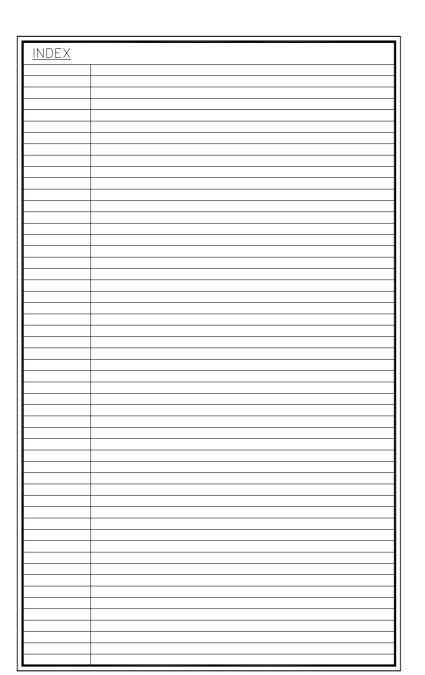
# MIDDLETON-RALE

RALEIGH - LOT 00.0031 THE FARM AT NEILL'S CREEK

(MODEL# 2183)

**ELEVATION 3- GR** 

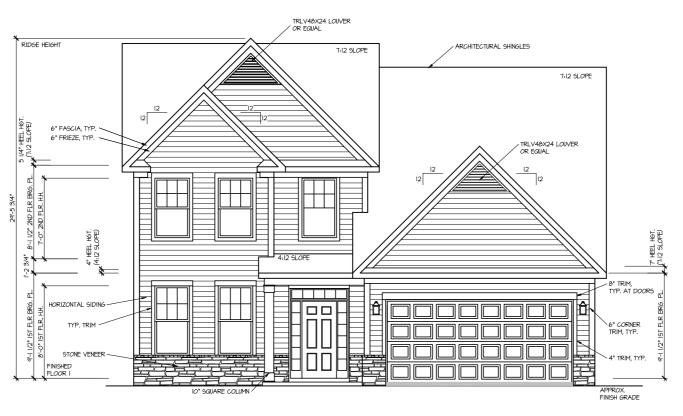




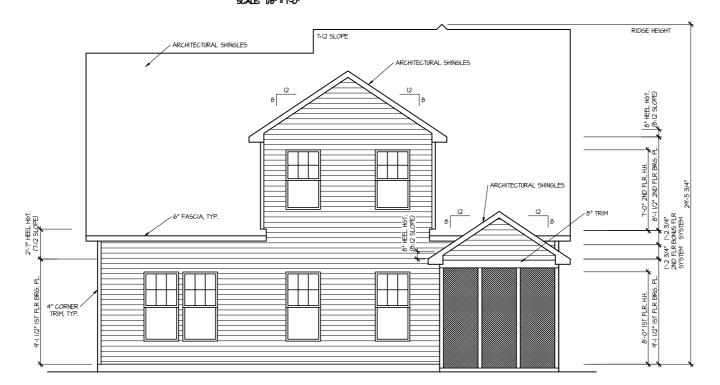
<u>AREA CALCULATIONS</u>		001/5050 /	
ELEVATION 3	HEATED	COVERED / UNHEATED	UNCOVERED
FIRST FLOOR	1495 SF	01111271128	OTTO TENED
GARAGE		417 SF	
FRONT PORCH - ELEVATION 3		45 SF	
SECOND FLOOR	692 SF		
OPTIONS			
BONUS ROOM	310 SF		
SCREEN PORCH	100.05	124 SF	
4' REAR EXTENSION	160 SF		
BONUS ROOM ADD W/ 4' REAR EXT	61 SF		
TOTAL	2718 SF	586 SF	
TOTAL	2/10 35	300 SF	
	1		

### 352 Peach Grove Way

LOT	SPECIFIC	
1	LOT 00.0031	THE FARM AT NEILL'S CREEK
		MIDDLETON REV. RALE 2 ELEVATION 3
2	ADDRESS	352 PEACH GROVE WAY LILLINGTON, NC 27546
	7.007.1200	and the state of t



### FRONT ELEVATION 3



REAR ELEVATION 3

MASIER PLAN INFORMATION

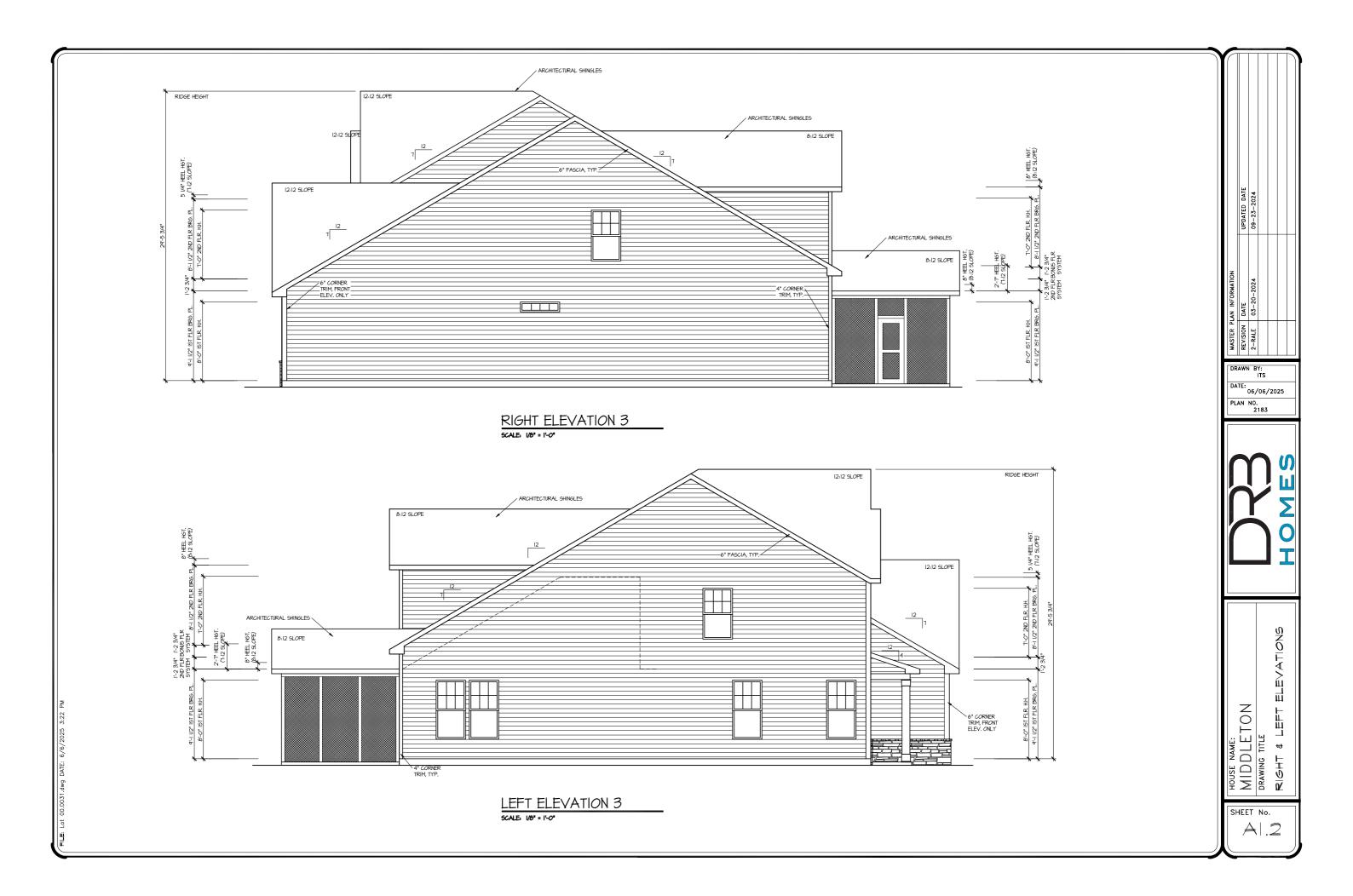
REVISION DATE

2 - RALE 03-20-2024 09-23-2024

DRAWN BY: ITS DATE: 06/06/2025 PLAN NO. 2183



HOUSE NAME:
MIDDLETON
DRAWING TITLE
FRONT & REAR ELEVATIONS



ROOF VENTILATION CALCULATIONS:
ROOF AREA = 202 SG. FT.
CORRULATIONS:
ROOF AREA = 202 SG. FT.
CORRULATIONS:
ROOF AREA = 202 SG. FT.
10 300 = 100 E. FT.
10 500 N IN TOP THERD = 350 - 361 SG. FT. (1 TO 300)
NET FREE AREA OF VENTES SOFT = 51 SG. IN / LINEAR FT.
NET FREE AREA OF RIDGE VENT = 16 SG. IN/ LINEAR FT.

LOVER VENTING. (BOTTOM 2/3 RDS)

T4 LINEAR FEET OF SOFFIT X 5.7 SQ. IN. = 2.49 SQ. FT.
UPPER VENTING. (TOP J. 28 DD)

30 LINEAR FEET OF RIDGE X ID SQ. IN. = 4.5 SQ. FT.
400 SQ. FT. EPIYERI SQS. - 60%

10 300 ALLOWED)

TOTAL ROOF VENTILATION. 143 SQ. FT. > 10.1 SQ. FT. (RQD)

8:12 SLOPE 8:12 SLOPE 8:12 SLOPE 8:12 SL*O*PE 7:12 SLOPE LINE OF SECOND FLOOR 16 LF RIDGE VENT 6 LF RIDGE VENT LINE OF SECOND FLOOR ATTIC ACCESS 7:12 SLOPE 12:12 SLOPE 12:12 5L*O*PE 12:12 SLOPE 12:12 SLOPE 12:12 / SLOPE 4"X4" PT P05T / CENTERLINE OF PORCH BEAM

ROOF PLAN ELEV. 3

HOUSE NAME:
MIDDLETON
DRAWING TITLE
ROOF PLAN

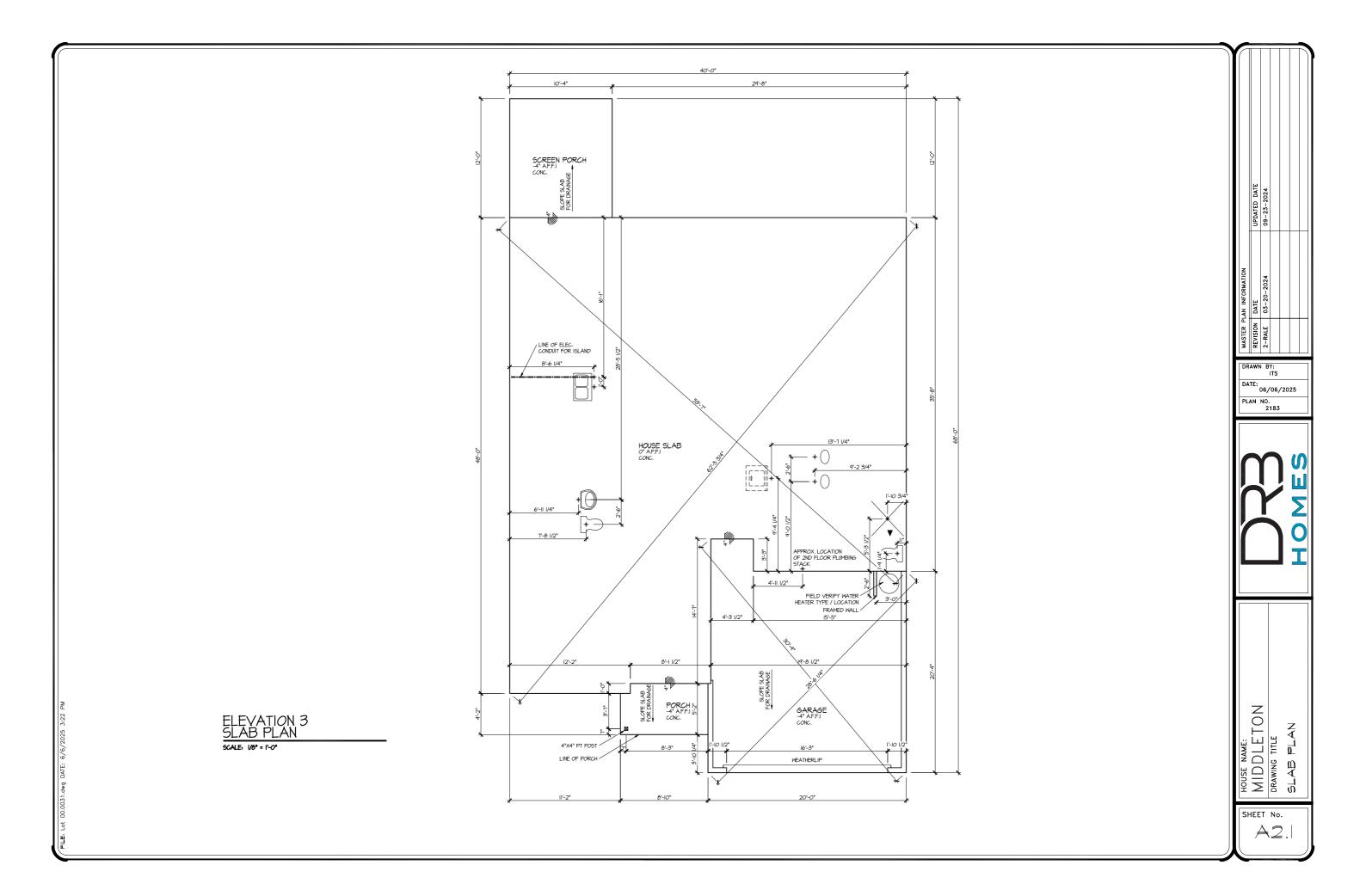
UPDATED DATE 09-23-2024

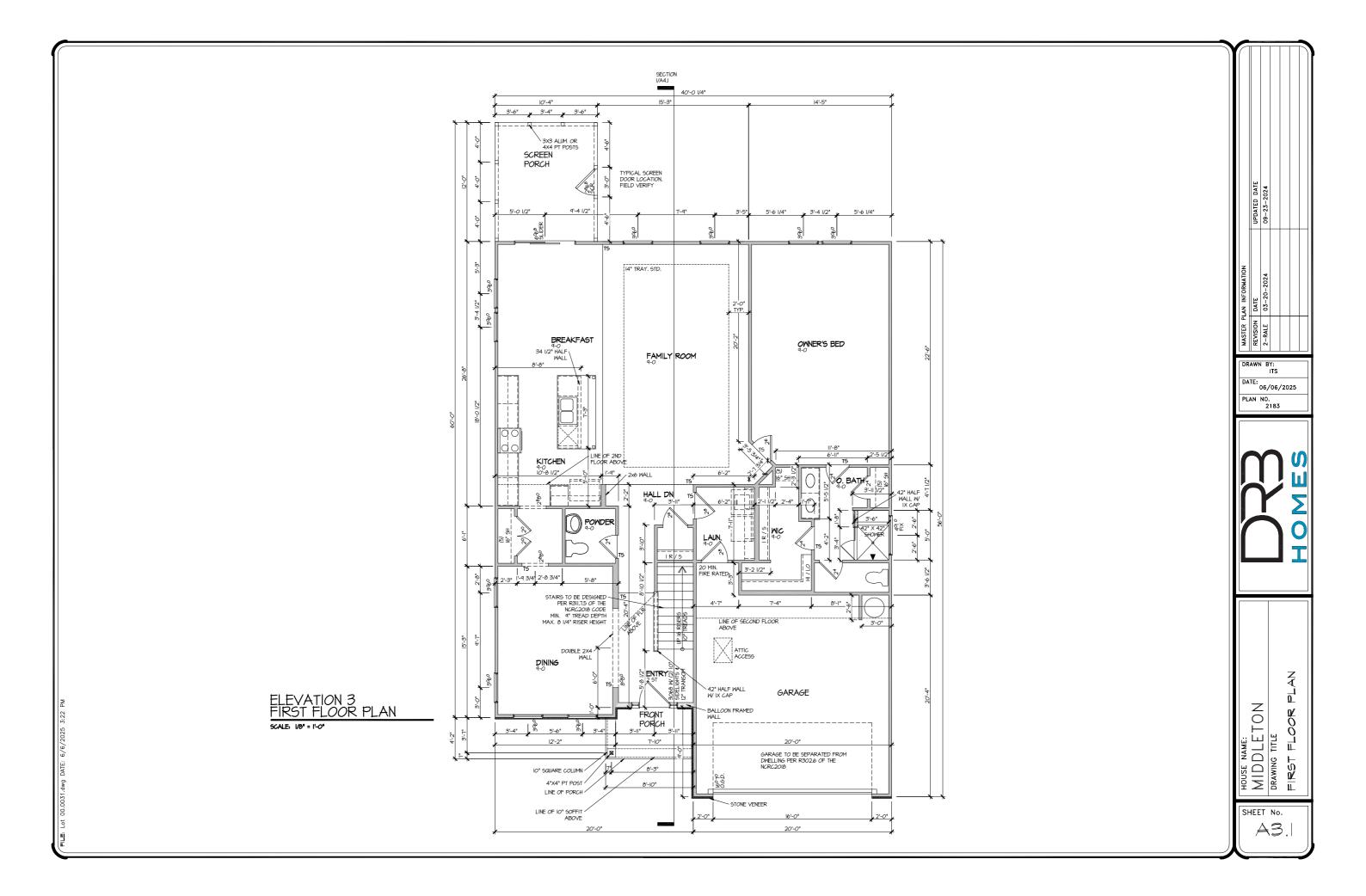
MASTER PLAN INFORMATION
REVISION DATE
2-RALE 03-20-2024

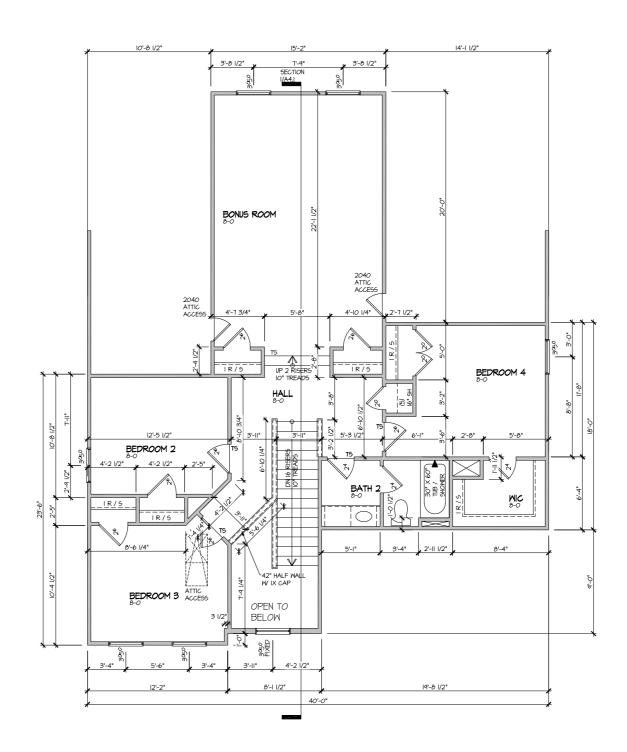
DRAWN BY:

DATE: 06/06/2025 PLAN NO. 2183

SHEET No. AI.3







ELEVATION 3 SECOND FLOOR PLAN SCALE 100" = 11-0"

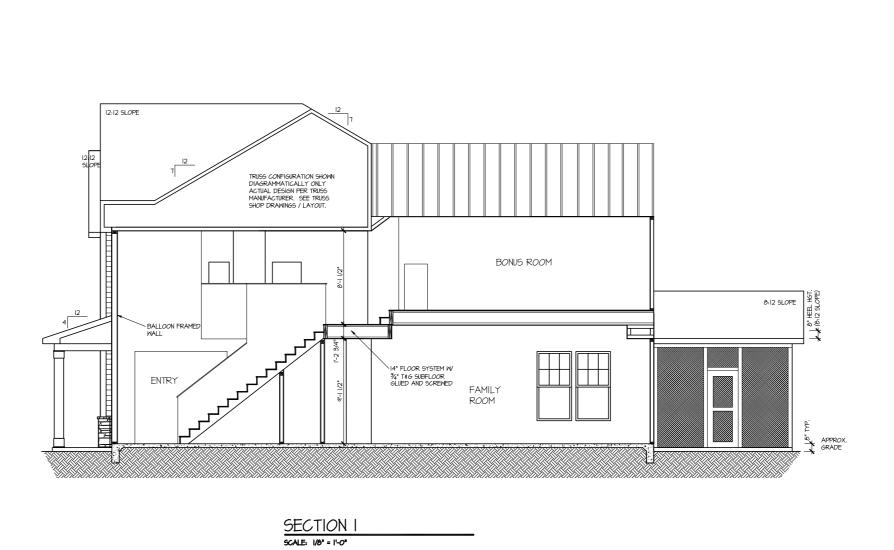
DRAWN BY:

DATE: 06/06/2025 PLAN NO. 2183



7 V V N HOUSE NAME:
MIDDLETON
DRAWING TITLE
SECOND FLOOR

SHEET No. A3.2

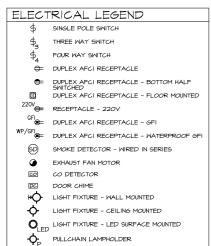


| MASTER PLAN INFORMATION | WPDATED DATE | CS-20-2024 | C9-23-2024 | C

DRAWN BY: ITS DATE: 06/06/2025 PLAN NO. 2183

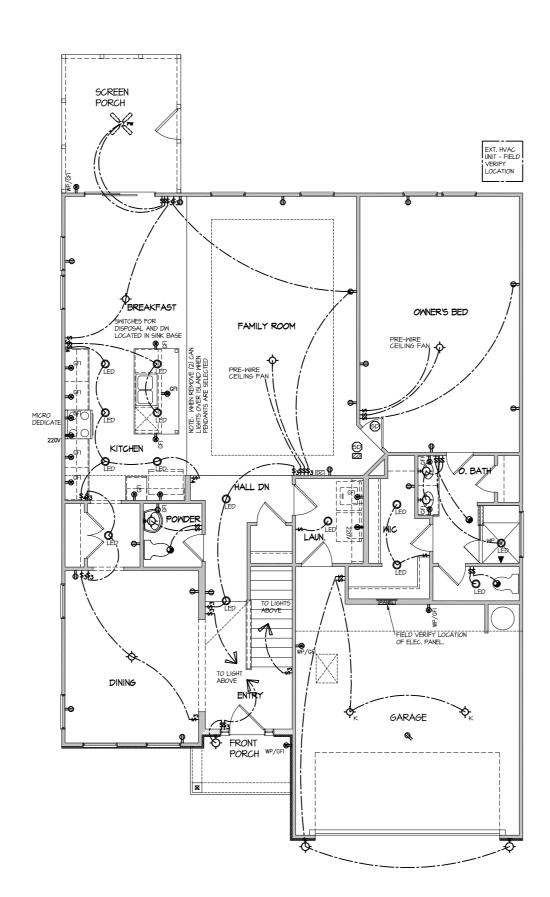


HOUSE NAME:
MIDDLETON
DRAWING TITLE
BUILDING SECTION



MOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.

- KEYLESS LAMPHOLDER



ELECTRICAL PLAN FIRST FLOOR - ELEV. 3 SCALE: 1/8" = 1'-0"



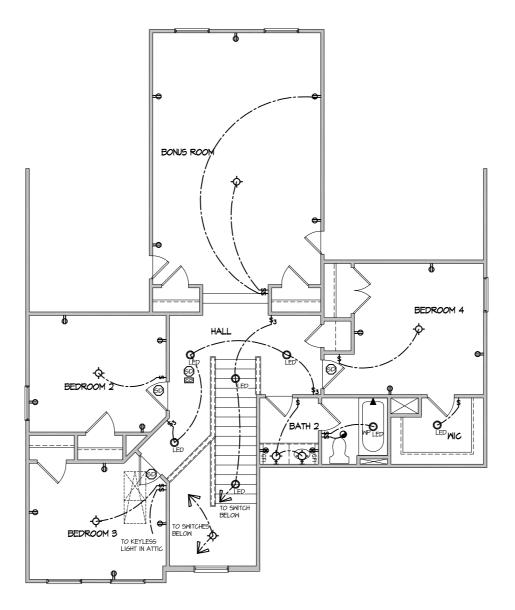
DATE: 06/06/2025 PLAN NO. 2183



0 ₹ 1 HOUSE NAME:
MIDDLETON
DRAWING TITLE
FIRST FLOOR EI

NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL PONER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.

PULLCHAIN LAMPHOLDER ♠ KEYLESS LAMPHOLDER



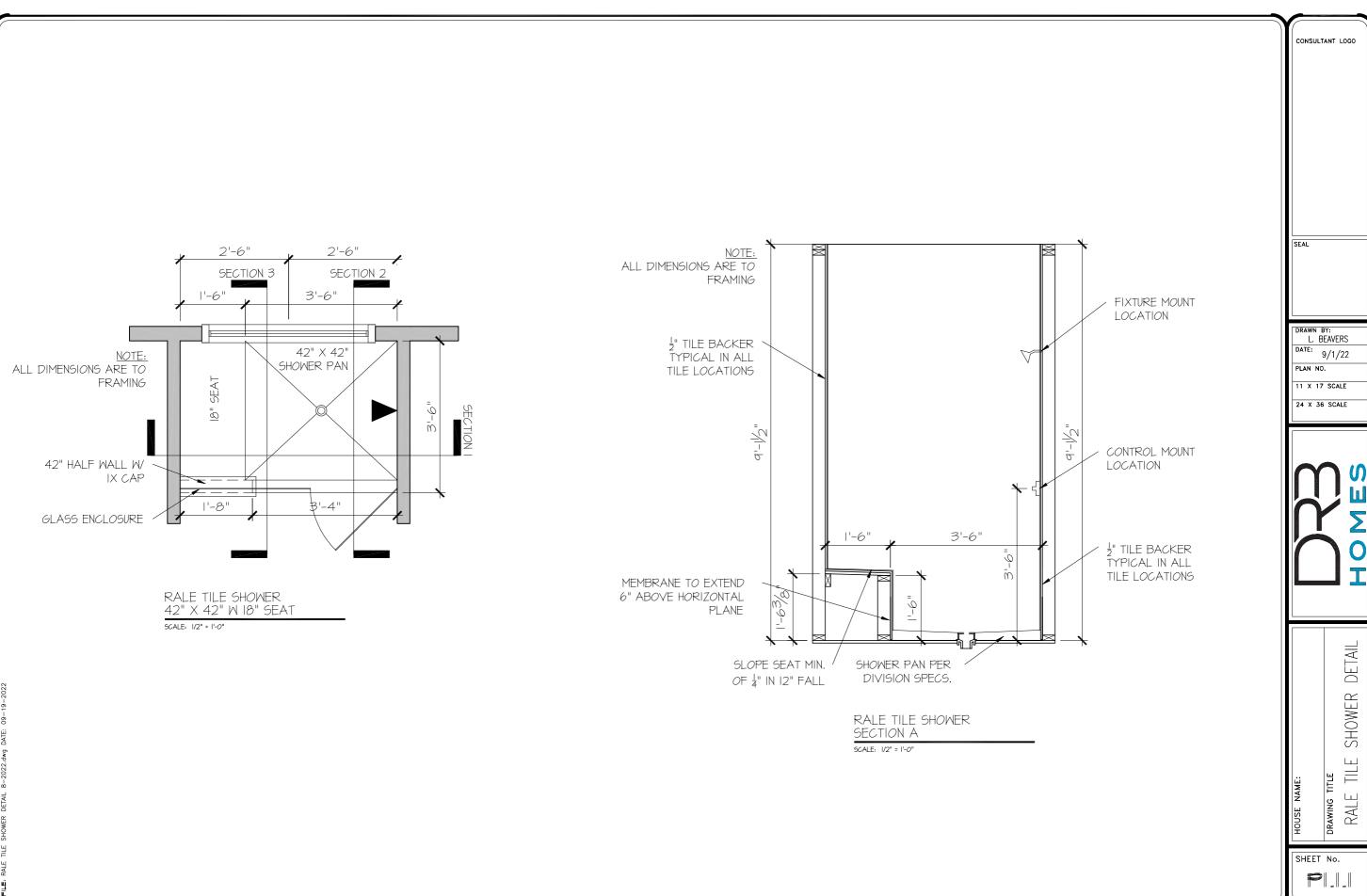
DRAWN BY:

DATE: 06/06/2025 PLAN NO. 2183



ELECTRICAL

HOUSE NAME:
MIDDLETON
DRAWING TITLE
SECOND FLOOR





DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE 24 X 36 SCALE



DETAIL SHOWER

RALE

DRAWING TITLE
RALE TILE

SHEET No.

PI.I.2

GLASS ENCLOSURE <u>-</u>P 42" HGT. HALF WALL W/ IX CAP BEYOND I" TILE BACKER V / LINE OF SEAT TYPICAL IN ALL BEYOND TILE LOCATIONS MEMBRANE TO EXTEND OVER CURB / MEMBRANE TO EXTEND (2) 2X4 PT CURB 6" ABOVE HORIZONTAL PLANE SHOWER PAN PER 3'-6" DIVISION SPECS. RALE TILE SHOWER

SECTION C

SCALE: 1/2" = 1'-0"

LINE OF TILE

TO CLG.

LINE OF TILE TO CLG. FIXTURE MOUNT LOCATION GLASS ENCLOSURE CONTROL MOUNT LOCATION "B-19 1 TILE BACKER TYPICAL IN ALL TILE LOCATIONS MEMBRANE TO EXTEND OVER CURB MEMBRANE TO EXTEND (2) 2X4 PT CURB 3'-6"

RALE TILE SHOWER SECTION B

SCALE: 1/2" = 1'-0"

6" ABOVE HORIZONTAL

SHOWER PAN PER

DIVISION SPECS.

PLANE

#### **FOUNDATION**

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, I2" MAX. FROM PLATE ENDS - UTILIZING: • I/2" DIA. ANCHOR BOLTS • 6'-0" O.C., 7" MIN. EMBEDMENT

  - (CONC.) 15" MIN EMBEDMENT (CMU) SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS @ 2'-8" O.C. (CMU)
- (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS) ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W/ CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 @ 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.:
  - 4,000 psi: ...... FOUNDATION WALLS 2,500 psi: ...... FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: ...... GARAGE & EXTERIOR SLABS ON GRADE
- BASEMENT FOUNDATION WALL DESIGN BASED ON:
- . 9' OR IO' HEIGHT (AS NOTED ON PLANS
- TALLER WALLS MUST BE ENGINEERED.

CLASSIFICATIONS (SC, ML-CL, OR CL).

- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN
- CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORGEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS
- FOR OPENINGS UP TO 36", PROVIDE MINIMUM IO" CONCRETE DEPTH OVER OPENING OR (3)2x10 w/ (2)2x6 JACK STUDS, U.N.O.
- LARGER OPENINGS SHALL BE PER PLAN.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT
- ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP
  - JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" O.C. (MAXIMUM)
  - JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I:I RATIO), WITH A MAXIMUM OF I:1.5 RATIO CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 \$ 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x8 x 16" LONG PT. PLATE ON TOP OF ALL CRAW SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS. FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE.
- DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

DESCRIPTION OF BLDG. ELEMENT 3"x0.131" NAILS

BLK'G. BTWN. JOISTS TO TOP PL. (3) TOENAILS

DOUBLE TOP PLATE LAP SPLICE (9) NAILS IN LAPPED AREA

DIST TO SOLE PLATE

DOUBLE STUD

E PLATE TO JOIST/BLK'G. D TO SOLE PLATE

FOP PLATE LAP @ CORNERS &

(ONLY ACCEPTABLE WHERE \* ARE SHOWN)

NTERSECTING WALLS

TOP OR SOLE PLATE TO STUD

• BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY

CONNECTION SPECIFICATIONS (TYP. U.N.O.)

3) TOENAILS

(2) NAILS

(2) NAILS

(3) NAILS **@** 4" o (2) TOENAILS

NAILS @ 24" o.c

2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS.

#### GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.
- DESIGN LOADS

DEAD = 7 PSF T.C., IO PSF B.C. LIVE = 16 PSF

LOAD DURATION FACTOR = 1.25

FLOOR LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

#### GENERAL FRAMING

- ALL TYP. NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS ALL NAILS SPECIFIED ARE MIN NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY, NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(I) FOR ALL CONNECTIONS, TYP. U.N.O.
- EXT & INT BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLAN • 16" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O.
  - WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED). ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED. WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX., U.N.O.)
- . HEADERS IN NON-LOAD BEARING WALLS SHALL BE (1)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING: 'L9L' - Fb=2325 psi; Fv=3I0 psi; E=I.55xI0^6 psi
- 'LVL' Fb=2600 psi; Fv=285 psi; E=2.0x10^6 psi 'PSL' - FB=2900 PSI: FV=290 PSI: E=2.0XI0^6 PSI
- M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THE OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR
- FOR 2 & 3 PLY BEAMS OF FOUAL WIDTH FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"X0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"X31/5" SIMPSON SDS SCREMS (OR 31/2" TRUSSLOK SCREMS) € 16" O/C. USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE. SOLID 3 1/2" OR 5 1/4" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 1/2"x6" SIMPSON SDS SCREWS (OR 6 3/4" TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREW 2" FROM EDGE, A SOLID 7" BEAM IS ACCEPTABLE.
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED UNO... • ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER W/ 3"X0.I3I"
- NAILS @ 24" O.C. (MIN.), EACH PLY. PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND./BEARING. BLOCKING TO MATCH POST ABOVE
- EASTEN 2× WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.

3"x0.120" NAILS

(3) TOENAILS\*

(3) TOENAII S\*

NAII 5 @ 16" 00

NAILS @ 16" 04

(3) NAILS @ 4" o. (3) TOENAILS\*

(3) NAILS TOENAILS @ 6" O.C

(II) NAILS IN LAPPED AREA

• ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4

#### FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED 1 /480 LIVE LOAD DEFLECTION CRITERIA (EXCLUDES MARBLE FLOORS - CONTACT M&K FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN, OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131 NAILS @ 6"o.c. @ PANEL EDGES & @ 12"o.c. FIELD. 2 3" × 0120" NAILS @ 4" OC @ PANEL EDGES & @ 8" OC FIELD
- 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- \$6 × 2" MIN. SCREWS @ 6" O.C. @ PANEL EDGES \$ @ 12" O.C. FIELD.

#### ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN EACH ROOF TRUSS TO TOP PLATE W SIMPSON H2.5T CLIP (OR APPROVED EQUAL) © ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES, (3) H2.5T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) w/ 2x4 LEDGER FASTENED TO:
  - RIM BOARD W (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) TRUSS VERTICALS w/ (3) 3"x0.131" NAILS  $\bullet$  19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 1/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS w/ 2 \* x 0.131" NAILS @ 6"04. @ PANEL EDGES & @ 12" O.C. FIELD.
- -w/2 🐉 x 0.120" NAILS 🙍 4"0 a. 🙍 PANEL FDGES 🕻 🗖 8" 0 a. FIELD. - W/ 2 🖥 x 0.113" NAILS @ 3"o.c. @ PANEL EDGES 🛊 @ 6" O.C. FIELD.

#### HOLD-DOWN SCHEDULE

SYMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC66B3 ALTERNATE
► HD-3	SIMPSON STHDI4/STHDI4RJ

\* UTILIZE THE SSTB24 ANCHOR BOLT @ ALL MONOSLAB & INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS. MINIMUM 24" MIN. FOOTING THICKNESS REQUIRED.

POXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE IO" (FOR 5/8" DIA.) OR 15" (FOR 1/8" DIA.) MIN EMBEDMENT INTO CONCRETE INSTALL PER MANUF. INSTRUCTIONS. MINIMUM 16" FOOTING THICKNESS REQ'D. DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE.

#### ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS OTED OTHERWISE ON PLAN.

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN AD JACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING: ROOF TRUSSES:

- 1/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS: 1/8" DEAD LOAD
- FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO
- LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAL LOAD. (NOT DIFFERENTIAL DEFLECTION)

#### ALTERNATE F.J MANUFACTURERS

FLOOR JOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED I-JOISTS DESIGN AND CONSTRUCTION GUIDE. MINIMUM JOIST PROPERTIES INCLUDING, BUT NOT LIMITED TO, ALLOWABLE SHEAR, ALLOWABLE MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I- MISTS ALL ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST CONNECTIONS ARE PER THE JOIST MANUFACTURER.

#### LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM:

0 MPH WIND IN 2018 NCSBC:RC (120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301211) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC

(SECTION 1609) & ASCE 7-10. AS PERMITTED BY R301.13 OF THE 2018 NGSBG:RG, OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R60212 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602.3.5 R802.II.

#### EXT. WALL SHEATHING SPECIFICATION

- 1/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W/ 2 %"x0.II3" NAILS @ 6" O.C. AT EDGES & @ 12" O.C. IN THE PANEL FIELD. TYP, U.N.O
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- PALT. STAPLE CONNECTION SPEC: 1½" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD.

#### BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 %" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 %" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

#### 3" O.C. EDGE NAILING

• AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING w/ 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING.

#### NOTES

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING.
- PRE-MANUFACTURED PANELIZED WALLS: FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWALL OR 3" O.C. OSB SHEARWALL. INDICATES HOLDOWN BELOW

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

### VENEER LINTEL SCHEDULE

'-0"	20 FT. MAX	L3"x3"x/4"
'-0"	3 FT. MAX	L3"x3"x¼"
	I2 FT. MAX	L4"x3"x/4"
	20 FT. MAX	L5"x3½"x%;"
'-o"	3 FT. MAX	L4"x4"x¼" *
	I2 FT. MAX	L5"x3½"x%;"
	I6 FT. MAX	L6"x31/2"x3%"
'-6"	I2 FT. MAX	L6"x3½"x%;"
,'-0"	2 FT. MAX	L7"x4"x½" **
	2 == 1/11/	

I LINILEST 94ML SUPPORT 2 %" - 3 ½" VENEER W 40 POF MAXIMUM WEIGHT. 16' SHALL HAVE 5" MIN, BEARING 16' SHALL NOT BE FASTINED BACK TO HEADER. 16' SHALL BE FASTINED BACK TO WOOD HEADER IN WALL **040**"0.0c.

 $w/k_2^*$  Dia. x 3  $k_2^*$  Long Lag screws in 2\* Long vertically slotted holes, max, veneer ht. Applies to any portion of Brick over the

OPENING.
ALL LINTELS SHALL BE LONG LEG VERTICAL.
WHEN SUPPORTING VIDEERS (3" MIDE THE EXTERIOR TOE OF THE
HORIZOATHAL LEG MAY BE CAT IN THE FIELD TO BE 3 1/4" MIDE OVER
THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT

STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT

#### LEGEND

- IIIIIIII INTERIOR BEARING WALL
- □□□□□□ BEARING WALL ABOVE
- BEAM / HEADER
- - INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

#### NON-BEARING HEADER SCHEDULE

SPAN	2x4 NON-BEARING PARTITION WALL	2x6 NON-BEARING PARTITION WALL
UP TO 3'-0"	(I)2x4 FLAT	(I)2x6 FLAT
UP TO 6'-0"	(2)2x4	(3)2x4
UP TO 8'-0"	(2)2x6	(3)2×6
UP TO 12'-0"	(2)2x8	(3)2×8

#### NOTES:

 ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 24" OC (MAX)

#### MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY
DETERMINE THE ERECTION PROCEDURES SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL. INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TOLERANCES.

#### ENGINEERED BEAM MATERIAL SCHEDULE

BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)13/4"x117/6" - H	3½"×II%" - H	(2)1¾"x11%" - H	(2)2xl2 + (I) ½"xll½" STEEL FLITCH PLATE - H	N/A
OOIA	(2)13/4"x117/6" - H	3½"×II%" - H	(2)1¾"x11%" - H	(2)2xl2 + (l) 片"xll片" STEEL FLITCH PLATE - H	N/A
002	(2)194"x18" - FT	51/4"×18" - FT	N/A	(3)2xl2 + (2) 片"xll片" STEEL FLITCH PLATES - FB	WI2xI9 - F
003	(2)13/4"×14" - F	3½"x14" - F	(2)19/4"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
004	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2x12 + (1) ¼"xII¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
005	(2)13/4"×14" - F	3½"x14" - F	(2)19/4"×14" - F	(2)2x12 + (1) ¼"xII¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
006	(2)13/4"×14" - F	3½"x14" - F	(2)19/4"×14" - F	(2)2x12 + (1) ¼"xII¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
001	(2)1¾"×11%" - F	3½"x11%" - F	(2)1¾"x11¾" - F	(2)2xi2 + (I) ¼"xII¼" STEEL FLITCH PLATE - F	WI0x12 - F
008	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
P00	(3)194"x18" - FT	5¼"xl8" - FT	N/A	(4)2xl2 + (3)火"xll以" STEEL FLITCH PLATES - FB	WI2x26 - F
010	(3)13/4"×20" - FT	5¼"×20" - FT	N/A	(4)2xl2 + (3) %"xll以" STEEL FLITCH PLATES - FB	WI2x35 - F
OII	(2)1¾"x11%" - FB	3½"x  %" - FB	(2)194"×14" - FB	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WIOxI2 - FB

- BEAM NOTATION: "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM "FB" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
- FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W (2) 3"XO.120" NAILS @ 8" O.C.
- FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN
- PLATES IN SUCCESSION W/ (2) 3"x0.120" NAILS @ 8" O.C.

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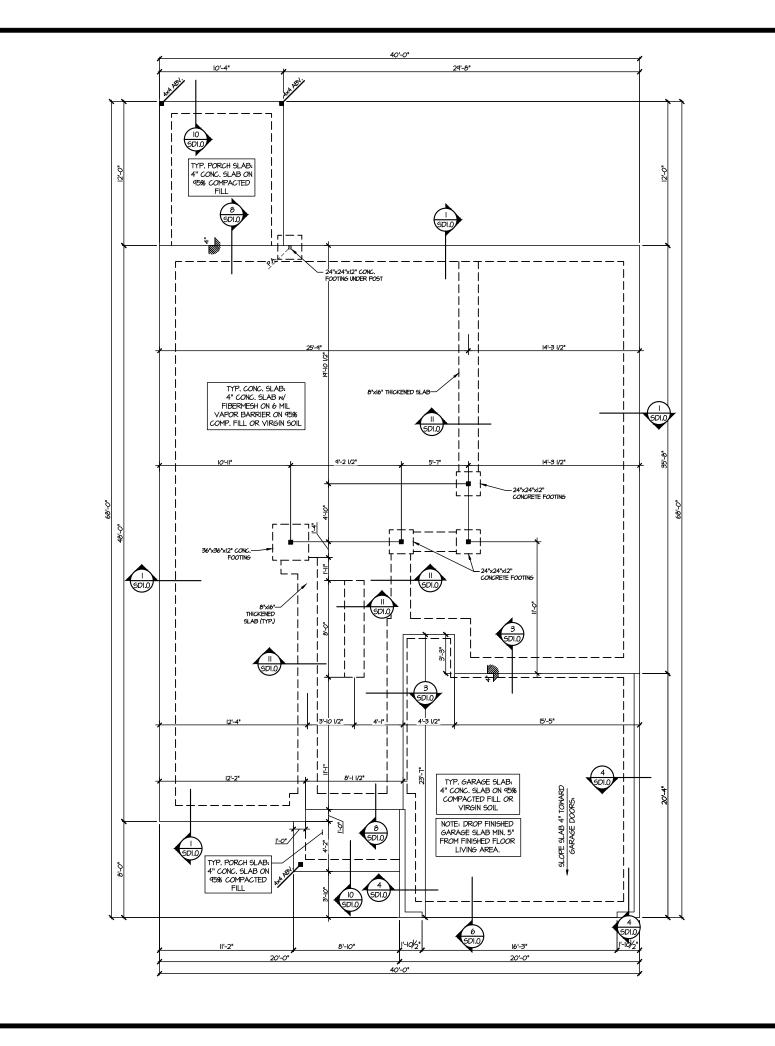
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M&K project number: 126-22076

JTR drawn by: issue date: 06-10-2

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### MONO SLAB FOUNDATION PLAN SCALE: 1/8"=1"-0"

#### LEGEND

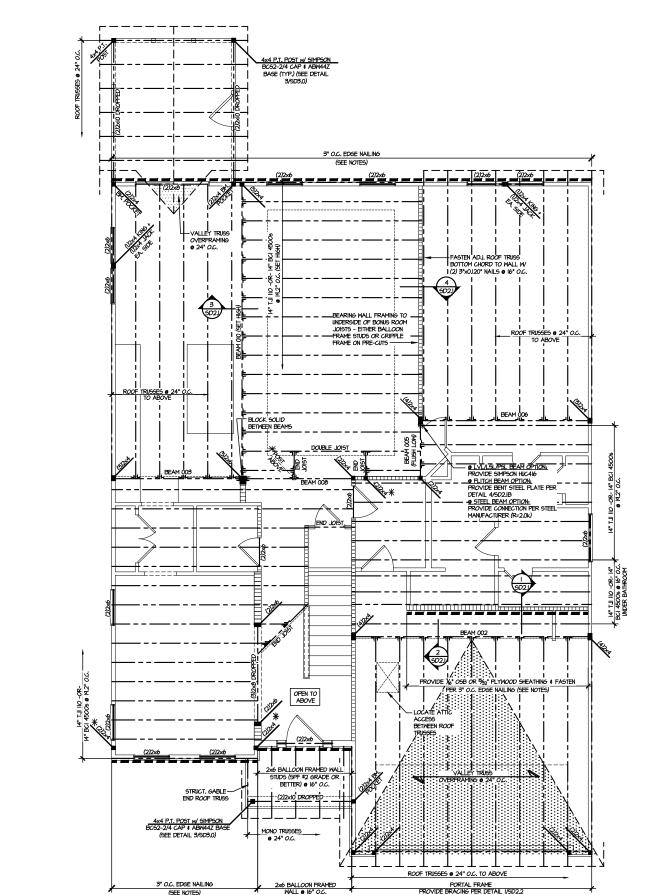
- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE,
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

NEILS OUNDATION P FARM AT NEII LOT 31 - MIDDLETON 3 RALEIGH, NC

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2ND FLOOR/LOW ROOF FRAMING PLAN SCALE: 1/8"=1'-0"

ENGINEERED BEAM MATERIAL SCHEDULE					
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)13/4"x113/6" - H	3½"xII%" - H	(2)13/4"×117/6" - H	(2)2xl2 + (l) 片"xl以" STEEL FLITCH PLATE - H	N/A
AIOO	(2)13/4"×117/6" - H	3½"xII%" - H	(2)1¾"×11½" - H	(2)2xl2 + (l) 片"xl以" STEEL FLITCH PLATE - H	N/A
002	(2)13/4"x18" - FT	5¼"xlô" - FT	N/A	(3)2xi2 + (2) 片"xii以" STEEL FLITCH PLATES - FB	WI2xI9 - F
003	(2)13/4"×14" - F	3½"x 4" - F	(2)134"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
004	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
005	(2)1¾"×14" - F	3½"x 4" - F	(2)194"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
006	(2)1¾"×14" - F	3½"xl4" - F	(2)134"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
007	(2)13/4"x117/6" - F	3½"xII%" - F	(2)1¾"x11½" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - F	WI0x12 - F
008	(2)13/4"×14" - F	3½"x 4" - F	(2)134"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
009	(3)1¾"x18" - FT	5¼"xl8" - FT	N/A	(4)2xl2 + (3) 片"xlik" STEEL FLITCH PLATES - FB	WI2x26 - F
010	(3)13/4"×20" - FT	5¼"×20" - FT	N/A	(4)2xl2 + (3) %"xll4" STEEL FLITCH PLATES - FB	WI2x35 - F
OII	(2)1¾"x11¾" - FB	3½"x11%" - FB	(2) % "x 4" - FB	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB	WIOxI2 - FB

- BEAM NOTATION: "F" INDICATES FLUSH BEAM "FT" INDICATES FLUSH TOP BEAM "FB" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM

- "J" INDICATES DROPPED DEPAM "I" INDICATES DROPPED OPENING HEADER
  REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
  REFER TO DETAIL E/SD2.0 FOR TYPICAL SITEL BEAM CONNECTIONS
  FOR FLUSH TOP DEAMS PROVIDE 28 STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN
  PLATES IN SUCCESSION W (2) 3"X0.120" NAILS @ 8" O.C.
- FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W/ (2) 3"x0.120" NAILS @ 8" O.C.

ALTERNATE F.J MANUFACTURERS

SD2.I REFERS TO SD2.IA FOR LYL/PSL/LSL BEAMS OR SD2.IB

FOR FLITCH BEAMS OR SD2.IC

FOR STEEL BEAMS

• FLOOR JOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED I-JOISTS DESIGN AND CONSTRUCTION GUIDE. MINIMUM JOIST PROPERTIES INCLUDING, BUT NOT LIMITED TO, ALLOWABLE SHEAR, ALLOWABLE MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I-JOISTS. ALL ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST CONNECTIONS ARE PER THE JOIST MANUFACTURER.

#### LEGEND

- INTERIOR BEARING WALL
- ullet ===== BEARING WALL ABOVE
- ---- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

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FARM AT NEIL LOT 31 - MIDDLETON 3 RALEIGH, NC

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M&K project number:

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REVISIONS:

126-22076

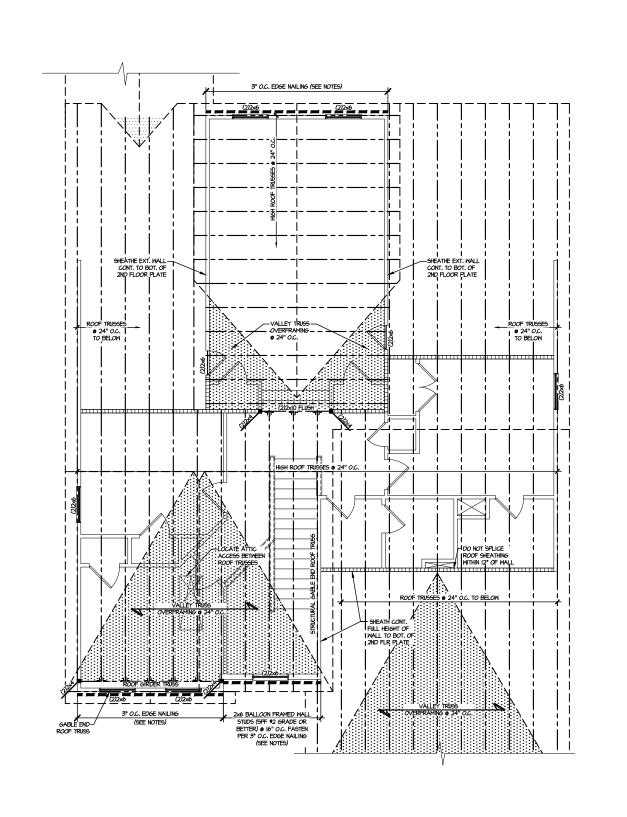
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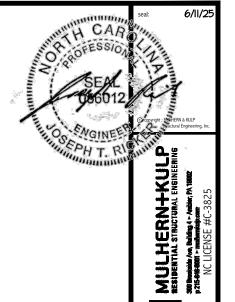
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M&K project number: 126-22076

JTR drawn by: issue date: 06-10-2

REVISIONS:

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CREEK HIGH ROOF FRAMING PLAN FARM AT NEIL'S (LOT 31 - MIDDLETON 3

ROOF

**S3.0** 

• IIIIII INTERIOR BEARING WALL □===□ BEARING WALL ABOVE • ---- BEAM / HEADER

LEGEND

• - INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

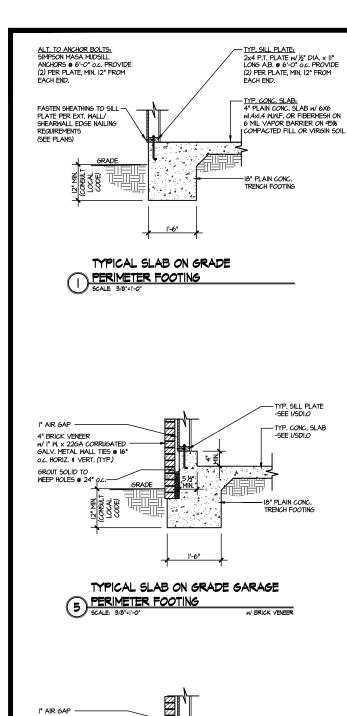
JL METAL HANGER

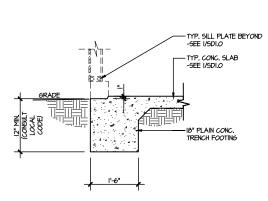
SCALE: 1/8"=1'-0"

\* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES \$ SCHEDULES





I" AIR GAP

GROUT SOLID TO WEEP HOLES @ 24

- TYP. SILL PLATE

-TYP. CONC. SLAB

-SEE I/SDI.O

-SEE I/SDI.O

18" PLAIN CONC.

4" BRICK VENEER w/ I" W. x 22GA CORRUGATED GALV. METAL WALL TIES

@ 16" o.c. HORIZ. & VERT. (TYP.)

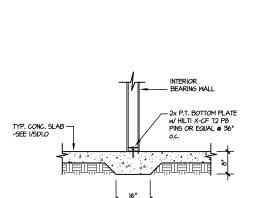
GRADE

TYPICAL SLAB ON GRADE

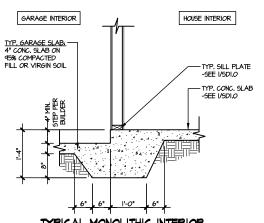
PERIMETER FOOTING

SCALE: 3/8"=1"-0"









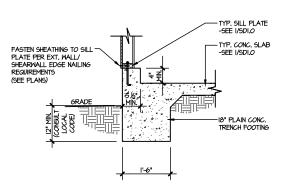


-TYP. SILL PLATE BEYOND -SEE I/SDI.O

- TYP. CONC. SLAB

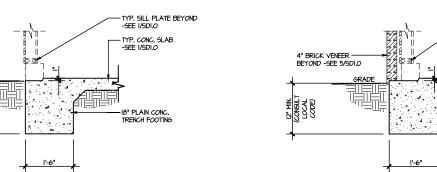
- IB" PLAIN CONC.

-SEE I/SDI.O



TYPICAL SLAB ON GRADE GARAGE PERIMETER FOOTING

SCALE: 3/8"=1"-0"



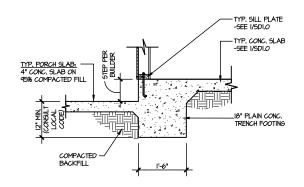
TYP, SILL PLATE -SEE I/SDI.O

- TYP. CONC. SLAB

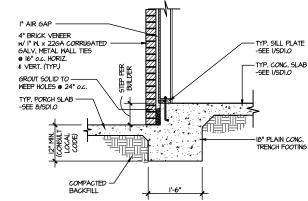
-18" PLAIN CONC.

-SEE I/SDI.O

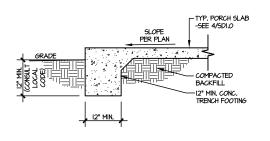




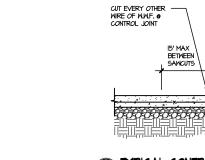
TYPICAL SLAB ON GRADE PERIMETER 8 FOOTING @ PORCH/PATIO SCALE: 3/8"=1'-0"











TYPICAL CONTROL JOINT SCALE: 9/8"=1"-0"

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

-CONC. SLAB -SEE PLAN  $\mathbb{Z}$  $\mathcal{S}$ NEIL Z O LOCATE @ 15'-O" o.c. MAX, OR CORNERS WHERE CRACKS LIKELY TO DEVELOP 1 AT MIDDLE ARM LOT

I" DEEP SAW CUT CONTROL JOINT WITHIN 24 HOURS OF POUR MID-WAY INTO SLAB

-LOCATE JOINTS @ SLAB CORNERS & OTHER AREAS

WHERE CRACKS ARE

6/11/2

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M&K project number: 126-22076

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REVISIONS

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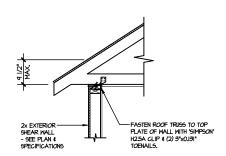
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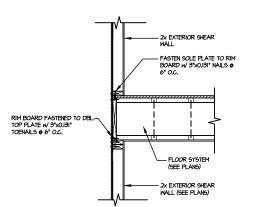
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#### TYPICAL SHEAR

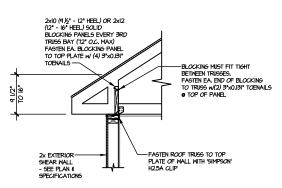
TRANSFER DETAIL @ ROOF

SCALE: 9/8"=1"-0" HEEL HEIGHT LESS THA HEEL HEIGHT LESS THAN 9 ½" NO BLOCKING REQ'D



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL

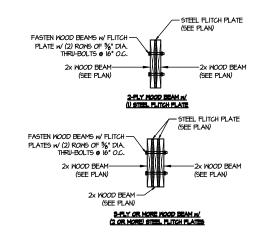
SCALE: 3/8'-1'-0'



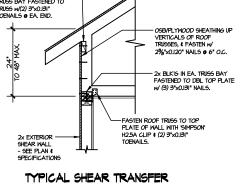
#### TYPICAL SHEAR

TRANSFER DETAIL @ ROOF
SCALE: 3/8"=1"-0" HEEL HEIGHT BETWEEN 9.)

HEEL HEIGHT BETWEEN 9 ½" - 16" BLOCKING REQ'D

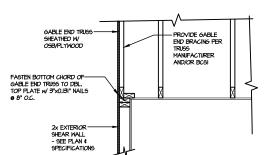


TYPICAL FLITCH BEAM CONNECTION DETAIL SCALE 9/4\*=1\*-0\*



DETAIL @ RAISED HEEL TRUSS

SCALE: 3/8':|'-0' HEEL HEIGHT UP TO 48" MAX.



TYPICAL GABLE END DETAIL

SCALE: 3/8°=1'-0"

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING Y

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M&K project number: 126-22076

**JTR** drawn by: **JAD** issue date: 06-10-2

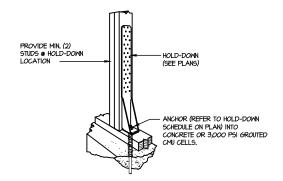
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 $\simeq$  $\Box$ DETAIL FARM AT NEIL'S LOT 31 - MIDDLETON 3 RALEIGH, NC Ŋ

—2x PLATE FASTENED TO TOP FLANGE OF STL. BM, W (2) ROWS OF HILTI XU-4TP8 PINS @ 16" O.C. PACK OUT STEEL BEAM WEB W-SOLID 2x MATERIAL & FASTEN w/ (2) ROWS OF 5/8" DIA. THRU-BOLTS @ 16" O.C. -2x PLATE FASTENED TO TOP FLANGE OF STL. BM. w/ (2) ROWS OF HILTI XU-47P8 PINS @ 16" o.c.

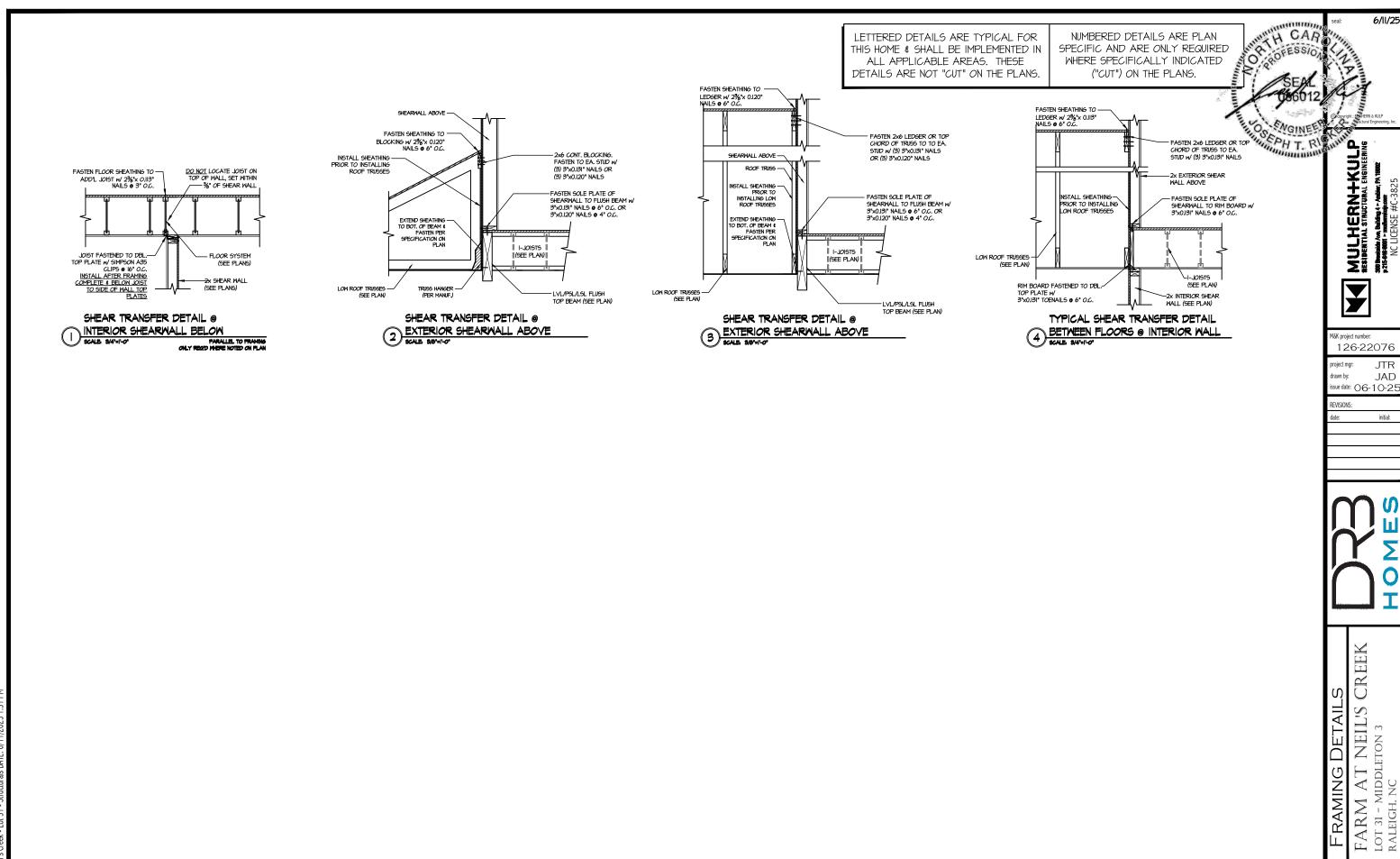
TYPICAL STEEL BEAM CONNECTION DETAIL



TYPICAL HOLD DOWN INSTALLATION
SCALE: N.T.S.

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

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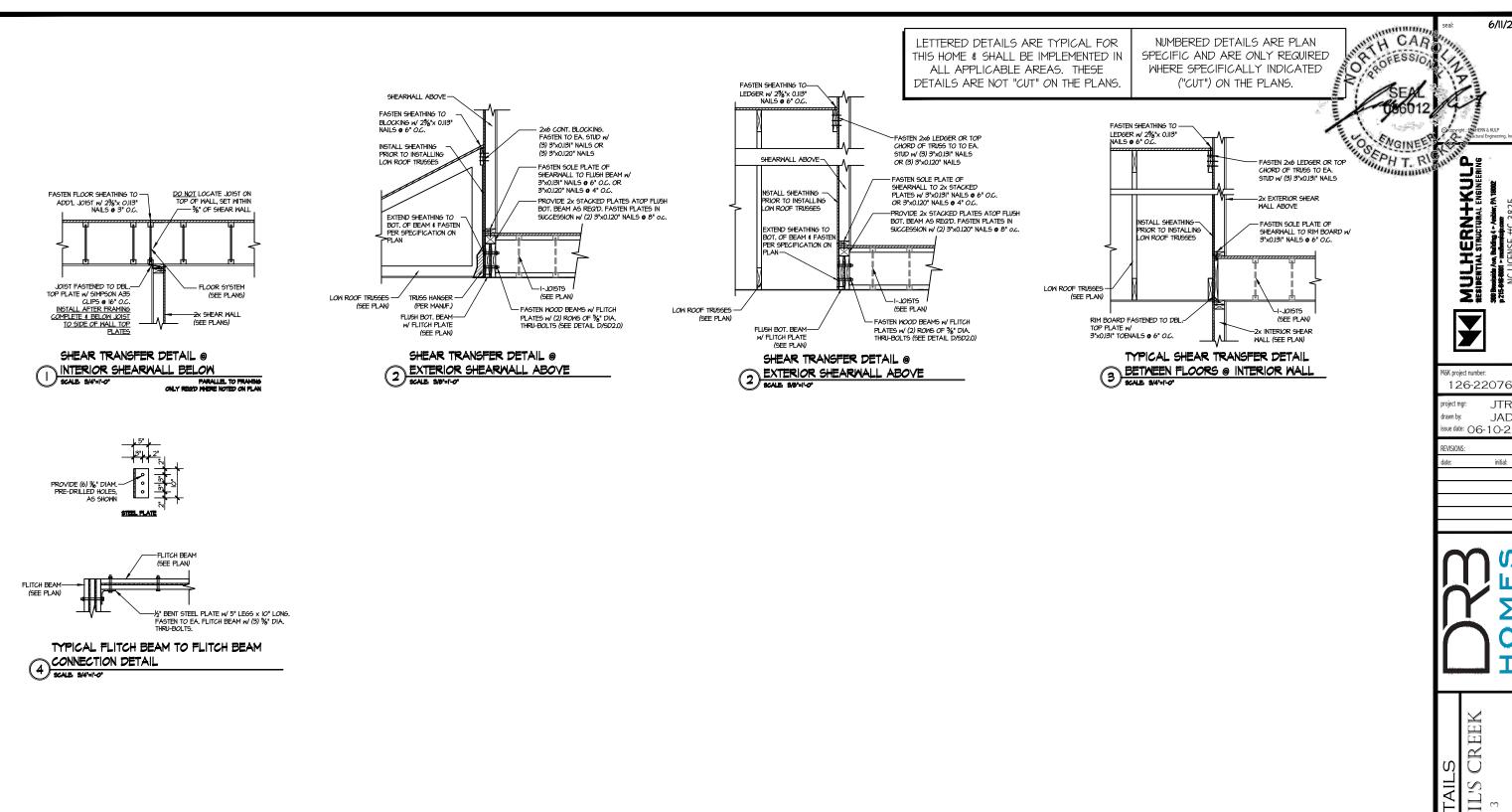
SD2.1A

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**JTR** 

**JAD** 

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NEIL rarm at NEI. Lot 31 - middleton 3 raleigh, nc

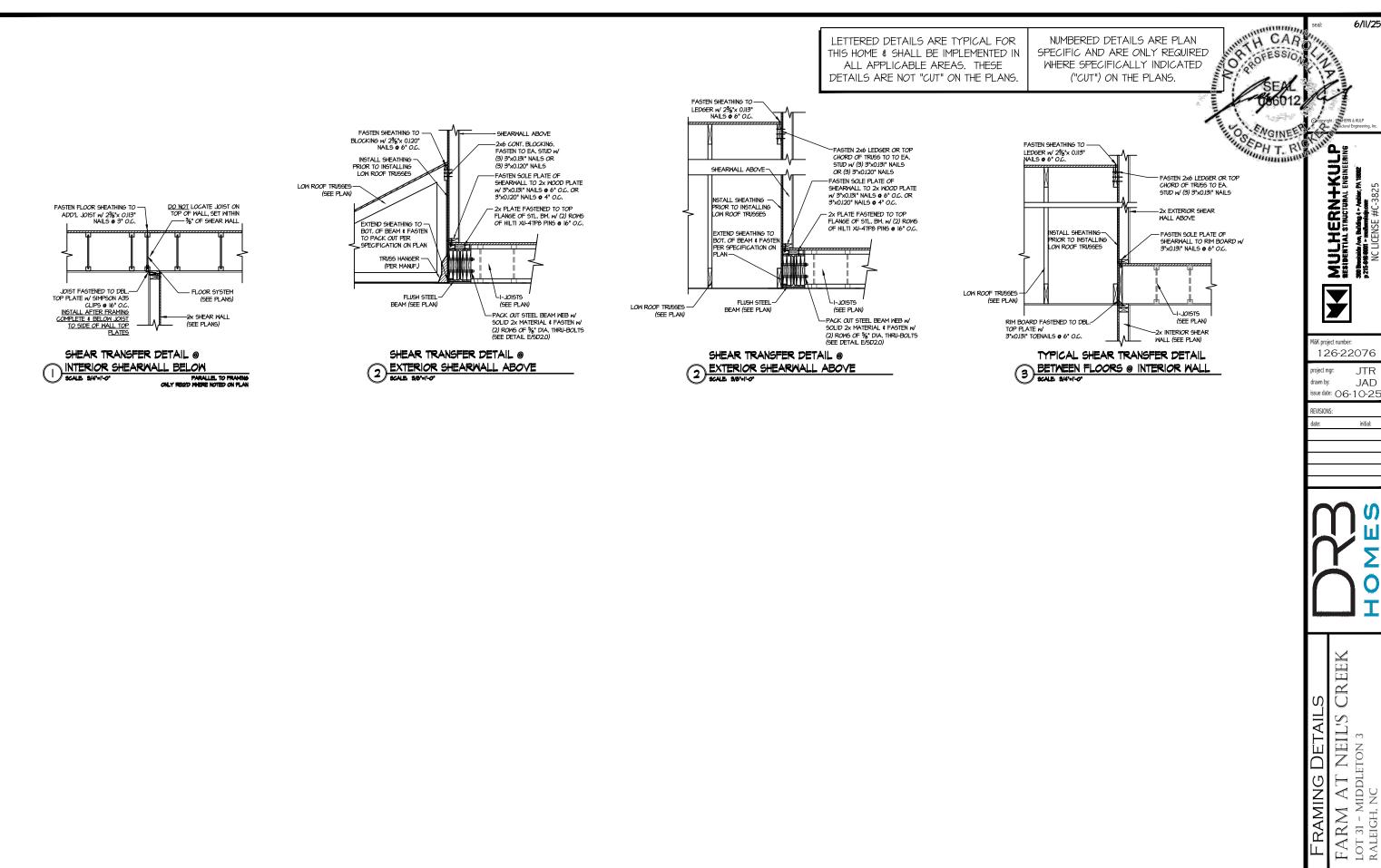
6/11/2

**JTR** 

**JAD** 

initial:

SD2.1B



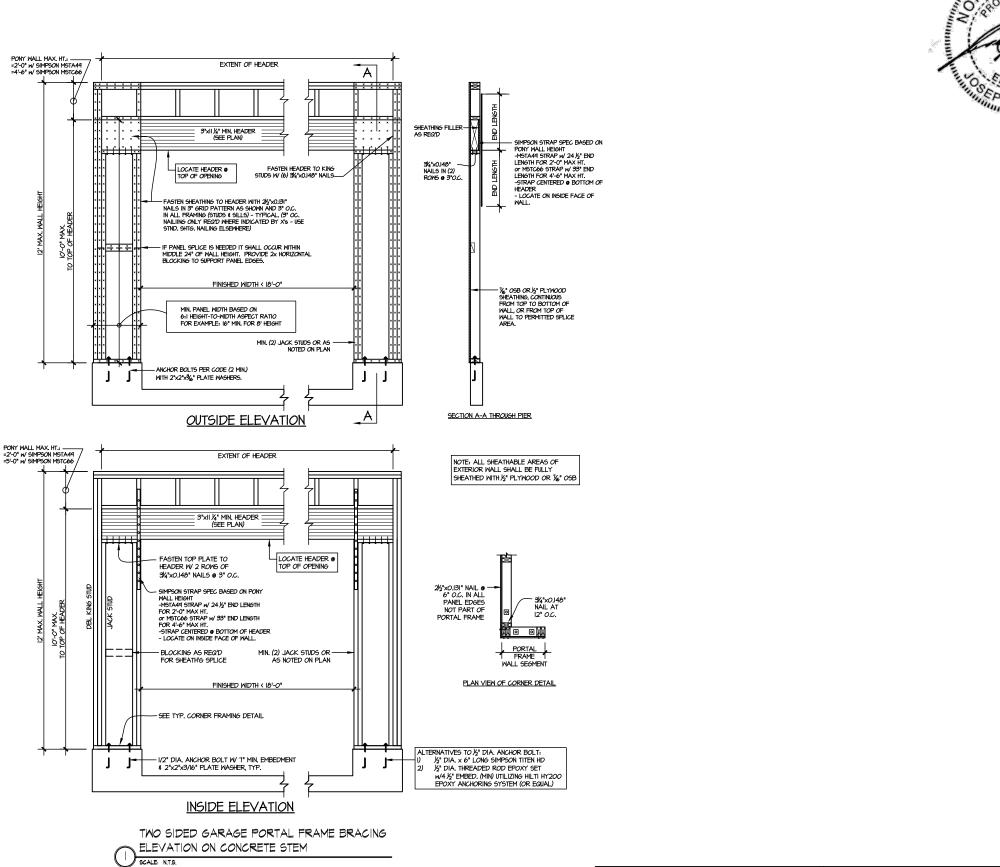
6/11/25

**JTR** 

**JAD** 

initial:

SD2.1C



LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

**SD2.2** 

RAMING

CREE

FARM AT NEIL'S LOT 31 - MIDDLETON 3 RALEIGH, NC

6/11/25

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING

Y

M&K project number: 126-22076

issue date: 06-10-2

drawn by:

REVISIONS:

**JTR** 

initial:

"H CAR

6/11/25

MUCHERN+KULP

RESIDENTIAL STRUCTURAL ENGINEERING Y

M&K project number:

126-22076

JTR drawn by: issue date: 06-10-2



FARM AT NEIL'S CREEK LOT 31 - MIDDLETON 3
RALEIGH, NC

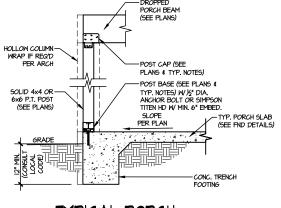
FRAMING DETAILS

**SD3.0** 

NUMBERED DETAILS ARE PLAN

SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED

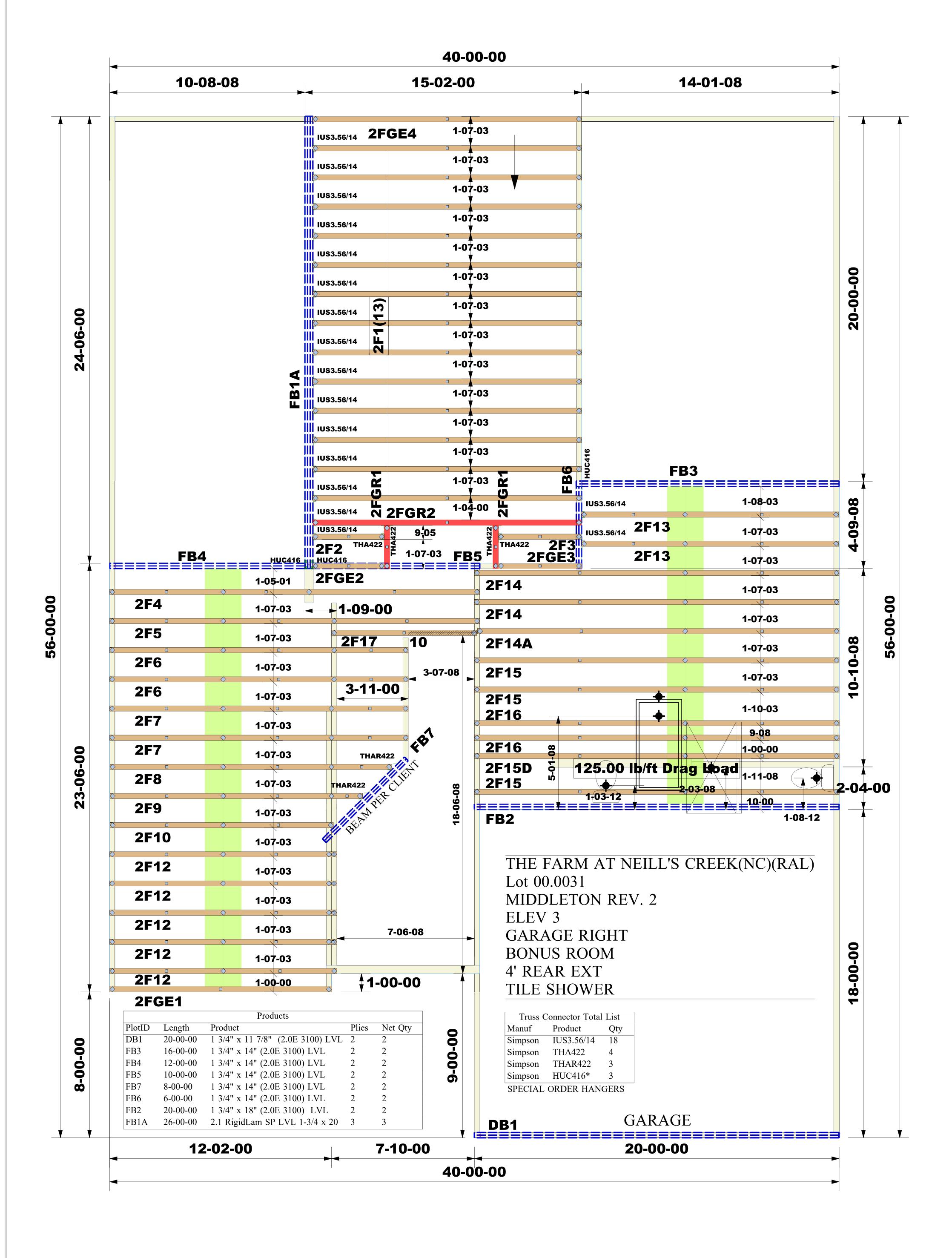
("CUT") ON THE PLANS.

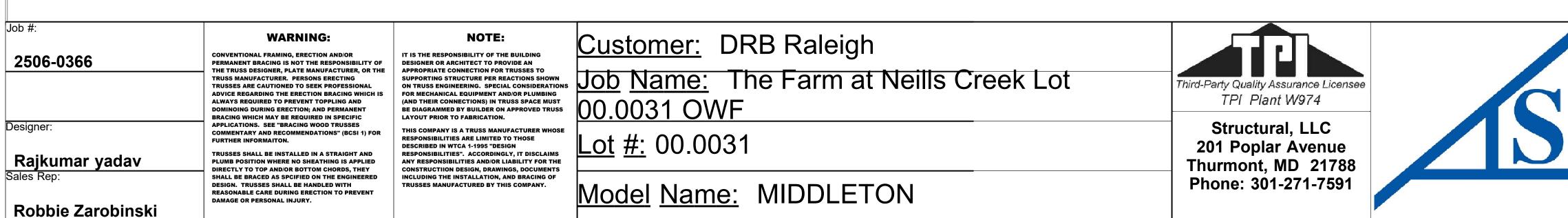


TYPICAL PORCH
POST CONNECTION DETAIL
SCALE: NONE SLAB ON GRADE SHOW SLAB ON GRADE SHOWN (SIM. e CRAWL & BSMT.)

> ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

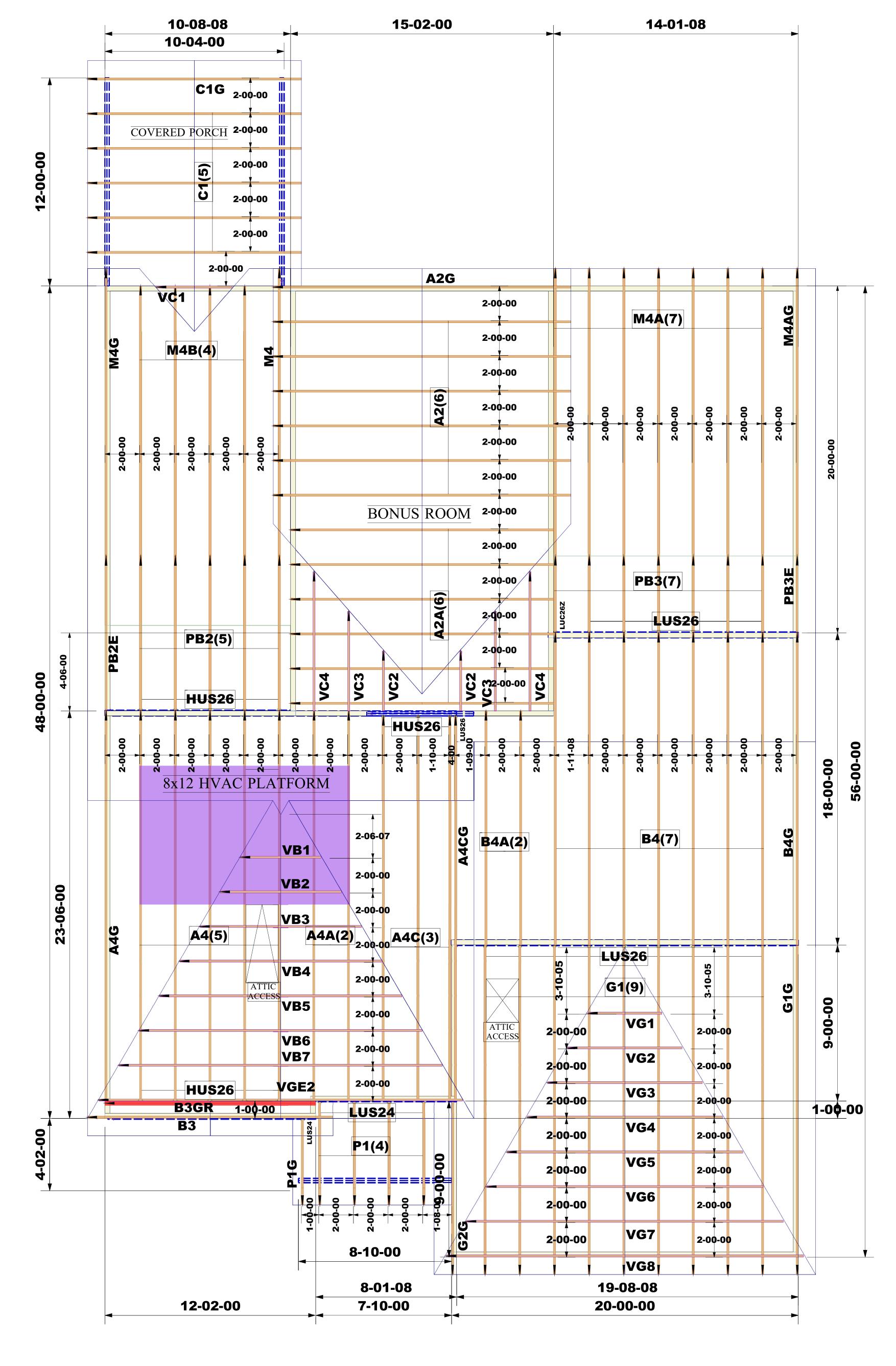
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN





## **ROOF TRUSS LAYOUT**

SCALE: NTS



THE FARM AT NEILL'S CREEK LOT 00.0031 (NC)(RAL) MIDDLETON REV.2 EL. 3 OPT.COVERED PORCH OPT.4' REAR EXTENSION **OPT.BONUS ROOM** 

Truss C	Connector Total	l List
Manuf	Product	Qty
Simpson	HUS26	14
Simpson	LUC26Z	1
Simpson	LUS24	7
Simpson	LUS26	18
Simpson	One H2.5A	120

NOTES:-\*EXTERIOR DIMENSIONS ARE TO STUD. \*TRUSSES @ 2' O/C U.N.O. \*INSTALL SIMPSON One H2.5A HURRICANE ANCHORS AT EACH BEARING POINTE.

Job #:	WARNING:	NOTE:	
2506-0367	CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	
Designer: Savvy Nath	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON.  TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE	L
Sales Rep:	DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT	CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.	<b> </b>

Robbie Zarobinski

GARAGE RIGHT

Customer: DRB Raleigh Job Name: The Farm at Neills Creek Lot 00.0031 Roof Lot #: 00.0031 Model Name: Middleton Rev. 2

