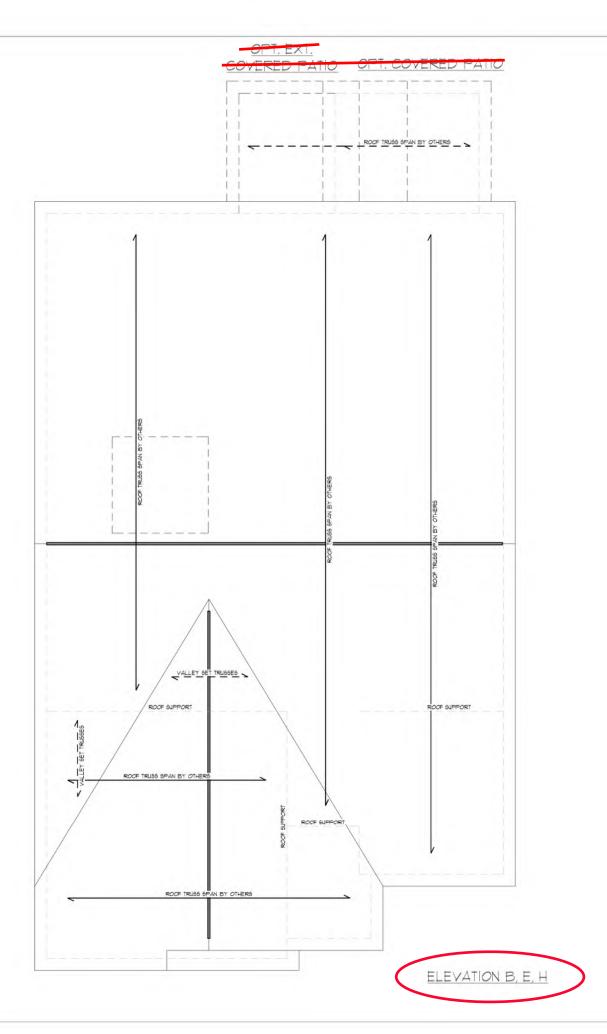
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN SCALE: 1/8" : 1"





Fax: 704.504.1125 www.summit-companies.com

SUMMIT

CE Engineering, Laboratory

& Testing, Inc.

No. F-1454

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OF AUTHORITIA

Raleigh Douglas Homes . Reliance Ave x, NC 21539 Framing Plan 土 Felfair Smith D 2520 R Apex, Roof I

## CURRENT DRAWING

DATE: 11/15/2022

BRIARWOOD

LOT 27

0530 WGINEER TO THE TO THE TOTAL MEMBERS OF

SCALE: 1/8"=1'-0"

PROJECT \*: 3832.T0041

DRAWN BY: EO

CHECKED BY: HKM

SEAL NAME OF THE PARTY OF THE P ORIGINAL DRAWING

DATE PROJECT .

09/24/2019 3832.303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

S5.1

market common	MN	MIN REQUIRED CONNECTION		
METHOD	MATERIAL	THICKNESS	© PANEL EDGES	NTERMEDIATE SUPPORTS
CS-USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS: 6 6" O.C.	6d COMMON NAILS: # 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** 6 1' O.C.	5d COOLER NAILS** 6 7' O.C.
USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS: 6 6 O.C.	6d COMMON NAILS+ # 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602J06.4	PER FIGURE R602J06.4

- 1) WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REGULØ FROM THE 2005 NITERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION REGULØ OF THE 2004 OR RESIDENTIAL CODE.

  2) WALLS ARE DESIGNED FOR SEISMIC ZONES ALC AND MAXIMUM WIND SPEEDS OF

- THE 1009 NC RESIDENTIAL CODE:

  JUNLIA SARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMM WIND SPEEDS OF
  130 MPH.

  BRACINS MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH
  IRC TABLE 860/10/4.

  NEFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OFFINING SIZES.

  ALL BRACED WALL PARELS SHALL BE FULL WALL HEIGHT AND SHALL NOT
  EXCEED IO FEET FOR ISOLATED PAREL METHOD AND IS FEET FOR CONTINUOUS
  SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

  MINIMUM PANEL LENGTH SHALL BE FOR TABLE REGIONS.

  THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS
  SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 10° GYPSIM BOARD (UND).

  FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
  ALL SHEATHABLE SHAPAGES INCLUDING INFILL AREAS BETWEEN BRACED WALL
  PAIRELS, ABOVE AND BELOW WALL OFFINISS, AND ON GABLE BND WALLS.

  PLOORS SHALL NOT BE CANTILLEVISTED MORE THAN 21° BETWORN FOR ROUND ATON
  OR BEARNING WALL BELOW WITHOUT ADDITIONAL ENSINEERING CALCULATIONS
  OF BEARNING WALL BELOW BEFORE WALL SHALL BE CORPORATED WHITH FIGHT REPORTS THE PROPRIED OF A
  BRACED WALL LINE COMPLY WITH RESECTION REPORTS.

  MASONRY OR CONCRETE SITH WALLS WITH A LENGTH OF AS' OR LESS
  SUFPORTING BY AND CONTROL WITH SECTION REPORTS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REPORTS FALL BE DESIGNED IN ACCORDANCE WITH SECTION REPORTS FALLE BETWEEN BEAUTING THAT BE PROPORTIAL PROCKAGES

  BRACED WALL PARKEL CONNECTIONS TO PROOF SHALL BE CONTRUCTED IN
  ACCORDANCE WITH SECTION REPORTS OF HALL BE CONTRUCTED IN
  ACCORDANCE WITH SECTION REPORTS OF SHALL BE CONTRUCTED IN
  ACCORDANCE WITH SECTION REPORTS.

(INO)

IO) ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

ID) ABBREVIATIONS.

GG & GTFSUM BOARD

CS - KOTSUM BOARD

CS - KOTSUM SHEATHED

FF = PORTIAL FRAME

FF = INDIREREED SOLUTION

BY SHADE STREET PORTIAL

STATE STATE STREET PORTIAL

STA W6P = WOOD STRUCTURAL PANEL ENG = ENGINEERED SOLUTION ENG-PF = ENGINEERED PORTAL FRAME

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED/REVISED ON BIOLOGY. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SWITHING THE PROVIDED ARE HADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SWITHIT ENGINEERING, LABORTORY I TESTIN, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS HADE TO CONSTRUCT SWITHING INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

CON	ITINUOUS SHEATHING ME	THOD
	REQUIRED	PROVIDED
BUL, 1-1	10.4	16.8
BUL 1-2	10,4	11,9
BWL 1-3	6.6	11.6
BUL 1-A	1.4	41/
BUL I-B	7.4	52.0

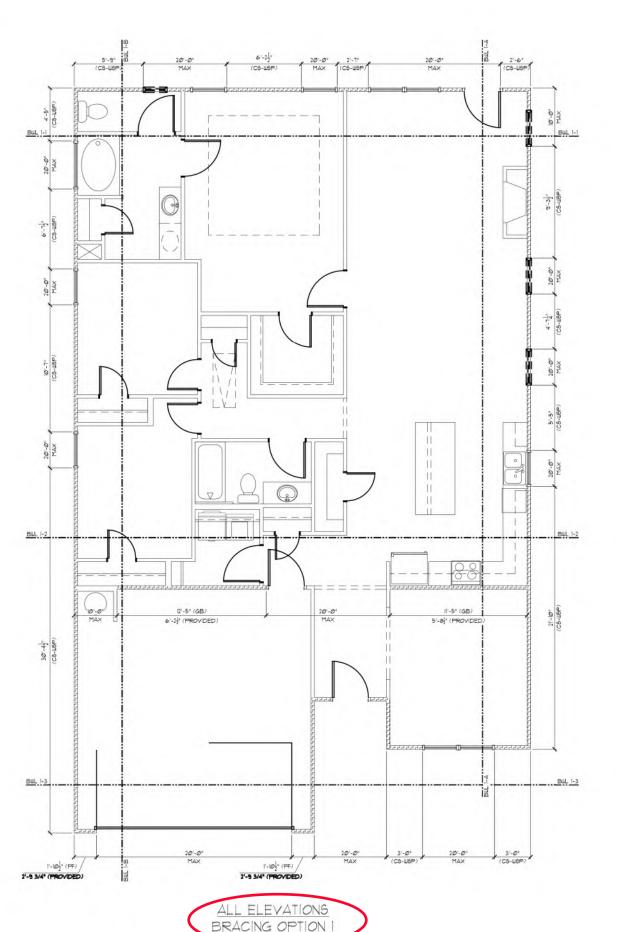
INSTALL ANY REQUIRED HOLDOWNS PER SECTION. R602.108 AND FIGURE R602.10.1 OF THE 2015 IRC.

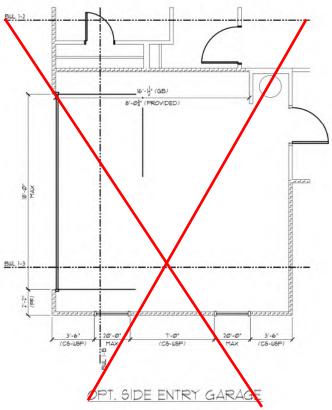
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BE BROUGHT TO THE IMMEDIATE ATTENTION OF SHIMMIT ENGINEERING LABORATORY & TESTING INC. FAILURE TO DO SO WILL VOID SUMMIT ELT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2015 IRC.

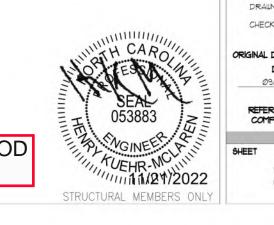
FIRST FLOOR BRACING PLAN SCALE: 1/8" : 1"





CON	TINUOUS SHEATHING ME	THOD
	REQUIRED	PROVIDED
BWL I-I	10.4	16.8
BWL 1-2	10.4	13.7
BUL 1-3	6.6	20.0
BWL I-A	1.4	41.1
BUL 1-B	1.4	36.9

BRIARWOOD LOT 27





2520 Whitehall Park Dr. Suite 250 Charlotte, NC 28273 Office: 704.504.1717 Fax: 704.504.1125 www.summit-companies.com



8 <u>o</u> <u>w</u> Homes Ave Bracing Douglas H Reliance Floor elfair Apex, Smith 1 2520 First

CURRENT DRAWING

DATE: 11/15/2022

SCALE: 1/8"=1'-@"

PROJECT \*: 3832.T0041

DRAWN BY: EO

CHECKED BY: HKM

ORIGINAL DRAWING

DATE PROJECT .

09/24/2019 3832.303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

S7.0

recorded formation	Contraction of	MN	REGUIRED CONNECTION			
METHOD	MATERIAL	THICKNESS			NTERMEDIATE SUPPORTS	
CS-USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS: 6 6" O.C.	6d COMMON NAILS* 9 12" O.C.		
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** 6 1' O.C.	5d COOLER NAILS** • 1' O.C.		
USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS: 6 6 O.C.	6d COMMON NAILS* # 12" O.C.		
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.06.4	PER FIGURE R602J06.4		

- 1) WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REGULØ FROM THE 2005 NITERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION REGULØ OF THE 2004 OR RESIDENTIAL CODE.

  2) WALLS ARE DESIGNED FOR SEISMIC ZONES ALC AND MAXIMUM WIND SPEEDS OF

- THE 1009 NC RESIDENTIAL CODE:

  JUNLIA SARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMM WIND SPEEDS OF
  130 MPH.

  BRACINS MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH
  IRC TABLE 860/10/4.

  NEFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OFFINING SIZES.

  ALL BRACED WALL PARELS SHALL BE FULL WALL HEIGHT AND SHALL NOT
  EXCEED IO FEET FOR ISOLATED PAREL METHOD AND IS FEET FOR CONTINUOUS
  SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

  MINIMUM PANEL LENGTH SHALL BE FOR TABLE REGIONS.

  THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS
  SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 10° GYPSIM BOARD (UND).

  FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
  ALL SHEATHABLE SHAPAGES INCLUDING INFILL AREAS BETWEEN BRACED WALL
  PAIRELS, ABOVE AND BELOW WALL OFFINISS, AND ON GABLE BND WALLS.

  PLOORS SHALL NOT BE CANTILLEVISED MORE THAN 20° BEYOND REACHED WALL
  PAIRELS, ABOVE AND BELOW WALL OFFINISS, AND ON GABLE BND WALLS.

  A BRACED WALL PERLOW WITHOUT ADDITIONAL ENSINEERING CALCULATIONS
  OF BEARNING WALL BELOW WITHOUT ADDITIONAL ENSINEERING CALCULATIONS
  OF BEARNING WALL BELOW WITHOUT ROPINISMS OF BRACED WALL FANDED

  THE DISTANCE BETWEEN ADJACENT EDGES OF BRACED WALL FANDES ALONG A
  BRACED WALL LANGUAGE HAVE SEEN WITHIN 10° PEET FROM EACH BND OF A
  BRACED WALL LANGUAGE HAVE SEEN WITHIN 10° PEET FROM EACH BND OF A
  BRACED WALL LANGUAGE HAVE SEEN WITHIN 10° PEET FROM EACH BND OF A
  BRACED WALL LOOPELY WITH RCS SECTION REQUISE.

  MASONRY OR CONCRETE SITH WALLS WITH A LENGTH OF 48° OR LESS
  SUFPORTING A BRACED WALL FANEL SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUISE SHEET DIS FROM DETAIL PACKAGES.

  MASONRY OR CONCRETE SITH WALLS WITH A LENGTH OF AS ORD THAT FIGURE REQUISE.

  MASONRY OR CONCRETE SITH WALLS WITH A LENGTH OF BRACED WALL FANEL SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUISE SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUISE SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUISE SHALL BE LOONECTIONS TO ROOT SHALL BE CONTRUCTED IN
  ACCORDANCE WITH SECTION REQUISE

(INO)

IO) ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

ID) ABBREVIATIONS.

GG & GTFSUM BOARD

CS - KOTSUM BOARD

CS - KOTSUM SHEATHED

FF = PORTIAL FRAME

FF = INDIREREED SOLUTION

BY SHADE STREET PORTIAL

STATE STATE STREET PORTIAL

STA W6P = WOOD STRUCTURAL PANEL ENG = ENGINEERED SOLUTION ENG-PF = ENGINEERED PORTAL FRAME

THESE PLANS ARE DESIGNED IN ACCORDINCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED/REVISED ON BIOLOGY. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SWITHING THE PROVIDED ARE HADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SWITHIT ENGINEERING, LABORTORY I TESTIN, INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS HADE TO CONSTRUCT SWITHING INC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

CON	TINUOUS SHEATHING ME	THOD
	REQUIRED	PROVIDED
BUL 1-1	10.4	16.8
BWL 1-2	10,4	11,9
BWL 1-3	6.6	11.6
BUL 1-A	7.4	16.8
BUL 1-B	7.4	11.2

INSTALL ANY REQUIRED HOLDOWNS PER SECTION. R602.108 AND FIGURE R602.10.1 OF THE 2015 IRC.

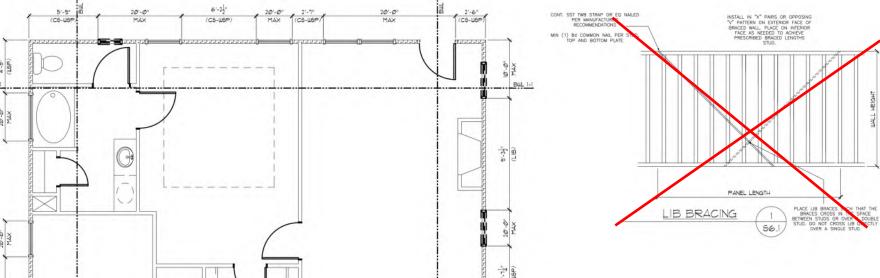
## STRUCTURAL MEMBERS ONLY

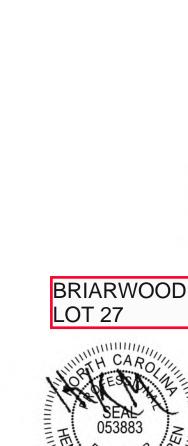
ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, INC. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR BRACING PLAN SCALE: 1/8" : 1"

SEE SHEET ST.Ø FOR NOTES AND MORE INFORMATION











8 <u>o</u> <u>8</u> Homes Ave Bracing Douglas H Reliance Floor 土 elfair Apex, First Smith 2520

## CURRENT DRAWING

DATE: 11/15/2022

SCALE: 1/8"=1'-0"

PROJECT \*: 3832.T0041

DRAWN BY: EO CHECKED BY: HKM

ORIGINAL DRAWING

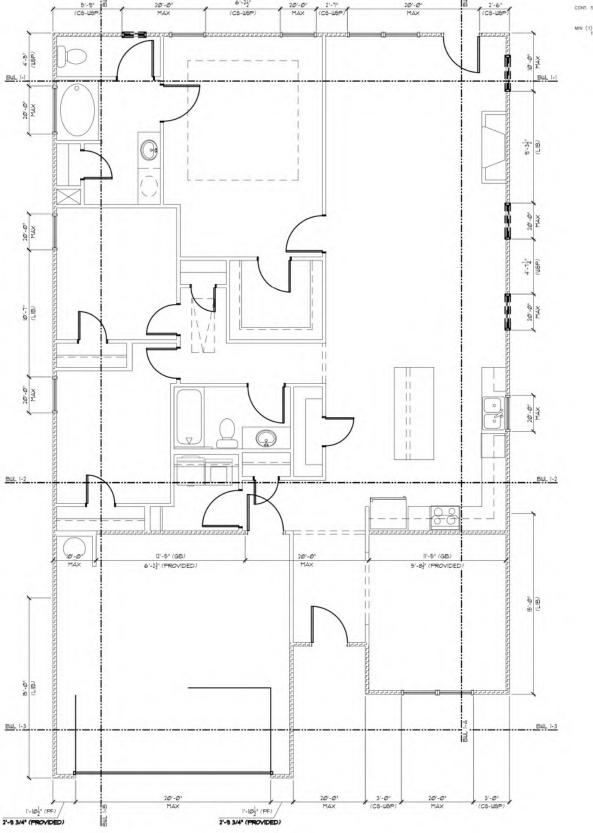
DATE PROJECT .

09/24/2019 3832.303

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

S7.1



ELEVATIONS

BRACING OPTION

### GENERAL STRUCTURAL NOTES:

- 1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, Inc. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- 7. This structure and all construction shall conform to all applicable sections of the international residential code.
- 8. This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

## FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones above 130mph)
- Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- 3. Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- 4. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- 7. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- 10. Crawl spaced to be graded level and clear of all debris
- 11. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

### CONCRETE

- 1. Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%
- 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- Concrete slabs on grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab—on—grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs—on—grade at a maximum of 15'—0" O.C. and in exterior slabs—on—grade at a maximum of 10'—0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- 9. Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (W.W.F.) for concrete slabs—on—grade shall be placed at mid—depth of slab. The W.W.F. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of W.W.F.

## CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs—on—grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- 5. Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315:
  "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

## WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
  - 2.1. E = 1,900,000 psi
  - 2.2. Fb = 2600 psi
  - 2.3. Fv = 285 psi
- 2.3. FV = 265 psi2.4. Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15.
   All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SPF#2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- 8. Individual studs forming a column shall be attached with one 10d nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached wth (3)10d nails @ 24" O.C.
- 10. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

### WOOD TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

## WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- 2. All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- 4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 5. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)—8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

## STRUCTURAL FIBERBOARD PANELS:

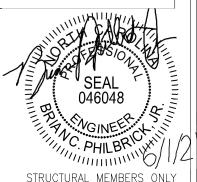
- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- 2. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- 3. Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

### EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

## STRUCTURĂL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- 2. All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shopt and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.





3070 HAMMOND BUSINESS PLACE, SUITE 171, RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



Notes and Specifications CLIENT Smith Douglas Homes 110 Village Trail, Suite 21 Woodstock, GA 30188

2

CURRENT DRAWING

Details

Standard

DATE: 4/29/2021

SCALE: NTS
PROJECT #: 3832-R2

DRAWN BY: HDK

CHECKED BY: BCP

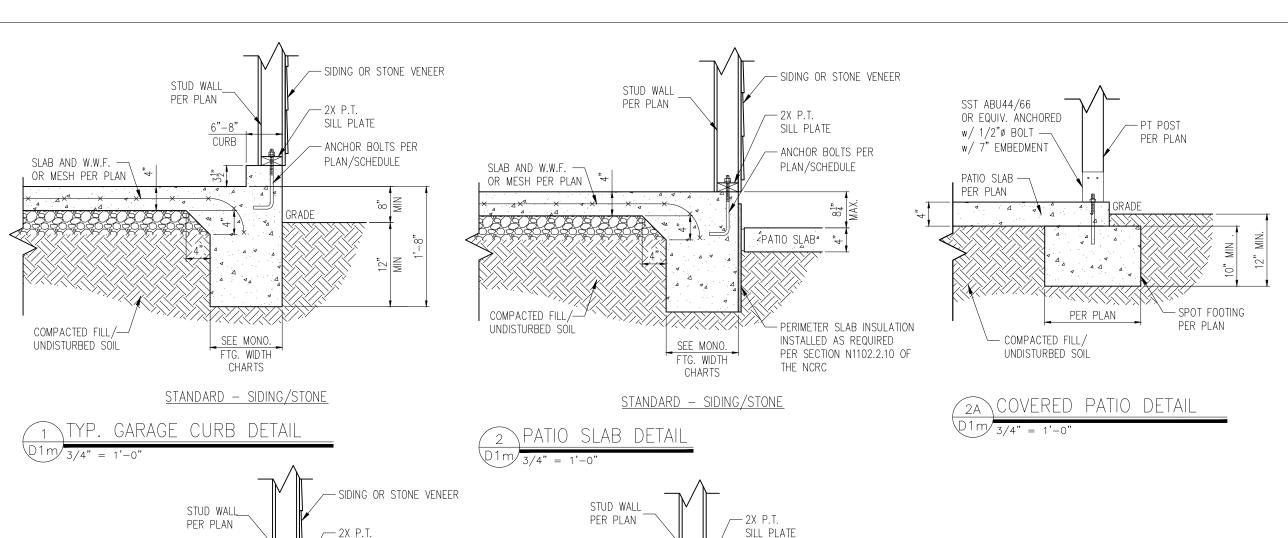
ORIGINAL DRAWING

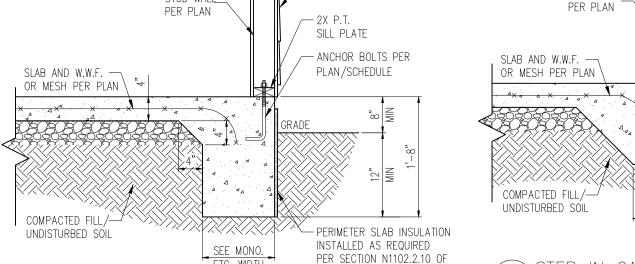
NO. DATE PROJECT #
0 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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CS2





THE NCRC



16"

## STANDARD - SIDING/STONE

FTG. WIDTH

CHARTS

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

## WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

## MONOLITHIC FOOTING WIDTH

ANCHOR BOLTS PER

PLAN/SCHEDULE

# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY				
	1500 PSF	2000 PSF	2500 PSF		
1 STORY - STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY — STD.	20"	16"	16"		
2 STORY - BRICK VENEER	25"*	21"*	21"*		
*5" BRICK LEDGE HAS BEEN A	*5" BRICK LEDGE HAS BEEN ADDED TO THE MONOLITHIC				

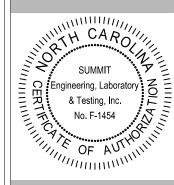
SEAL 046048

OF PHILBRUSH

OF FOOTING WIDTH FOR BRICK SUPPORT STRUCTURAL MEMBERS ONLY



SUITE 171, RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



21 Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Standard Details Monolithic

2

CURRENT DRAWING

DATE: 4/29/2021

SCALE: NTS

PROJECT #: 3832-R2

DRAWN BY: HDK

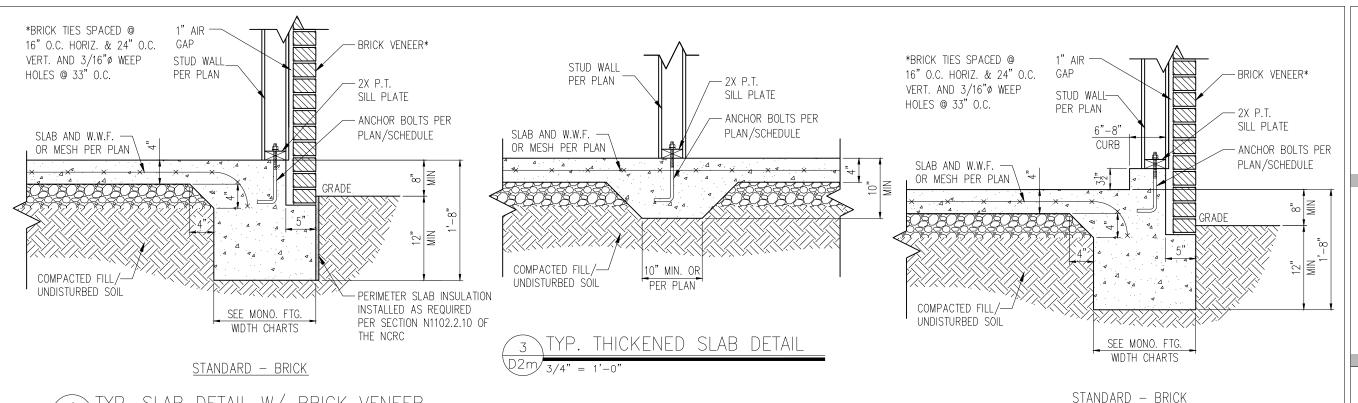
CHECKED BY: BCP

ORIGINAL DRAWING

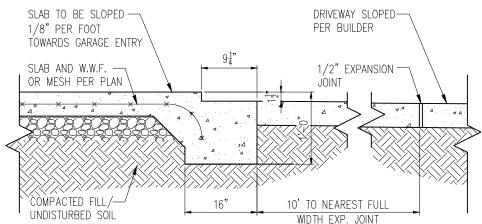
DATE PROJECT # 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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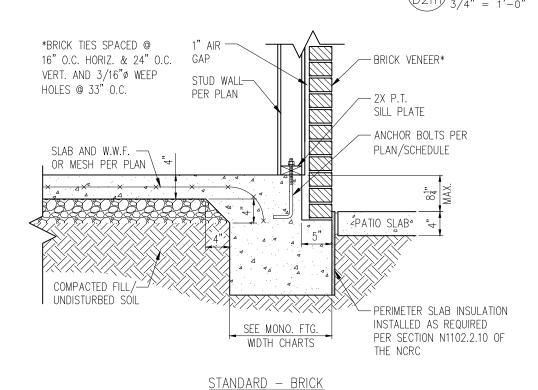
TYP. SLAB DETAIL W/ BRICK VENEER



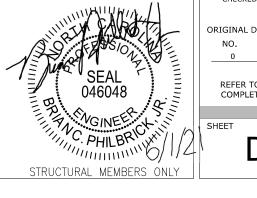




- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



PATIO SLAB DETAIL W/ BRICK VENEER



TYP. GARAGE CURB DETAIL

W/ BRICK VENEER

3070 HAMMOND BUSINESS PLACE,

SUITE 171, RALEIGH, NC 27603

OFFICE: 919.380.9991

FAX: 919.380.9993

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SUMMIT REPRESENTATION OF AUTHORITION OF AUTHORITION

2 21 Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Details Monolithic Standard

CURRENT DRAWING

DATE: 4/29/2021

SCALE: NTS

PROJECT #: 3832-R2

DRAWN BY: HDK

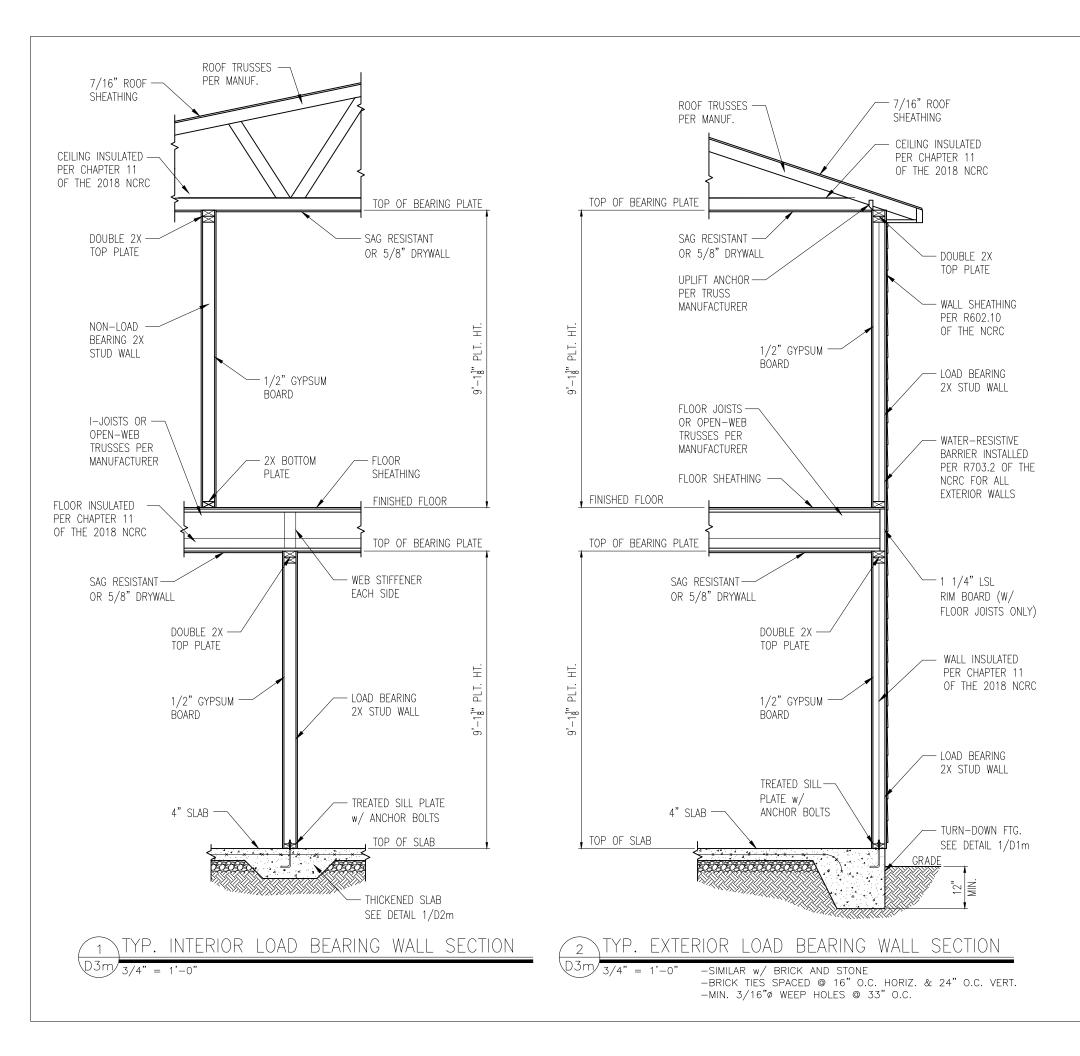
CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D<sub>2</sub>m





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SUMMIT REGISTER & Testing, Inc.
No. F-1454
OF AUTIMITIAL O

Standard Details

Monolithic Slab Details

Smith Douglas Homes
110 Village Trail, Suite 21
Woodstock, GA 30188

2

INFORMATION. CURRENT DRAWING

DATE: 4/29/2021

SCALE: NTS

PROJECT #: 3832-R2

DRAWN BY: HDK
CHECKED BY: BCP

ORIGINAL DRAWING

NO. DATE PROJECT #
0 1/7/16 3832

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SHEET

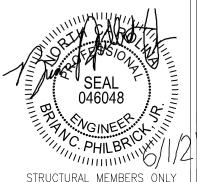
D3m

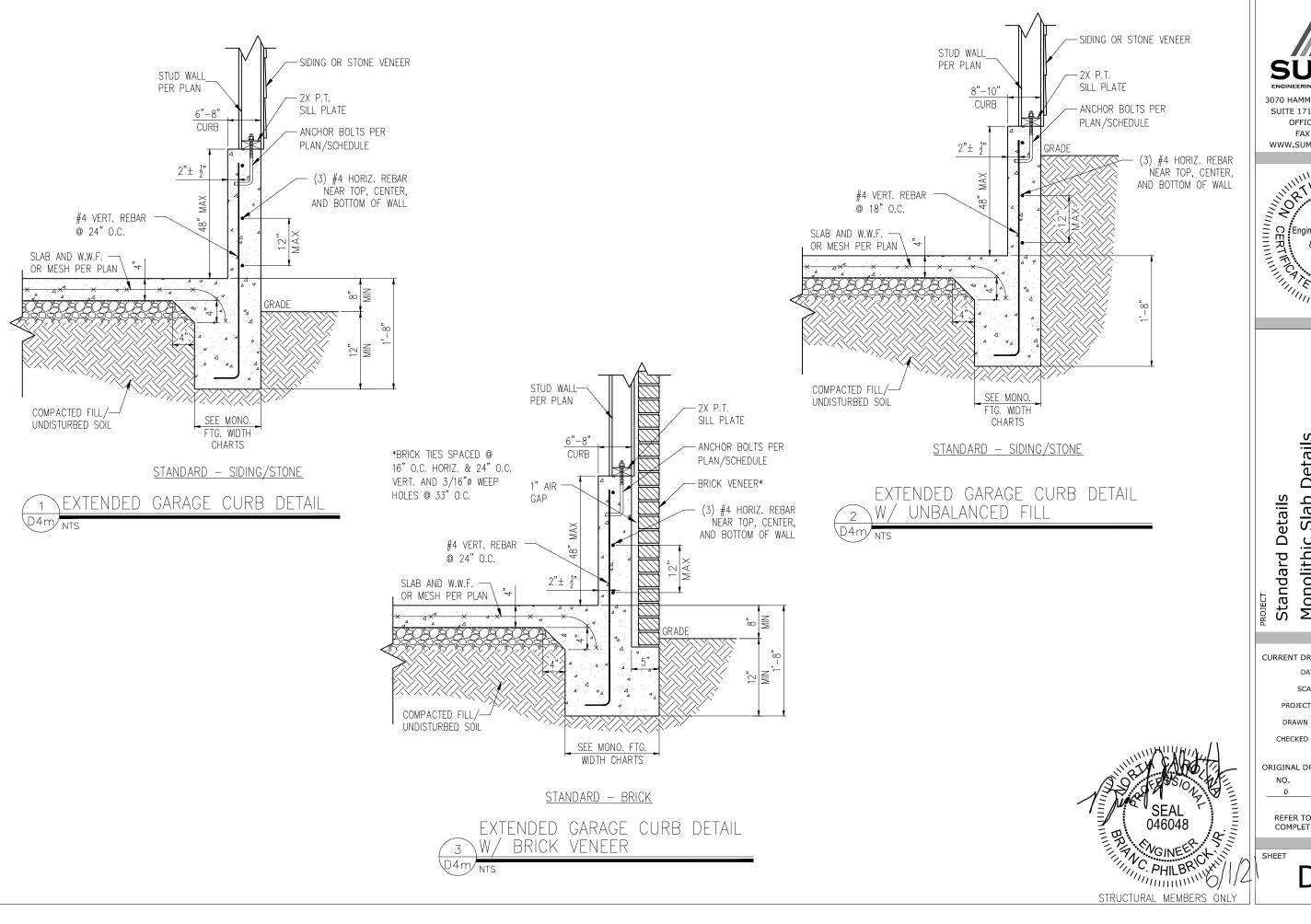
MOTES

REFER TO GENERAL NOTES &
 SPECIFICATIONS ON SHEET CS2
 FOR ADDITIONAL INFORMATION.

2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.

3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.





W/ BRICK VENEER

D4m/<sub>NTS</sub>



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21 Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Standard Details Monolithic

2

CURRENT DRAWING

DATE: 4/29/2021

SCALE: NTS

PROJECT #: 3832-R2

DRAWN BY: HDK

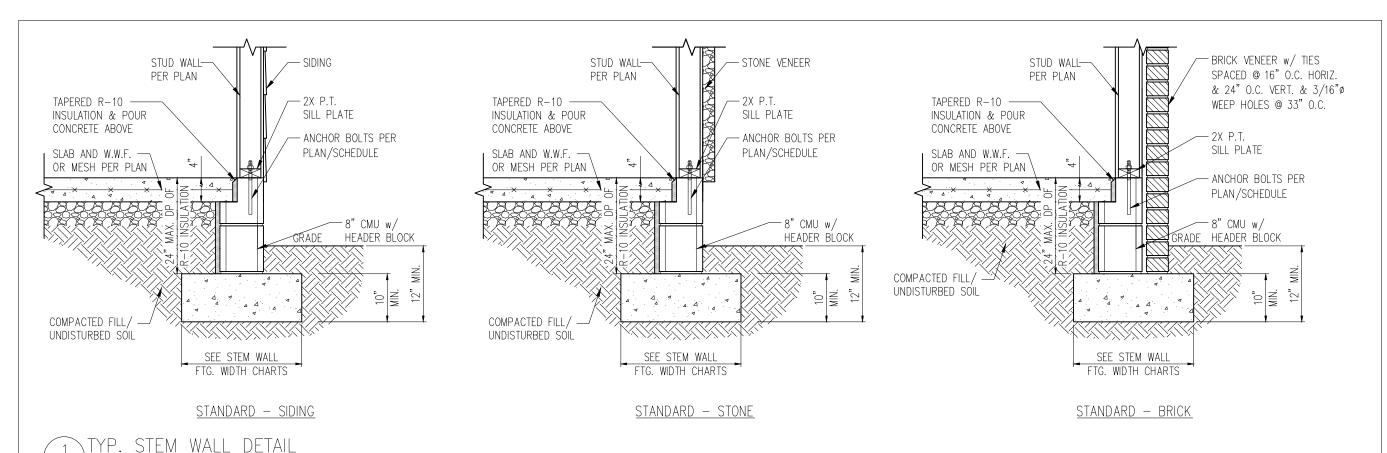
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ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

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D4m



## STEM WALL FOOTING WIDTH

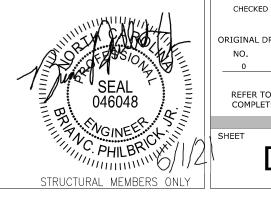
JIEM WILL FOOTING WIDTH					
# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY				
	1500 PSF 2000 PSF 2500 PSF				
1 STORY — STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY — STD.	20"	16"	16"		
2 STORY – BRICK VENEER	25"*	21"*	21"*		
*5" BRICK LEDGE HAS BEEN ADDED TO THE STEM WALL					
FOOTING WIDTH FOR BRICK S	UPPORT				

## WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
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21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Stemwall Details CLIENT
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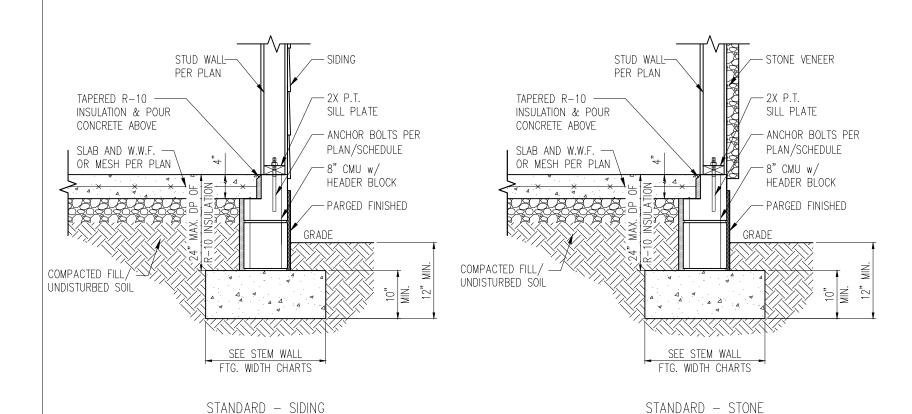
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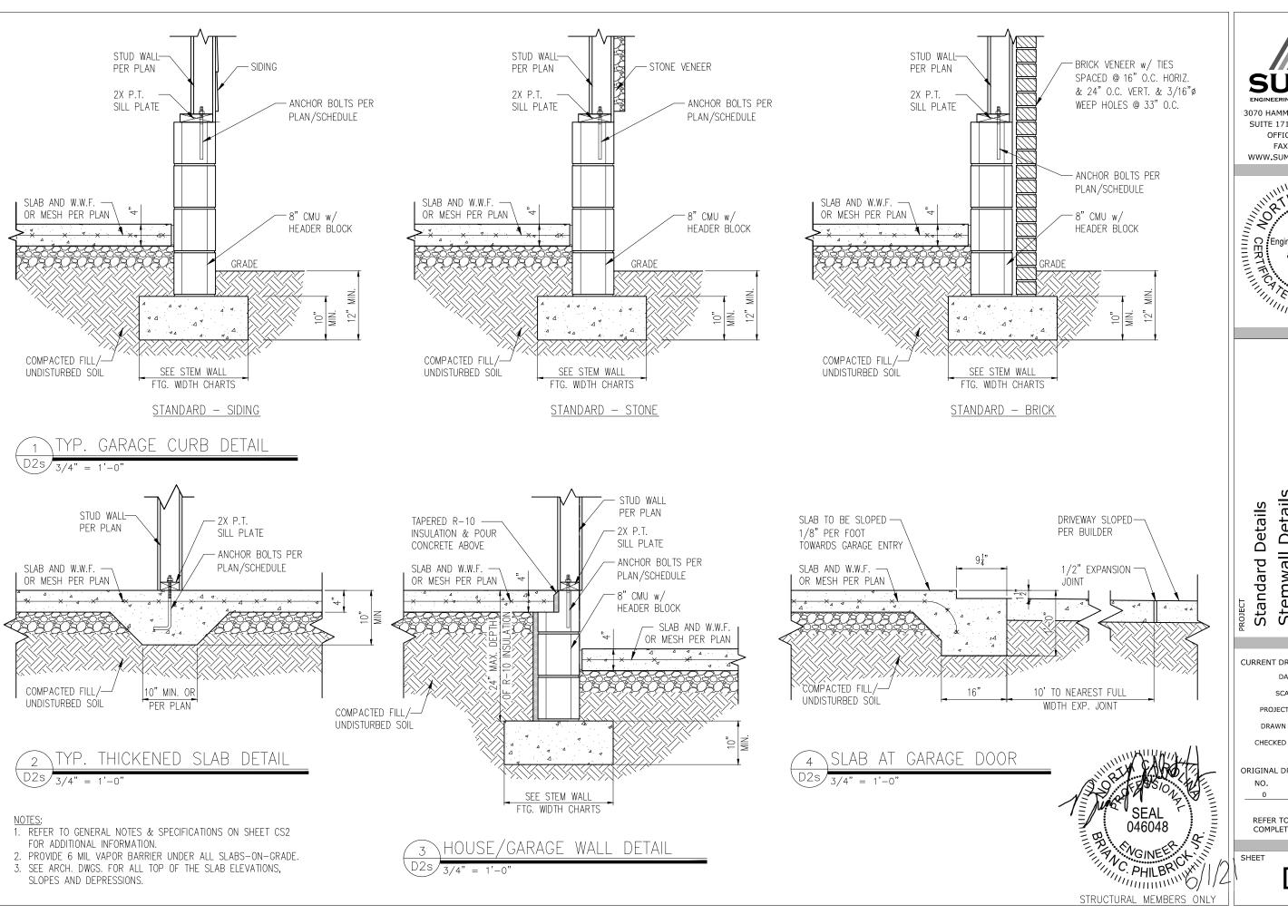
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STEM WALL DETAIL w/ PARGED FINISH



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DATE: 4/29/2021

SCALE: NTS

PROJECT #: 3832-R2 DRAWN BY: HDK

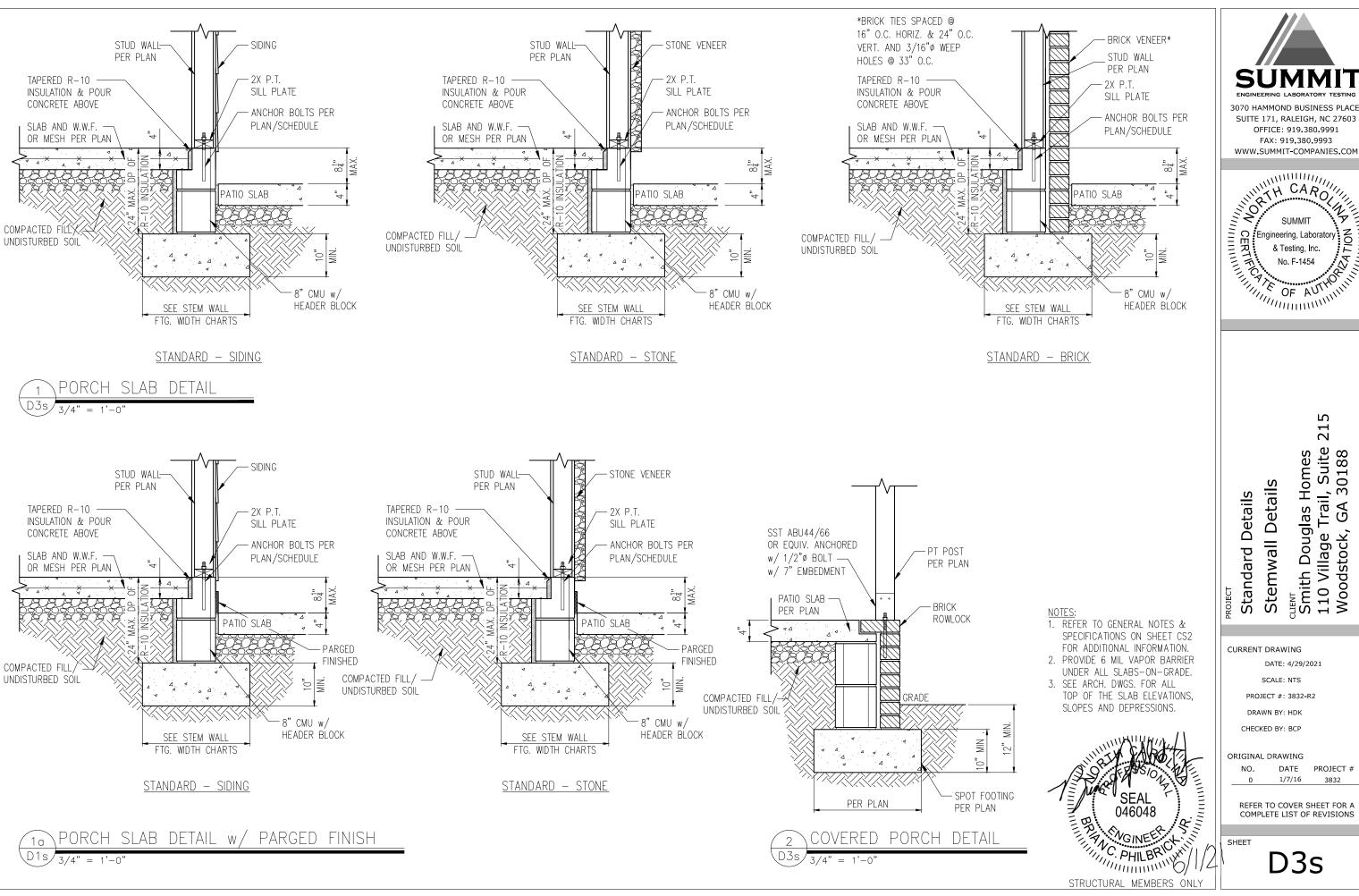
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PROJECT #: 3832-R2

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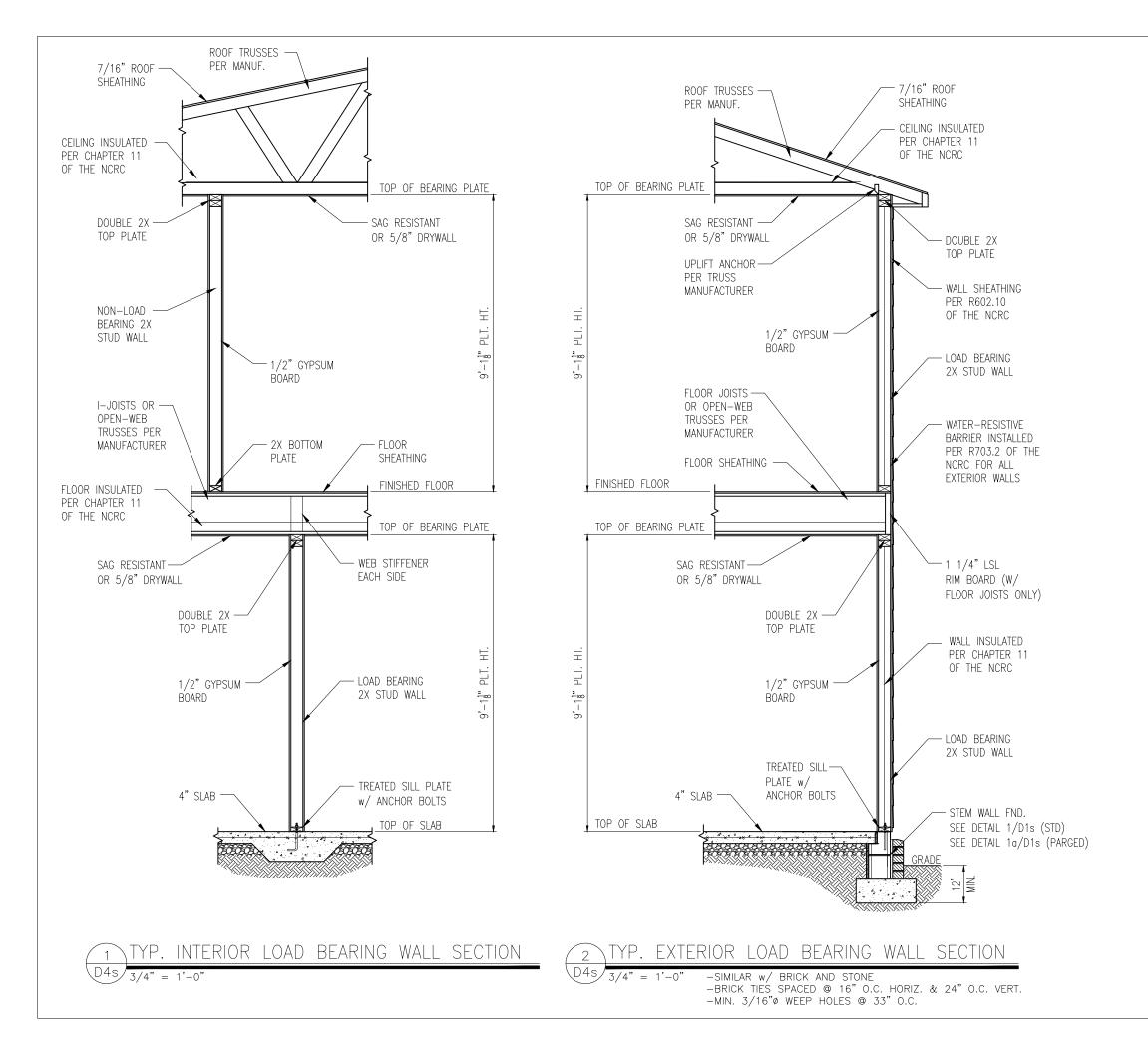
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SCALE: NTS

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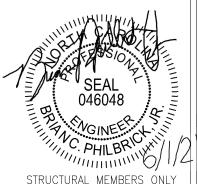
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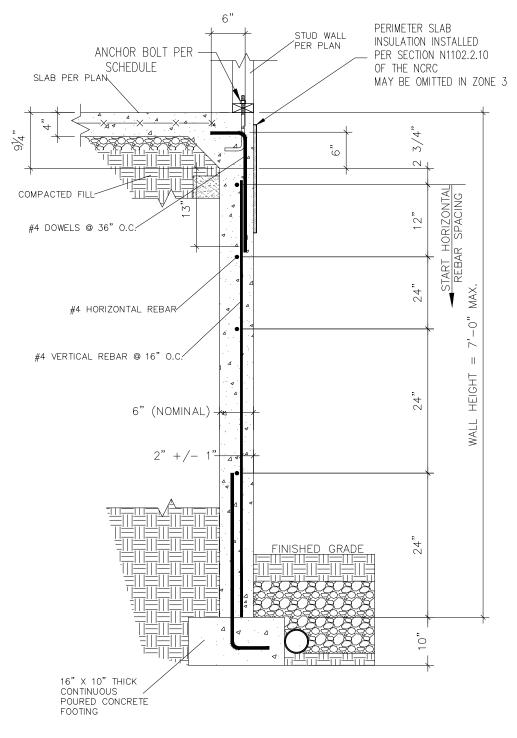
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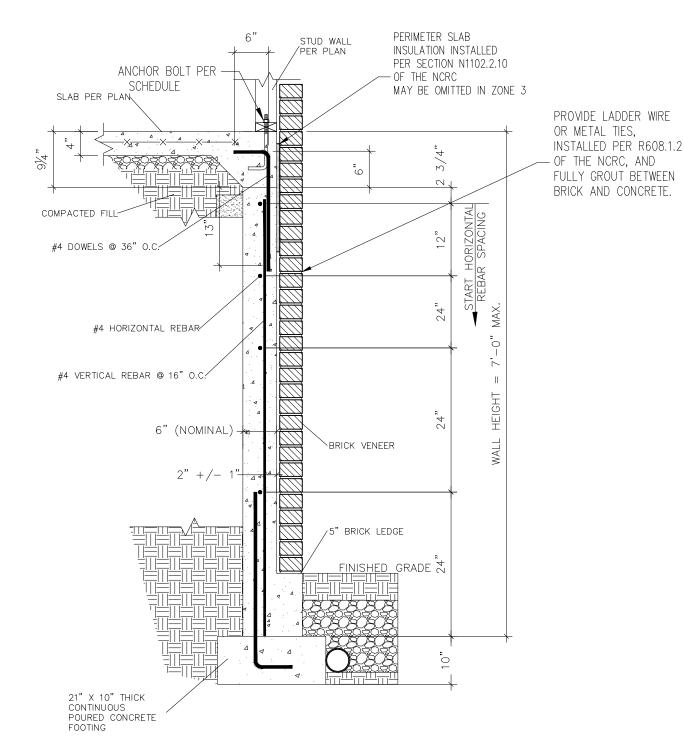
D4s

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
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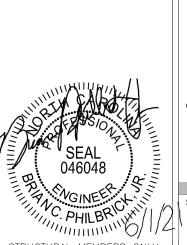






SUBWALL FOUNDATION W/ BRICK VENEER

3/4" = 1'-0"



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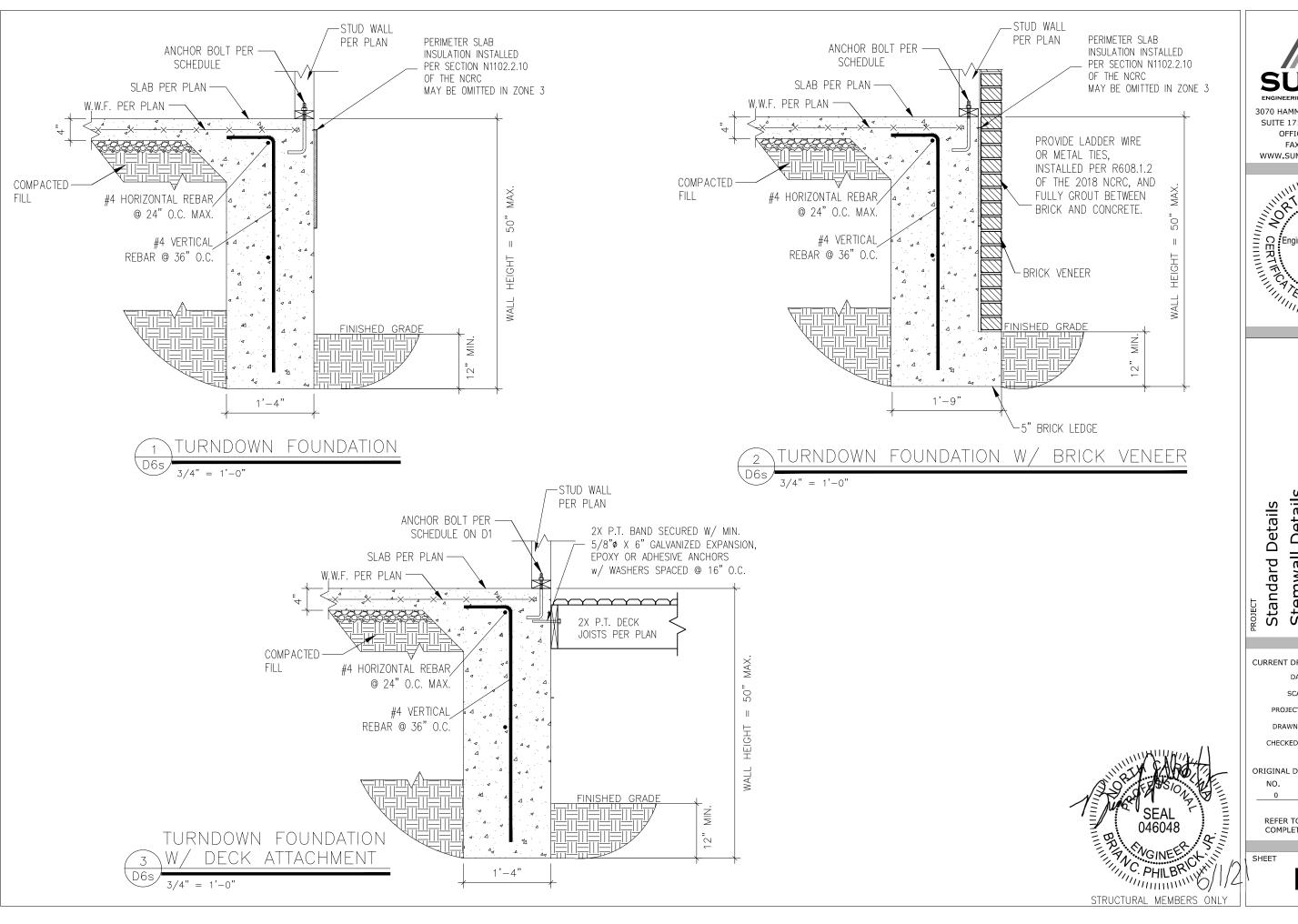
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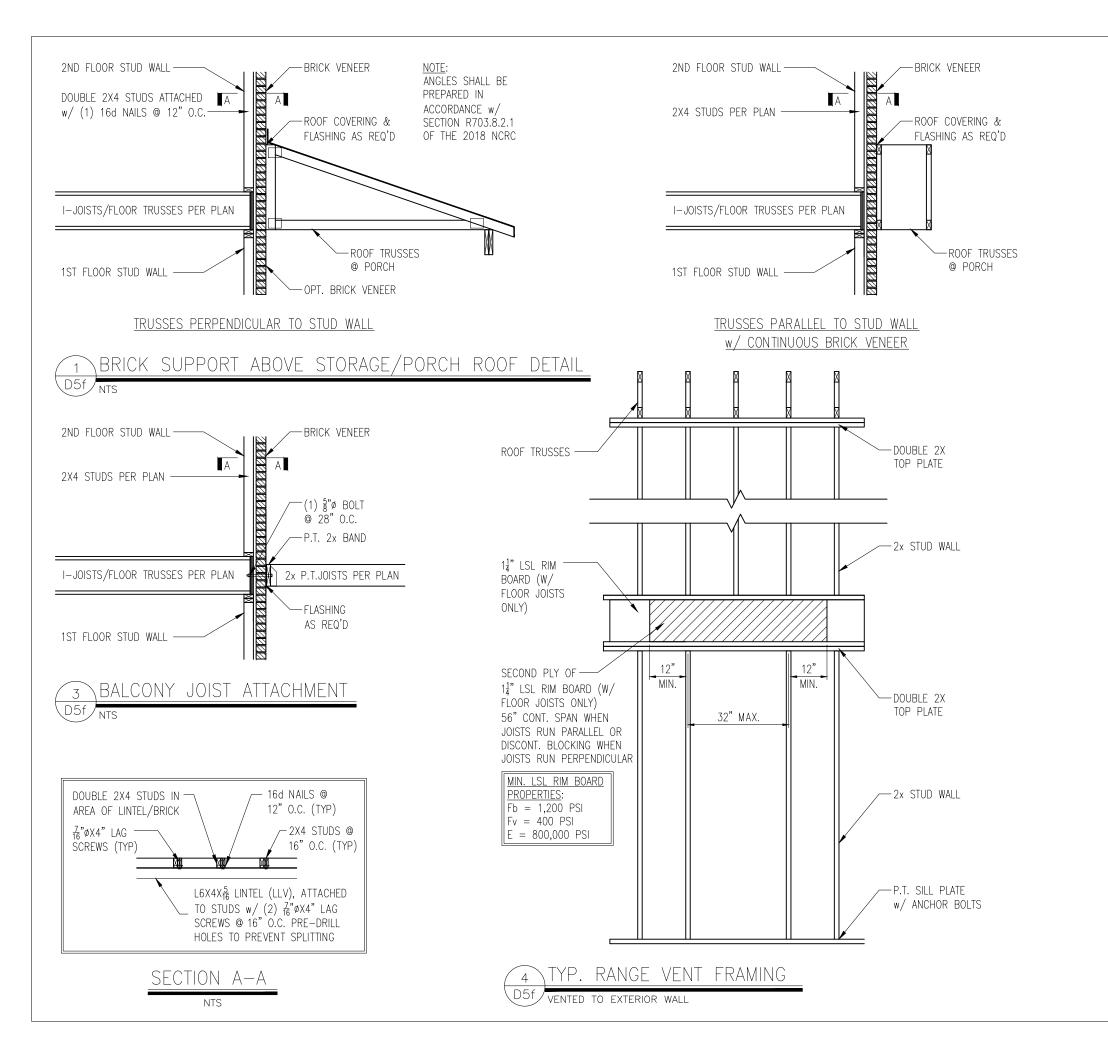
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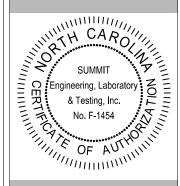
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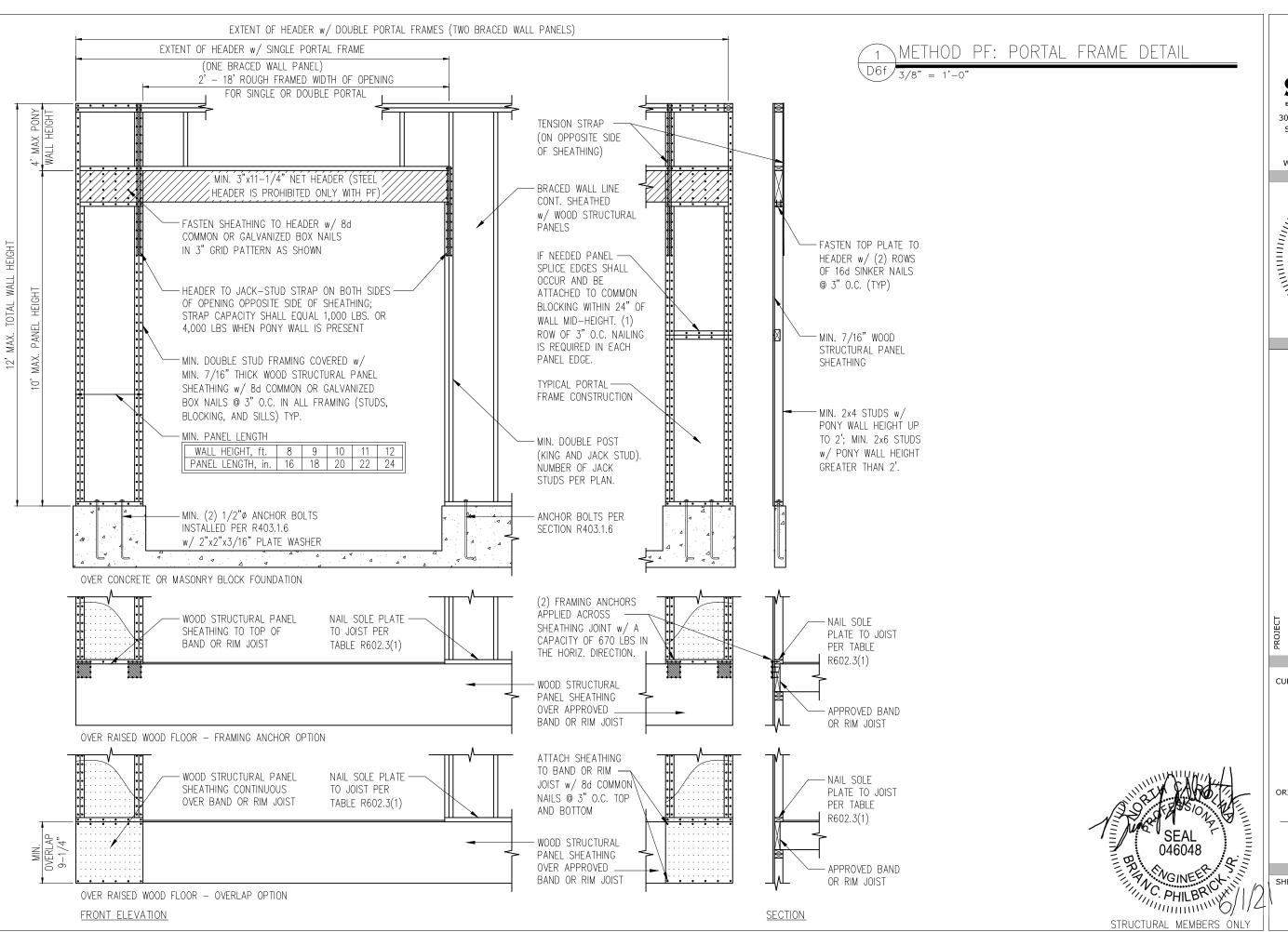
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21

110 Village T Woodstock,

## Bracing Douglas Homes illage Trail, Suite S stock, GA 30188 Details Details Framing Standard CLIENT Smith 1 110 Vill

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DATE: 4/29/2021

SCALE: NTS

PROJECT #: 3832-R2

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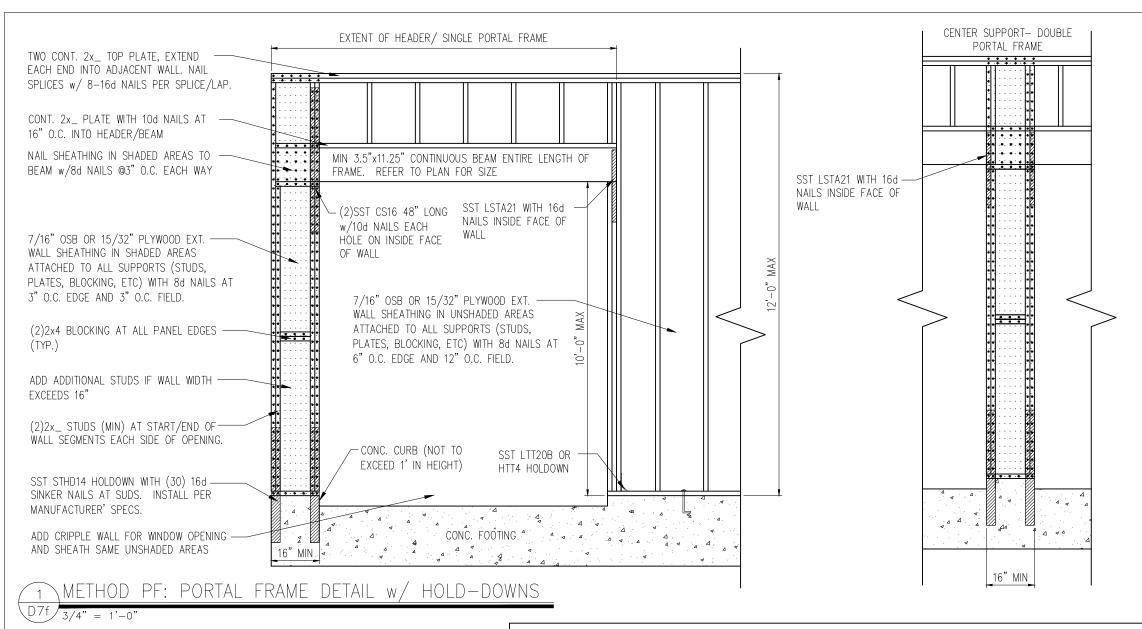
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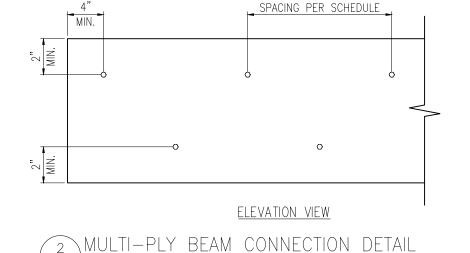
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MINIMUM FASTENING 31/2" WIDE 51/4" WIDE 7" WIDE REQUIREMENTS FOR TOP- AND SIDE-LOADED **MEMBERS** FASTENER TYPE LVL DEPTH 2-Ply 13/4" 3-Ply 13/4"  $1^{3}/4'' + 3^{1}/2''$ 4-Ply 13/4" 2-Ply 13/4" + 31/2" 2-Ply 31/2" 3 rows @ 12" o.c. 3 rows @ 12" o.c. (ES) 3 rows @ 12" o.c. 71/4"≤d<14" 3 rows @ 12" o.c. (ES)  $10d (0.128'' \times 3'')$ `Nails d≥14" 4 rows @ 12" o.c. 4 rows @ 12" o.c. (ES) 4 rows @ 12" o.c. 4 rows @ 12" o.c. (ES) 71/4"≤d<14" 2 rows @ 12" o.c. 2 rows @ 12" o.c. (ES) 2 rows @ 12" o.c. 2 rows @ 12" o.c. (ES) 16d (0.162" x 3½") 3 rows @ 12" o.c. d≥14" 3 rows @ 12" o.c. (ES) 3 rows @ 12" o.c. 3 rows @ 12" o.c. (ES) 2 rows @ 24" o.c. 2 rows @ 24" o.c. 2 rows @ 24" o.c. ½" Through Bolts SDS 1/4" x 31/2", WS35, 2 rows @ 24" o.c. 2 rows @ 24" o.c. (ES) 2 rows @ 24" o.c. 2 rows @ 24" o.c. (ES) 33/8" TrussLok d≥71/4" SDS 1/4" x 6", WS6 2 rows @ 24" o.c. (ES) 2 rows @ 24" o.c. 5" TrussLok 2 rows @ 24" o.c. 6¾" TrussLok



- I.All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- 2. Minimum fastening requirements for depths less than 71/4" require special consideration. Please contact your technical representative
- 3. Three general rules for staggering or offsetting for a certain fastener schedule: (I) if staggering or offsetting is not referenced, then none is required;
- (2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener
- (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



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## Bracing Douglas Homes illage Trail, Suite Stock, GA 30188 Details Details Framing Standard Smith |

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110 Village T Woodstock,

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SCALE: NTS

PROJECT #: 3832-R2

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DATE PROJECT # 1/7/16 3832

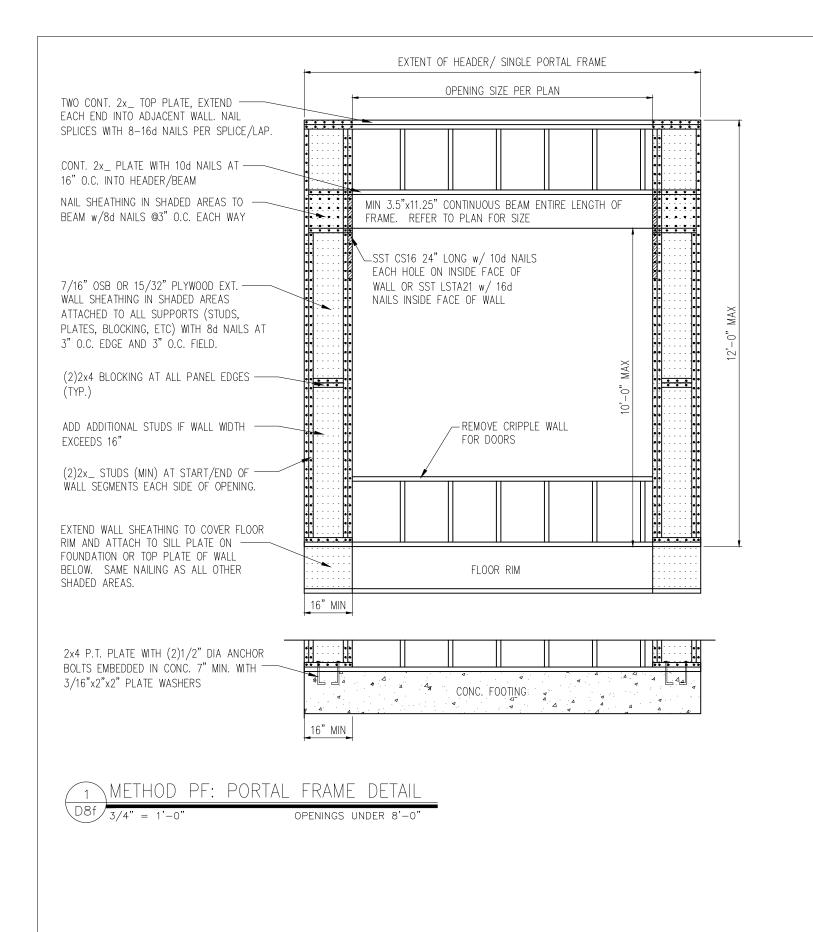
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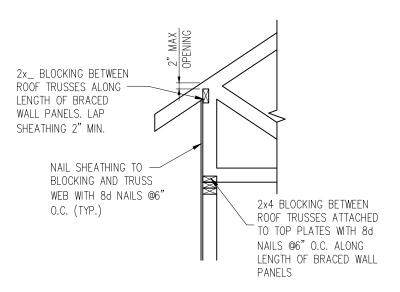
NGINEER

PHILBROWN

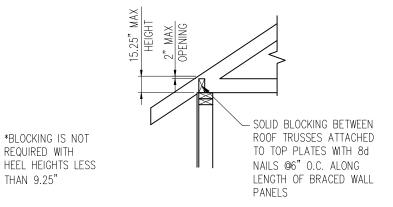
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D7f





## HEEL HEIGHT GREATER THAN 15.25"

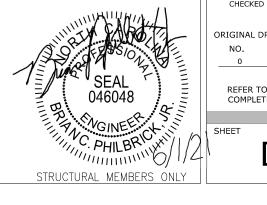


HEEL HEIGHT LESS THAN 15.25" \*

YP. WALL PANEL TO ROOF TRUSS CONNECTION

REQUIRED WITH

THAN 9.25"





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PROJECT #: 3832-R2

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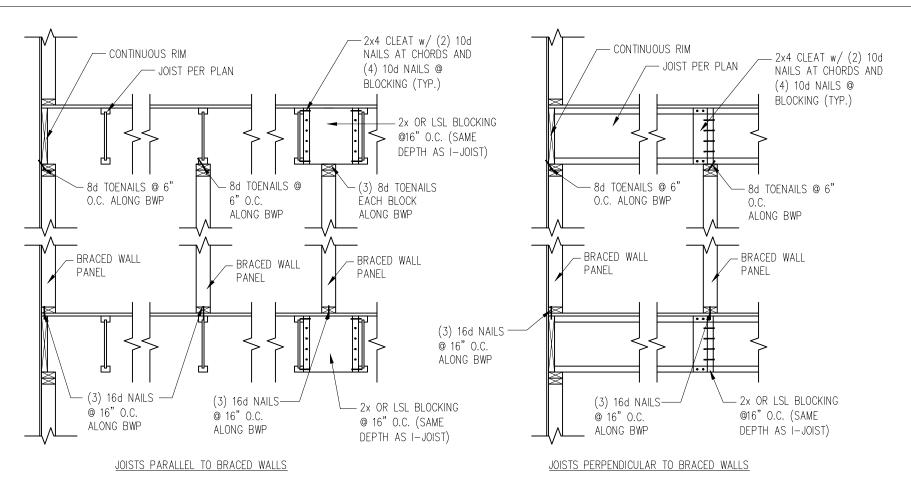
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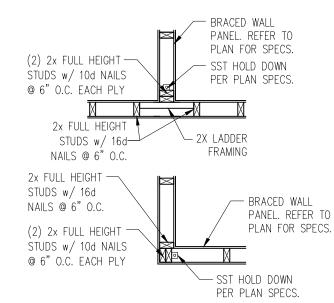
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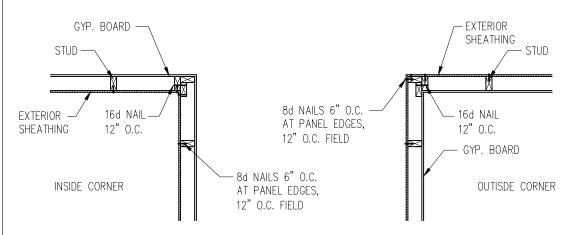


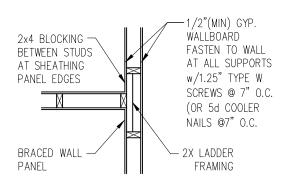
TYP. HOLD DOWN DETAIL

D9f

1" = 1'-0"

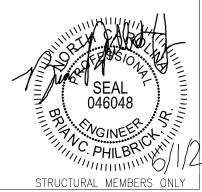
## 1 TYP. WALL PANEL TO FLOOR/CEILING CONNECTION





TYP. EXTERIOR CORNER FRAMING

3 INTERIOR 3-STUD WALL INTERSECTION
D9f 1" = 1'-0"





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Framing Details - Bracing
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Woodstock, GA 30188

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