| | LIVE LOAD DEAD LOAD DEFLECTION (PSF) | | | | | | |
|--------------------|--------------------------------------|----|-------|-------|--|--|--|
| | ` ´ | | | | | | |
| FLOOR (primary) | 40 10 L/360 L/240 | | | | | | |
| FLOOR (secondary) | 40 | 10 | L/360 | L/240 | | | |
| ATTIC (w/ storage) | 20 | 10 | L/240 | L/180 | | | |
| ATTIC (no access) | 10 | 5 | L/240 | L/180 | | | |
| EXTERNAL BALCONY | 40 | 10 | L/360 | L/240 | | | |
| ROOF | 20 | 10 | L/240 | L/180 | | | |
| ROOF TRUSS | 20 20 L/240 L/180 | | | | | | |
| WIND LOAD | BASED ON 120 MPH (EXPOSURE B) | | | | | | |
| SEISMIC | BASED ON SEISMIC ZONES A, B & C | | | | | | |

STRUCTURAL NOTES:

- 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
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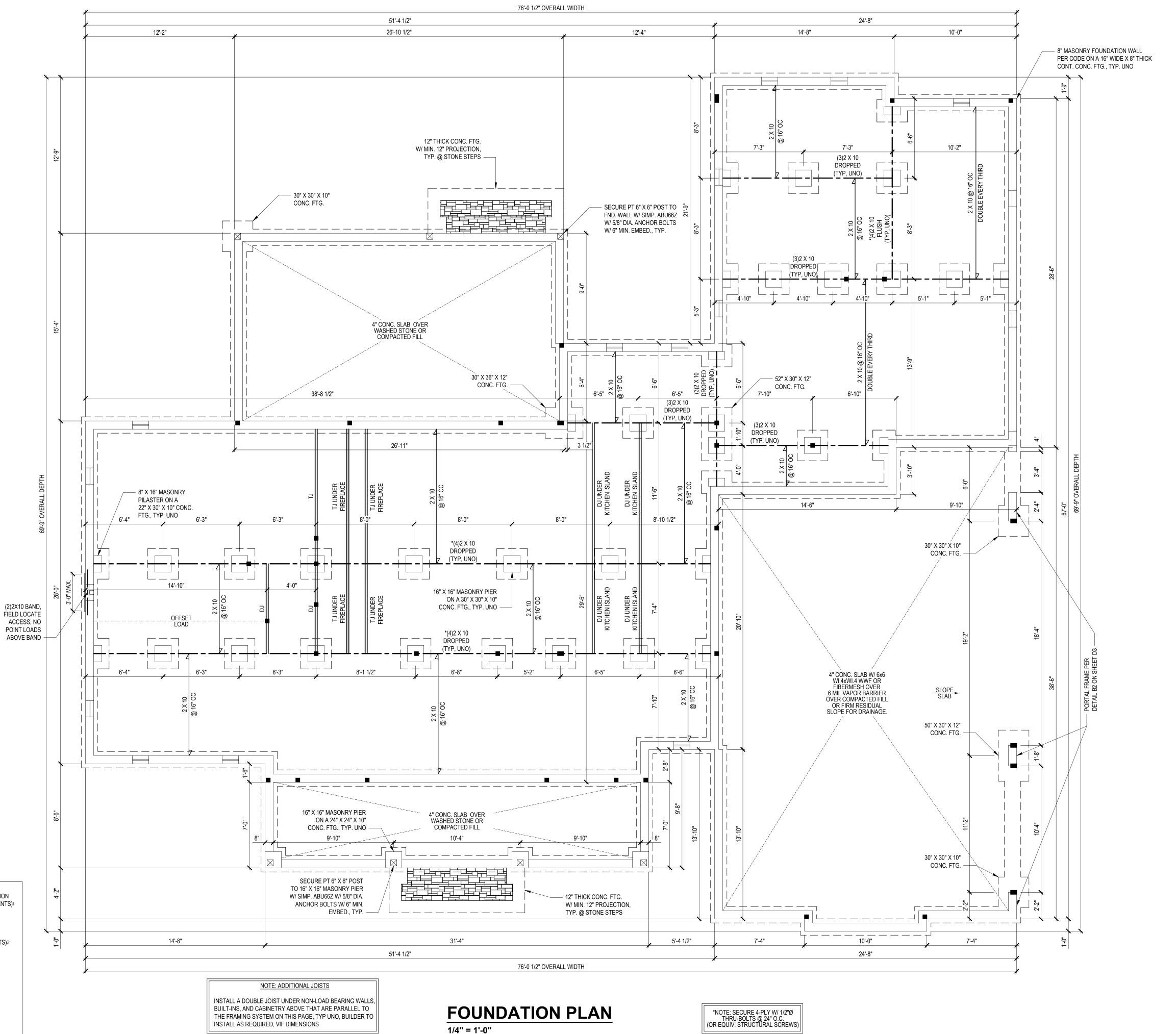
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- 3) ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND
 Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)
 (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
 ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)

 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
 TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE
 WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS
- 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).

 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- (UNO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT.
 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT 9) ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASCAURY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
- LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.

 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



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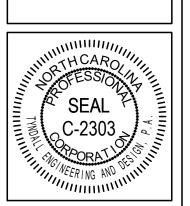


ENGINEERING & DESIGN, P.A.

TOTO 775-1200 = p.019778-9659

TOTO 775-1200 = p.019778-9659

WWW.syndallengineering.com



LLISON STEWART

FEWART RESIDENCE

FOUNDATION PLAN ST FLOOR FRAMING

Project #:
2501-010037

Date:
03/13/2025

Engineered By:
JA

DWG. Checked By:

PAT

Scale:

SEE PLAN

REVISIONS

Date: Remarks

No. Date: Remarks

2
3
4

Sheet Number

S1

1 **of** 7

2352 SQ. FT. OF CRAWL SPACE / 150 = 15.68 SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION 15.68 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ. FT. PER VENT = 18.0 VENTS REQ'D (BASED ON 8" X 16" VENTS)¹

-OR-

2352 SQ. FT. OF CRAWL SPACE / 1500 = 1.57 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 1.57 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 2.0 VENTS REQ'D (BASED ON 8" X 16" VENTS)2

) VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON PLAN, HOWEVER VENTS SHALL BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS AND TO PREVENT DEAD AIR POCKETS.

THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 OF THE CRAWL SPACE GROUND AREA WHERE THE REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS VENTILATION OF THE CRAWL SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. ONE FOUNDATION VENT SHALL BE WITHIN 3 FEET OF EACH CORNER OF THE BUILDING. TO PREVENT RAINWATER ENTRY WHEN THE CRAWL SPACE IS BUILT ON A SLOPED SITE, THE UPHILL FOUNDATION WALLS MAY BE CONSTRUCTED WITHOUT WALL VENT OPENINGS. VENT DAMS SHALL BE PROVIDED WHEN THE BOTTOM OF THE FOUNDATION VENT OPENING IS LESS THAN 4 INCHES ABOVE THE FINISHED

WALL VENTED CRAWL SPACES REQUIRE FULL COVERAGE GROUND VAPOR RETARDERS.

NO SCALE *

→ CRAWL SPACE VENTILATION CALCULATION

DESIGN LOADS

| | LIVE LOAD DEAD LOAD DEFLECTION (PSF) (PSF) | | | | | |
|--------------------|--|----|-------|-------|--|--|
| | LL TL | | | | | |
| FLOOR (primary) | 40 10 L/360 L/240 | | | | | |
| FLOOR (secondary) | 40 | 10 | L/360 | L/240 | | |
| ATTIC (w/ storage) | 20 | 10 | L/240 | L/180 | | |
| ATTIC (no access) | 10 | 5 | L/240 | L/180 | | |
| EXTERNAL BALCONY | 40 10 L/360 L/240 | | | | | |
| ROOF | 20 | 10 | L/240 | L/180 | | |
| ROOF TRUSS | 20 20 L/240 L/180 | | | | | |
| WIND LOAD | BASED ON 120 MPH (EXPOSURE B) | | | | | |
| SEISMIC | BASED ON SEISMIC ZONES A, B & C | | | | | |

STRUCTURAL NOTES:

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- ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM) ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
- ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER) 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS
- 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)

14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018

- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
- LEAST HORIZONTAL DIMENSION. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
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STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF
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- $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

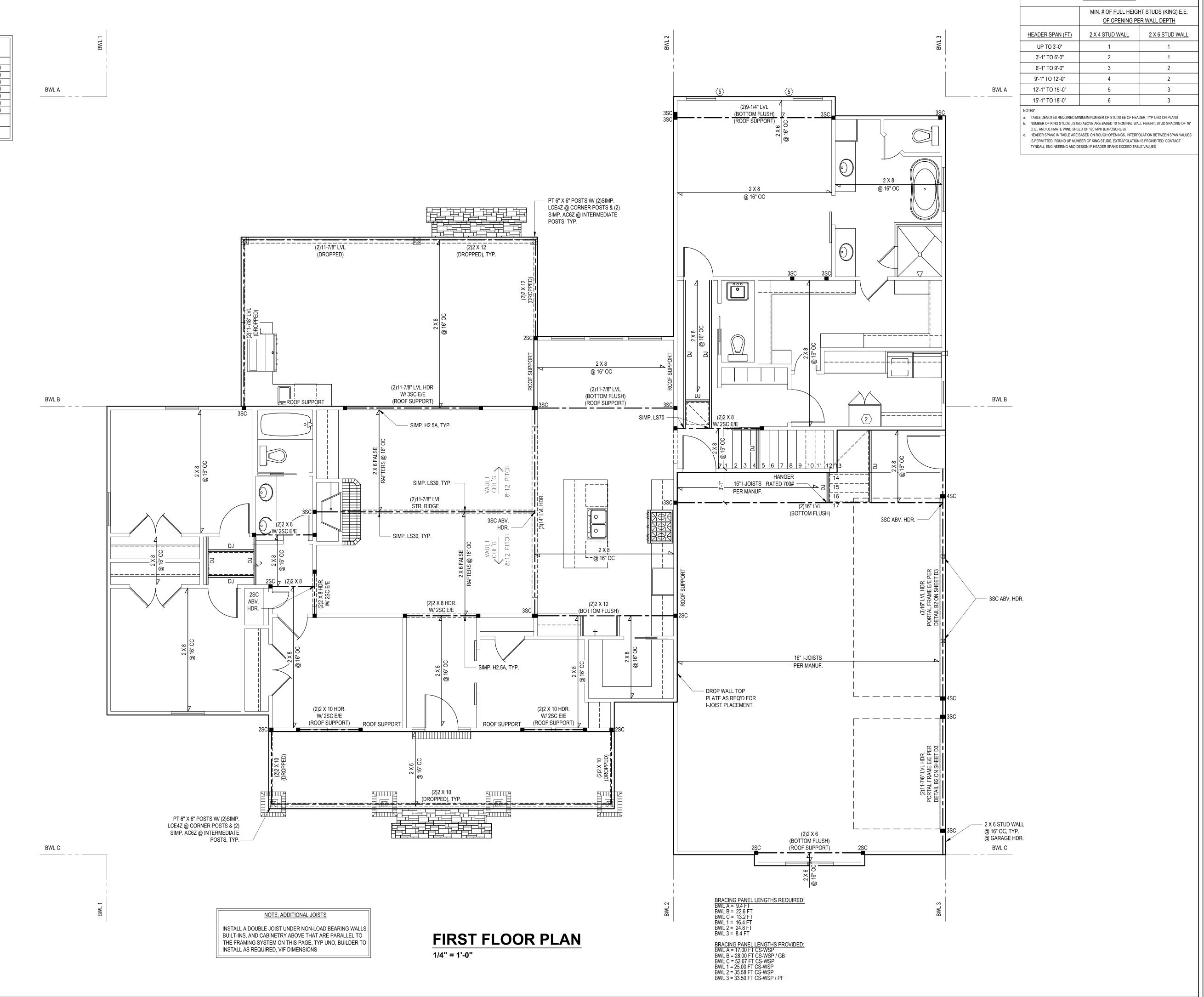
MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL

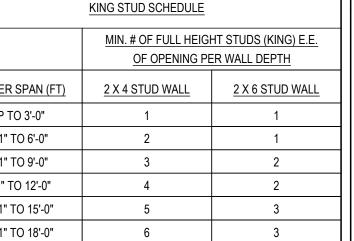
- 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF

WALL HEIGHT 4 SHEATH INTERIOR & EXTERIOR

BE AS FOLLOWS:

- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE





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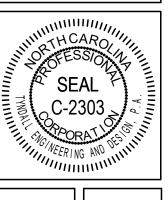
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recommendations, etc. presented in these documents were





2501-010037 03/13/2025 Engineered By: DWG. Checked By: PAT SEE PLAN

REVISIONS Date:

Sheet Number

2 of 7

DESIGN LOADS

| | LIVE LOAD DEAD LOAD DEFLECTION (PSF) | | | | | |
|--------------------|--------------------------------------|----|-------|-------|--|--|
| | (r s.) LL TL | | | | | |
| FLOOR (primary) | 40 10 L/360 L/240 | | | | | |
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BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT - 30" ADJACENT TO OPENINGS GREATER THAN

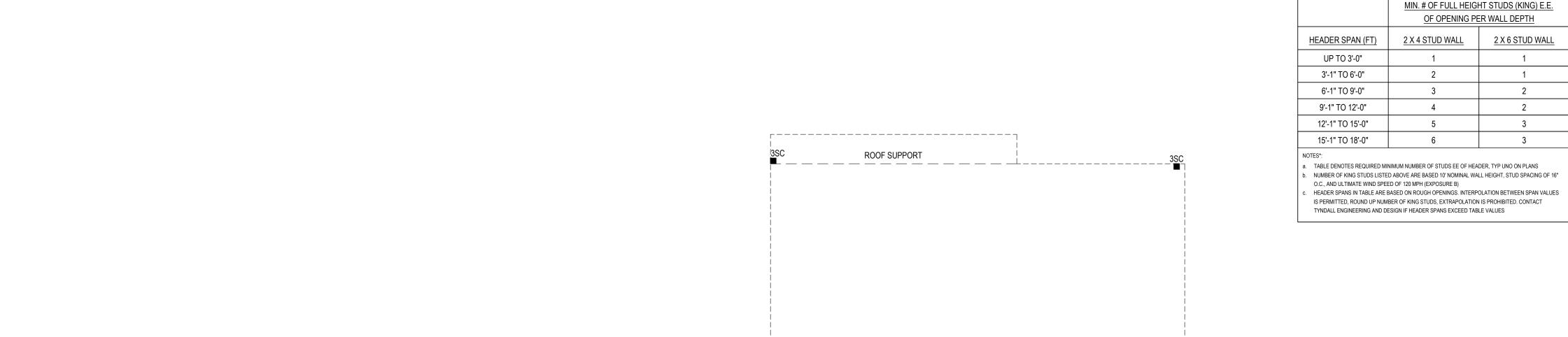
67% AND LESS THAN 85% OF WALL HEIGHT.

- 48" FOR OPENINGS GREATER THAN 85% OF

4 SHEATH INTERIOR & EXTERIOR

WALL HEIGHT

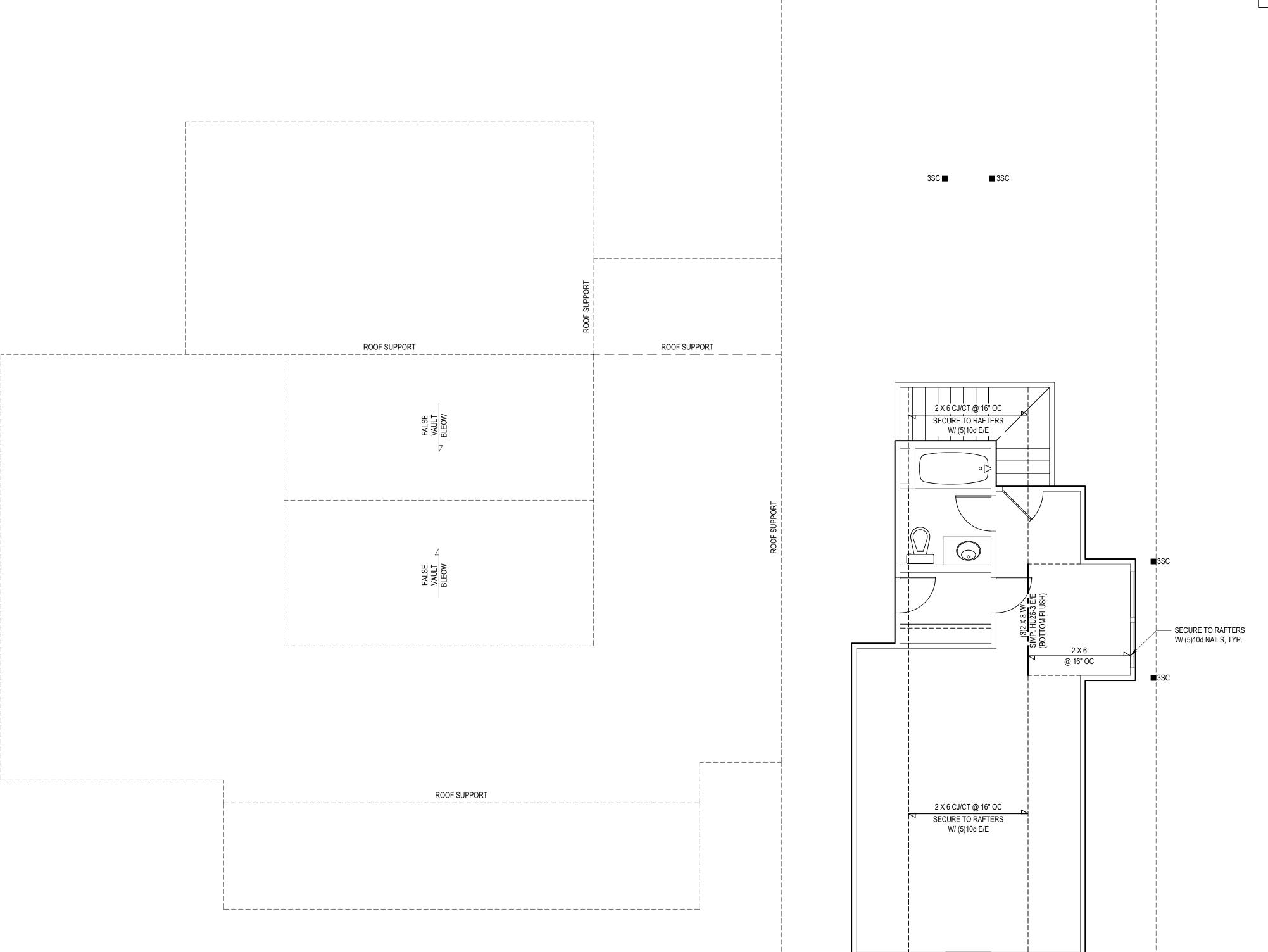
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- (5) MINIMUM 800# HOLD-DOWN DEVICE



ROOF SUPPORT

L-----

L-----



SECOND FLOOR PLAN

1/4" = 1'-0"

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KING STUD SCHEDULE



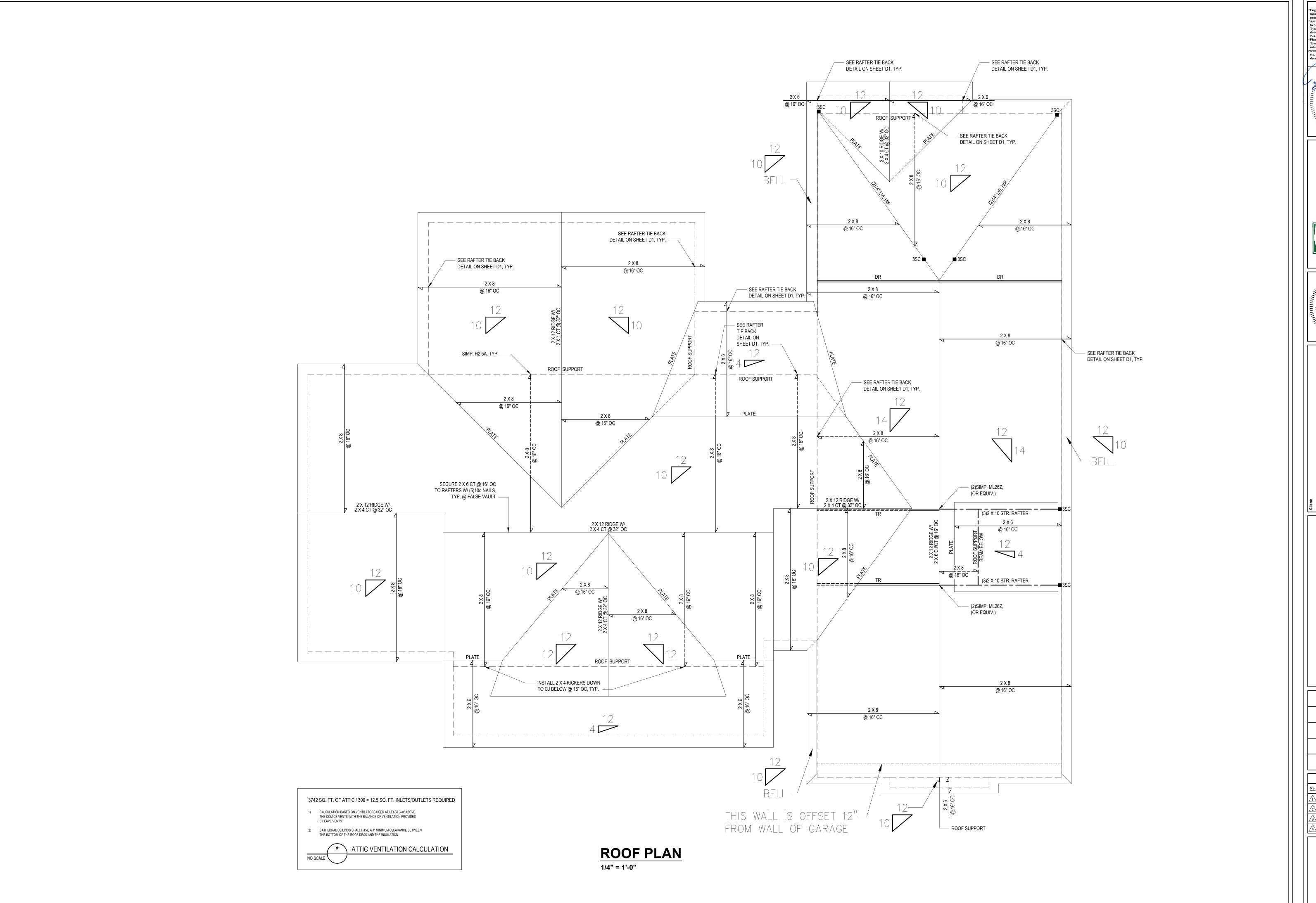


2501-010037 03/13/2025 Engineered By: DWG. Checked By:

PAT SEE PLAN

REVISIONS Date:

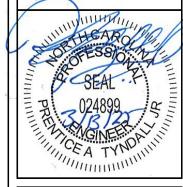
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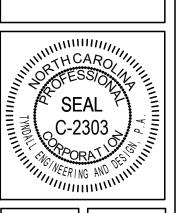


ENGINEERING & DESIGN, P.A.

199 778-1200 = 1919 778-9488

199 \$hipwash Drive = Germer = North Caroline = 27339

www.tyndellengineering.com



ON STEWART
ART RESIDENCE

ROOF PLAN

Project #:

2501-010037

Date:

03/13/2025

Engineered By:

JA

DWG. Checked By:

PAT
Scale:
SEE PLAN

 REVISIONS

 No.
 Date:
 Remarks

 1
 2

Sheet Number

S4

4 of 7

STRUCTURAL NOTES

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DESIGN LOADS:

| | LIVE LOAD (PSF) | LIVE LOAD DEAD LOAD DEFLECTION (PSF) (PSF) | | | | | |
|---------------------------|--------------------|--|-------|-------|--|--|--|
| | (* 5.) | LL | | | | | |
| ALL FLOORS | 40 | 10 | L/360 | L/240 | | | |
| ATTIC (w/ walk up stairs) | 30 | 10 | L/360 | L/240 | | | |
| ATTIC (pull down access) | 20 | 10 | L/240 | L/180 | | | |
| ATTIC (no access) | 10 | 5 | L/240 | L/180 | | | |
| EXTERNAL BALCONY | 40 | 10 | L/360 | L/240 | | | |
| ROOF | 20 | 10 | L/240 | L/180 | | | |
| ROOF TRUSS | 20 | 20 | L/240 | L/180 | | | |
| WIND LOAD | | BASED ON 120 MPH (EXPOSURE B) | | | | | |
| SEISMIC | | SEISMIC ZONES A, B & C | | | | | |

- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO. ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL. ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.) ALL LSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.) ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
- ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2"Ø x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- 12) WALL AND ROOF CLADDING VALUES:

WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE. ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

- 39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12 36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12
- 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12 **MEAN ROOF HEIGHT 30'-0" OR LESS
- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 14) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 20) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

DEFINITIONS FOR COMMON ABBREVIATIONS

| ALT | = | ALTERNATE | MANUF | = | MANUFACTURER |
|-------|---|-----------------------|-------|---|------------------------|
| CANT | = | CANTILEVER | MAX | = | MAXIMUM |
| CJ | = | CEILING JOIST | MIN | = | MINIMUM |
| CMU | = | CONCRETE MASONRY UNIT | NOM | = | NOMINAL |
| COL | = | COLUMN | O.C. | = | ON CENTER |
| CONC | = | CONCRETE | PL | = | POINT LOAD |
| CONT | = | CONTINUOUS | PT | = | PRESSURE TREATED |
| CT | = | COLLAR TIE | REINF | = | REINFORCED |
| DBL | = | DOUBLE | REQ'D | = | REQUIRED |
| DIA | = | DIAMETER | RJ | = | ROOF JOIST |
| DJ | = | DOUBLE JOIST | RS | = | ROOF SUPPORT |
| DR | = | DOUBLE RAFTER | SC | = | STUD COLUMN |
| DSP | = | DOUBLE STUD POCKET | SCH | = | SCHEDULE |
| EA | = | EACH | SPEC | = | SPECIFIED |
| EE | = | EACH END | TH | = | THICK |
| FJ | = | FLOOR JOIST | TJ | = | TRIPLE JOIST |
| FND | = | FOUNDATION | TRTD | = | TREATED |
| FTG | = | FOOTING | TSP | = | TRIPLE STUD POCKET |
| GALV | = | GALVANIZED | TYP | = | TYPICAL |
| HORIZ | = | HORIZONTAL | UNO | = | UNLESS NOTED OTHERWISE |
| HT | = | HEIGHT | W | = | WIDE FLANGE BEAM |
| JSC | = | JACK STUD | WWF | = | WELDED WIRE FABRIC |
| KS | = | KING STUD | XJ | = | EXTRA JOIST |
| | | | | | |

MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

| POST SIZE | MAX. POST HEIGHT** |
|-----------|--------------------|
| 4 x 4 | 8'-0" |
| 6 x 6 | 20'-0" |
| *** | OVER 20'-0" |
| | |

- * THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
- FROM TOP OF FOOTING TO BOTTOM OF GIRDER DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF
- THESE METHODS: A. THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS
- ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4) ABOVE. LATERAL BRACING IS NOT REQUIRED. B. 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN
- BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED BOLT AT EACH END OF THE BRACE.
- C. FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

| POST SIZE | MAX. TRIBUTARY AREA | MAX. POST HEIGHT | EMBEDMENT DEPTH | CONCRETE DIAMETER |
|-----------|------------------------|---------------------|--------------------|----------------------|
| 4 x 4 | 48 SQ. FT. | 4'-0" | 2'-6" | 1'-0" |
| 6 x 6 | 120 SQ. FT. | 6'-0" | 3'-6" | 1'-8" |

BEAM SPLICE

BEAM PER PLAN

POST PER

- D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8"Ø HOT
- DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER. FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.

SIMP. CS16 COIL STRAP

EACH SIDE OF BEAM -

POST CAP

PER PLAN -

BEAM PER PLAN

| 3 0.35 0.55 0.30 38 or 30 15 or 5/10 cont 19 5/13 f 0 5/13 4 0.35 0.55 0.30 38 or 30 15 or 5/10 cont 19 10/15 10 10/15 | CLIMATE ZONES | FENESTRATION U-FACTOR b,j | SKYLIGHT ^b U-FACTOR | GLAZED FENESTRATION SHGC ^{b,<u>k</u>} | CEILING ^m R-VALUE | WOOD FRAMED WALL R-VALUE | MASS WALL R-VALUE | FLOOR R-VALUE | BASEMENT ^{c,} º WALL R-VALUE | SLAB ^d R-VALUE AND DEPTH | CRAWL SPACE CWALL R-VALUE |
|---|------------------|------------------------------|-----------------------------------|--|---------------------------------|---|------------------------------------|------------------------|---|---|---------------------------|
| 0.35 0.55 0.30 cont j 13 + 2.5 h 5/10 cont 19 10/15 10 10/15 | 3 | 0.35 | 0.55 | 0.30 | 38 or 30 cont | 15 or 13 + 2.5 | <u>5/13 or</u> <u>5/10 cont</u> | 19 | <u>5/13</u> ^f | 0 | 5/13 |
| 5 38 or 30 19, or 13 + 5 1 13/17 or | 4 | 0.35 | 0.55 | 0.30 | | h 1 | | 19 | <u>10/15</u> | 10 | 10/15 |
| 0.35 NR Cont or 15 + 3 h 13/12.5 cont 30 9 10/15 10 10/19 | 5 | 0.35 | 0.55 | NR | 38 or 30 cont | ⁿ 19, or 13 + 5 or 15 + 3 | 13/17 <u>or</u> 13/12.5 cont | 30 ⁹ | <u>10/15</u> | 10 | 10/19 |

TABLE N1102.1 CLIMATE ZONES 3-5

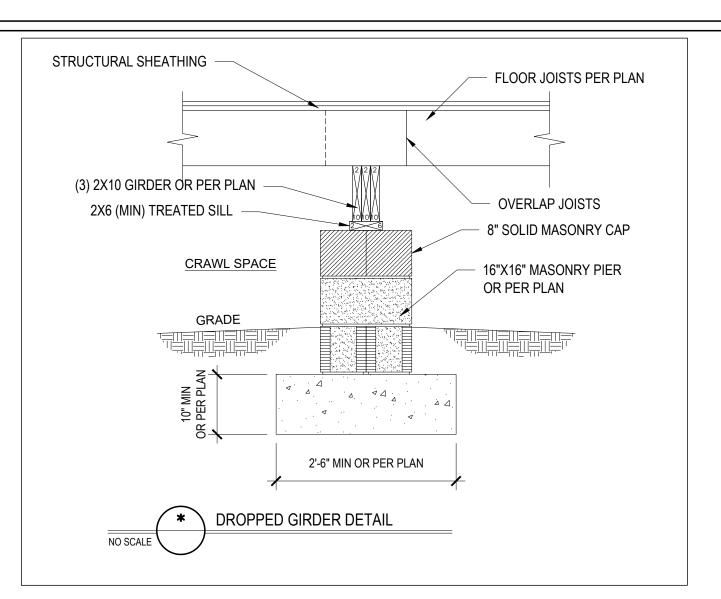
NO SCALE

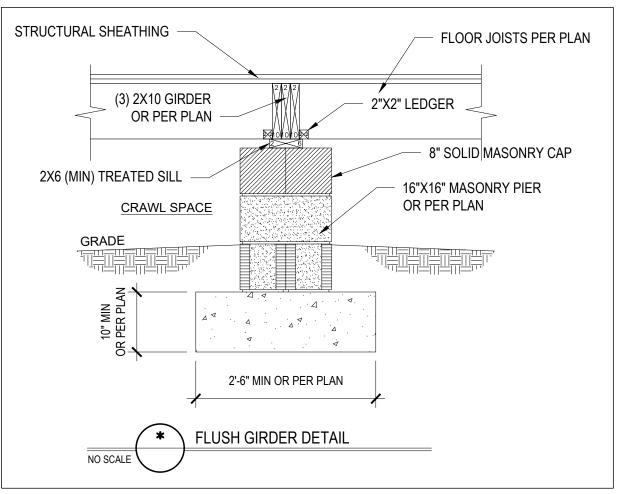
- 1. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.
- b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIEN' (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.
- c. $\underline{"10/15"}$ MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME
- OR R-15 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL. d. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM
- OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. R-5 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS.
- f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE N1101.7 AND TABLE N1101.7.
- g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
- h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 INSULATED SHEATHING. "15+3" MEANS R-15 CAVITY INSULATION. PLUS R-3 INSULATED SHEATHING. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR,
- INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVITY
- INSULATION PLUS R-2.5 SHEATHING i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL.
- j. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH OF THE ATTIC ROOF DECK.
- m. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE. n. R-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2X4 WALL IS NOT DEEMED TO COMPLY.

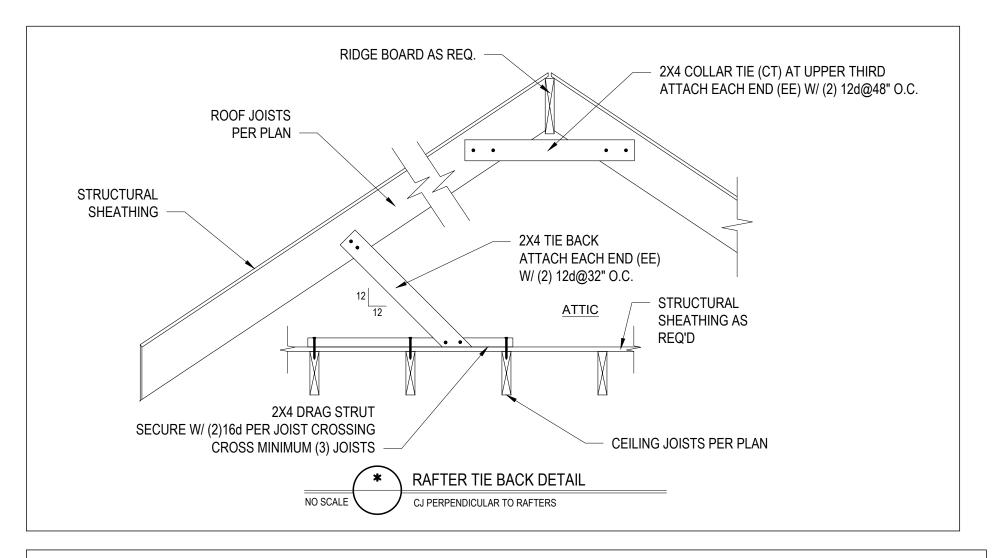
9. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

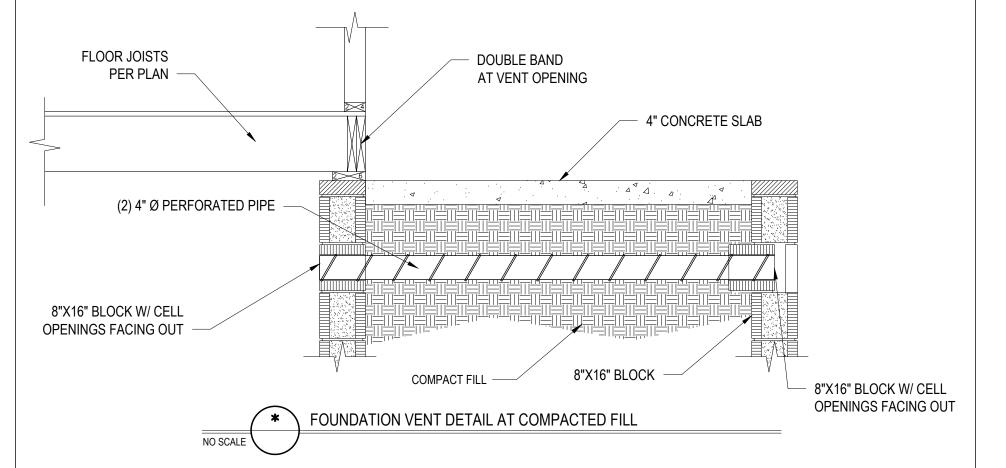
BOTTOM FLUSH BEAM PER PLAN -- INSTALL 2 X 4 KICKERS TO JOISTS W/ (2)10d 2 X 4 DRAG STRUT, NAILS @ 4'-0" OC (MAX) SECURE W/ (2)16d PER JOIST CROSSING CROSS MINIMUM (2) JOISTS -HANGERS AS REQ'D CEILING / FLOOR JOISTS PER PLAN -* LATERAL BEAM BRACING WHEN BOTTOM FLUSH BEAMS ARE ≥ TWICE THE DEPTH OF THE ADJACENT JOISTS

BEAM SPLICE AT COLUMN

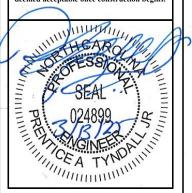


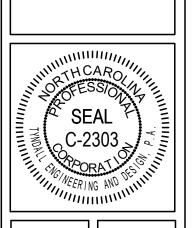






ocedures or safety precaution. Any deviations or discrepancies on plans ar to be brought to the immediate attention of Please review these documents carefully Tyndall Engineering & Design, P.A. will interpret that all dimensions, etc. presented in these documents were

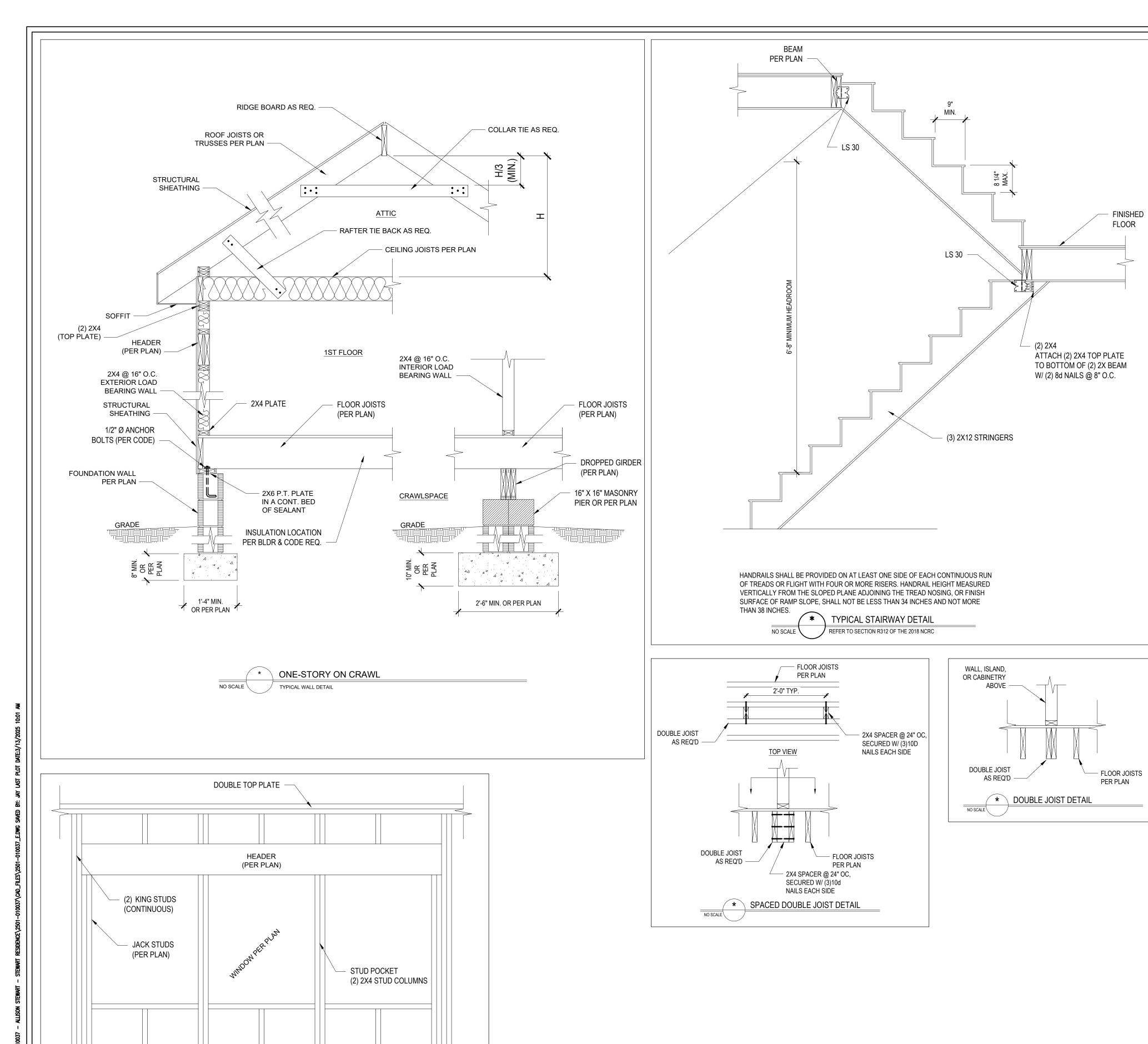




2501-010037 03/13/2025 **Engineered By:** DWG. Checked By:

PAT SEE PLAN REVISIONS Date:

Sheet Number



SOLE PLATE

* TYPICAL WINDOW STUD POCKET DETAIL

NO SCALE

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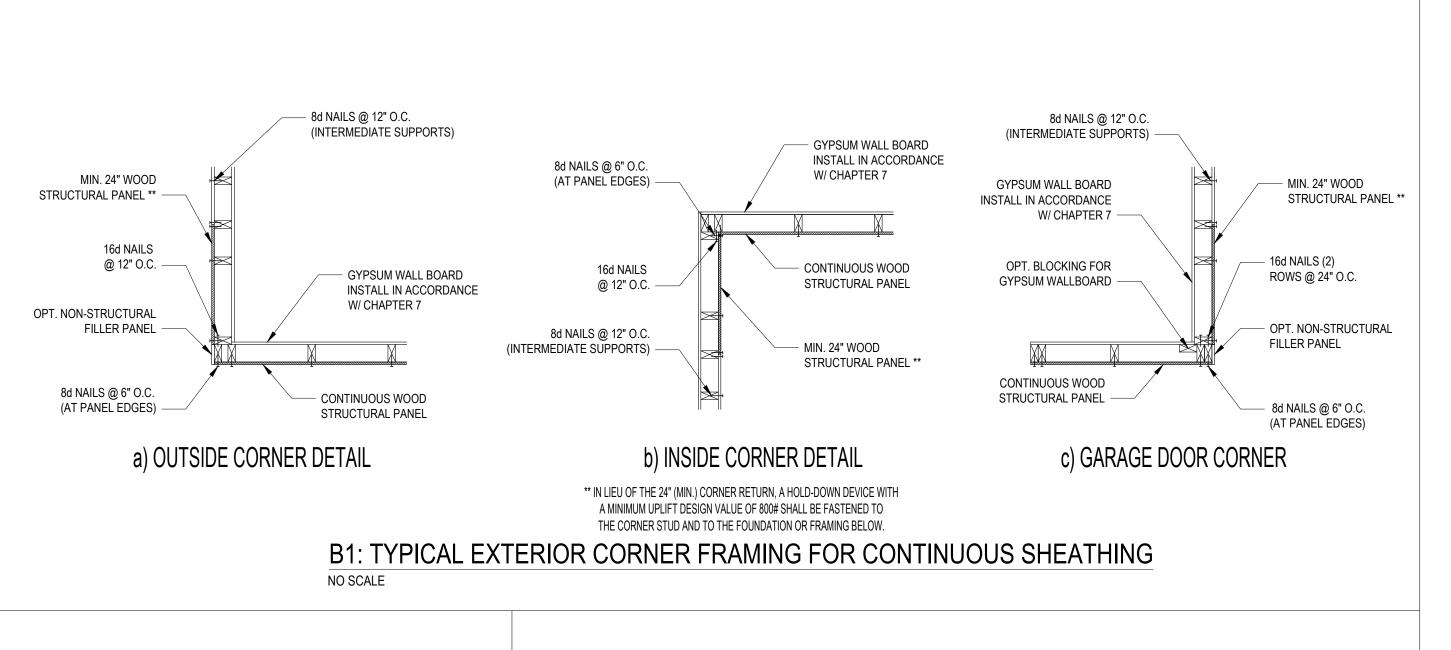
*Please review these documents carefully. Tyndall Engineering & Design, P.A. will Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction beg STEWART STANDARD DETAILS 2501-010037 03/13/2025 Engineered By: DWG. Checked By: PAT SEE PLAN **REVISIONS**



Sheet Number

D2

6 of 7



STRUCTURAL SHEATHING NOTES

 DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.

 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC
 BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL

1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.

4. INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)

2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0"

3 3/8" WOOD STRUCTURAL PANEL)WSP) SECURE W/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS

(ISOLATED PANELS) OR 4'-0" (CONTINUOUS

5. EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD

AS PRESCRIBED IN SECTION R602.10.3 (UNO)
6. ALL SHEATHABLE SURFACES OF EXTERIOR WALLS
(INCLUDING AREAS ABOVE AND BELOW OPENINGS AND
GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED
WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH
A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE
SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6"

O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

7. MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:

- 24" ADJACENT TO OPENINGS NOT MORE THAN 67%

OF WALL HEIGHT
- 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT
- 48" FOR OPENINGS GREATER THAN 85% OF WALL

4 SHEATH INTERIOR AND EXTERIOR

8. FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3 (4). IN LIEU OF A CORNER RETURN, EITHER A MINIMUM 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR

FRAMING BELOW

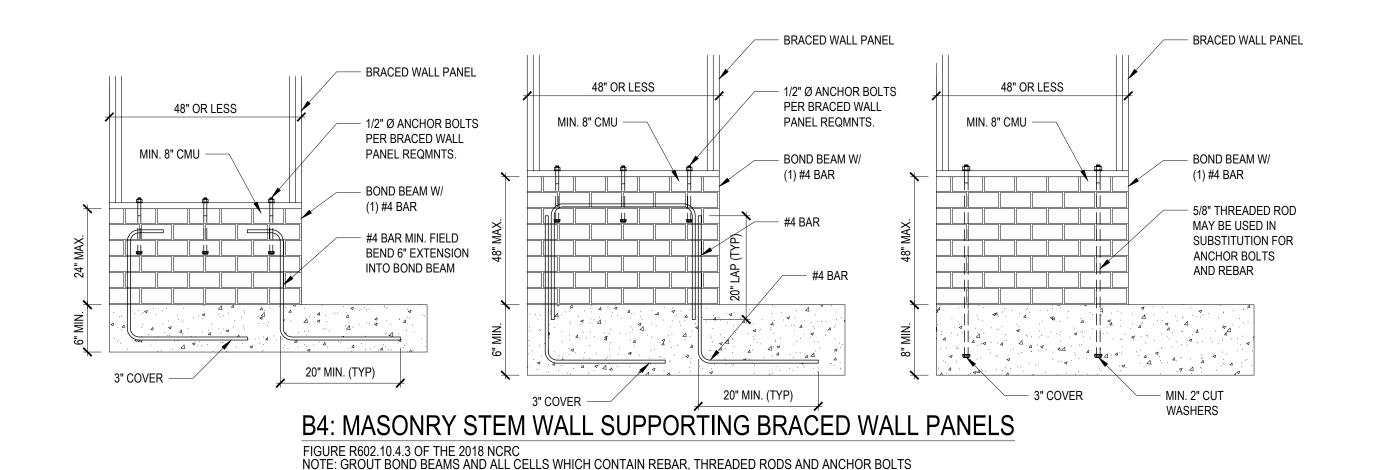
(5) MINIMUM 800# HOLD-DOWN DEVICE

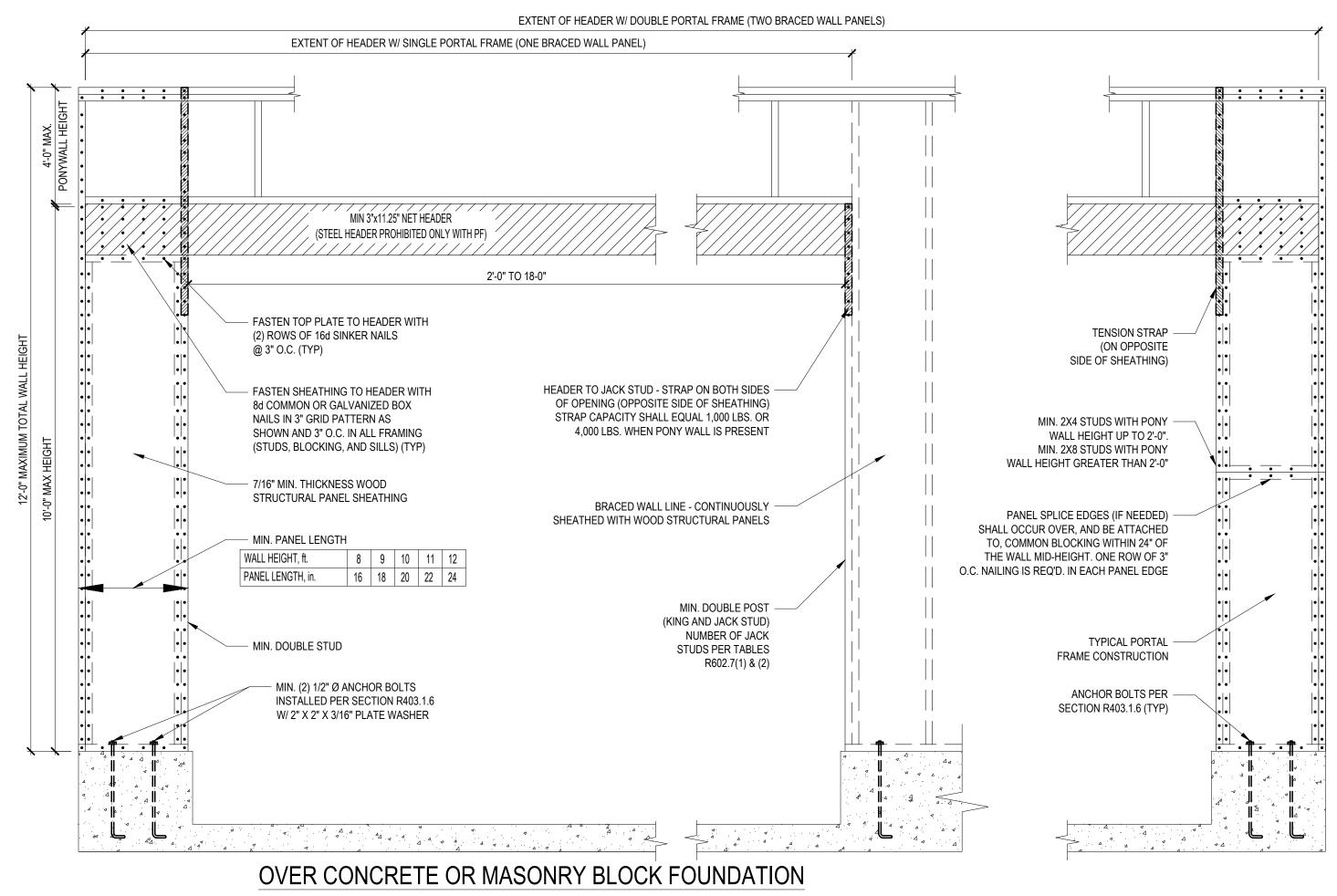
| | | | REQUIRED | CONNECTION |
|--------|--------------------------|----------------|-------------------------------|-------------------------------|
| METHOD | MATERIAL | MIN. THICKNESS | @ PANEL EDGES | @ INTERMEDIATE SUPPORTS |
| CS-WSP | WOOD STRUCTURAL PANEL | 3/8" | 6d COMMON NAILS @ 6" O.C. | 6d COMMON NAILS @ 12" O.C. |
| GB | GYPSUM BOARD | 1/2" | 5d COOLER NAIL** @ 7" O.C. | 5d COOLER NAIL** @ 7" O.C. |
| WSP | WOOD STRUCTURAL PANEL | 3/8" | 6d COMMON NAILS @ 6" O.C. | 6d COMMON NAILS @ 12" O.C. |

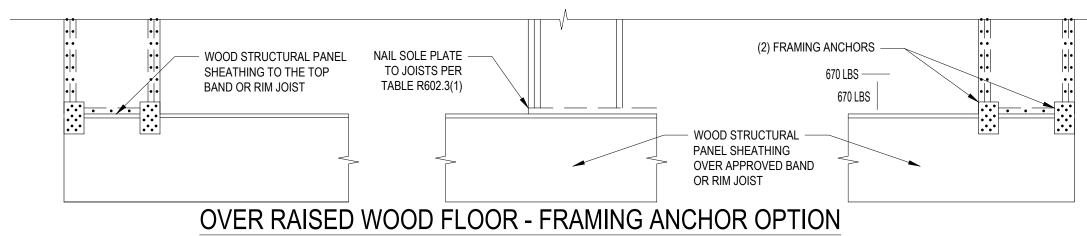
**OR EQUIVALENT PER TABLE R702.3.5

B3: BRACE WALL PANEL CONNECTIONS

NO SCALE







OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION

(WHEN PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST)

ATTACH SHEATHING TO BAND
OR RIM JOIST WITH 8d COMMON
NAILS 3" O.C. TOP AND BOTTOM

WOOD STRUCTURAL
PANEL SHEATHING
OVER APPROVED BAND
OR RIM JOIST

OVER RAISED WOOD FLOOR - OVERLAP OPTION (WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

B2: METHOD PF: PORTAL FRAME CONSTRUCTION
FIGURE R602.10.1

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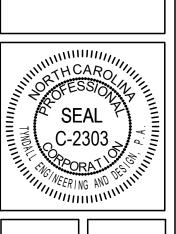


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ALLISON STEWART

Plan:
STEWART RESIDENCE

SHEATHING DETAILS

Project #:
2501-010037

Date:
03/13/2025

Engineered By:
JA

DWG. Checked By:
PAT

Scale:

 REVISIONS

 No.
 Date:
 Remarks

 1
 2

 3
 3

SEE PLAN

Sheet Number

D3