

RE: 2412-1161-D - Stonefield Rev 3-Elev 4-Roof

Trenco  
 818 Soundside Rd  
 Edenton, NC 27932

**Site Information:**

Project Customer: DRB Raleigh Project Name: DRB Raleigh Model Track  
 Lot/Block: Subdivision: DRB Raleigh  
 Model: Stonefield Rev 3  
 Address:  
 City: State: NC

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: IRC2021/TPI2014  
 Wind Code: ASCE 7-16  
 Wind Speed: 120 mph  
 Roof Load: 40.0 psf  
 Mean Roof Height (feet): 25

Design Program: MiTek 20/20 25.2  
 Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-16  
 Floor Load: N/A psf  
 Exposure Category: B

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I74084129	E1GE	6/11/25	35	I74084163	D1GE	6/11/25
2	I74084130	E1	6/11/25	36	I74084164	A1GR	6/11/25
3	I74084131	P1GE	6/11/25	37	I74084165	B1GR	6/11/25
4		P1B	6/11/25	38	I74084166	B1J	6/11/25
5	I74084133	P1A	6/11/25	39	I74084167	B1I	6/11/25
6	I74084134	G1GE	6/11/25	40	I74084168	B1H	6/11/25
7	I74084135	G1	6/11/25	41	I74084169	B1G	6/11/25
8	I74084136	G1A	6/11/25		I74084170	B1F	6/11/25
9	I74084137	E1A	6/11/25	43	I74084171	B1E	6/11/25
10	I74084138	E1AGE	6/11/25	44	I74084172	B1D	6/11/25
11	I74084139	G1DGE	6/11/25	45	I74084173	B1C	6/11/25
12	I74084140	G1C	6/11/25	46	I74084174	B1B	6/11/25
13		G1B	6/11/25	47	I74084175	B1A	6/11/25
14	I74084142	M1I	6/11/25	48	I74084176	B1	6/11/25
	I74084143	M1H	6/11/25	49	I74084177	A1D	6/11/25
16	I74084144	M1F	6/11/25	50	I74084178	A1C	6/11/25
17	I74084145	M1E	6/11/25		I74084179	A1B	6/11/25
18	I74084146	M1K	6/11/25	52	I74084180	A1A	6/11/25
19	I74084147	M1J	6/11/25	53	I74084181	A1	6/11/25
20	I74084148	P1AGE	6/11/25	54	I74084182	V4	6/11/25
21	I74084149	P1C	6/11/25	55	I74084183	V5	6/11/25
22		V7	6/11/25	56	I74084184	V6	6/11/25
23	I74084151	V8	6/11/25	57	I74084185	V1	6/11/25
	I74084152	P1D	6/11/25	58	I74084186	V2	6/11/25
25	I74084153	E1D	6/11/25	59	I74084187	V3	6/11/25
26	I74084154	E1C	6/11/25		I74084188	M1G	6/11/25
27	I74084155	V10	6/11/25	61	I74084189	M1B	6/11/25
28	I74084156	V9	6/11/25	62	I74084190	M1A	6/11/25
29	I74084157	P1BGE	6/11/25	63	I74084191	M1D	6/11/25
30	I74084158	E1BGE	6/11/25	64	I74084192	M1C	6/11/25
31	I74084159	H1GE	6/11/25	65	I74084193	M1	6/11/25
32	I74084160	C1GR	6/11/25	66	I74084194	M2G	6/11/25
	I74084161	D1GR	6/11/25	67	I74084195	M2B	6/11/25
34	I74084162	C1GE	6/11/25	68	I74084196	M2A	6/11/25

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Structural, LLC.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 11, 2025

RE: 2412-1161-D - Stonefield Rev 3-Elev 4-Roof

Trenco  
818 Soundside Rd  
Edenton, NC 27932

No.	Seal#	Truss Name	Date
69	I74084197	M2D	6/11/25
70	I74084198	M2C	6/11/25
71	I74084199	M2	6/11/25
72	I74084200	PB1	6/11/25
73	I74084201	B1L	6/11/25
74	I74084202	B1O	6/11/25
75	I74084203	B1P	6/11/25
76		B1N	6/11/25
77	I74084205	B1M	6/11/25
78	I74084206	PB1B	6/11/25
79	I74084207	PB1A	6/11/25
80	I74084208	B1K	6/11/25
81	I74084209	B2GR	6/11/25
82	I74084210	C2GR	6/11/25
83	I74084211	B1S	6/11/25
84	I74084212	M1N	6/11/25
85		B1Q	6/11/25
86	I74084214	M1L	6/11/25
87	I74084215	M1M	6/11/25
88	I74084216	B1R	6/11/25
89	I74084217	B3GR	6/11/25

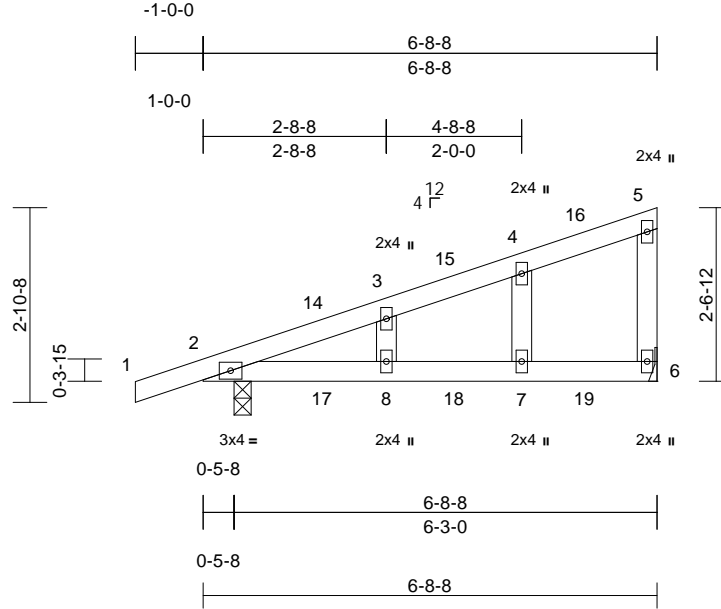
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084131
	P1GE	Monopitch Structural Gable	2	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Scale = 1:34.1

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.14	7-8	>566	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.22	7-8	>360	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.12	7-8	>666	240		
BCDL	10.0										Weight: 28 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	10-0-0 oc bracing.

#### REACTIONS

(size)	2=0-3-0, 6= Mechanical
Max Horiz	2=61 (LC 15)
Max Uplift	2=-65 (LC 12), 6=-38 (LC 12)
Max Grav	2=436 (LC 40), 6=371 (LC 45)

#### FORCES

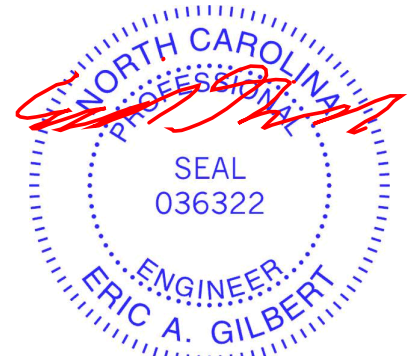
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/26, 2-3=-105/250, 3-4=-83/54, 4-5=-64/67, 5-6=-311/111
BOT CHORD	2-8=-209/132, 7-8=-33/35, 6-7=-33/35
WEBS	4-7=-128/123, 3-8=-126/125

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 6-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 6 and 65 lb uplift at joint 2.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord.

LOAD CASE(S) Standard



June 11, 2025

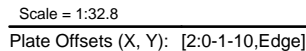
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY  
**TRENCO**  
A MiTek Affiliate

818 Soundside Road  
Edenton, NC 27932

Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:36 Page: 1  
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<b>LUMBER</b>		6) Plates checked for a plus or minus 5 degree rotation about its center.
TOP CHORD	2x4 SP No.2	7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
BOT CHORD	2x4 SP SS	8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0"-0" tall by 2'-0"-0" wide will fit between the bottom chord and any other members.
WEBS	2x4 SP No.3	9) Refer to girder(s) for truss to truss connections.
<b>BRACING</b>		10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 4.
TOP CHORD	Structural wood sheathing directly applied, except end verticals.	11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
BOT CHORD	10'-0" oc bracing.	12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
<b>REACTIONS</b>	(size) 2=0-3-0, 4= Mechanical Max Horiz 2=61 (LC 15) Max Uplift 2=-65 (LC 12), 4=-38 (LC 12) Max Grav 2=436 (LC 40), 4=371 (LC 43)	13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord.
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/26, 2-3=-141/243, 3-4=-311/137	
BOT CHORD	2-4=-217/135	
<b>NOTES</b>		
1) Unbalanced roof live loads have been considered for this design.		
2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDF=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp. B; Enclosed: MWERS (envelope) and C.C.		

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinstr.org](http://www.tpinstr.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcacomponents.com](http://www.sbcacomponents.com)).



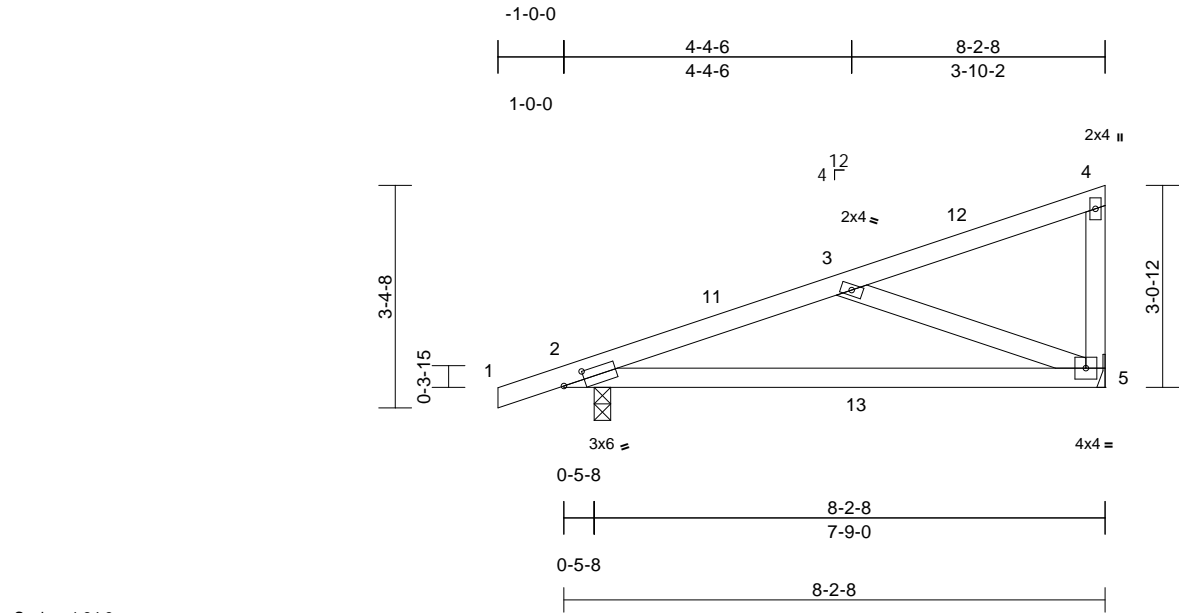
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084133
	P1A	Monopitch	4	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



Scale = 1:34.9

Plate Offsets (X, Y): [2:0-3-14,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	-0.23	5-10	>422	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.80	Vert(CT)	-0.31	5-10	>311	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.07	5-10	>999	240		
BCDL	10.0										Weight: 36 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP SS

WEBS 2x4 SP No.3

**BRACING**

TOP CHORD Structural wood sheathing directly applied, except end verticals.

BOT CHORD 10-0-0 oc bracing.

**REACTIONS** (size) 2=0-3-0, 5= Mechanical

Max Horiz 2=74 (LC 15)

Max Uplift 2=-73 (LC 12), 5=-49 (LC 12)

Max Grav 2=462 (LC 40), 5=402 (LC 44)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-0/26, 2-3=-534/257, 3-4=-66/51, 4-5=-279/79

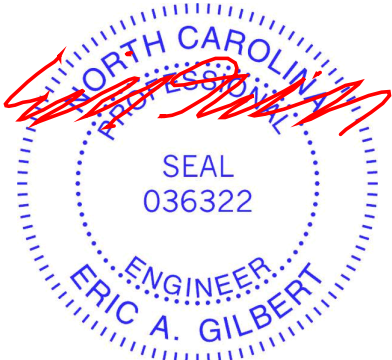
BOT CHORD 2-5=-335/503

WEBS 3-5=-534/326

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 8-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
  - Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord.

**LOAD CASE(S)** Standard



June 11,2025

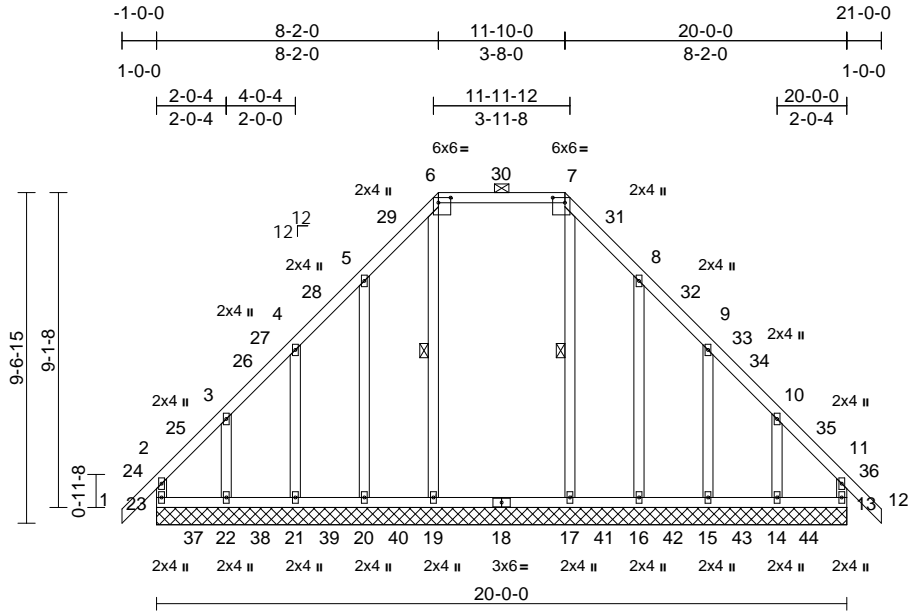


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084134
	G1GE	Hip Supported Gable	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



Scale = 1:66.8

Plate Offsets (X, Y): [6:0-4-4,0-1-12], [7:0-4-4,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(CT)	0.00	13	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
Weight: 141 lb FT = 20%											

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 6-19, 7-17

#### REACTIONS

(size)	13=20-0-0, 14=20-0-0, 15=20-0-0, 16=20-0-0, 17=20-0-0, 19=20-0-0, 20=20-0-0, 21=20-0-0, 22=20-0-0, 23=20-0-0
Max Horiz	23=165 (LC 15)
Max Uplift	13=53 (LC 13), 14=74 (LC 17), 15=29 (LC 17), 16=55 (LC 17), 20=55 (LC 16), 21=28 (LC 16), 22=78 (LC 16), 23=76 (LC 12)
Max Grav	13=320 (LC 97), 14=326 (LC 96), 15=338 (LC 95), 16=320 (LC 94), 17=378 (LC 93), 19=378 (LC 92), 20=320 (LC 91), 21=338 (LC 90), 22=326 (LC 89), 23=320 (LC 88)

#### FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-23=-300/70, 1-2=0/66, 2-3=-151/119, 3-4=-127/104, 4-5=-146/167, 5-6=-218/256, 6-7=-170/203, 7-8=-218/256, 8-9=-146/166, 9-10=-109/104, 10-11=-132/98, 11-12=0/66, 11-13=-300/51
BOT CHORD	22-23=-75/91, 21-22=-75/91, 20-21=-75/91, 19-20=-75/91, 17-19=-75/93, 16-17=-76/91, 15-16=-76/91, 14-15=-76/91, 13-14=-76/91

#### WEBS

6-19=-293/14, 7-17=-293/14, 5-20=-281/100, 4-21=-284/94, 3-22=-281/109, 8-16=-281/100, 9-15=-284/94, 10-14=-281/108
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#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -1-0-0 to 2-0-4, Exterior(2N) 2-0-4 to 8-2-0, Corner (3R) 8-2-0 to 11-2-0, Exterior(2N) 11-2-0 to 11-10-0, Corner(3R) 11-10-0 to 14-10-0, Exterior(2N) 14-10-0 to 21-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 23, 53 lb uplift at joint 13, 55 lb uplift at joint 20, 28 lb uplift at joint 21, 78 lb uplift at joint 22, 55 lb uplift at joint 16, 29 lb uplift at joint 15 and 74 lb uplift at joint 14.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



June 11,2025

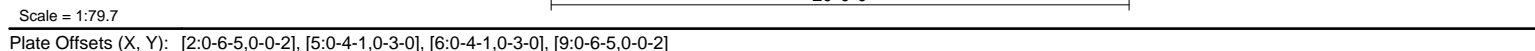
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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818 Soundside Road  
Edenton, NC 27932

Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:31 Page: 1  
ID:h7aEAQbDO3e ImCN9pf3QPzqv80-RfC?PsB70Hg3NSqPqnL8w3ulTXbGKwKCDoi7J4zJC?f



<b>LUMBER</b>		3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP SS	
WEBS	2x4 SP No.3	
SLIDER	Left 2x6 SP No.2 -- 1-6-0, Right 2x6 SP No.2 -- 1-6-0	4) Unbalanced snow loads have been considered for this design.
<b>BRACING</b>		5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
TOP CHORD	Structural wood sheathing directly applied or 4-9-6 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-6.	6) Provide adequate drainage to prevent water ponding.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	7) All plates are MT20 plates unless otherwise indicated.
<b>REACTIONS</b> (size) 2=0-3-8, 9=0-3-8		8) Plates checked for a plus or minus 5 degree rotation about its center.
	Max Horiz 2=165 (LC 15)	9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
	Max Grav 2=1190 (LC 49), 9=1190 (LC 51)	10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
<b>FORCES</b> (lb) - Maximum Compression/Maximum Tension		11) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
TOP CHORD	1-2=0/58, 2-4=-1259/95, 4-5=-1013/146, 5-6=-605/146, 6-7=-1013/146, 7-9=-1259/95, 9-10=0/58	12) Graphical purlin representation does not depict the size
BOT CHORD	2-13=-110/884, 11-13=0/657, 9-11=-46/813	
WEBS	4-13=-343/143, 5-13=-50/423, 6-11=-50/423, 7-11=-343/143	

- |  |   |   |
|--|---|---|
| <b>NOTES</b>   | or the orientation of the purlin along the top and/or bottom chord. |  |
| 1) Unbalanced roof live loads have been considered for this design.<br>2) Wind: ASCE 7-16; Vult=120mph (3-second gust)<br>Vasd=95mph; TCdL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 9-4-11, Exterior(2E) 9-4-11 to 10-7-5, Exterior(2R) 10-7-5 to 13-7-5, Interior (1) 13-7-5 to 21-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60 | <b>LOAD CASE(S)</b> Standard  |   |

June 11.2025

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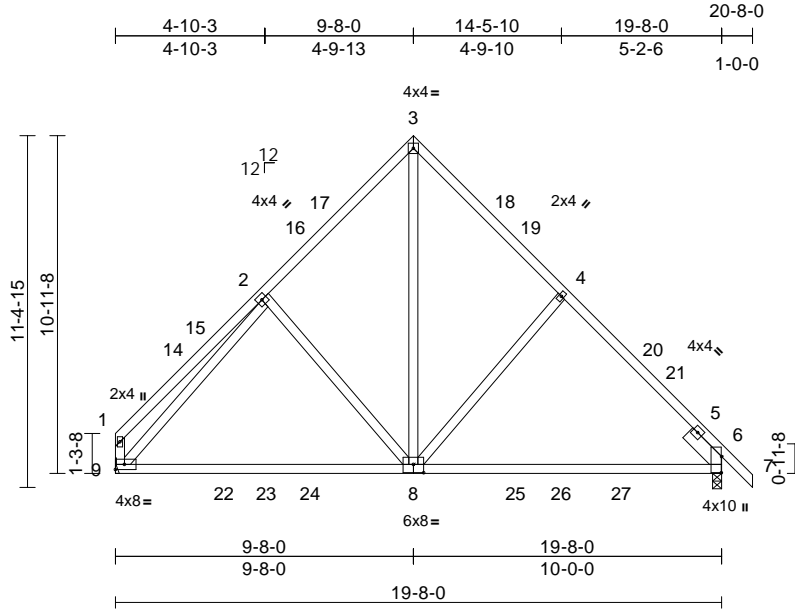
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084136
	G1A	Common	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:31  
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Page: 1



Scale = 1:74.7

Plate Offsets (X, Y): [6:0-6-5,0-0-2], [8:0-4-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.82	Vert(LL)	-0.45	8-9	>521	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.70	Vert(CT)	-0.59	8-9	>398	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(CT)	-0.04	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.03	8-12	>999	240		
BCDL	10.0											
											Weight: 125 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Right 2x6 SP No.2 -- 1-6-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-5 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 9-9-4 oc bracing.

**REACTIONS** (size) 6=0-3-8, 9= Mechanical  
Max Horiz 9=186 (LC 14)  
Max Grav 6=943 (LC 31), 9=892 (LC 31)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-371/85, 2-3=-753/139, 3-4=-753/141, 4-6=-1046/89, 6-7=0/58, 1-9=-374/68  
BOT CHORD 6-9=-48/637  
WEBS 3-8=-96/681, 4-8=-259/149, 2-8=-201/156, 2-9=-612/38

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust)  
Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-5-12 to 3-5-12, Interior (1) 3-5-12 to 10-0-0, Exterior(2R) 10-0-0 to 13-0-0, Interior (1) 13-0-0 to 21-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

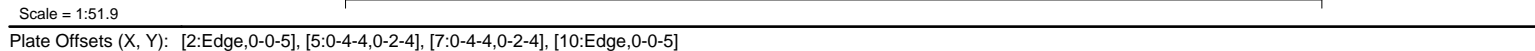
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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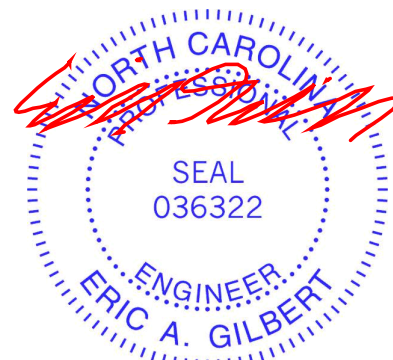
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Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:32 Page: 1  
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<b>LUMBER</b>		
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
WEBS	2x4 SP No.3	
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0	
<b>BRACING</b>		
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-7.	
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
<b>REACTIONS</b>		
(size)	2=0-3-8, 10=0-3-8	
Max Horiz	2=-80 (LC 10)	
Max Uplift	2=-178 (LC 12), 10=-179 (LC 13)	
Max Grav	2=1909 (LC 37), 10=1912 (LC 37)	
<b>FORCES</b>		
(lb) - Maximum Compression/Maximum Tension		
TOP CHORD	1-2=0/59, 2-4=-2345/280, 4-5=-2265/294, 5-6=-2332/300, 6-7=-2332/300, 7-8=-2270/296, 8-10=-2350/282, 10-11=0/59	
BOT CHORD	2-15=-248/1865, 14-15=-243/1863, 12-14=-199/1867, 10-12=-180/1869	
WEBS	4-15=-193/96, 5-15=-91/639, 5-14=-142/676, 6-14=-471/58, 7-14=-140/670, 7-12=-94/646, 8-12=-193/97	
<b>NOTES</b>		
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:		
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.		
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.		
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.		



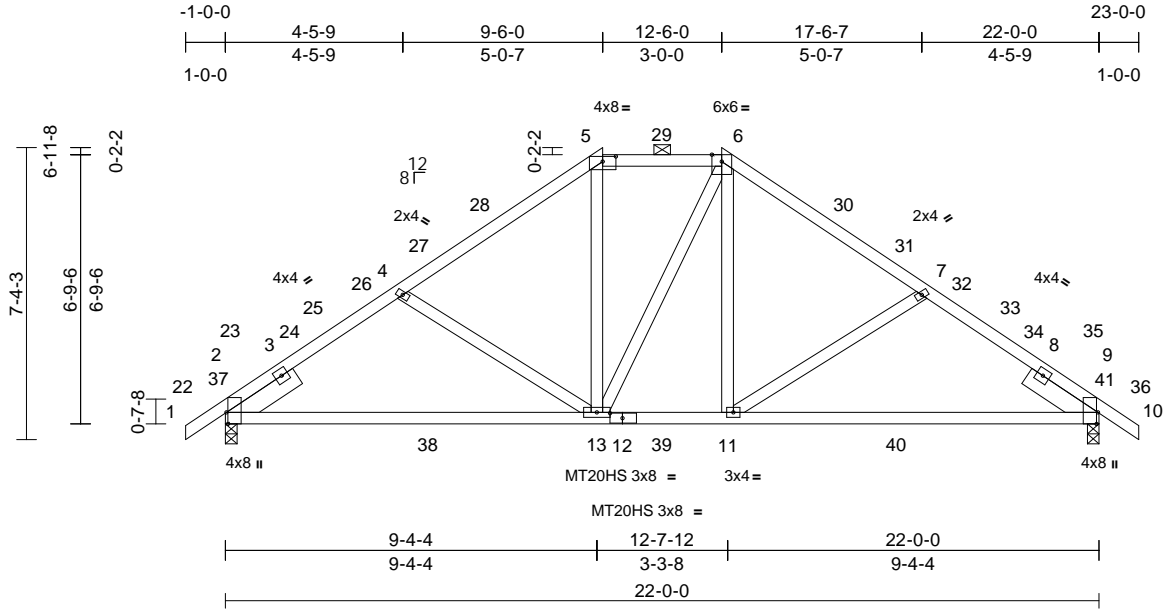
June 11.2025

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084140
	G1C	Hip	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:31  
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Page: 1



Scale = 1:58

Plate Offsets (X, Y): [2:0-3-8,Edge], [5:0-4-0,0-1-9], [9:0-3-8,Edge], [12:0-3-12,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.24	11-20	>999	360	MT20 244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.34	11-20	>757	240	MT20HS 187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(CT)	0.03	9	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.01	11-13	>999	240	
BCDL	10.0										
										Weight: 130 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Left 2x6 SP No.2 -- 2-0-0, Right 2x6 SP No.2 -- 2-0-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (5-10-10 max.); 5-6.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size) 2=0-3-8, 9=0-3-8  
Max Horiz 2=-111 (LC 14)  
Max Grav 2=1226 (LC 41), 9=1226 (LC 41)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/52, 2-4=-1486/53, 4-5=-1193/55, 5-6=-879/83, 6-7=-1192/60, 7-9=-1486/47, 9-10=0/52  
BOT CHORD 2-13=0/1187, 11-13=0/878, 9-11=0/1187  
WEBS 4-13=-356/78, 5-13=0/299, 6-13=-143/147, 6-11=0/404, 7-11=-358/79

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 1-0-0 to 2-0-0, Interior (1) 2-0-0 to 9-6-0, Exterior(2E) 9-6-0 to 12-6-0, Exterior(2R) 12-6-0 to 16-8-15, Interior (1) 16-8-15 to 23-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

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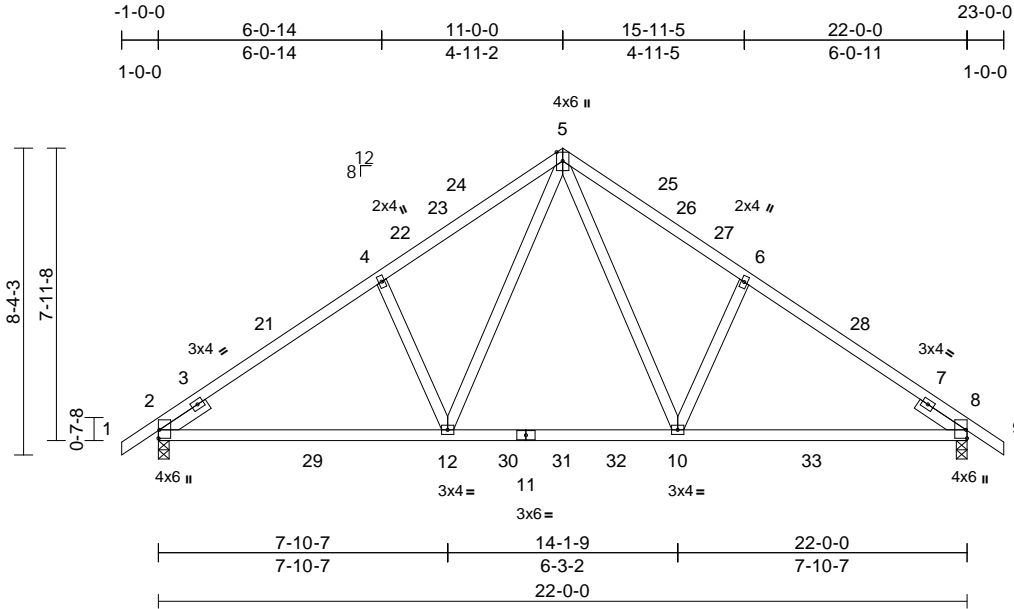
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084141
	G1B	Common	3	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:31

Page: 1

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Scale = 1:62.7									
Plate Offsets (X, Y): [2:Edge,0-0-5], [8:Edge,0-0-5]									
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.60	Vert(LL)	-0.18 12-15	>999	360
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.49	Vert(CT)	-0.24 12-15	>999	240
TCDL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.02 8	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.02 12-15	>999	240
BCDL	10.0								
									<b>PLATES</b> MT20
									<b>GRIP</b> 244/190
									Weight: 117 lb FT = 20%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

**BRACING**  
TOP CHORD Structural wood sheathing directly applied.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size) 2=0-3-8, 8=0-3-8  
Max Horiz 2=-128 (LC 14)  
Max Grav 2=1038 (LC 30), 8=1038 (LC 31)

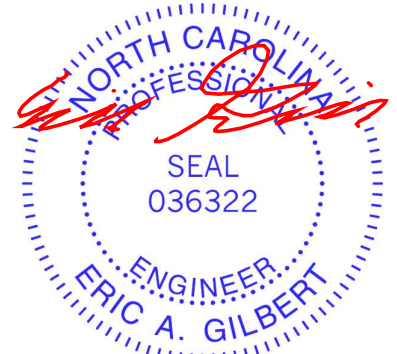
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/52, 2-4=-1238/65, 4-5=-1150/117, 5-6=-1150/117, 6-8=-1239/65, 8-9=0/52  
BOT CHORD 2-12=0/1054, 10-12=0/736, 8-10=0/980  
WEBS 5-10=-29/554, 6-10=-286/115, 5-12=-29/555, 4-12=-286/115

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 11-0-0, Exterior(2R) 11-0-0 to 14-0-0, Interior (1) 14-0-0 to 23-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.



June 11,2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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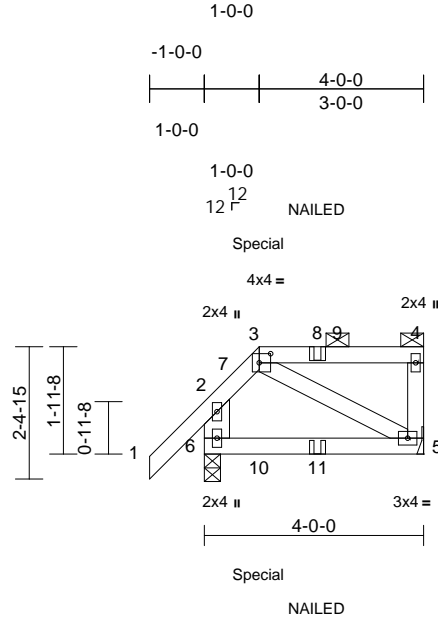
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	M1I	Half Hip Girder	2	1	Job Reference (optional)
					I74084142

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



Scale = 1:42

Plate Offsets (X, Y): [3:0-2-8,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	-0.03	5-6	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.50	Vert(CT)	-0.04	5-6	>999	240		
TCDL	10.0	Rep Stress Incr	NO	WB	0.03	Horz(CT)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.00	5-6	>999	240		
BCDL	10.0										Weight: 23 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3 \*Except\* 6-2:2x6 SP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (size) 5= Mechanical, 6=0-3-8  
Max Horiz 6=53 (LC 11)  
Max Uplift 5=-11 (LC 9), 6=-11 (LC 12)  
Max Grav 5=316 (LC 54), 6=333 (LC 51)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/89, 2-3=-242/34, 3-4=-45/10,  
4-5=-281/16, 2-6=-296/34

BOT CHORD 5-6=-35/104  
WEBS 3-5=-108/46

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- Provide adequate drainage to prevent water ponding.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 26 lb down and 91 lb up at 1-0-0 on top chord, and 7 lb down and 19 lb up at 1-0-0 on bottom chord. The design/ selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (lb/ft)

Vert: 1-2=-51, 2-3=-51, 3-4=-61, 5-6=-20  
Concentrated Loads (lb)  
Vert: 3=26 (B), 10=3 (B), 11=-5 (B)



June 11,2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

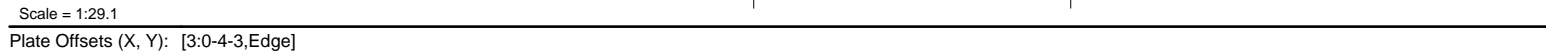
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcacomponents.com](http://www.sbcacomponents.com))

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June 11, 2025



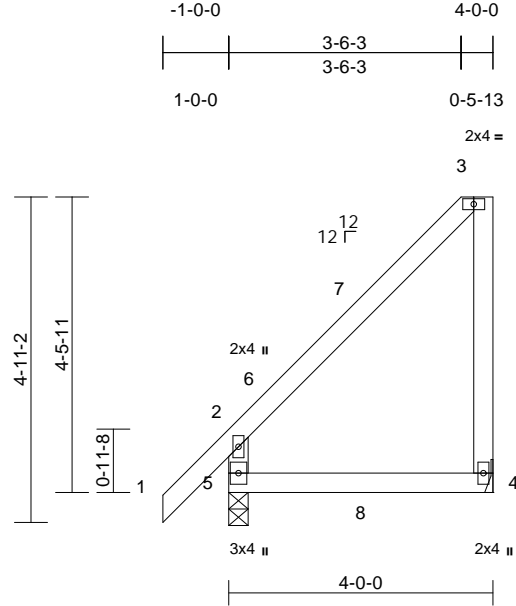
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084144
	M1F	Half Hip	2	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:33

Page: 1

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Scale = 1:34.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	-0.04	4-5	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.52	Vert(CT)	-0.05	4-5	>834	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR		Wind(LL)	0.01	4-5	>999	240		
BCDL	10.0										Weight: 23 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size)	4= Mechanical, 5=0-3-8
Max Horiz	5=122 (LC 13)
Max Uplift	4=-51 (LC 13)
Max Grav	4=324 (LC 42), 5=354 (LC 40)

#### FORCES

(lb) - Maximum Compression/Maximum Tension	
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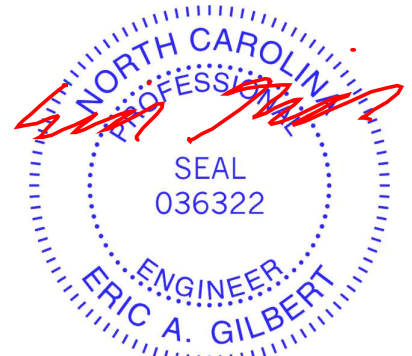
TOP CHORD	1-2=0/66, 2-3=-152/159, 3-4=-284/142, 2-5=-319/76
BOT CHORD	4-5=-59/73

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



June 11,2025

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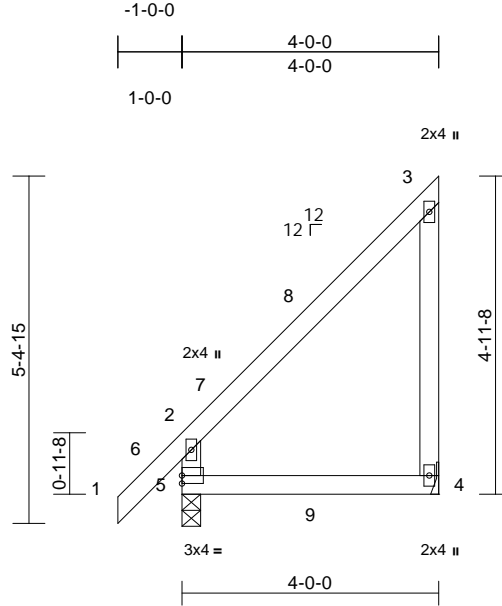
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	M1E	Jack-Closed	4	1	Job Reference (optional)
					I74084145

Structural, LLC, Thurmont, MD - 21788,

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Scale = 1:35.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	-0.04	4-5	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.52	Vert(CT)	-0.05	4-5	>834	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR		Wind(LL)	0.02	4-5	>999	240		
BCDL	10.0											
											Weight: 24 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS (size) 4= Mechanical, 5=0-3-8

Max Horiz	5=122 (LC 15)
Max Uplift	4=-51 (LC 13)
Max Grav	4=324 (LC 45), 5=354 (LC 40)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	2-5=-319/129, 1-2=0/66, 2-3=-227/140, 3-4=-284/264
BOT CHORD	4-5=-74/69

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11,2025

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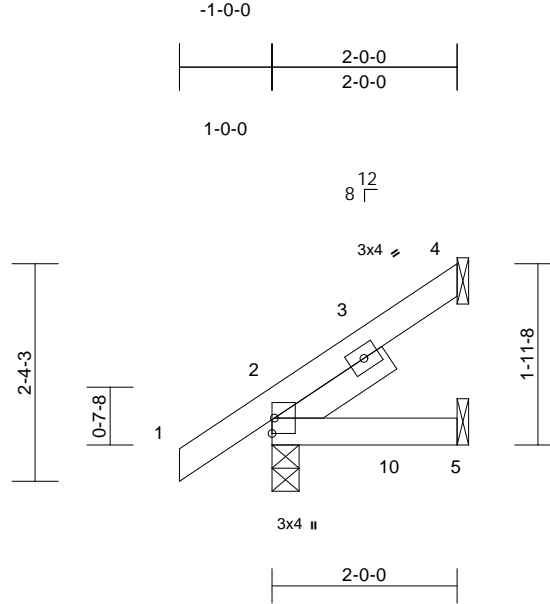
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084146
	M1K	Jack-Open	2	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	0.00	5-8	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	-0.01	5-8	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP		Wind(LL)	0.00	5-8	>999	240		
BCDL	10.0										Weight: 11 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 1-4-12

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

<b>REACTIONS</b>	(size)	2=0-3-8, 4= Mechanical, 5= Mechanical
	Max Horiz	2=40 (LC 16)
	Max Uplift	4=15 (LC 16)
	Max Grav	2=318 (LC 41), 4=270 (LC 40), 5=270 (LC 45)

#### FORCES

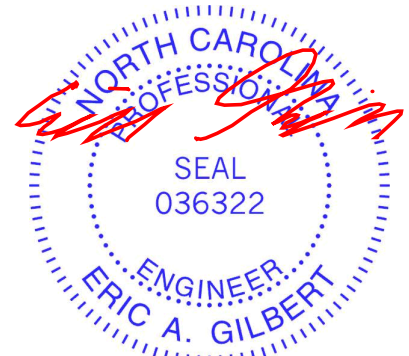
(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-4=163/54
BOT CHORD	2-5=0/0

#### NOTES

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
  - Refer to girder(s) for truss to truss connections.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 4.
  - This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- LOAD CASE(S)** Standard



June 11,2025

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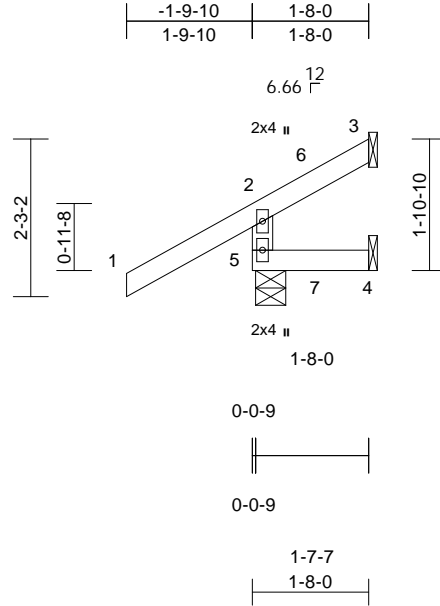
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084147
	M1J	Jack-Open	2	1	Job Reference (optional)	

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Scale = 1:33

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	0.00	4-5	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.17	Vert(CT)	0.00	4-5	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR		Wind(LL)	0.00	4-5	>999	240		
BCDL	10.0										Weight: 9 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 1-8-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 3= Mechanical, 4= Mechanical,  
5=0-5-3  
Max Horiz 5=35 (LC 13)  
Max Uplift 3=-56 (LC 22), 4=-17 (LC 22)  
Max Grav 3=251 (LC 42), 4=260 (LC 45),  
5=347 (LC 40)

**FORCES** (lb) - Maximum Compression/Maximum Tension

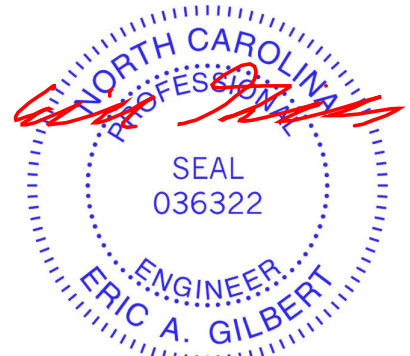
TOP CHORD 2-5=-324/169, 1-2=0/77, 2-3=-84/52  
BOT CHORD 4-5=0/0

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 4 and 56 lb uplift at joint 3.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11,2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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Edenton, NC 27932

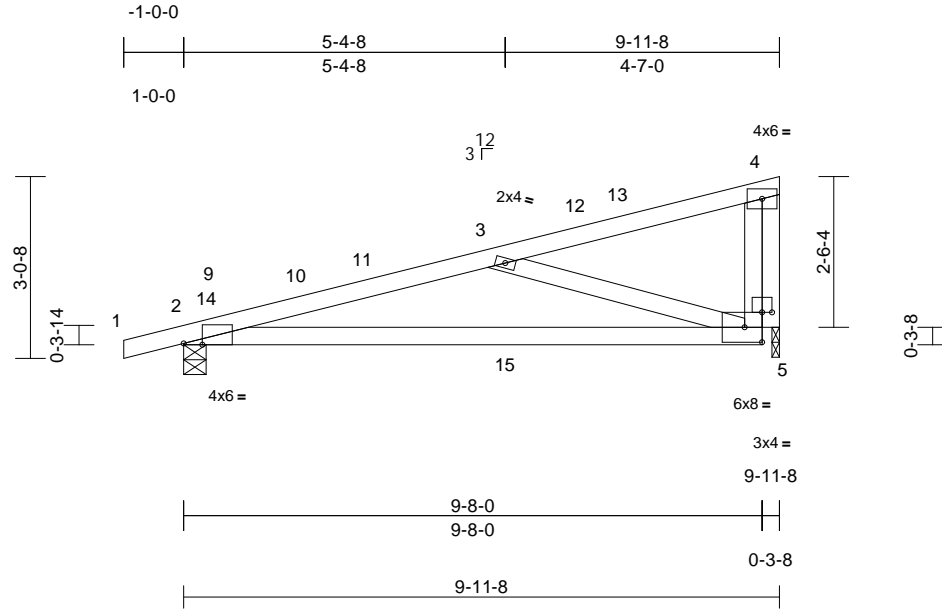
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084152
	P1D	Monopitch	7	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:36

Page: 1

ID:Thk8SZjCvEOIGwFyxWod3NzIH13-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:38.5

Plate Offsets (X, Y): [2:0-3-12,Edge], [5:0-2-0,0-0-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.28	5-8	>410	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.87	Vert(CT)	-0.42	5-8	>274	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.12	5-8	>945	240		
BCDL	10.0										Weight: 44 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
OTHERS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.  
BOT CHORD 10-0-0 oc bracing.

**REACTIONS** (size) 2=0-4-8, 5=0-1-8  
Max Horiz 2=63 (LC 15)  
Max Uplift 2=-80 (LC 12), 5=-62 (LC 12)  
Max Grav 2=467 (LC 50), 5=445 (LC 44)

**FORCES** (lb) - Maximum Compression/Maximum Tension

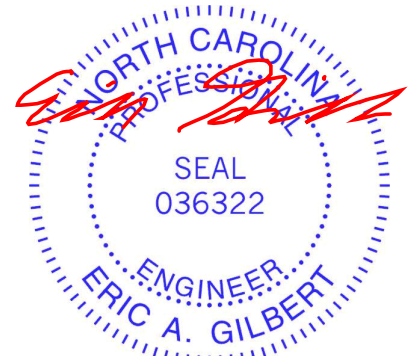
TOP CHORD 1-2=0/20, 2-3=-838/384, 3-4=-164/69, 4-5=-272/66  
BOT CHORD 2-5=-442/809  
WEBS 3-5=-758/387

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 9-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord.

**LOAD CASE(S)** Standard



June 11,2025

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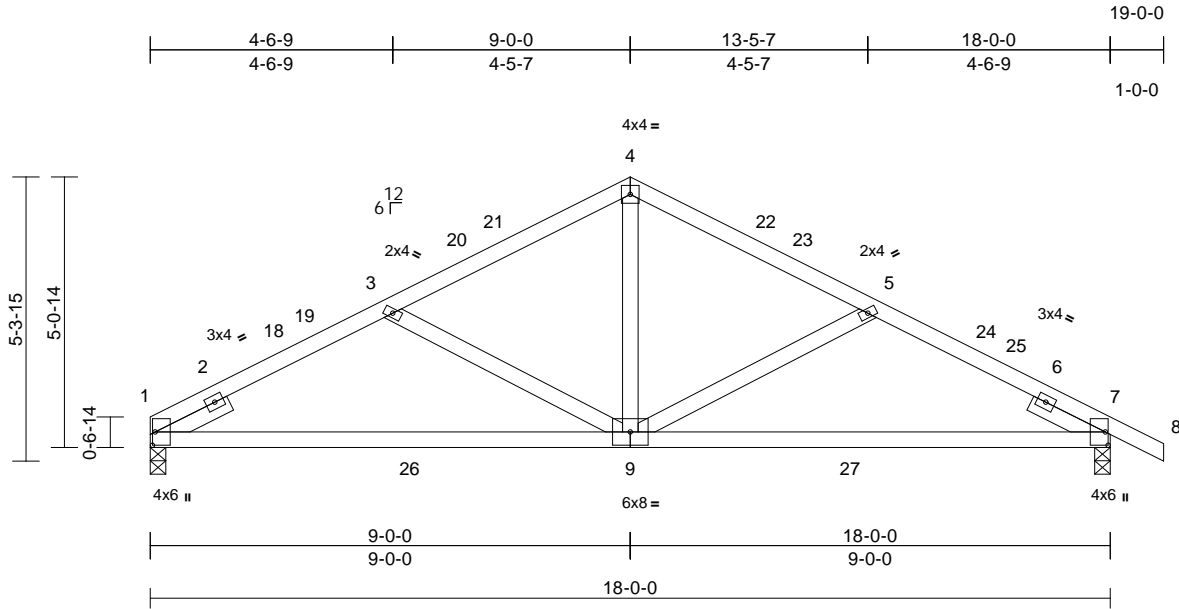


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084153
	E1D	Common	6	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:30  
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Page: 1



Scale = 1:43.2

Plate Offsets (X, Y): [1:0-3-0,0-0-10], [7:0-3-0,0-0-10]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.55	Vert(LL)	-0.26	9-12	>828	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.33	9-12	>638	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.19	Horz(CT)	0.02	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.02	9-12	>999	240		
BCDL	10.0											
Weight: 85 lb											FT = 20%	

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied.  
BOT CHORD Rigid ceiling directly applied.

#### REACTIONS

(size) 1=0-3-8, 7=0-3-8  
Max Horiz 1=-45 (LC 21)  
Max Grav 1=706 (LC 2), 7=779 (LC 2)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

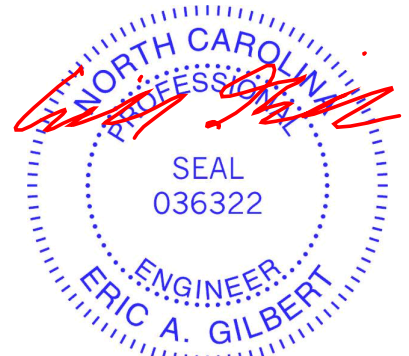
TOP CHORD 1-3=-1135/175, 3-4=-839/136, 4-5=-838/131, 5-7=-1138/170, 7-8=0/42  
BOT CHORD 1-7=-86/929  
WEBS 4-9=0/493, 5-9=-338/116, 3-9=-342/119

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12 to 9-0-0, Exterior(2R) 9-0-0 to 12-0-0, Interior (1) 12-0-0 to 19-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



June 11, 2025

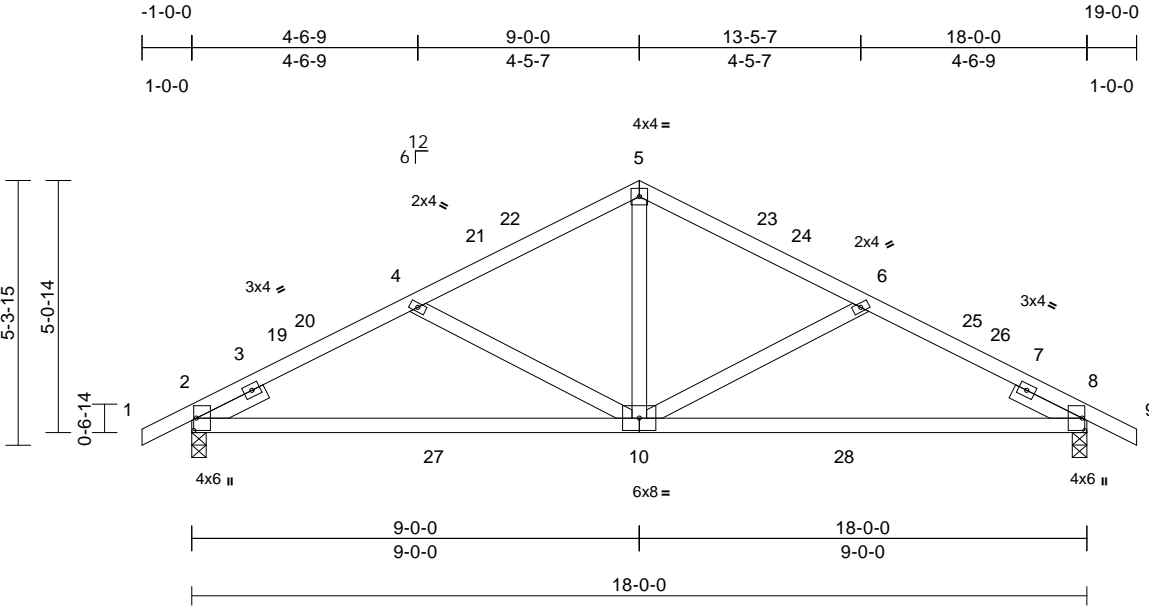
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084154
	E1C	Common	1	1	Job Reference (optional)	



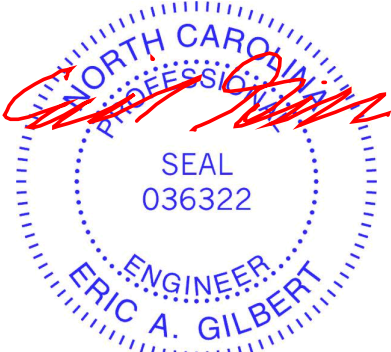
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Plate Offsets (X, Y): [2:0-3-0,0-0-10], [8:0-3-0,0-0-10]												
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in	(loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.55	Vert(LL)	-0.26	10-13	>829	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.33	10-13	>643	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.19	Horz(CT)	0.02	8	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.02	10	>999	240		
BCDL	10.0										Weight: 86 lb	FT = 20%

<b>LUMBER</b>	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP SS
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0
<b>BRACING</b>	
TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
<b>REACTIONS</b>	
(size)	2=0-3-8, 8=0-3-8
Max Horiz	2=-43 (LC 14)
Max Grav	2=777 (LC 2), 8=777 (LC 2)
<b>FORCES</b>	
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/42, 2-4=-1138/166, 4-5=-833/130, 5-6=-833/130, 6-8=-1138/166, 8-9=0/42
BOT CHORD	2-8=-82/916
WEBS	5-10=0/490, 6-10=-338/116, 4-10=-338/116

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 9-0-0, Exterior(2R) 9-0-0 to 12-0-0, Interior (1) 12-0-0 to 19-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
  - Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard



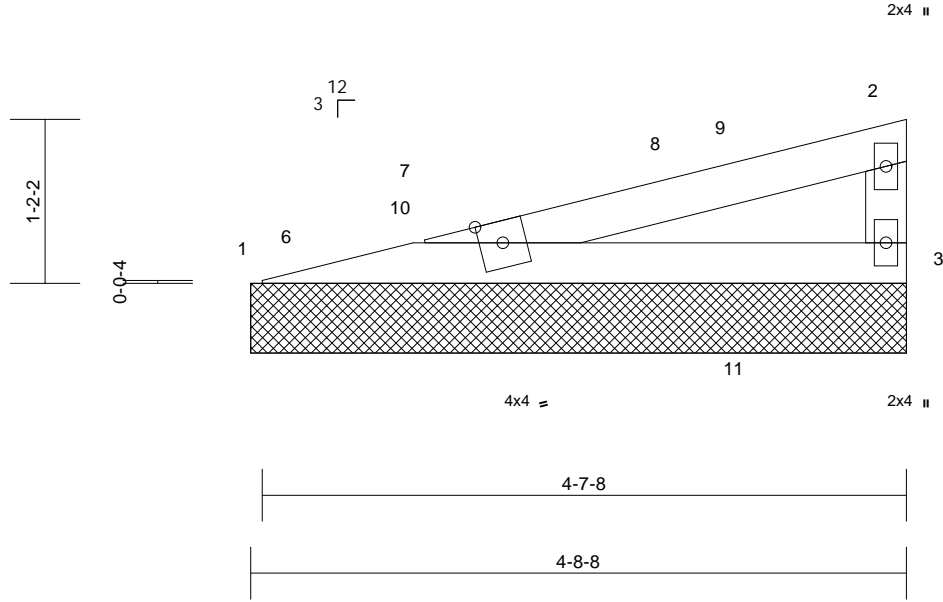
June 11,2025

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084155
	V10	Valley	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:38  
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Page: 1



Scale = 1:16.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.77	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.01	3	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS							
BCDL	10.0									Weight: 13 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.3  
WEBS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size) 1=4-8-8, 3=4-8-8  
Max Horiz 1=23 (LC 13)  
Max Grav 1=344 (LC 43), 3=344 (LC 42)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-706/186, 2-3=-300/81

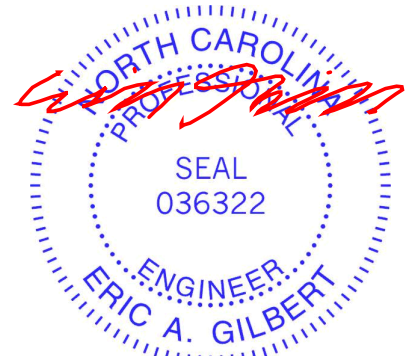
BOT CHORD 1-3=-248/684

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 4-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.

- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

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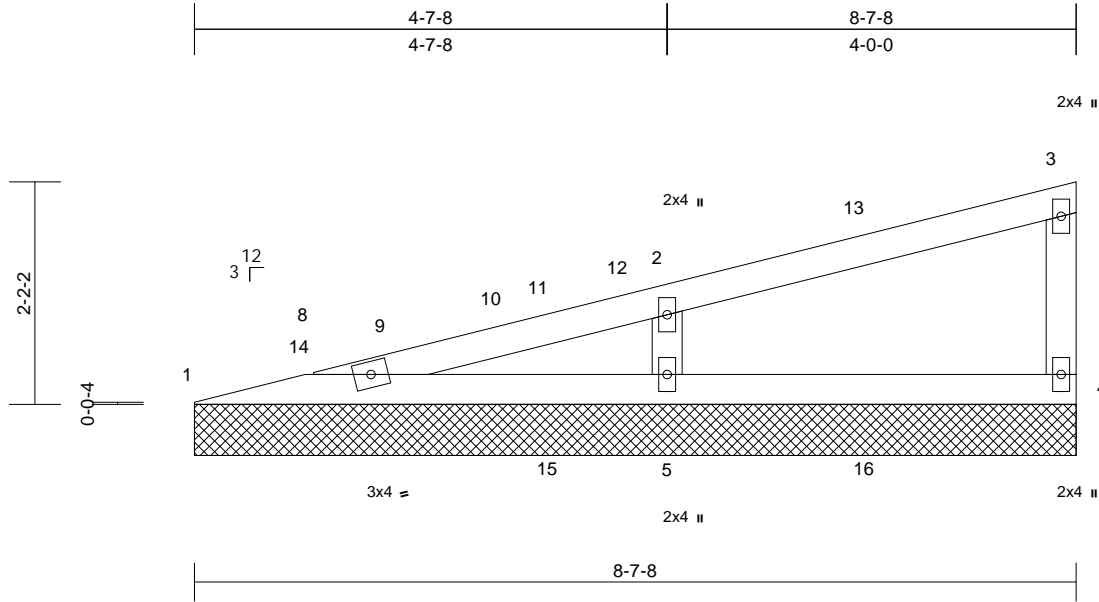
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084156
	V9	Valley	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in	(loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.83	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 28 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.3
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	Rigid ceiling directly applied.

#### REACTIONS

(size)	1=8-7-8, 4=8-7-8, 5=8-7-8
Max Horiz	1=48 (LC 13)
Max Grav	1=327 (LC 44), 4=308 (LC 52), 5=469 (LC 22)

#### FORCES

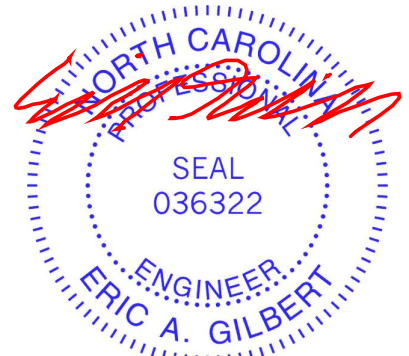
	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=-558/67, 2-3=-49/33, 3-4=-280/80
BOT CHORD	1-5=-79/538, 4-5=-27/30
WEBS	2-5=-359/180

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-0 to 3-1-0, Interior (1) 3-1-0 to 8-6-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard



June 11,2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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Edenton, NC 27932

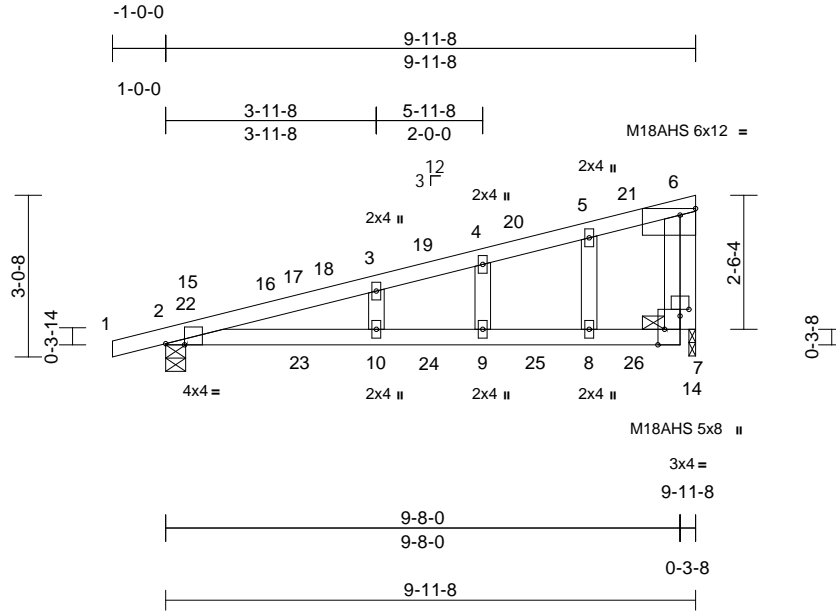
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	P1BGE	Monopitch Supported Gable	1	1	Job Reference (optional)
					I74084157

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Scale = 1:43.3

Plate Offsets (X, Y): [2:0-4-4,Edge], [6:Edge,0-1-8], [7:0-3-8,Edge], [7:0-2-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.24	9-10	>488	360	M18AHS 186/179
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.46	9-10	>260	240	MT20 244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	14	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.26	9-10	>462	240	
BCDL	10.0										
Weight: 43 lb FT = 20%											

#### LUMBER

TOP CHORD 2x4 SP SS  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.2  
OTHERS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.  
BOT CHORD 10-0-0 oc bracing.

#### REACTIONS

(size) 2=0-4-8, 14=0-1-8  
Max Horiz 2=61 (LC 12)  
Max Uplift 2=-77 (LC 12), 14=-65 (LC 12)  
Max Grav 2=472 (LC 57), 14=435 (LC 46)

#### FORCES

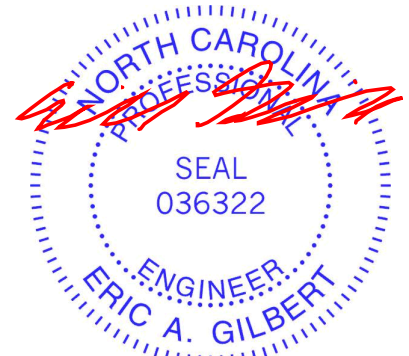
(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/20, 2-3=-414/156, 3-4=-373/134, 4-5=-357/147, 5-6=-320/152, 6-7=-100/347  
BOT CHORD 2-10=-199/383, 9-10=-185/358, 8-9=-185/358, 7-8=-185/358  
WEBS 5-8=-84/165, 4-9=-137/113, 3-10=-122/130, 6-14=-440/219

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 9-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 14.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord.

LOAD CASE(S) Standard



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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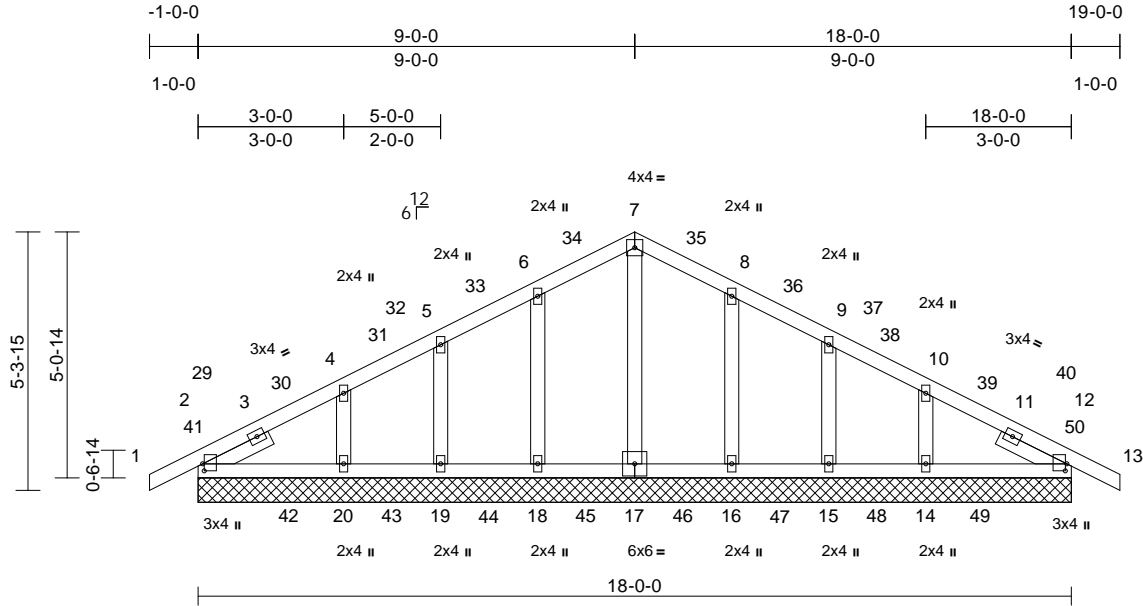
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084158
	E1BGE	Common Supported Gable	1	1	Job Reference (optional)	

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Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:30

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Scale = 1:47.5									
Plate Offsets (X, Y): [2:0-1-12,0-0-6], [12:0-1-12,0-0-6]									
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in	(loc)	l/defl
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	n/a	-	n/a
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	n/a	-	n/a
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.00	2	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS					
BCDL	10.0								
Weight: 94 lb FT = 20%									

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

**BRACING**  
TOP CHORD Structural wood sheathing directly applied.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size)  
2=18-0-0, 12=18-0-0, 14=18-0-0, 15=18-0-0, 16=18-0-0, 17=18-0-0, 18=18-0-0, 19=18-0-0, 20=18-0-0  
Max Horiz 2=43 (LC 15)  
Max Uplift 14=-13 (LC 17), 15=-4 (LC 17), 16=-6 (LC 17), 18=-7 (LC 16), 19=-4 (LC 16), 20=-16 (LC 16)  
Max Grav 2=335 (LC 71), 12=335 (LC 81), 14=367 (LC 79), 15=321 (LC 78), 16=337 (LC 77), 17=315 (LC 76), 18=337 (LC 75), 19=321 (LC 74), 20=367 (LC 73)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/36, 2-4=-248/41, 4-5=-78/61, 5-6=-80/103, 6-7=-101/145, 7-8=-101/145, 8-9=-80/102, 9-10=-78/62, 10-12=-248/41, 12-13=0/36  
BOT CHORD 2-20=-10/68, 19-20=-10/68, 18-19=-10/68, 16-18=-10/68, 15-16=-10/68, 14-15=-10/68, 12-14=-10/68  
WEBS 7-17=-242/8, 6-18=-285/72, 5-19=-279/68, 4-20=-307/97, 8-16=-285/71, 9-15=-279/68, 10-14=-307/96

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 9-0-0, Corner (3R) 9-0-0 to 12-0-0, Exterior(2N) 12-0-0 to 19-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 18, 4 lb uplift at joint 19, 16 lb uplift at joint 20, 6 lb uplift at joint 16, 4 lb uplift at joint 15 and 13 lb uplift at joint 14.

- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 12.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

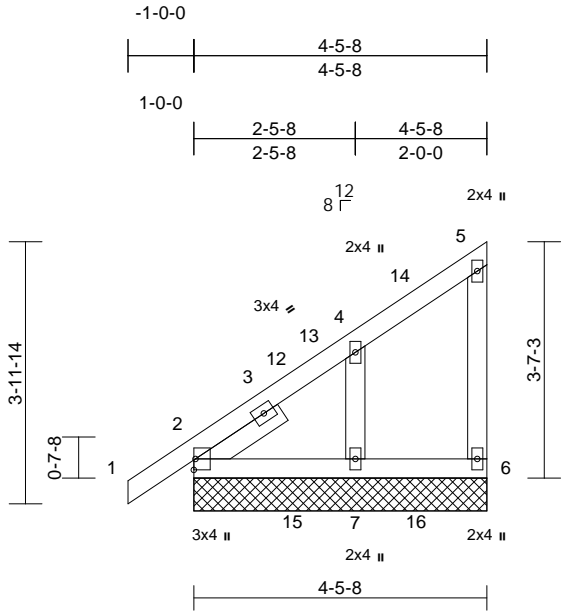
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	H1GE	Monopitch Supported Gable	1	1	Job Reference (optional)
					I74084159



Scale = 1:35											
Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.21	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	2	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS							
BCDL	10.0									Weight: 26 lb	FT = 20%

<b>LUMBER</b>		
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
WEBS	2x4 SP No.3	
OTHERS	2x4 SP No.3	
SLIDER	Left 2x4 SP No.3 -- 1-6-0	
<b>BRACING</b>		
TOP CHORD	Structural wood sheathing directly applied, except end verticals.	
BOT CHORD	Rigid ceiling directly applied.	
<b>REACTIONS</b>		
(size)	2=4-5-8, 6=4-5-8, 7=4-5-8	
Max Horiz	2=84 (LC 15)	
Max Uplift	6=-10 (LC 41), 7=-26 (LC 16)	
Max Grav	2=318 (LC 47), 6=279 (LC 53), 7=355 (LC 52)	
<b>FORCES</b>		
(lb) - Maximum Compression/Maximum Tension		
TOP CHORD	1-2=0/45, 2-4=-213/149, 4-5=-96/76, 5-6=-265/79	
BOT CHORD	2-7=-47/63, 6-7=-47/63	
WEBS	4-7=-300/217	

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 4-3-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Plates checked for a plus or minus 5 degree rotation about its center.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 6 and 26 lb uplift at joint 7.
- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 14) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard



June 11,2025

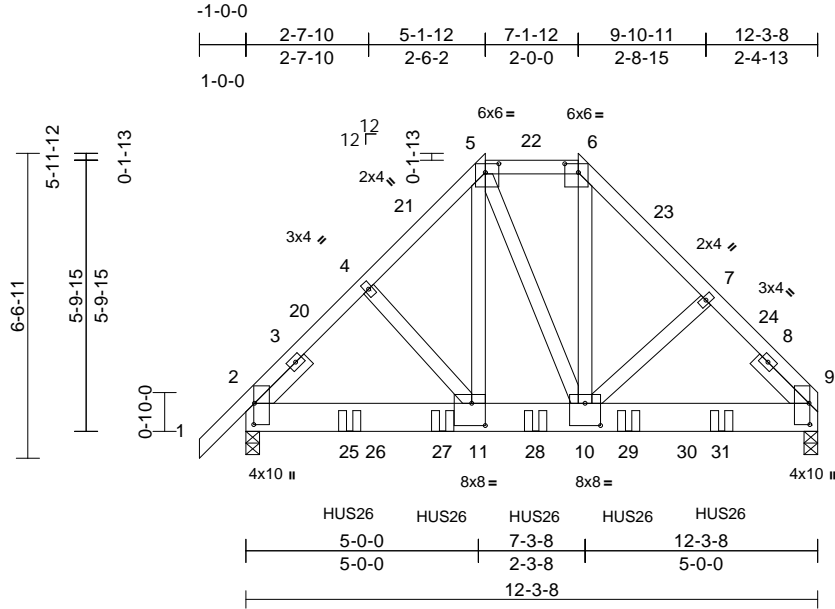
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084161
	D1GR	Hip Girder	1	2	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:29

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Scale = 1:49.5

Plate Offsets (X, Y): [2:0-5-8,0-0-3], [5:0-3-8,0-2-4], [6:0-3-8,0-2-4], [9:0-5-8,0-0-3], [10:0-4-0,0-5-12], [11:0-3-8,0-5-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	-0.04	11-18	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.38	Vert(CT)	-0.08	11-18	>999	240		
TCDL	10.0	Rep Stress Incr	NO	WB	0.67	Horz(CT)	0.01	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.00	11	>999	240		
BCDL	10.0											
											Weight: 205 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x8 SP DSS
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-3-3 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size)	2=0-3-8, 9=0-3-8
Max Horiz	2=91 (LC 9)
Max Grav	2=5571 (LC 38), 9=5214 (LC 38)

#### FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/83, 2-4=-5130/0, 4-5=-5002/0, 5-6=-3479/0, 6-7=-5025/0, 7-9=-5135/0
BOT CHORD	2-11=0/3543, 10-11=0/3459, 9-10=0/3510
WEBS	5-11=0/3234, 5-10=-820/281, 6-10=0/3226, 4-11=-144/163, 7-10=-47/226

#### NOTES

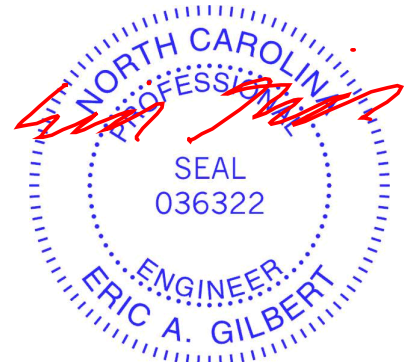
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.  
Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-5-0 oc.  
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Use Simpson Strong-Tie HUS26 (14-10d Girder, 6-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-2-12 from the left end to 6-2-12 to connect truss(es) to front face of bottom chord.
- Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 8-2-12 from the left end to 10-2-12 to connect truss(es) to front face of bottom chord.

- Fill all nail holes where hanger is in contact with lumber.

#### LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (lb/ft)  
Vert: 1-5=-51, 5-6=-61, 6-9=-51, 12-16=-20  
Concentrated Loads (lb)  
Vert: 25=-1998 (F), 27=-2007 (F), 28=-2014 (F), 29=-1715 (F), 31=-1608 (F)



June 11,2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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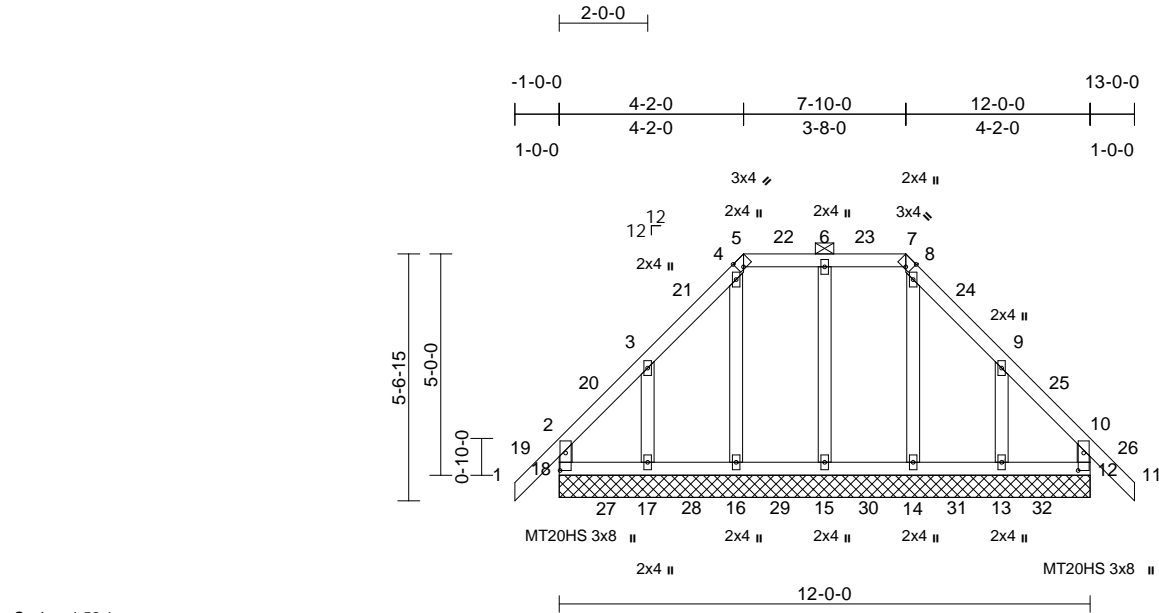
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084162
	C1GE	Hip Supported Gable	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:28  
ID:jiMONWMO0I8A0RfniNVMyjzqten-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:52.1

Plate Offsets (X, Y): [5:0-1-8,Edge], [7:0-1-8,Edge], [12:0-4-12,0-1-8], [18:0-4-12,0-1-8]												
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in	(loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	n/a	-	n/a	999	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.00	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS								
BCDL	10.0											
											Weight: 73 lb	FT = 20%

<b>LUMBER</b>	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.3
<b>BRACING</b>	
TOP CHORD	Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied.
<b>REACTIONS</b>	(size) 12=12-0-0, 13=12-0-0, 14=12-0-0, 15=12-0-0, 16=12-0-0, 17=12-0-0, 18=12-0-0
	Max Horiz 18=98 (LC 15)
	Max Uplift 12=14 (LC 13), 13=59 (LC 17), 17=61 (LC 16), 18=27 (LC 12)
	Max Grav 12=315 (LC 89), 13=329 (LC 88), 14=332 (LC 87), 15=333 (LC 86), 16=332 (LC 85), 17=329 (LC 84), 18=315 (LC 83)
<b>FORCES</b>	
	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	2-18=-295/104, 1-2=0/83, 2-3=-137/105, 3-4=-87/150, 4-5=-114/124, 5-6=-72/146, 6-7=-72/146, 7-8=-215/123, 8-9=-89/157, 9-10=-137/92, 10-11=0/83, 10-12=-295/110
BOT CHORD	17-18=-48/95, 16-17=-48/95, 15-16=-48/95, 14-15=-48/95, 13-14=-48/95, 12-13=-48/95
WEBS	6-15=-280/16, 4-16=-273/7, 3-17=-283/161, 8-14=-266/7, 9-13=-283/160

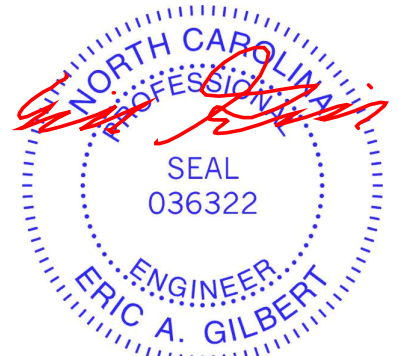
**NOTES**

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 4-2-0, Corner (3R) 4-2-0 to 7-2-0, Exterior(2N) 7-2-0 to 7-10-0, Corner (3R) 7-10-0 to 10-10-0, Exterior(2N) 10-10-0 to 13-0-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 18, 14 lb uplift at joint 12, 61 lb uplift at joint 17 and 59 lb uplift at joint 13.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11,2025

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818 Soundside Road  
Edenton, NC 27932



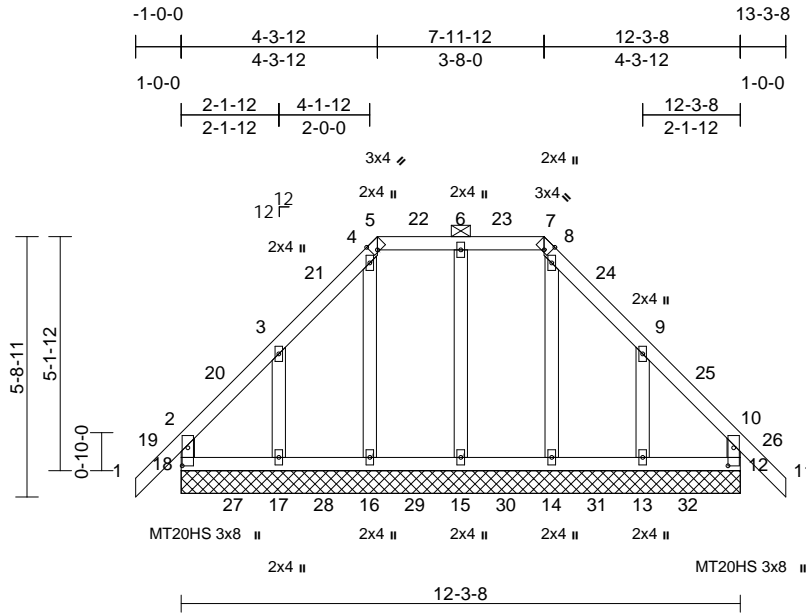
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084163
	D1GE	Hip Supported Gable	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:29

Page: 1

ID:CR\_djUnLmVZCjpqWGDdZt5zqteE-RfC?PsB70Hq3NSgPqnL8w3uITXbGKwRcDoi7J4zJC?i



Scale = 1:50.7																
Plate Offsets (X, Y): [5:0-1-8,Edge], [7:0-1-8,Edge], [12:0-4-12,0-1-8], [18:0-4-12,0-1-8]																
<b>Loading</b>		(psf)	<b>Spacing</b>		2-0-0	<b>CSI</b>		<b>DEFL</b>			in	(loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL (roof)		20.0	Plate Grip DOL		1.15	TC		0.22	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
Snow (Pf/Pg)		20.4/20.0	Lumber DOL		1.15	BC		0.21	Vert(CT)	n/a	-	n/a	999	MT20HS	187/143	
TCDL		10.0	Rep Stress Incr		YES	WB		0.13	Horz(CT)	0.00	12	n/a	n/a			
BCLL		0.0*	Code		IRC2021/TPI2014	Matrix-MR										
BCDL		10.0														
														Weight: 75 lb	FT = 20%	

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

<b>REACTIONS</b>	(size)	12=12-3-8, 13=12-3-8, 14=12-3-8, 15=12-3-8, 16=12-3-8, 17=12-3-8, 18=12-3-8
	Max Horiz	18=101 (LC 14)
	Max Uplift	12=15 (LC 13), 13=61 (LC 17), 17=63 (LC 16), 18=26 (LC 12)
	Max Grav	12=317 (LC 89), 13=334 (LC 88), 14=331 (LC 87), 15=335 (LC 86), 16=331 (LC 85), 17=334 (LC 84), 18=317 (LC 83)

#### FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	2-18=-297/115, 1-2=0/83, 2-3=-140/107, 3-4=-95/168, 4-5=-112/129, 5-6=-76/157, 6-7=-76/157, 7-8=-215/129, 8-9=-97/175, 9-10=-140/96, 10-11=0/83, 10-12=-297/121
BOT CHORD	17-18=-48/89, 16-17=-48/89, 15-16=-48/89, 14-15=-48/89, 13-14=-48/89, 12-13=-48/89
WEBS	6-15=-281/17, 4-16=-274/14, 3-17=-286/170, 8-14=-270/14, 9-13=-286/168

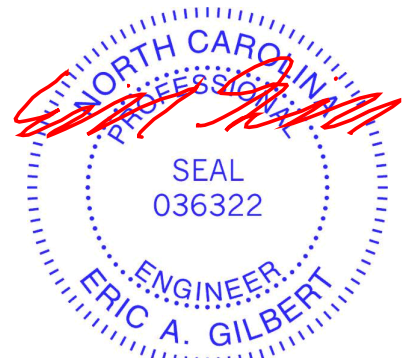
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -1-0-0 to 2-1-12, Exterior(2N) 2-1-12 to 4-3-12, Corner(3R) 4-3-12 to 7-3-12, Exterior(2N) 7-3-12 to 7-11-12, Corner(3R) 7-11-12 to 10-11-12, Exterior(2N) 10-11-12 to 13-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) Plates checked for a plus or minus 5 degree rotation about its center.
- 10) Gable requires continuous bottom chord bearing.
- 11) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 12) Gable studs spaced at 2-0-0 oc.
- 13) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 14) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- 15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 18, 15 lb uplift at joint 12, 63 lb uplift at joint 17 and 61 lb uplift at joint 13.
- 16) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

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818 Soundside Road  
Edenton, NC 27932

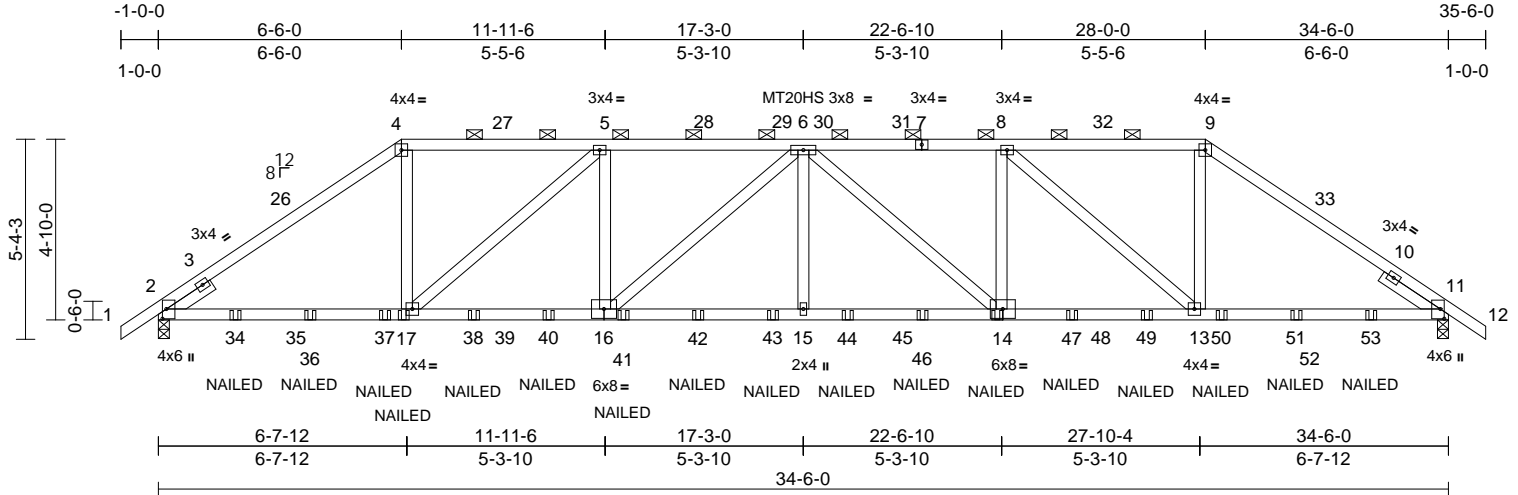


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084164
	A1GR	Hip Girder	1	2	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:18  
ID:5d0R4qU3yt\_HyZ0RM3zpmzqu2s-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrcDoi7J4zJC?f

Page: 1



Scale = 1:61.6

Plate Offsets (X, Y): [2:0-3-2,0-1-4], [11:0-3-2,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.24	15-16	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.36	15-16	>999	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.54	Horz(CT)	0.11	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.12	15-16	>999	240		
BCDL	10.0											
Weight: 372 lb											FT = 20%	

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 -- 1-4-12, Right 2x4 SP No.3 -- 1-9-10

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-5 oc purlins, except 2-0-0 oc purlins (5-0-1 max.): 4-9.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size) 2=0-3-8, 11=0-3-8  
Max Horiz 2=-80 (LC 10)  
Max Uplift 2=-351 (LC 9), 11=-322 (LC 8)  
Max Grav 2=3064 (LC 36), 11=2976 (LC 36)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/59, 2-4=-4608/627, 4-5=-3846/539, 5-6=-5708/774, 6-8=-5647/754, 8-9=-3727/499, 9-11=-4463/579, 11-12=0/59  
BOT CHORD 2-17=-525/3773, 15-17=-823/6255, 13-15=-823/6255, 11-13=-416/3656  
WEBS 4-17=-332/2277, 5-17=-2461/373, 5-16=-178/1051, 6-16=-719/110, 6-15=-90/608, 6-14=-799/136, 8-14=-195/1105, 8-13=-2538/400, 9-13=-305/2201

#### NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.  
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.  
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.

- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.

- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.

#### LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (lb/ft)  
Vert: 1-4=-51, 4-9=-61, 9-12=-51, 18-22=-20  
Concentrated Loads (lb)  
Vert: 17=-175 (B), 14=-175 (B), 34=-189 (B), 36=-151 (B), 37=-175 (B), 38=-175 (B), 40=-175 (B), 41=-175 (B), 42=-175 (B), 43=-175 (B), 44=-175 (B), 46=-175 (B), 47=-175 (B), 49=-175 (B), 50=-175 (B), 51=-151 (B), 53=-189 (B)



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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**TRENCO**  
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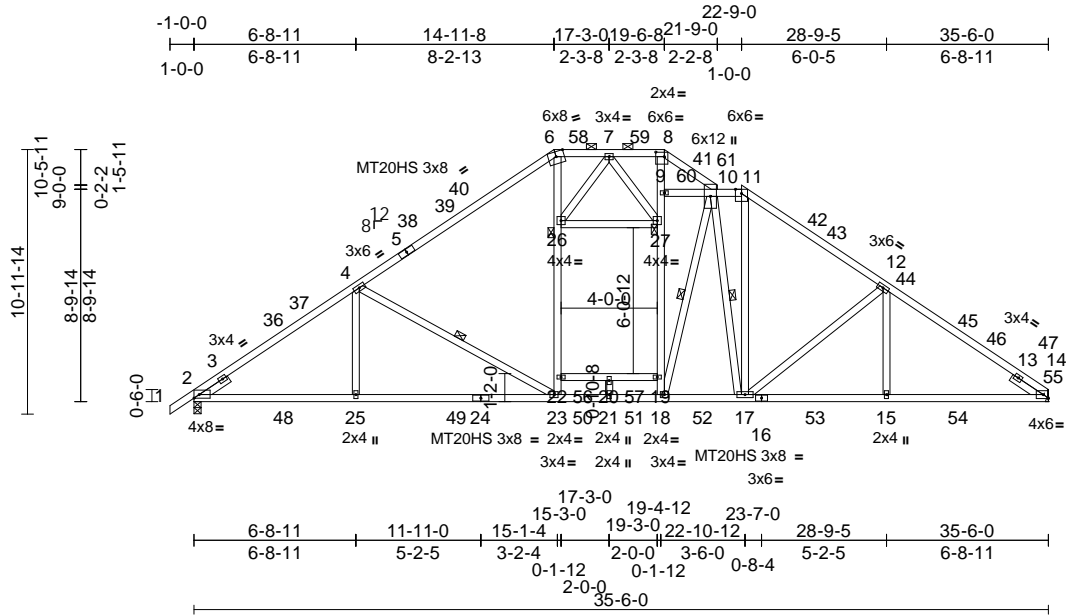
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084172
	B1D	Attic	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:21  
ID:evDsELfBtcxbCxRmOvBJJmzqtMK-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKwRCDoi7J4zJC?i

Page: 1



Scale = 1:95.7

Plate Offsets (X, Y): [2:Edge,0-1-12], [8:0-4-4,0-2-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.95	Vert(LL)	-0.31	23-25	>999	360	MT20HS	187/143
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.46	23-25	>917	240	MT20	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.09	14	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.05	17-18	>999	240		
BCDL	10.0											
											Weight: 252 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2 \*Except\* 5-6:2x4 SP SS  
BOT CHORD 2x4 SP SS \*Except\* 22-19:2x4 SP No.2  
WEBS 2x4 SP No.3 \*Except\* 6-23,8-18,26-27:2x4 SP No.2  
SLIDER Left 2x4 SP No.3 -- 1-4-13, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (3-6-5 max.): 6-8, 9-11.  
BOT CHORD Rigid ceiling directly applied.  
WEBS 1 Row at midpt 10-17, 10-18, 4-23  
JOINTS 1 Brace at Jt(s): 26, 27

#### REACTIONS

(size) 2=0-3-8, 14= Mechanical  
Max Horiz 2=169 (LC 15)  
Max Grav 2=1892 (LC 58), 14=1871 (LC 60)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

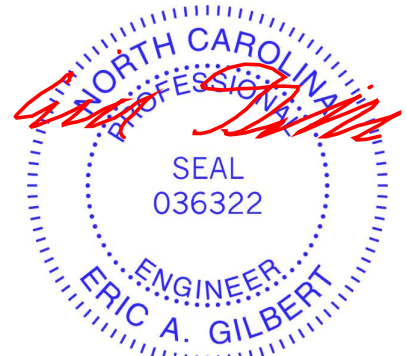
TOP CHORD 1-2=0/52, 2-4=-2792/38, 4-6=-2184/95, 8-10=-1965/130, 11-12=-2321/98, 12-14=-2828/48, 6-7=-1690/127, 7-8=-1639/121, 9-10=-316/118, 10-11=-1815/110  
BOT CHORD 2-25=0/2375, 23-25=0/2375, 21-23=0/1695, 18-21=0/1695, 17-18=0/1875, 15-17=0/2271, 14-15=0/2271, 20-22=-6/33, 19-20=-6/33  
WEBS 22-23=0/723, 22-26=0/782, 6-26=0/678, 18-19=-53/984, 19-27=-41/1044, 9-27=-33/972, 8-9=-37/1026, 26-27=-139/96, 11-17=0/767, 20-21=-110/34, 7-27=-176/108, 7-26=-169/252, 10-17=-389/94, 10-18=-870/101, 4-25=0/381, 4-23=-747/102, 12-15=0/367, 12-17=-623/86

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 15-0-0, Exterior(2R) 15-0-0 to 18-6-10, Interior (1) 18-6-10 to 19-6-8, Exterior(2E) 19-6-8 to 21-7-0, Interior (1) 21-7-0 to 22-9-0, Exterior(2R) 22-9-0 to 26-3-10, Interior (1) 26-3-10 to 35-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



June 11,2025

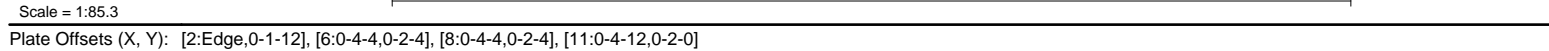
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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818 Soundside Road  
Edenton, NC 27932

Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:20 Page: 1  
ID:bl7?QBZlaRbGUTl91w4JJGzqsoH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWRcDoi7J4CJc?f

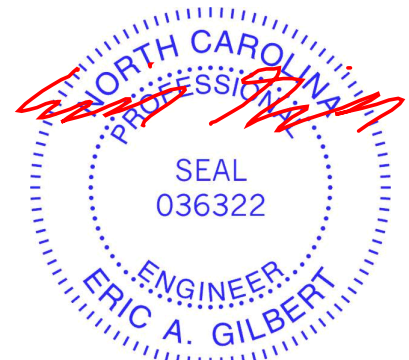


<b>LUMBER</b>		2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. I; Exp B; Enclosed; MWFRS (envelope) and C-C	14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
TOP CHORD	2x4 SP No.2	Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 12-11-8, Exterior(2R) 12-11-8 to 16-6-2, Interior (1) 16-6-2 to 21-6-8, Exterior(2E) 21-6-8 to 24-6-15, Interior (1) 24-6-15 to 25-9-0, Exterior(2R) 25-9-0 to 29-3-10, Interior (1) 29-3-10 to 35-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60	15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
BOT CHORD	2x4 SP SS *Except* 22-20:2x4 SP No.2		16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
WEBS	2x4 SP No.3 *Except* 6-23,8-18,26-27:2x4 SP No.2	3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL =	17) Attic room checked for L/360 deflection.
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0		<b>LOAD CASE(S)</b> Standard
<b>BRACING</b>			
TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-3-14 max.): 6-8, 9-11.		
BOT CHORD	Rigid ceiling directly applied.		
WEBS	1 Row at midpt 26-27		

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust)  
Vasd=95mph; TCDD=6.0psf; BCDL=6.0psf; h=25ft; Cat.  
II; Exp B; Enclosed; MWFRS (envelope) and C-C  
Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to  
12-11-8, Exterior(2R) 12-11-8 to 16-6-2, Interior (1)  
16-6-2 to 21-6-8, Exterior(2E) 21-6-8 to 24-6-15, Interior  
(1) 24-6-15 to 25-9-0, Exterior(2R) 25-9-0 to 29-3-10,  
Interior (1) 29-3-10 to 35-5-4 zone; cantilever left and  
right exposed ; end vertical left and right exposed;C-C  
for members and forces & MWFRS for reactions shown;  
Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCDL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15  
Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL =  
1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially  
Exp.; Ce=1.0; Cs=1.00; Ct=1.0; Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this  
design.
- 5) This truss has been designed for greater of min roof live  
load of 12.0 psf or 2.00 times flat roof load of 15.4 psf or  
overhangs non-concurrent with other live loads.
- 6) 250.0lb AC unit load placed on the bottom chord, 17-3-0  
from left end, supported at two points, 5-0-0 apart.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) Plates checked for a plus or minus 5 degree rotation  
about its center.
- 10) This truss has been designed for a 10.0 psf bottom  
chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf  
on the bottom chord in all areas where a rectangle  
3-06-00 tall by 2-00-00 wide will fit between the bottom  
chord and any other members.
- 12) Bottom chord live load (20.0 psf) and additional bottom  
chord dead load (20.0 psf) applied only to room. 21-22,  
20-21
- 13) Refer to girder(s) for truss to roof connections.

- 14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 17) Attic room checked for L/360 deflection.

**LOAD CASE(S)** Standard



June 11, 2025

**NOTES**

1) Unbalanced roof live loads have been considered for this design.

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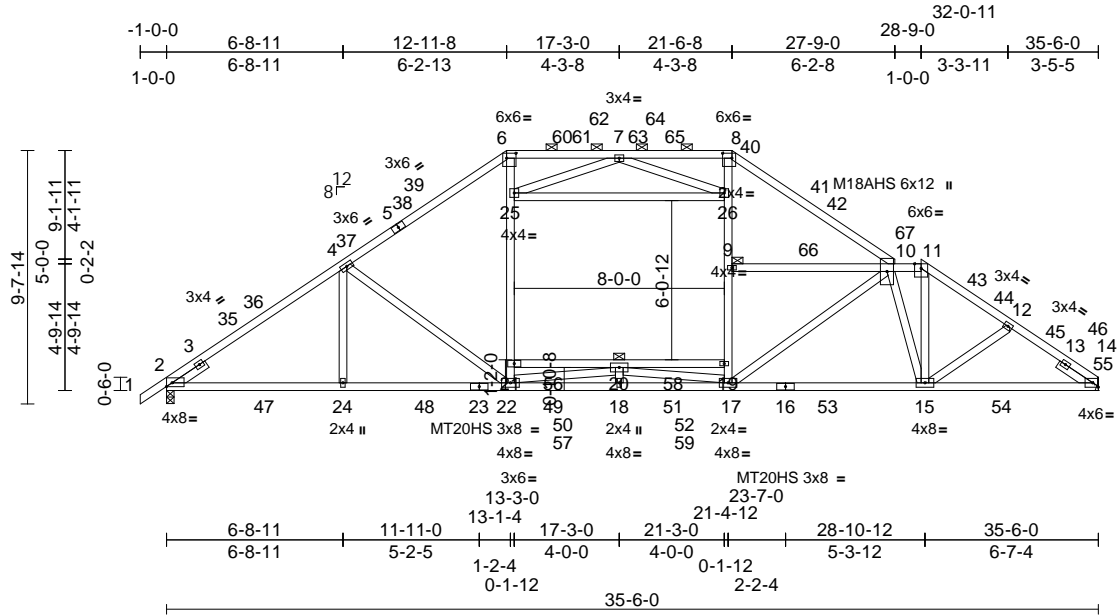
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084174
	B1B	Attic	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:20  
ID:1cFYIWIIPASJtQ43fVjmq7zqssW-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKwRCDoi7J4zJC?f

Page: 1



Scale = 1:87.8									
Plate Offsets (X, Y): [2:Edge,0-1-12], [6:0-4-4,0-2-4], [8:0-4-4,0-2-4]									
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.93	Vert(LL)	-0.29 15-17	>999	360
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.59	Vert(CT)	-0.49 15-17	>867	240
TCDL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(CT)	0.11 14	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.10 15-17	>999	240
BCDL	10.0								
									<b>PLATES</b>
									<b>GRIP</b>
									MT20 244/190
									MT20HS 187/143
									M18AHS 186/179
									Weight: 251 lb FT = 20%

**LUMBER**  
TOP CHORD 2x4 SP No.2 \*Except\* 5-6:2x4 SP SS  
BOT CHORD 2x4 SP SS \*Except\* 21-19:2x4 SP No.2  
WEBS 2x4 SP No.3 \*Except\* 6-22,8-17,25-26:2x4 SP No.2  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

**BRACING**  
TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (3-5-8 max.): 6-8, 9-11.  
BOT CHORD Rigid ceiling directly applied.  
JOINTS 1 Brace at Jt(s): 9

**REACTIONS** (size) 2=0-3-8, 14= Mechanical  
Max Horiz 2=147 (LC 15)  
Max Grav 2=2035 (LC 46), 14=2027 (LC 46)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/52, 2-4=-3031/0, 4-6=-2571/0, 8-10=-2646/0, 11-12=-2942/0, 12-14=-3087/0, 6-7=-2027/0, 7-8=-2135/0, 9-10=-76/23, 10-11=-2394/0  
BOT CHORD 2-24=0/2431, 22-24=0/2431, 18-22=0/3070, 17-18=0/3070, 15-17=0/2771, 14-15=0/2484, 20-21=-3/417, 19-20=-92/176  
WEBS 21-22=0/849, 21-25=0/995, 6-25=0/1014, 17-19=0/968, 9-19=0/1134, 9-26=0/1148, 8-26=0/1140, 25-26=-55/573, 18-20=0/327, 20-22=-1545/0, 17-20=-1402/0, 4-24=0/321, 4-22=-544/125, 7-25=-640/135, 7-26=-508/172, 11-15=0/1434, 12-15=-131/144, 10-15=-1341/0, 10-17=-895/68

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 12-11-8, Exterior(2R) 12-11-8 to 16-6-2, Interior (1) 16-6-2 to 21-6-8, Exterior(2R) 21-6-8 to 25-1-2, Interior (1) 25-1-2 to 28-9-0, Exterior(2R) 28-9-0 to 32-2-8, Interior (1) 32-2-8 to 35-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) 250.0lb AC unit load placed on the bottom chord, 17-3-0 from left end, supported at two points, 5-0-0 apart.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) Plates checked for a plus or minus 5 degree rotation about its center.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Bottom chord live load (20.0 psf) and additional bottom chord dead load (20.0 psf) applied only to room. 20-21, 19-20
- 13) Refer to girder(s) for truss to truss connections.

- 14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
  - 15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
  - 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
  - 17) Attic room checked for L/360 deflection.
- LOAD CASE(S)** Standard



June 11,2025

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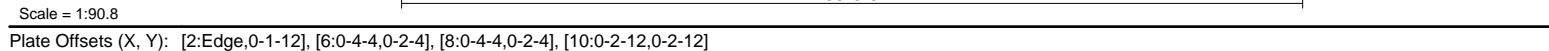
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ID:W5Q6LOguXeNK5PP3fxsEzIzqtVK-RfC?PsB70Hq3NSgPqnL8w3u1tXhGKwRCDoi7J4JcZ?#



<b>LUMBER</b>		2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 12-11-8, Exterior(2R) 12-11-8 to 16-6-2, Interior (1) 16-6-2 to 21-6-8, Exterior(2R) 21-6-8 to 25-1-2, Interior (1) 25-1-2 to 31-9-0, Exterior(2E) 31-9-0 to 35-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60	4) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
TOP CHORD	2x4 SP No.2 *Except* 5-6:2x4 SP SS		
BOT CHORD	2x4 SP SS *Except* 22-20:2x4 SP No.2		
WEBS	2x4 SP No.3 *Except* 6-23,8-18,26-27:2x4 SP No.2		15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0		16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
<b>BRACING</b>			17) Attic room checked for L/360 deflection.
TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-5-1 max.): 6-8, 11-12.	3) TLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0	<b>LOAD CASE(S)</b> Standard
BOT CHORD	Rigid ceiling directly applied.	4) Unbalanced snow loads have been considered for this design.	
WEBS	1 Row at midpt 9-18	5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.	
JOINTS	1 Brace at Jt(s): 11	6) 250.0lb AC unit load placed on the bottom chord, 17-3-0 from left end, supported at two points, 5-0-0 apart.	
<b>REACTIONS</b>	(size) 2=0-3-8, 14= Mechanical Max Horiz 2=147 (LC 15) Max Grav 2=2042 (LC 46), 14=2018 (LC 46)	7) Provide adequate drainage to prevent water ponding.	
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension	8) All plates are MT20 plates unless otherwise indicated.	
TOP CHORD	1-2=0/52, 2-4=-3041/0, 4-6=-2581/0, 8-9=-2642/0, 9-10=-3428/0, 12-14=-3109/0, 6-7=-2035/0, 7-8=-2162/0, 10-11=-36/0, 10-12=-2455/0	9) Plates checked for a plus or minus 5 degree rotation about its center.	
BOT CHORD	2-25=0/2440, 23-25=0/2440, 19-23=0/3061, 18-19=0/3061, 16-18=0/2726, 15-16=0/3321, 14-15=0/2544, 21-22=-9/415, 20-21=-118/268	10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	
WEBS	4-25=0/321, 4-23=-544/125, 22-23=0/844, 22-26=0/990, 6-26=0/1019, 18-20=0/918, 20-27=0/1084, 8-27=0/1076, 26-27=-44/589, 7-26=-644/123, 7-27=-511/161, 19-21=0/331, 21-23=-1542/0, 18-21=-1421/0, 12-15=0/1560, 11-16=0/696, 9-11=0/710, 9-18=-844/76, 10-15=-1801/0, 10-16=-790/0	11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.	
		12) Bottom chord live load (20.0 psf) and additional bottom chord dead load (20.0 psf) applied only to room. 21-22, 20-21	

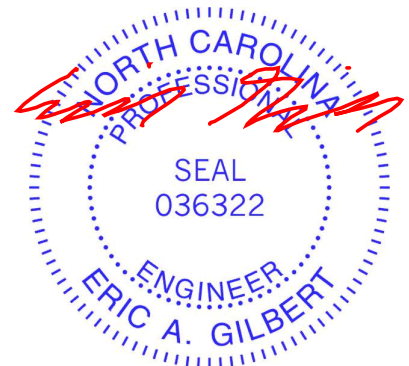
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust)  
Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.  
II; Exp B; Enclosed; MWFRS (envelope) and C-C  
Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to  
12-11-8, Exterior(2R) 12-11-8 to 16-6-2, Interior (1)  
16-6-2 to 21-6-8, Exterior(2R) 21-6-8 to 25-1-2, Interior  
(1) 25-1-2 to 31-9-0, Exterior(2E) 31-9-0 to 35-5-4 zone;  
cantilever left and right exposed ; end vertical left and  
right exposed; C-C for members and forces & MWFRS  
for reactions shown; Lumber DOL=1.60 plate grip  
DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15  
Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL =  
1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially  
Exp.; Ce=1.0; Cs=1.00; Ct=1.10; Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this  
design.
- 5) This truss has been designed for greater of min roof live  
load of 12.0 psf or 2.00 times flat roof load of 15.4 psf or  
overhangs non-concurrent with other live loads.
- 6) 250.0lb AC unit load placed on the bottom chord, 17-3-0  
from left end, supported at two points, 5-0-0 apart.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) Plates checked for a plus or minus 5 degree rotation  
about its center.
- 10) This truss has been designed for a 10.0 psf bottom  
chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf  
on the bottom chord in all areas where a rectangle  
3-06-00 tall by 2-00-00 wide will fit between the bottom  
chord and any other members.
- 12) Bottom chord live load (20.0 psf) and additional bottom  
chord dead load (20.0 psf) applied only to room. 21-22,  
20-21
- 13) Refer to girder(s) for truss to truss connections.

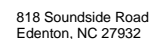
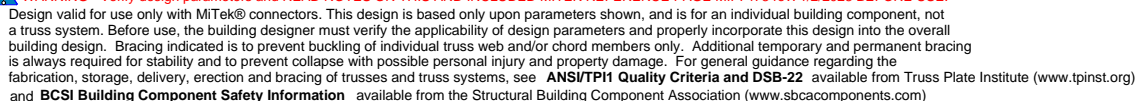
- 14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 17) Attic room checked for L/360 deflection.

**LOAD CASE(S)**      Standard

LOAD CASE(S) Standard

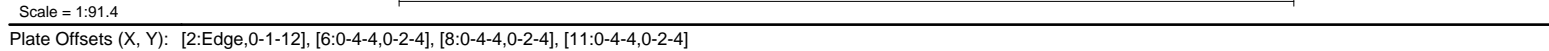


June 11, 2025





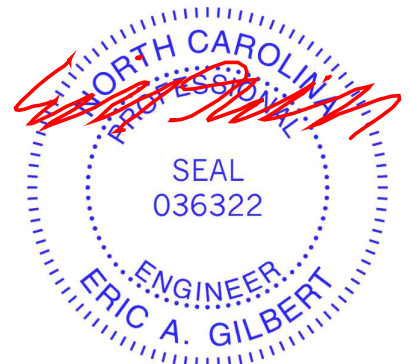
Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:18 Page: 1  
ID:NNrpy8wJwMij\_HdvBNzfEzqtG-RfC?PsB70Hg3NSgPqnL8w3ulTXbGKwRCDoi7J4zJC?f



<b>LUMBER</b>		2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 12-11-8, Exterior(2R) 12-11-8 to 16-6-2, Interior (1) 16-6-2 to 21-6-8, Exterior(2R) 21-6-8 to 25-1-2, Interior (1) 25-1-2 to 34-9-0, Exterior(2E) 34-9-0 to 35-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60	3) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12. This connection is for uplift only and does not consider lateral forces.
TOP CHORD	2x4 SP No.2 *Except* 5-6:2x4 SP SS		
BOT CHORD	2x4 SP SS *Except* 20-18:2x4 SP No.2		14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
WEBS	2x4 SP No.3 *Except* 6-21,8-16,24-25:2x4 SP No.2		15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
SLIDER	Left 2x4 SP No.3 -- 1-6-0		16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
<b>BRACING</b>			17) Attic room checked for L/360 deflection.
TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-8-8 max.): 6-8, 10-11.	3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0	<b>LOAD CASE(S)</b> Standard
BOT CHORD	Rigid ceiling directly applied.	4) Unbalanced snow loads have been considered for this design.	
<b>REACTIONS</b>	(size) 2=0-3-8, 12=1-3-8, 13=1-3-8 Max Horiz 2=146 (LC 15) Max Uplift 12=-208 (LC 98) Max Grav 2=1951 (LC 46), 12=160 (LC 100), 13=2233 (LC 46)	5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.	
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension	6) 250.0lb AC unit load placed on the bottom chord, 17-3-0 from left end, supported at two points, 5-0-0 apart.	
TOP CHORD	1-2=0/52, 2-4=-2894/0, 4-6=-2416/0, 6-7=-1963/0, 7-8=-1881/0, 8-9=-2392/0, 9-10=-2774/0, 10-11=-255/257, 11-12=-134/265	7) Provide adequate drainage to prevent water ponding.	
BOT CHORD	2-23=0/2319, 21-23=0/2319, 17-21=0/2926, 16-17=0/2926, 14-16=0/2197, 13-14=-53/525, 12-13=-229/72, 19-20=-88/263, 18-19=-15/355	8) All plates are MT20 plates unless otherwise indicated.	
WEBS	4-23=0/331, 4-21=-568/122, 20-21=0/791, 20-24=0/954, 6-24=0/949, 16-18=0/765, 18-25=0/906, 8-25=0/935, 10-13=-2171/0, 11-13=-97/251, 24-25=-46/585, 17-19=0/333, 7-24=-548/146, 7-25=-615/126, 9-14=-137/185, 9-16=-447/132, 16-19=-1528/0, 10-14=0/1808, 19-21=-1449/0	9) Plates checked for a plus or minus 5 degree rotation about its center.	
		10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	
		11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.	
		12) Bottom chord live load (20.0 psf) and additional bottom chord dead load (20.0 psf) applied only to room. 19-20, 18-19	

1) Unbalanced roof live loads have been considered for this design.

June 11, 2025

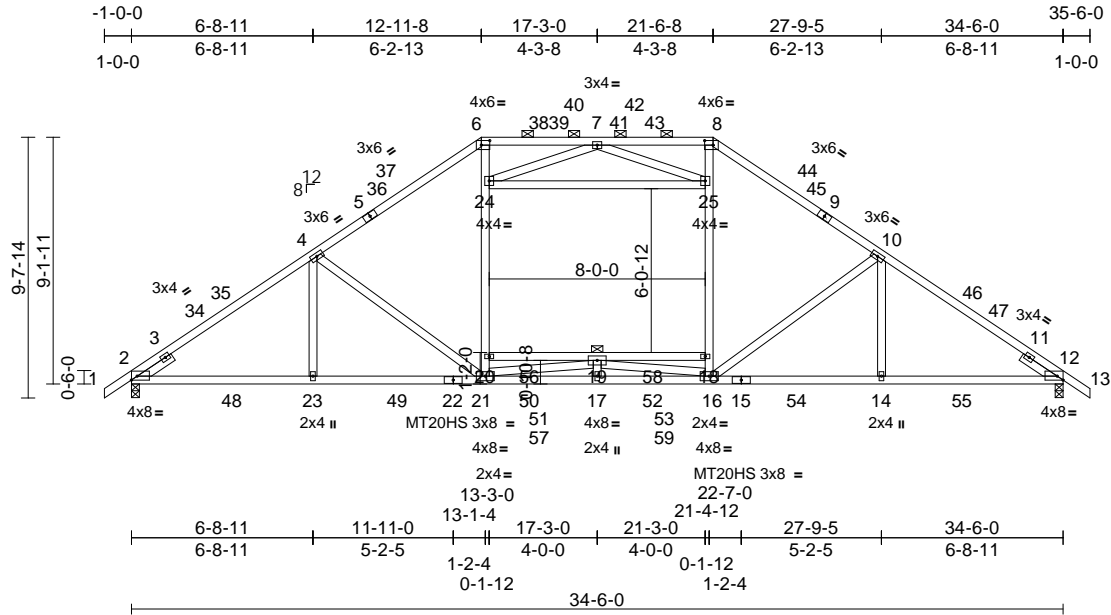


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084177
	A1D	Attic	3	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:17  
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Page: 1



Scale = 1:85.3

Plate Offsets (X, Y): [2:Edge,0-1-12], [6:0-3-12,0-2-0], [8:0-3-12,0-2-0], [12:Edge,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.22	14-16	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.35	14-16	>999	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(CT)	0.11	12	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.08	21-23	>999	240		
BCDL	10.0											
											Weight: 229 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2 \*Except\* 5-6,8-9:2x4 SP SS  
BOT CHORD 2x4 SP SS \*Except\* 20-18:2x4 SP No.2  
WEBS 2x4 SP No.3 \*Except\* 6-21,8-16,24-25:2x4 SP No.2  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (3-8-8 max.): 6-8.  
BOT CHORD Rigid ceiling directly applied.

#### REACTIONS

(size) 2=0-3-8, 12=0-3-8  
Max Horiz 2=149 (LC 14)  
Max Grav 2=1982 (LC 42), 12=1982 (LC 42)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/52, 2-4=-2944/0, 4-6=-2472/0, 6-7=-1984/0, 7-8=-1985/0, 8-10=-2472/0, 10-12=-2944/0, 12-13=0/52  
BOT CHORD 2-23=0/2360, 21-23=0/2360, 17-21=0/2967, 16-17=0/2967, 14-16=0/2360, 12-14=0/2360, 19-20=-57/325, 18-19=-57/325  
WEBS 4-23=0/328, 4-21=-559/124, 20-21=0/809, 20-24=0/965, 6-24=0/972, 16-18=0/809, 18-25=0/965, 8-25=0/972, 10-16=-559/124, 10-14=0/328, 24-25=-46/578, 17-19=0/330, 7-24=-572/143, 7-25=-572/143, 19-21=-1480/0, 16-19=-1480/0

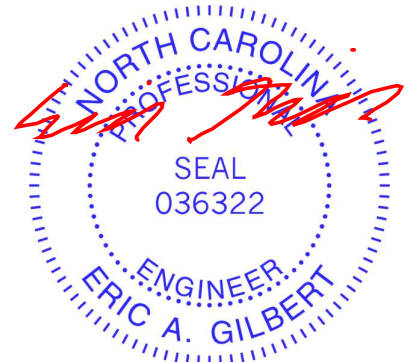
#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-6, Interior (1) 2-5-6 to 12-11-8, Exterior(2R) 12-11-8 to 16-4-14, Interior (1) 16-4-14 to 21-6-8, Exterior(2R) 21-6-8 to 24-11-14, Interior (1) 24-11-14 to 35-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 250.0lb AC unit load placed on the bottom chord, 17-3-0 from left end, supported at two points, 5-0-0 apart.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bottom chord live load (20.0 psf) and additional bottom chord dead load (20.0 psf) applied only to room. 19-20, 18-19
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcacomponents.com](http://www.sbcacomponents.com))

ENGINEERING BY  
**TRENCO**  
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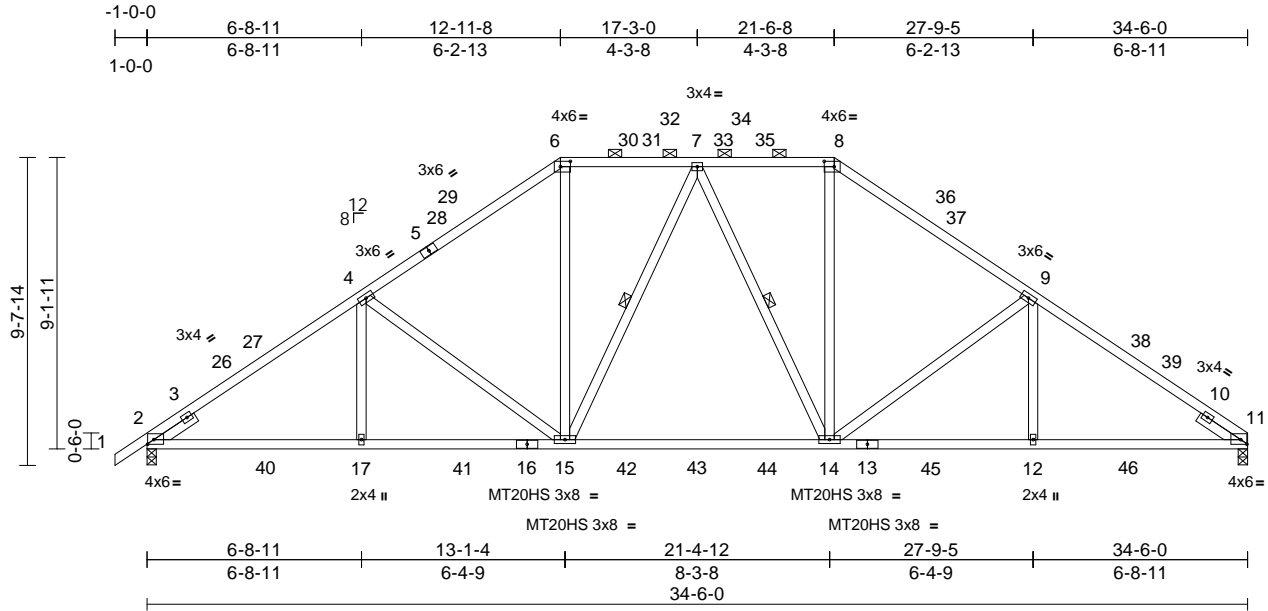
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084178
	A1C	Piggyback Base	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:17  
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Page: 1



Scale = 1:72.2

Plate Offsets (X, Y): [2:0-0-0,0-0-0], [2:0-2-5,0-1-12], [6:0-3-12,0-2-0], [8:0-3-12,0-2-0], [11:0-2-5,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	-0.27	14-15	>999	360	MT20 244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.59	Vert(CT)	-0.42	14-15	>987	240	MT20HS 187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.08	11	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.03	14-15	>999	240	
BCDL	10.0										
Weight: 206 lb FT = 20%											

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (4-6-8 max.): 6-8.  
BOT CHORD Rigid ceiling directly applied.  
WEBS 1 Row at midpt 7-15, 7-14

#### REACTIONS

(size) 2=0-3-8, 11=0-3-8  
Max Horiz 2=145 (LC 13)  
Max Grav 2=1688 (LC 49), 11=1633 (LC 51)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/52, 2-4=-2402/81, 4-6=-1908/130,  
6-7=-1481/144, 7-8=-1481/143,  
8-9=-1909/132, 9-11=-2406/84

BOT CHORD 2-17=-8/2009, 15-17=-8/2009, 14-15=0/1492,  
12-14=0/1934, 11-12=0/1934

WEBS 4-17=0/353, 4-15=-644/92, 6-15=0/677,  
7-15=-252/95, 7-14=-251/94, 8-14=0/678,  
9-14=-650/93, 9-12=0/355

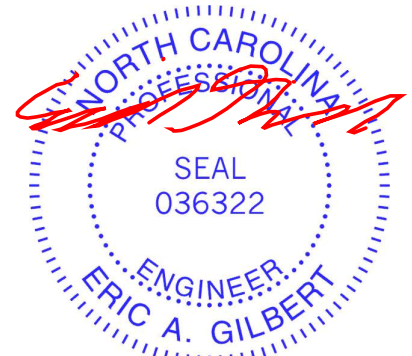
#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-6, Interior (1) 2-5-6 to 12-11-8, Exterior(2R) 12-11-8 to 16-4-14, Interior (1) 16-4-14 to 21-6-8, Exterior(2R) 21-6-8 to 24-11-14, Interior (1) 24-11-14 to 34-4-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



June 11, 2025

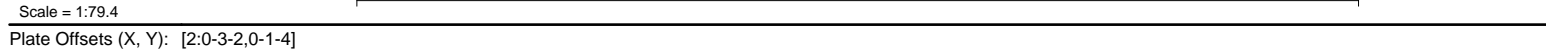
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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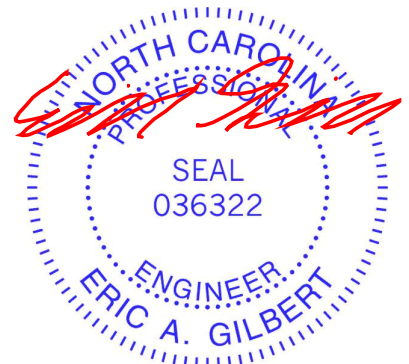
ENGINEERING BY  
**TRENCO**  
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818 Soundside Road  
Edenton, NC 27932

Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:17 Page: 1  
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<b>LUMBER</b>			
TOP CHORD	2x4 SP No.2 *Except*	5-6,7-8:	2x4 SP SS
BOT CHORD	2x4 SP SS		
WEBS	2x4 SP No.3		
SLIDER	Left 2x4 SP No.3 --	1-6-0,	Right 2x4 SP No.3 -- 1-6-0
<b>BRACING</b>			
TOP CHORD	Structural wood sheathing directly applied, except		
	2-0-0 oc purlins (4-1-3 max.):		6-7.
BOT CHORD	Rigid ceiling directly applied.		
WEBS	1 Row at midpt	4-15,	6-14, 9-14
<b>REACTIONS</b>	(size)	2=0-3-8,	11=0-3-8
	Max Horiz	2=169 (LC 13)	
	Max Grav	2=1736 (LC 49),	11=1679 (LC 51)
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension		
TOP CHORD	1-2=0/52, 2-4=-2489/0,	4-6=-1879/69,	
	6-7=-1426/90, 7-9=-1877/82,	9-11=-2491/0	
BOT CHORD	2-17=0/2117, 15-17=0/2117,	14-15=0/1473,	
	12-14=0/1999, 11-12=0/1999		
WEBS	4-17=0/396, 4-15=-767/72,	6-15=0/659,	
	6-14=-166/169, 7-14=0/647,	9-14=-771/74,	
	9-12=0/396		
<b>NOTES</b>			
1)	Unbalanced roof live loads have been considered for this design.		
2)	Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-6, Interior (1) 2-5-6 to 15-3-12, Exterior(2E) 15-3-12 to 19-2-4, Exterior(2R) 19-2-4 to 24-0-13, Interior (1) 24-0-13 to 34-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60		
3)	TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15; Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0		
4)	Unbalanced snow loads have been considered for this design.		
5)	This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.		
6)	Provide adequate drainage to prevent water ponding.		
7)	All plates are MT20 plates unless otherwise indicated.		
8)	Plates checked for a plus or minus 5 degree rotation about its center.		
9)	This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.		
10)	* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.		
11)	This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.		
12)	This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.		
13)	Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.		
<b>LOAD CASE(S)</b>	Standard		



June 11, 2025

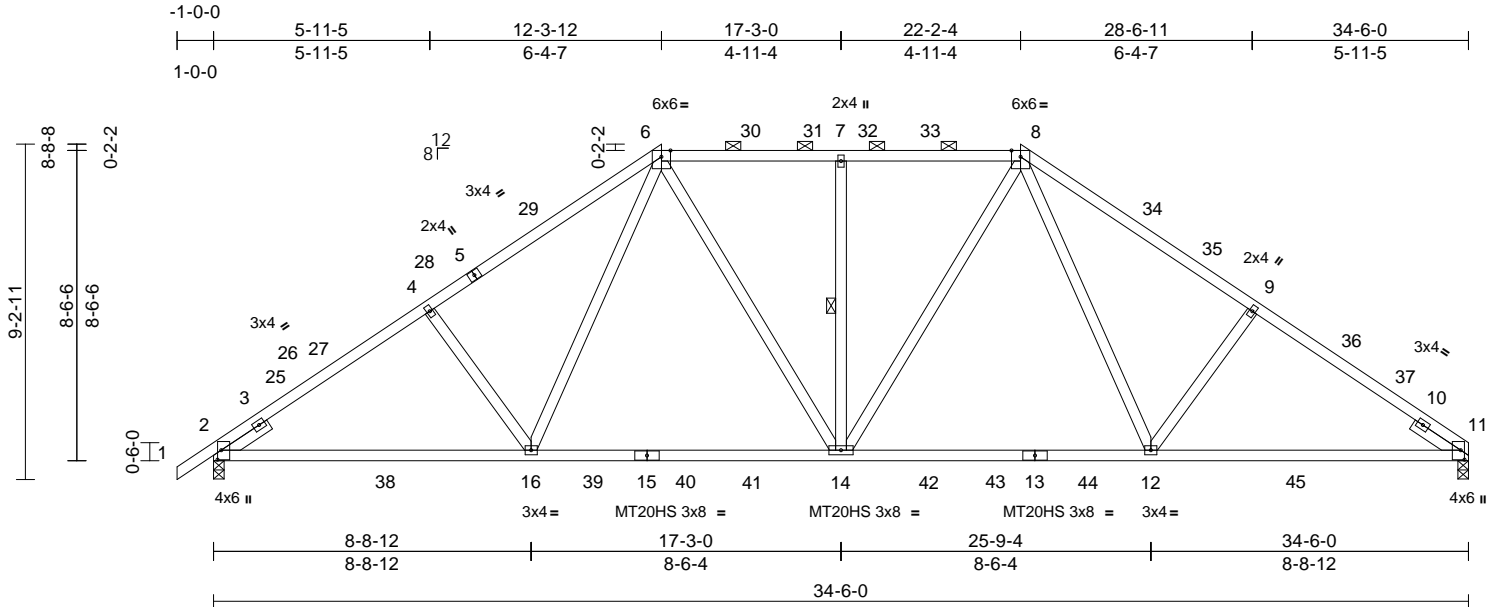


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084180
	A1A	Hip	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:17  
ID:06RDXpl5c?7alz7X4HTXZ5zqu4h-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:63.3

Plate Offsets (X, Y): [2:0-3-2,0-1-4], [11:0-3-2,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.33	12-14	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.62	Vert(CT)	-0.46	14-16	>893	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.07	11	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.03	14-16	>999	240		
BCDL	10.0											
											Weight: 197 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP SS
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD	Structural wood sheathing directly applied, except
	2-0-0 oc purlins (4-2-12 max.): 6-8.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 7-14

#### REACTIONS

(size)	2=0-3-8, 11=0-3-8
Max Horiz	2=137 (LC 13)
Max Grav	2=1704 (LC 49), 11=1648 (LC 51)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/52, 2-4=-2432/12, 4-6=-2247/51, 6-7=-1677/68, 7-8=-1677/69, 8-9=-2252/50, 9-11=-2438/10
BOT CHORD	2-16=0/2030, 14-16=0/1517, 12-14=0/1510, 11-12=0/1970
WEBS	6-16=0/709, 4-16=-438/107, 6-14=-35/490, 7-14=-538/55, 8-14=-35/489, 8-12=0/717, 9-12=-446/108

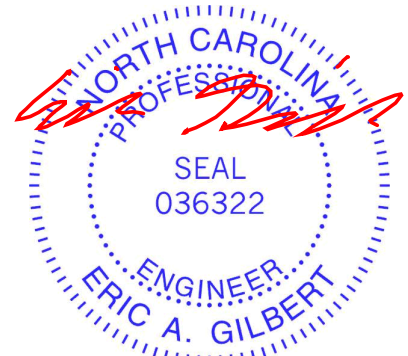
#### NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-6, Interior (1) 2-5-6 to 12-3-12, Exterior(2R) 12-3-12 to 17-3-0, Interior (1) 17-3-0 to 22-2-4, Exterior(2R) 22-2-4 to 27-0-13, Interior (1) 27-0-13 to 34-4-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



June 11, 2025

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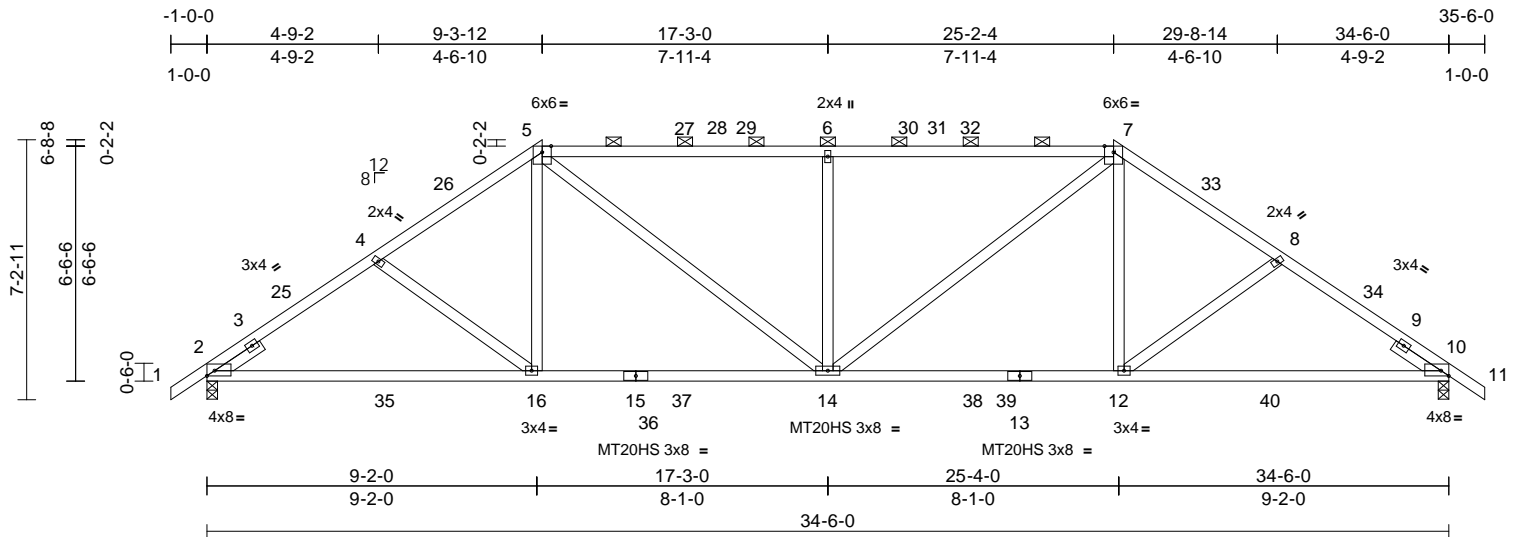
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084181
	A1	Hip	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:15

Page: 1

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Scale = 1:64

Plate Offsets (X, Y): [2:Edge,0-1-12], [10:Edge,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.69	Vert(LL)	-0.29	12-14	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.41	12-14	>998	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(CT)	0.08	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.04	14	>999	240		
BCDL	10.0											
Weight: 189 lb											FT = 20%	

#### LUMBER

TOP CHORD 2x4 SP No.2 \*Except\* 5-7:2x4 SP SS  
BOT CHORD 2x4 SP SS  
WEBS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (3-5-3 max.): 5-7.  
BOT CHORD Rigid ceiling directly applied.

#### REACTIONS

(size) 2=0-3-8, 10=0-3-8  
Max Horiz 2=109 (LC 15)  
Max Grav 2=1631 (LC 47), 10=1631 (LC 47)

#### FORCES

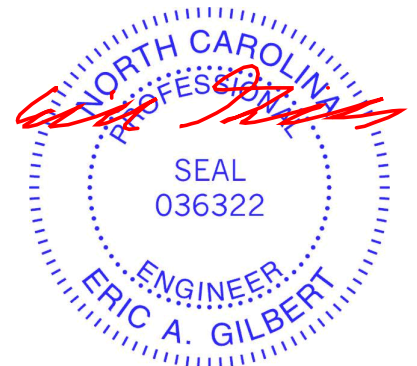
(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/59, 2-4=-2212/2, 4-5=-2160/6, 5-6=-2441/15, 6-7=-2441/15, 7-8=-2160/6, 8-10=-2212/2, 10-11=0/59  
BOT CHORD 2-16=0/1810, 14-16=0/1805, 12-14=0/1805, 10-12=0/1798  
WEBS 4-16=-312/86, 5-16=0/509, 5-14=-43/832, 6-14=-872/88, 7-14=-43/832, 7-12=0/509, 8-12=-312/86

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-6, Interior (1) 2-5-6 to 9-3-12, Exterior(2R) 9-3-12 to 14-2-5, Interior (1) 14-2-5 to 25-2-4, Exterior(2R) 25-2-4 to 29-10-6, Interior (1) 29-10-6 to 35-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



June 11, 2025

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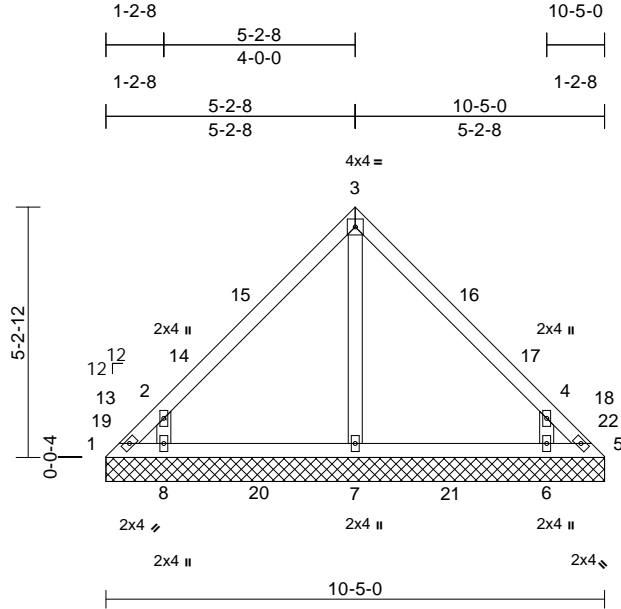
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084182
	V4	Valley	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:37

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Scale = 1:48.1

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.69	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.10	Horiz(TL)	0.00	5	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS							
BCDL	10.0									Weight: 45 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10'-0" oc bracing.

#### REACTIONS

(size)	1=10-5-0, 5=10-5-0, 6=10-5-0, 7=10-5-0, 8=10-5-0
Max Horiz	1=80 (LC 15)
Max Uplift	1=-97 (LC 54), 5=-97 (LC 55), 6=-67 (LC 17), 8=-70 (LC 16)
Max Grav	1=252 (LC 45), 5=252 (LC 51), 6=417 (LC 42), 7=376 (LC 60), 8=417 (LC 41)

#### FORCES

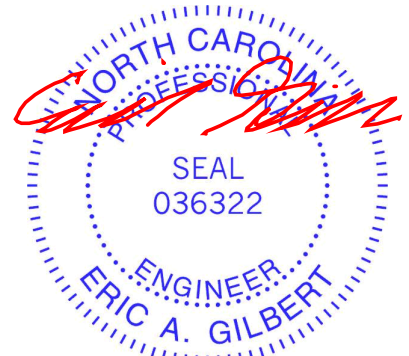
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-193/139, 2-3=-206/116, 3-4=-206/109, 4-5=-185/139
BOT CHORD	1-8=-88/104, 7-8=-17/68, 6-7=-17/68, 5-6=-88/104
WEBS	3-7=-203/0, 2-8=-488/347, 4-6=-488/346

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-4 to 3-0-4, Interior (1) 3-0-4 to 5-2-12, Exterior(2R) 5-2-12 to 8-2-12, Interior (1) 8-2-12 to 10-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4'-0" oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06"-00 tall by 2'-00"-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 1, 97 lb uplift at joint 5, 70 lb uplift at joint 8 and 67 lb uplift at joint 6.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



June 11, 2025

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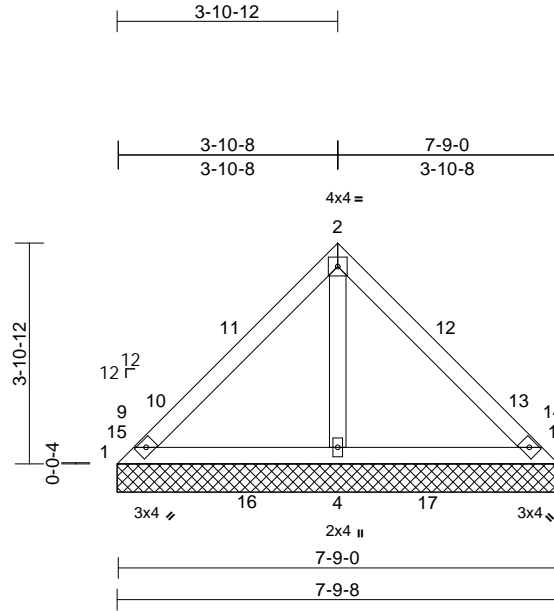
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	V5	Valley	1	1	174084183
Job Reference (optional)					

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:38  
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Page: 1



Scale = 1:40.7

<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in	(loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.67	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 31 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 7-9-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

#### REACTIONS

(size)	1=7-9-8, 3=7-9-8, 4=7-9-8
Max Horiz	1=59 (LC 13)
Max Uplift	1=-59 (LC 41), 3=-59 (LC 40), 4=-6 (LC 16)
Max Grav	1=268 (LC 43), 3=268 (LC 47), 4=579 (LC 40)

#### FORCES

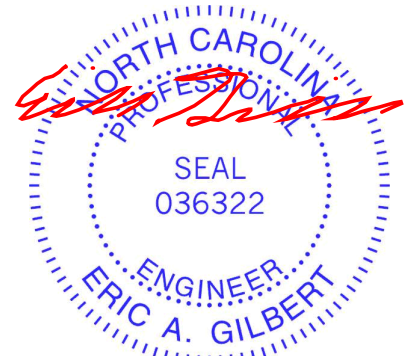
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-203/275, 2-3=-203/275
BOT CHORD	1-4=-187/128, 3-4=-187/128
WEBS	2-4=-420/121

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 3-10-12, Exterior(2R) 3-10-12 to 7-1-1, Interior (1) 7-1-1 to 7-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 1, 59 lb uplift at joint 3 and 6 lb uplift at joint 4.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11,2025

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818 Soundside Road  
Edenton, NC 27932

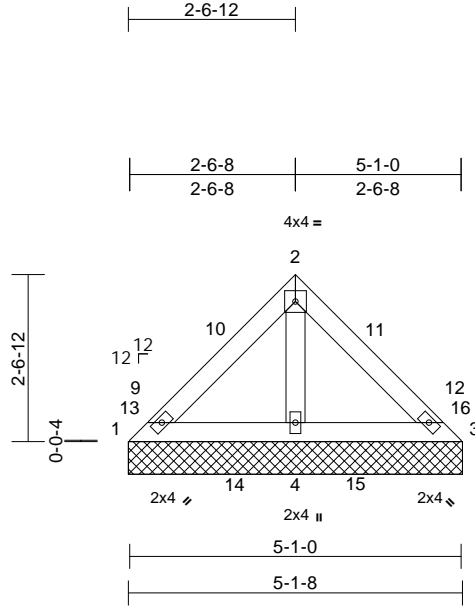
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084184
	V6	Valley	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.27	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.36	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	4	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 20 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-1-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

#### REACTIONS

(size)	1=5-1-8, 3=5-1-8, 4=5-1-8
Max Horiz	1=-38 (LC 12)
Max Uplift	1=-28 (LC 46), 3=-28 (LC 44)
Max Grav	1=277 (LC 43), 3=277 (LC 47), 4=412 (LC 49)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-210/176, 2-3=-210/176
BOT CHORD	1-4=-82/134, 3-4=-82/134
WEBS	2-4=-272/126

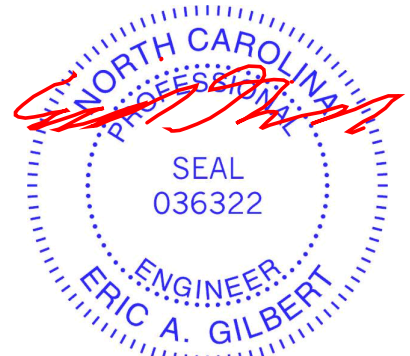
#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1 and 28 lb uplift at joint 3.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

#### LOAD CASE(S)

Standard



June 11, 2025

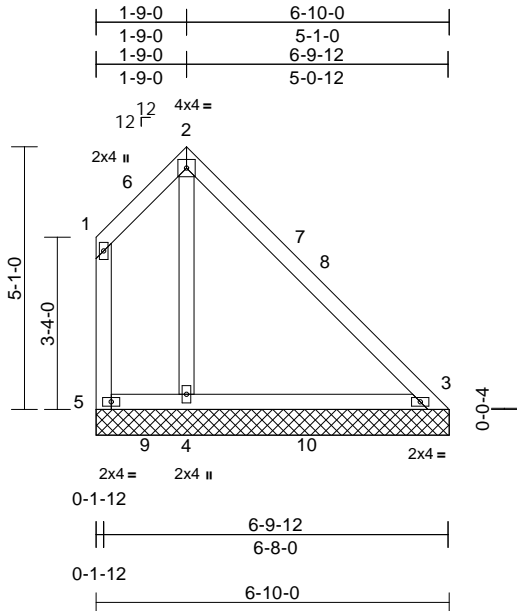
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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	V1	Valley	1	1	174084185
Job Reference (optional)					



Scale = 1:44.6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.61	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-P								
BCDL	10.0										Weight: 35 lb	FT = 20%

<b>LUMBER</b>	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
<b>BRACING</b>	
TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
<b>REACTIONS</b>	
(size)	3=6-10-0, 4=6-10-0, 5=6-10-0
Max Horiz	5=-103 (LC 12)
Max Uplift	4=-7 (LC 12), 5=-97 (LC 46)
Max Grav	3=337 (LC 44), 4=426 (LC 46), 5=259 (LC 40)
<b>FORCES</b>	
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-5=-269/90, 1-2=-107/112, 2-3=-130/117
BOT CHORD	4-5=-126/130, 3-4=-126/130
WEBS	2-4=-316/124

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 1-9-0, Exterior(2R) 1-9-0 to 4-9-0, Interior (1) 4-9-0 to 6-5-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5, 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 5 and 7 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11,2025



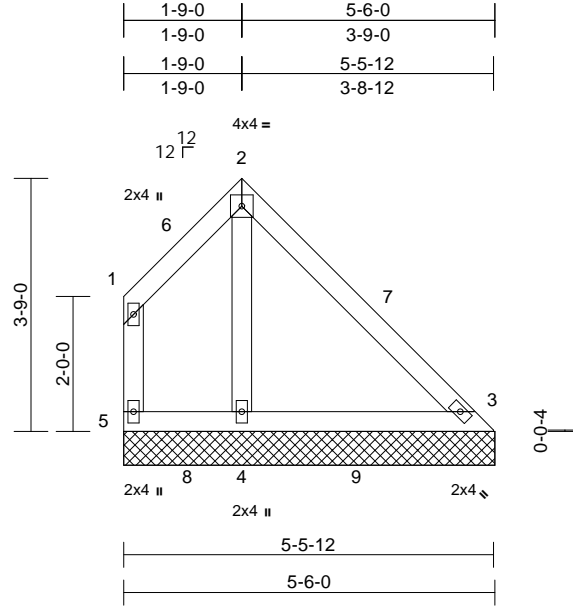
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	V2	Valley	1	1	Job Reference (optional)
					I74084186

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Scale = 1:34.1

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.59	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.70	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-P							
BCDL	10.0									Weight: 26 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.3
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-6-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size)	3=5-6-0, 4=5-6-0, 5=5-6-0
Max Horiz	5=-70 (LC 14)
Max Uplift	5=-50 (LC 45)
Max Grav	3=315 (LC 43), 4=373 (LC 42), 5=272 (LC 39)

#### FORCES

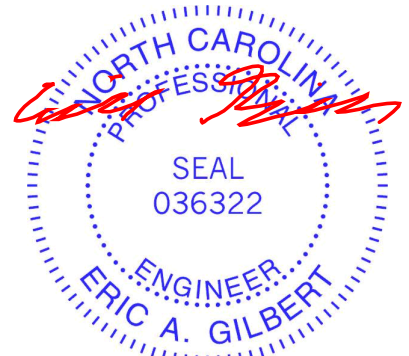
	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-5=-269/70, 1-2=-99/99, 2-3=-117/101
BOT CHORD	4-5=-73/75, 3-4=-73/75
WEBS	2-4=-303/64

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 1-9-0, Exterior(2R) 1-9-0 to 4-9-0, Interior (1) 4-9-0 to 5-1-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 5.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



June 11,2025

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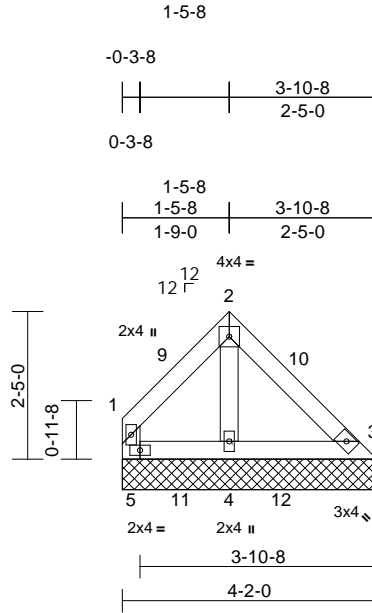
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	V3	Valley	1	1	174084187
Job Reference (optional)					

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Scale = 1:37.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.97	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	3	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 18 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-2-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied.

#### REACTIONS

(size)	3=4-2-0, 4=4-2-0, 5=4-2-0
Max Horiz	5=-33 (LC 12)
Max Uplift	3=-68 (LC 46), 5=-272 (LC 46)
Max Grav	3=3 (LC 16), 4=740 (LC 46), 5=240 (LC 43)

#### FORCES

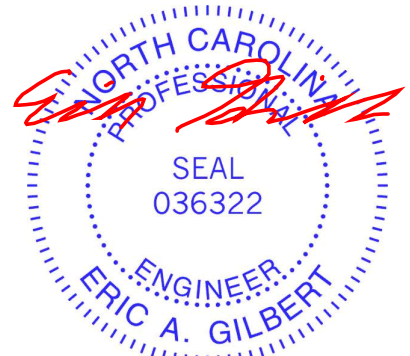
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-99/99, 2-3=-123/108
BOT CHORD	4-5=-30/33, 3-4=-30/33
WEBS	2-4=-282/7, 1-5=-269/42

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- 5) Unbalanced snow loads have been considered for this design.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 4-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 3, 272 lb uplift at joint 5 and 68 lb uplift at joint 3.
- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



June 11,2025

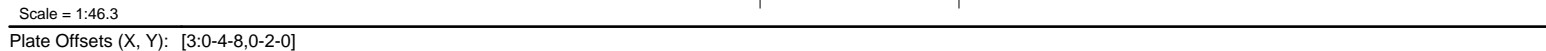
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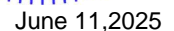
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<b>LUMBER</b>		Provide adequate drainage to prevent water ponding.	1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
TOP CHORD	2x4 SP No.2	7) Plates checked for a plus or minus 5 degree rotation about its center.	Uniform Loads (lb/ft)
BOT CHORD	2x4 SP No.2		Vert: 1-3=-51, 3-4=-61, 5-6=-20
WEBS	2x4 SP No.3	8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	Concentrated Loads (lb)
<b>BRACING</b>		9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.	Vert: 3=26 (B), 10=3 (B), 11=-2 (B)
TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.	10) Refer to girder(s) for truss to truss connections.	
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	11) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.	
<b>REACTIONS</b>	(size) 5= Mechanical, 6=0-3-8 Max Horiz 6=47 (LC 11) Max Uplift 5=-8 (LC 9), 6=-8 (LC 12) Max Grav 5=318 (LC 53), 6=334 (LC 54)	12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 5.	
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension	13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.	
TOP CHORD	1-2=0/83, 2-3=-179/39, 3-4=-64/7, 4-5=-285/18	14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.	
BOT CHORD	5-6=-47/22	15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.	
WEBS	3-5=-3/73, 2-6=-300/31	16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.	
<b>NOTES</b>		17) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 25 lb down and 89 lb up at 1-0-0 on top chord, and 7 lb down and 20 lb up at 1-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.	
1) Unbalanced roof live loads have been considered for this design.		18) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).	
2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60			
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0			
4) Unbalanced snow loads have been considered for this design.			
5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.			



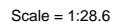
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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Edenton, NC 27932

Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:32 Page: 1  
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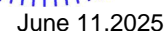
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Plate Offsets (X, Y): [3:0-4-3,Edge]

<b>LUMBER</b>	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* 6-2:2x6 SP No.2
<b>BRACING</b>	
TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
<b>REACTIONS</b>	(size) 5= Mechanical, 6=0-3-8 Max Horiz 6=80 (LC 13) Max Uplift 5=18 (LC 13) Max Grav 5=321 (LC 62), 6=354 (LC 55)
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/89, 2-3=-201/37, 3-4=-46/44, 4-5=-266/17, 2-6=-316/72
BOT CHORD	5-6=-71/103
WEBS	3-5=-185/91

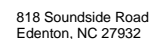
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) Plates checked for a plus or minus 5 degree rotation about its center.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 5.
- 12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



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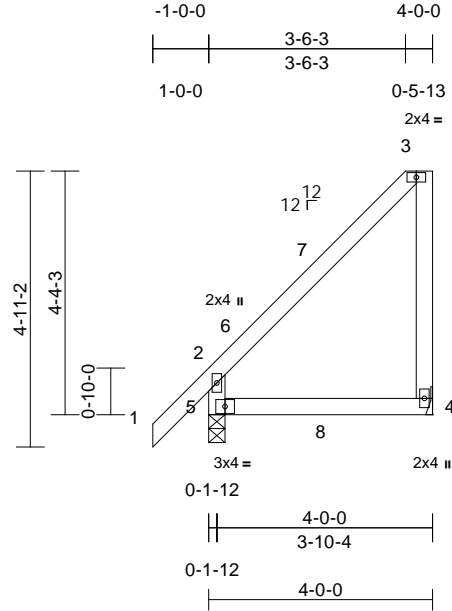


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084190
	M1A	Half Hip	2	1	Job Reference (optional)	

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	-0.04	4-5	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.52	Vert(CT)	-0.05	4-5	>838	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR		Wind(LL)	0.01	4-5	>999	240		
BCDL	10.0										Weight: 23 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

<b>REACTIONS</b>	(size) 4= Mechanical, 5=0-3-8
	Max Horiz 5=119 (LC 13)
	Max Uplift 4=-47 (LC 13)
	Max Grav 4=324 (LC 42), 5=354 (LC 40)

<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/66, 2-3=-197/137, 3-4=-284/215, 2-5=-319/131
BOT CHORD	4-5=-65/67

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11, 2025

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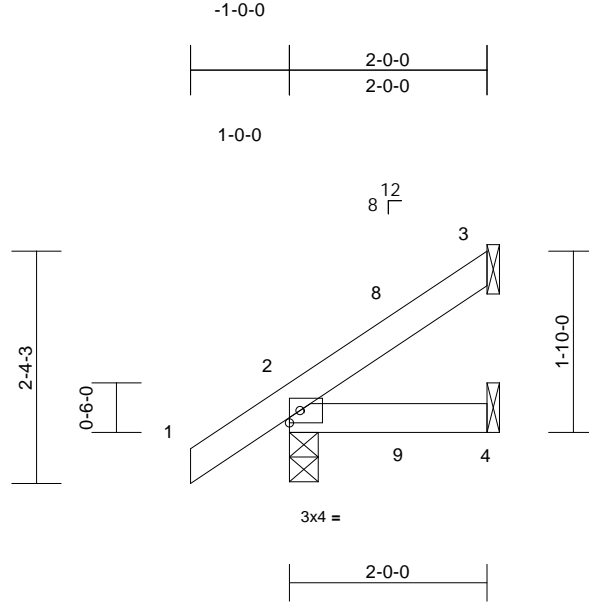
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084191
	M1D	Jack-Open	3	1	Job Reference (optional)	

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<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in	(loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	-0.01	4-7	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.01	4-7	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP		Wind(LL)	0.00	4-7	>999	240		
BCDL	10.0										Weight: 9 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 2=0-3-8, 3= Mechanical, 4= Mechanical  
 Max Horiz 2=40 (LC 16)  
 Max Uplift 3=-15 (LC 16)  
 Max Grav 2=318 (LC 36), 3=270 (LC 40), 4=271 (LC 45)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-90/68

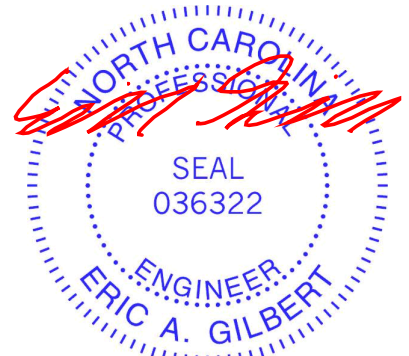
BOT CHORD 2-4=-46/61

#### NOTES

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 5 degree rotation about its center.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 3.
- 10) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11,2025

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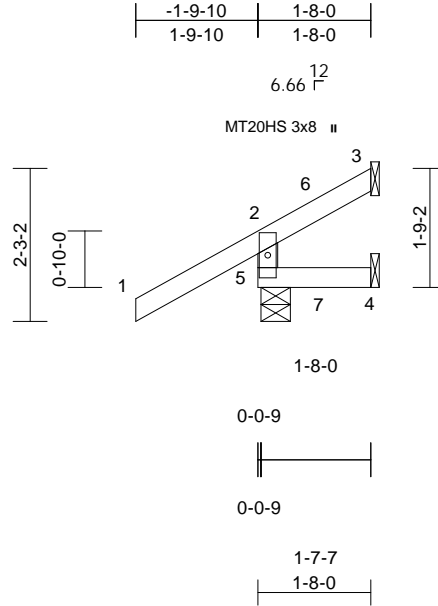
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	M1C	Jack-Open	3	1	Job Reference (optional)
					I74084192

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Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:33

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Scale = 1:34

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	0.00	4-5	>999	360	MT20HS	187/143
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.17	Vert(CT)	0.00	4-5	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR		Wind(LL)	0.00	4-5	>999	240		
BCDL	10.0										Weight: 9 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 1-8-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size)	3= Mechanical, 4= Mechanical,
	5=0-5-3
Max Horiz	5=34 (LC 16)
Max Uplift	3=-53 (LC 22), 4=-19 (LC 22), 5=-3 (LC 16)
Max Grav	3=251 (LC 42), 4=260 (LC 45), 5=347 (LC 40)

#### FORCES

(lb) - Maximum Compression/Maximum Tension	
--	--

TOP CHORD	2-5=-324/172, 1-2=0/77, 2-3=-84/51
BOT CHORD	4-5=0/0

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 4, 53 lb uplift at joint 3 and 3 lb uplift at joint 5.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

**LOAD CASE(S)** Standard



June 11, 2025

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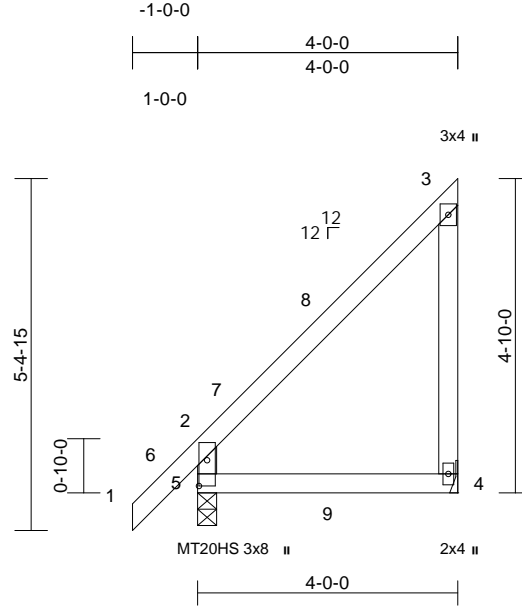
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	I74084193
	M1	Jack-Closed	22	1	Job Reference (optional)	

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Scale = 1:35.4

Plate Offsets (X, Y): [5:0-4-12,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	Vert(LL)	-0.04	4-5	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.49	Vert(CT)	-0.05	4-5	>945	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.01	4-5	>999	240		
BCDL	10.0										Weight: 24 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size) 4= Mechanical, 5=0-3-8  
Max Horiz 5=119 (LC 15)  
Max Uplift 4=-47 (LC 13)  
Max Grav 4=324 (LC 42), 5=354 (LC 40)

**FORCES** (lb) - Maximum Compression/Maximum Tension

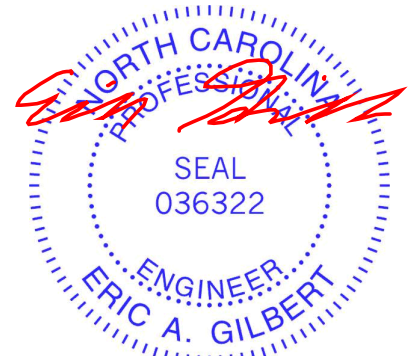
TOP CHORD 2-5=-319/131, 1-2=0/66, 2-3=-214/131, 3-4=-284/250  
BOT CHORD 4-5=-77/72

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcacomponents.com](http://www.sbcacomponents.com))

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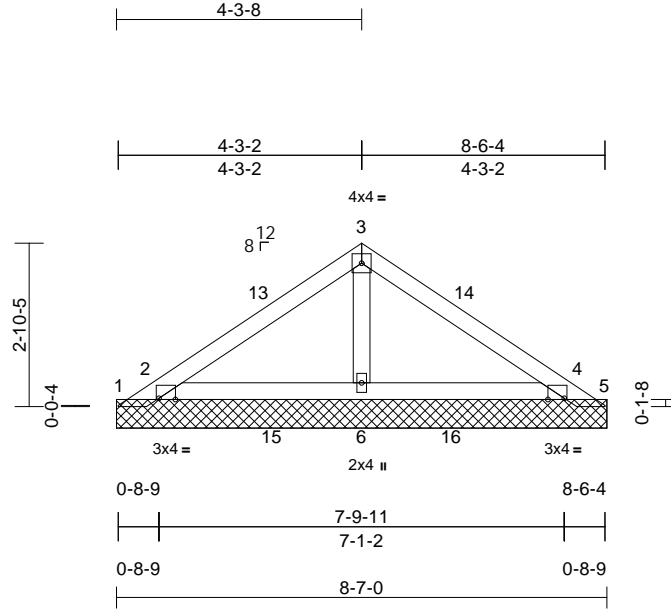
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084200
	PB1	Piggyback	12	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:37  
ID:czVbfbWvpKqNBq7hTQskfrztCA-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:40.3

Plate Offsets (X, Y): [2:0-3-7,Edge], [4:0-3-7,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.54	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS							
BCDL	10.0										
										Weight: 29 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.3  
OTHERS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size) 1=8-7-0, 2=8-7-0, 4=8-7-0, 5=8-7-0, 6=8-7-0  
Max Horiz 1=43 (LC 15)  
Max Uplift 1=-362 (LC 42), 2=-35 (LC 16), 4=-34 (LC 17), 5=-359 (LC 43)  
Max Grav 1=170 (LC 40), 2=638 (LC 42), 4=626 (LC 43), 5=173 (LC 52), 6=365 (LC 59)

**FORCES** (lb) - Maximum Compression/Maximum Tension

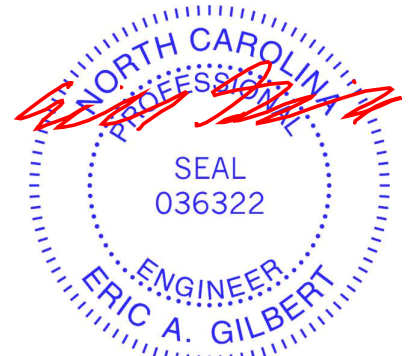
TOP CHORD 1-2=-53/212, 2-3=-154/167, 3-4=-154/166, 4-5=-50/211  
BOT CHORD 2-6=-149/52, 4-6=-149/52  
WEBS 3-6=-237/13

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior (1) 3-3-5 to 4-3-8, Exterior(2R) 4-3-8 to 7-5-3, Interior (1) 7-5-3 to 8-3-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 2, 34 lb uplift at joint 4, 362 lb uplift at joint 1, 359 lb uplift at joint 5, 35 lb uplift at joint 2 and 34 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

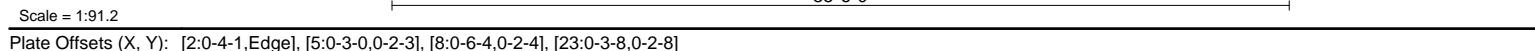
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:24 Page: 1  
ID:Z3hGCMOeQIPr1zCsrIh6szqpvu-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWwRCDoi7J4Jc?#



<b>LUMBER</b>					Unbalanced roof live loads have been considered for this design.	13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
<b>TOP CHORD</b>	2x4 SP No.2				Wind: ASCE 7-16; Vult=120mph (3-second gust)	14) This truss design requires that a minimum of 7/16"
<b>BOT CHORD</b>	2x4 SP SS *Except* 2-26;2x6 SP DSS, 23-22;2x4 SP No.3, 22-14,20-17;2x4 SP No.2				Vasd=95mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 13-6-0, Exterior(2R) 13-6-0 to 18-6-4, Interior (1) 18-6-4 to 22-0-8, Exterior(2R) 22-0-8 to 27-0-12, Interior (1) 27-0-12 to 35-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60	structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
<b>WEBS</b>	2x4 SP No.3 *Except* 6-21,7-16,27-28;2x4 SP No.2				TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0	15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
<b>SLIDER</b>	Right 2x4 SP No.3 -- 1-6-0				4) Unbalanced snow loads have been considered for this design.	16) Attic room checked for L/360 deflection.
<b>BRACING</b>					5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.	<b>LOAD CASE(S)</b> Standard
<b>TOP CHORD</b>	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-10-5 max.): 5-8.				6) Provide adequate drainage to prevent water ponding.	
<b>BOT CHORD</b>	Rigid ceiling directly applied.				7) All plates are MT20 plates unless otherwise indicated.	
<b>WEBS</b>	1 Row at midpt      3-23, 21-27, 8-16				8) Plates checked for a plus or minus 5 degree rotation about its center.	
<b>JOINTS</b>	1 Brace at Jt(s): 27, 28				9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	
<b>REACTIONS</b>	(size)                  2=0-3-8, 12=Mechanical Max Horiz    2=151 (LC 13) Max Grav     2=1908 (LC 50), 12=1824 (LC 52)				10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.	
<b>FORCES</b>	(Ib) - Maximum Compression/Maximum Tension				11) Refer to girder(s) for truss to truss connections.	
<b>TOP CHORD</b>	1-2=0/45, 2-3=-3705/0, 3-5=-2451/0, 5-6=-1925/41, 6-7=-1712/45, 7-8=-1780/42, 8-10=-2158/32, 10-12=-2716/0				12) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.	
<b>BOT CHORD</b>	2-25=0/3005, 25-26=0/334, 24-25=0/3017, 23-24=0/3072, 22-23=-122/184, 21-22=-165/44, 19-21=0/1761, 16-19=0/1761, 15-16=0/1688, 13-15=0/2182, 12-13=0/2182, 18-20=0/25, 17-18=0/25					
<b>WEBS</b>	3-23=-1286/37, 21-23=0/2177, 6-23=-39/1070, 20-21=-1005/0, 20-27=-934/0, 6-27=-922/5, 16-17=-335/83, 17-28=-308/100, 7-28=-308/100, 8-16=-77/550, 8-15=-2/641, 27-28=-40/32, 18-19=-79/46, 7-27=-127/35, 5-23=0/1057, 10-13=0/389, 10-15=-768/63, 24-26=0/342, 3-26=0/759					

NOTES

June 11, 2025

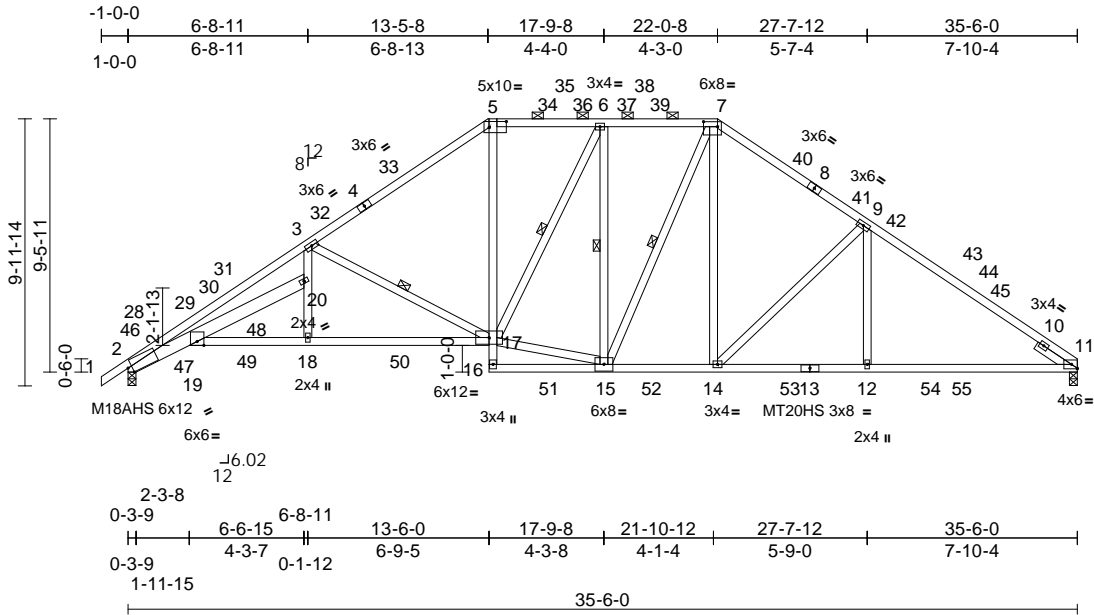


Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084202
	B10	Piggyback Base	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:26  
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Page: 1



Scale = 1:86.2									
Plate Offsets (X, Y): [2:0-1-13,0-2-13], [5:0-7-12,0-2-11], [7:0-6-4,0-2-4]									
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.26 19-20	>999	360
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.44 19-20	>971	240
TCDL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(CT)	0.27 11	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.09 19-20	>999	240
BCDL	10.0								
									<b>PLATES</b>
									<b>GRIP</b>
									MT20 244/190
									M18AHS 186/179
									MT20HS 187/143
									Weight: 241 lb FT = 20%

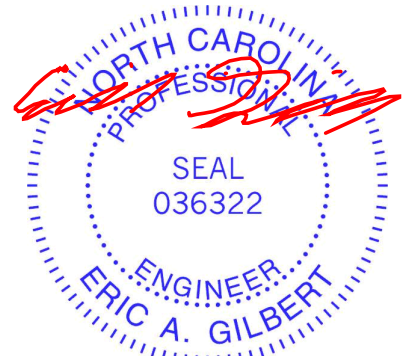
**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS \*Except\* 2-20:2x6 SP DSS, 5-16:2x4 SP No.3, 16-13:2x4 SP No.2  
WEBS 2x4 SP No.3  
SLIDER Right 2x4 SP No.3 -- 1-6-0  
**BRACING**  
TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (4-3-14 max.): 5-7. Rigid ceiling directly applied.  
BOT CHORD  
WEBS 1 Row at midpt 3-17, 6-15, 7-15, 6-17  
**REACTIONS** (size) 2=0-3-8, 11=0-3-8  
Max Horiz 2=151 (LC 13)  
Max Grav 2=1776 (LC 49), 11=1708 (LC 51)  
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/45, 2-3=-3410/0, 3-5=-2201/48, 5-6=-1694/81, 6-7=-1571/85, 7-9=-1950/75, 9-11=-2492/5  
BOT CHORD 2-19=0/2771, 19-20=0/316, 18-19=0/2778, 17-18=0/2831, 16-17=0/173, 5-17=0/831, 15-16=-124/126, 14-15=0/1518, 12-14=0/1992, 11-12=0/1992  
WEBS 3-17=-1242/42, 7-14=0/689, 18-20=0/333, 3-20=0/721, 9-12=0/382, 9-14=-744/65, 6-15=-483/74, 7-15=-65/307, 15-17=0/1561, 6-17=-32/343

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 13-7-12, Exterior(2R) 13-7-12 to 18-8-0, Interior (1) 18-8-0 to 22-0-8, Exterior(2R) 22-0-8 to 27-0-12, Interior (1) 27-0-12 to 35-4-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

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Edenton, NC 27932

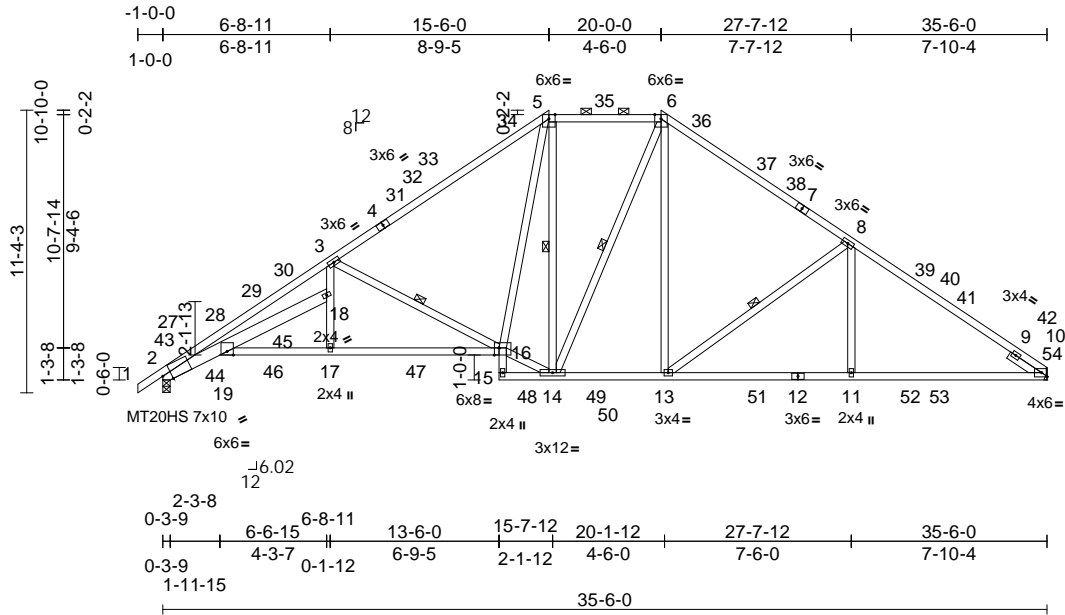
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084203
	B1P	Hip	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:26

Page: 1

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Scale = 1:92.5									
Plate Offsets (X, Y): [2:0-4-1,Edge], [16:0-2-4,Edge]									
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.25 18-19	>999	360
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.77	Vert(CT)	-0.42 18-19	>999	240
TCDL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(CT)	0.28 10	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.09 18-19	>999	240
BCDL	10.0								
									<b>PLATES</b>
									<b>GRIP</b>
									MT20 244/190
									MT20HS 187/143
									Weight: 236 lb FT = 20%

**LUMBER**  
TOP CHORD 2x4 SP SS \*Except\* 5-6:2x4 SP No.2  
BOT CHORD 2x4 SP SS \*Except\* 2-18:2x6 SP DSS, 16-15:2x4 SP No.3  
WEBS 2x4 SP No.3  
SLIDER Right 2x4 SP No.3 -- 1-6-0

**BRACING**  
TOP CHORD Structural wood sheathing directly applied, except  
2-0-0 oc purlins (3-11-5 max.): 5-6.  
Rigid ceiling directly applied.  
BOT CHORD  
WEBS 1 Row at midpt 3-16, 5-14, 6-14, 8-13

**REACTIONS** (size) 2=0-3-8, 10= Mechanical  
Max Horiz 2=171 (LC 13)  
Max Grav 2=1844 (LC 49), 10=1787 (LC 51)

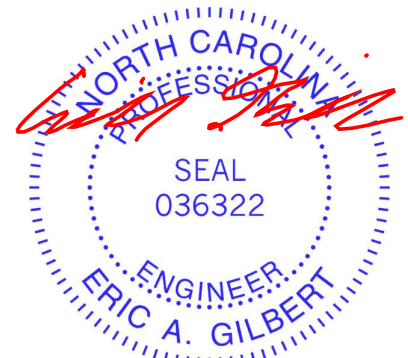
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/45, 2-3=-3636/0, 3-5=-2367/55, 5-6=-1522/113, 6-8=-2002/86, 8-10=-2679/2  
BOT CHORD 2-19=0/2974, 18-19=0/318, 17-19=0/3004, 16-17=0/3060, 15-16=-71/138, 14-15=-182/31, 13-14=0/1544, 11-13=0/2156, 10-11=0/2156  
WEBS 3-16=-1338/71, 14-16=0/1770, 5-16=0/1542, 5-14=-705/54, 6-14=-164/174, 6-13=0/749, 8-13=-860/75, 8-11=0/406, 17-18=0/338, 3-18=0/714

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 15-6-0, Exterior(2E) 15-6-0 to 20-0-0, Exterior(2R) 20-0-0 to 25-0-4, Interior (1) 25-0-4 to 35-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) Plates checked for a plus or minus 5 degree rotation about its center.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Refer to girder(s) for truss to truss connections.
- 12) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

- 14) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcacomponents.com](http://www.sbcacomponents.com))

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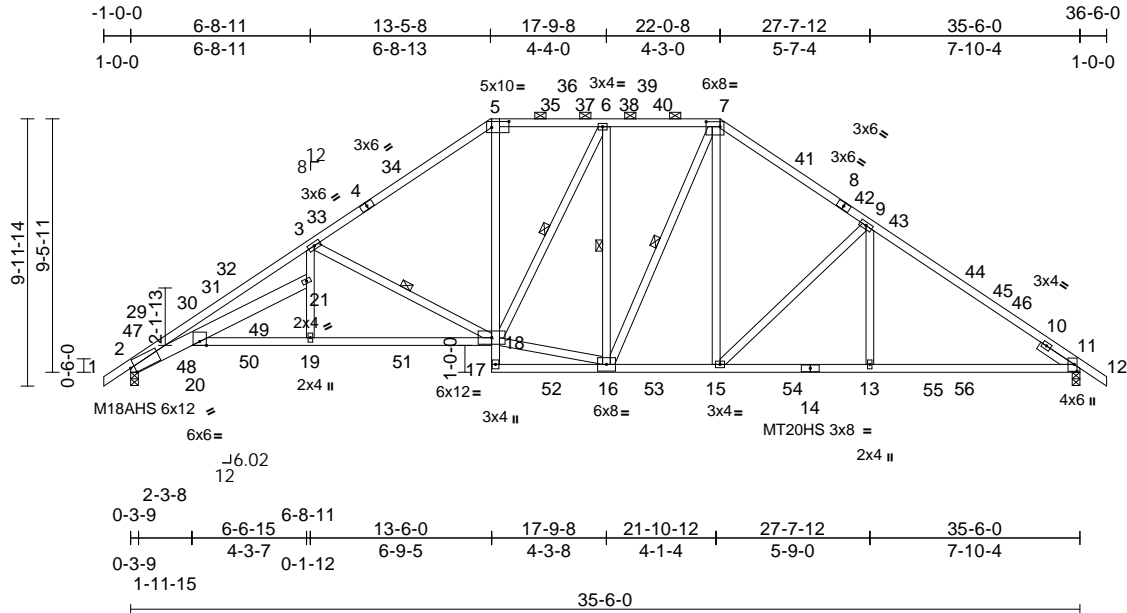
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084204
	B1N	Piggyback Base	2	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:25

Page: 1

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Scale = 1:86.2									
Plate Offsets (X, Y): [2:0-1-13,0-2-13], [5:0-7-12,0-2-11], [7:0-6-4,0-2-4], [11:0-3-2,0-1-4]									
<b>Loading</b>	(psf)	<b>Spacing</b>	2-0-0	<b>CSI</b>		<b>DEFL</b>	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.26 20-21	>999	360
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.44 20-21	>972	240
TCDL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.27 11	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.09 20-21	>999	240
BCDL	10.0								
									<b>PLATES</b>
									<b>GRIP</b>
									MT20
									MT18AHS
									MT20HS
									Weight: 242 lb FT = 20%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP SS \*Except\* 2-21:2x6 SP DSS, 5-17:2x4 SP No.3, 17-14:2x4 SP No.2  
WEBS 2x4 SP No.3  
SLIDER Right 2x4 SP No.3 -- 1-6-0  
**BRACING**  
TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (4-3-14 max.): 5-7. Rigid ceiling directly applied.  
BOT CHORD Rigid ceiling directly applied.  
WEBS 1 Row at midpt 3-18, 6-16, 7-16, 6-18  
**REACTIONS** (size) 2=0-3-8, 11=0-3-8  
Max Horiz 2=156 (LC 14)  
Max Grav 2=1776 (LC 49), 11=1763 (LC 51)  
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/45, 2-3=-3411/0, 3-5=-2199/46, 5-6=-1692/75, 6-7=-1569/83, 7-9=-1947/70, 9-11=-2486/0, 11-12=0/52  
BOT CHORD 2-20=0/2778, 20-21=0/317, 19-20=0/2785, 18-19=0/2838, 17-18=0/173, 5-18=0/830, 16-17=-124/126, 15-16=0/1516, 13-15=0/1986, 11-13=0/1986  
WEBS 3-18=-1243/41, 7-15=0/686, 9-13=0/381, 9-15=-738/64, 6-16=-484/72, 7-16=-64/307, 19-21=0/333, 3-21=0/722, 16-18=0/1560, 6-18=-31/345

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 13-7-12, Exterior(2R) 13-7-12 to 18-8-0, Interior (1) 18-8-0 to 22-0-8, Exterior(2R) 22-0-8 to 27-0-12, Interior (1) 27-0-12 to 36-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

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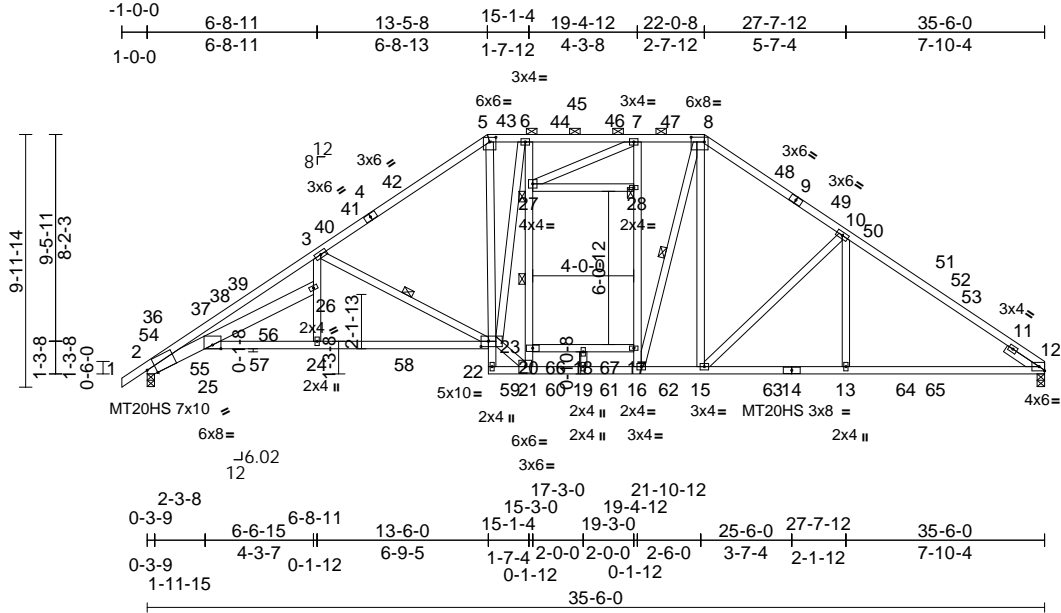
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084205
	B1M	Attic	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.20 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:25

Page: 1

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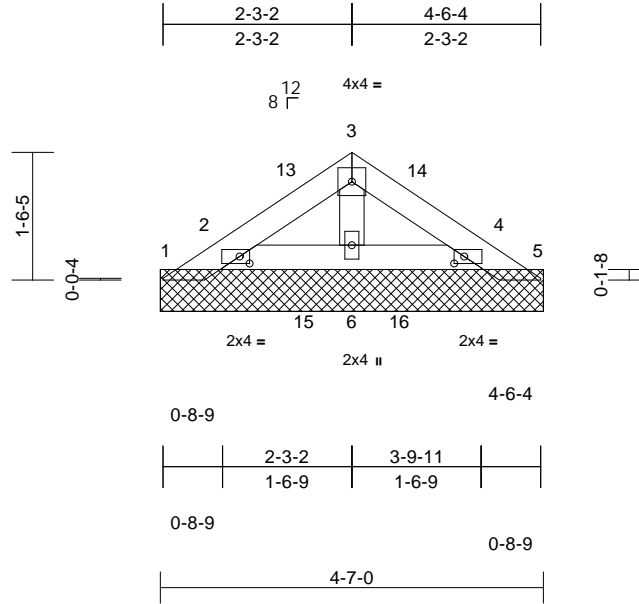
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	PB1B	Piggyback	1	1	Job Reference (optional)
					I74084206

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Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:37

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Scale = 1:27.6

Plate Offsets (X, Y): [2:0-1-7,Edge], [4:0-1-7,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.19	Vert(TL)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horiz(TL)	0.00	10	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS							
BCDL	10.0										
										Weight: 14 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.3  
WEBS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied.  
BOT CHORD Rigid ceiling directly applied.

**REACTIONS** (size) 1=4-7-0, 2=4-7-0, 4=4-7-0, 5=4-7-0, 6=4-7-0  
Max Horiz 1=-21 (LC 14)  
Max Uplift 1=-81 (LC 42), 2=-1 (LC 16), 4=-3 (LC 17), 5=-79 (LC 43)  
Max Grav 1=249 (LC 40), 2=343 (LC 48), 4=333 (LC 44), 5=251 (LC 52), 6=309 (LC 59)

**FORCES** (lb) - Maximum Compression/Maximum Tension

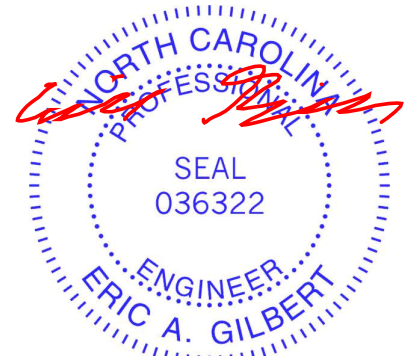
TOP CHORD 1-2=-25/56, 2-3=-118/33, 3-4=-119/33, 4-5=-9/55  
BOT CHORD 2-6=-18/47, 4-6=-18/47  
WEBS 3-6=-210/7

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Plates checked for a plus or minus 5 degree rotation about its center.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 2, 3 lb uplift at joint 4, 81 lb uplift at joint 1, 79 lb uplift at joint 5, 1 lb uplift at joint 2 and 3 lb uplift at joint 4.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

**LOAD CASE(S)** Standard



June 11, 2025

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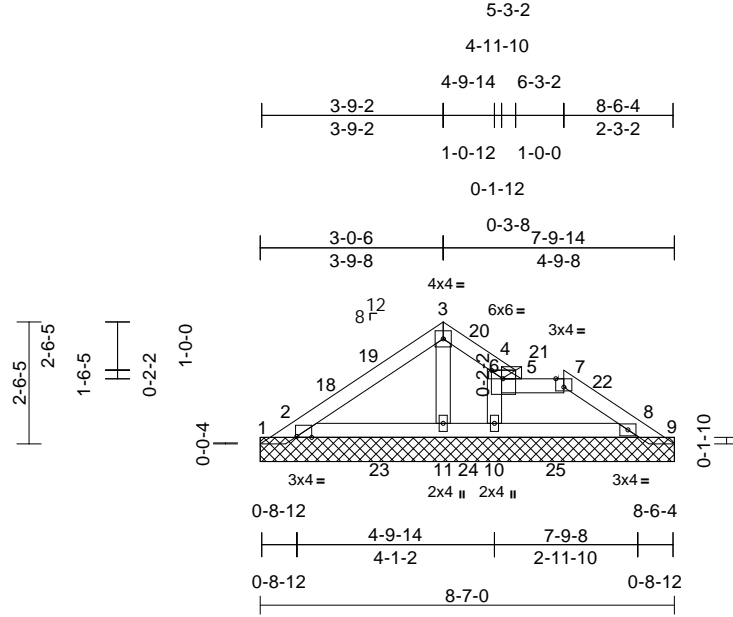
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084207
	PB1A	Piggyback	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1

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Scale = 1:47.7

Plate Offsets (X, Y): [2:0-3-11,Edge], [5:0-2-14,0-2-2], [7:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.43	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.40	Horiz(TL)	0.00	15	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS							
BCDL	10.0										
Weight: 30 lb FT = 20%											

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.3
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

#### BRACING

TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD	Rigid ceiling directly applied.

REACTIONS	(size)	1=8-7-0, 2=8-7-0, 8=8-7-0, 9=8-7-0, 10=8-7-0, 11=8-7-0
	Max Horiz	1=39 (LC 13)
	Max Uplift	1=-272 (LC 66), 2=-17 (LC 16), 9=-171 (LC 70), 10=-13 (LC 17), 11=-176 (LC 79)
	Max Grav	1=195 (LC 64), 2=529 (LC 66), 8=478 (LC 70), 9=222 (LC 82), 10=339 (LC 91), 11=300 (LC 90)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-45/163, 2-3=-207/120, 3-4=-214/74, 4-5=-20/249, 5-6=-357/52, 5-7=-277/41, 7-8=-353/41, 8-9=-6/107
BOT CHORD	2-11=-101/159, 10-11=0/159, 8-10=-65/274
WEBS	3-11=-224/209, 6-10=-297/89, 4-6=-310/89

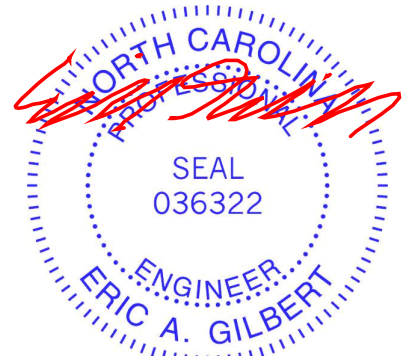
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior (1) 3-3-5 to 3-9-8, Exterior(2E) 3-9-8 to 5-3-8, Interior (1) 5-3-8 to 6-3-8, Exterior(2E) 6-3-8 to 8-3-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- 5) Unbalanced snow loads have been considered for this design.
- 6) Provide adequate drainage to prevent water ponding.
- 7) Plates checked for a plus or minus 5 degree rotation about its center.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 4-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 2, 272 lb uplift at joint 1, 171 lb uplift at joint 9, 176 lb uplift at joint 11, 13 lb uplift at joint 10 and 17 lb uplift at joint 2.

- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 14) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 15) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



June 11,2025

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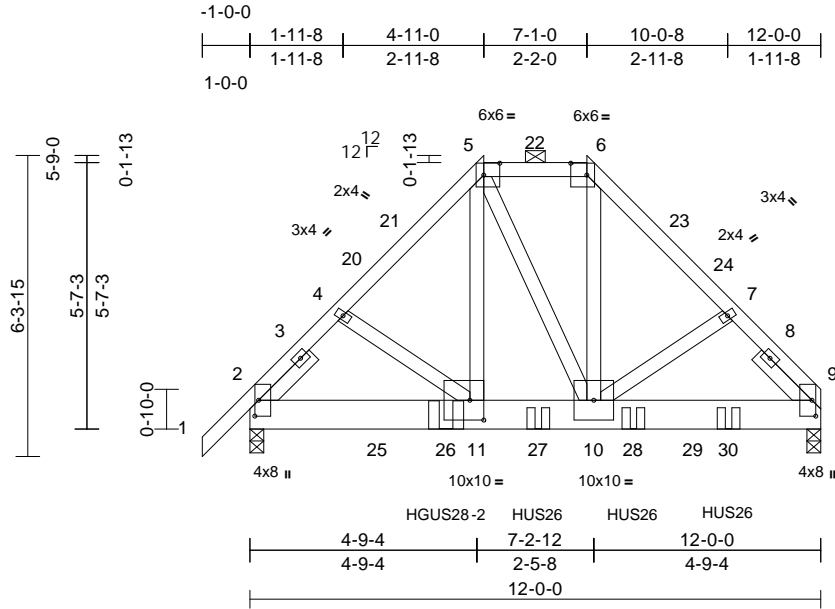
818 Soundside Road  
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Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084210
	C2GR	Hip Girder	1	3	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	-0.03	11-18	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.21	Vert(CT)	-0.04	11-18	>999	240		
TCDL	10.0	Rep Stress Incr	NO	WB	0.41	Horz(CT)	0.01	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.01	11-18	>999	240		
BCDL	10.0											
											Weight: 299 lb	FT = 20%

#### LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x8 SP DSS
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

#### BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size)	2=0-3-8, 9=0-3-8
Max Horiz	2=87 (LC 87)
Max Grav	2=4359 (LC 37), 9=4920 (LC 47)

#### FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/83, 2-4=-4631/0, 4-5=-4586/0, 5-6=-3222/0, 6-7=-4651/0, 7-9=-4685/0
BOT CHORD	2-11=0/3053, 10-11=0/3169, 9-10=0/3144
WEBS	5-11=-362/2884, 5-10=-536/574, 6-10=0/3031, 4-11=-117/400, 7-10=-105/330

#### NOTES

- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.  
Bottom chords connected as follows: 2x8 - 4 rows staggered at 0-5-0 oc.  
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Use Simpson Strong-Tie HGUS28-2 (36-10d Girder, 6-10d Truss) or equivalent at 4-1-8 from the left end to connect truss(es) to back face of bottom chord.
- Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 6-0-12 from the left end to 10-0-12 to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

#### LOAD CASE(S)

- Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (lb/ft)  
Vert: 1-5=-51, 5-6=-61, 6-9=-51, 12-16=-20  
Concentrated Loads (lb)  
Vert: 26=-3232 (B), 27=-1449 (B), 28=-1545 (B), 30=-1610 (B)



June 11,2025

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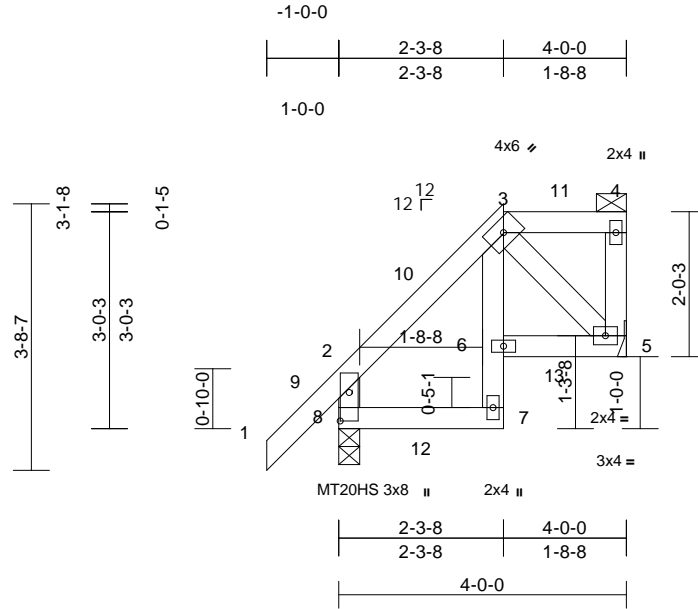
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	M1N	Half Hip	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:35

Page: 1

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Scale = 1:32

Plate Offsets (X, Y): [8:0-4-12,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.01	7	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.01	7	>999	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.00	5-6	>999	240		
BCDL	10.0										Weight: 25 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2 \*Except\* 7-3:2x4 SP No.3  
WEBS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (size) 5= Mechanical, 8=0-3-8  
Max Horiz 8=69 (LC 13)  
Max Uplift 5=-16 (LC 13)  
Max Grav 5=324 (LC 58), 8=354 (LC 55)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/83, 2-3=-236/39, 3-4=-30/30, 4-5=-267/28, 2-8=-332/136

BOT CHORD 7-8=-97/112, 6-7=-27/266, 3-6=0/255, 5-6=-116/146

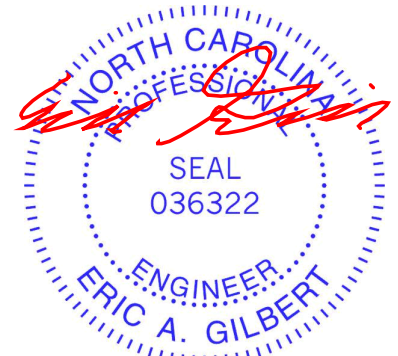
WEBS 3-5=-207/141

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 5.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**LOAD CASE(S)** Standard



June 11, 2025

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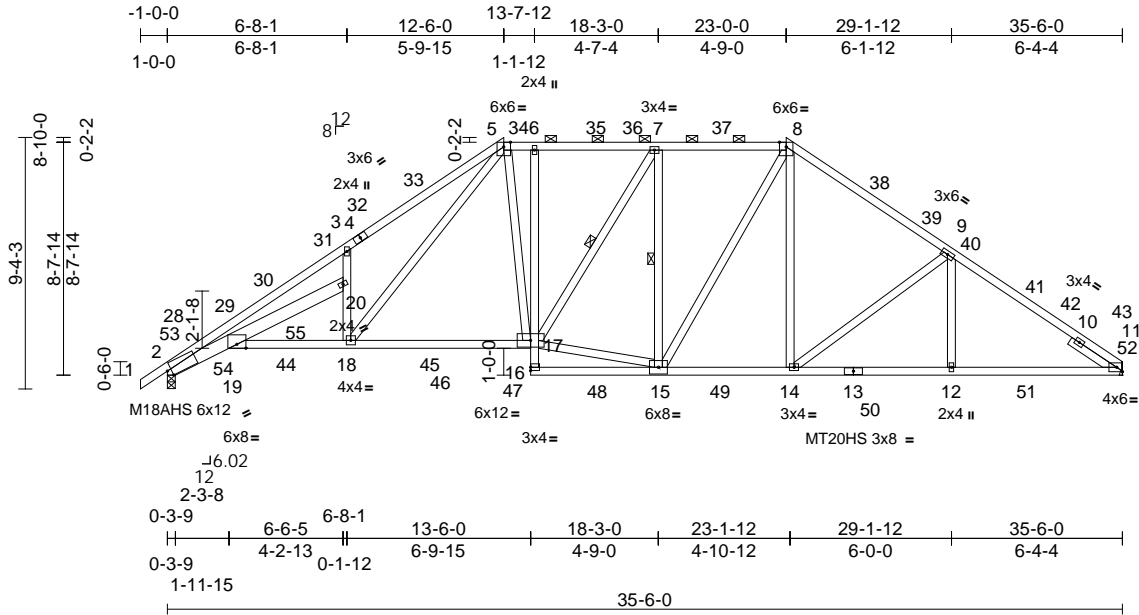
818 Soundside Road  
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084213
	B1Q	Hip	1	1	Job Reference (optional)	

Structural, LLC, Thurmont, MD - 21788,

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Page: 1



Scale = 1:85.6

Plate Offsets (X, Y): [2:0-1-12,0-2-13]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.28	19-20	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.97	Vert(CT)	-0.47	19-20	>908	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(CT)	0.28	11	n/a	n/a	M18AHS	186/179
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.10	19-20	>999	240		
BCDL	10.0											
											Weight: 247 lb	FT = 20%

#### LUMBER

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2 \*Except\* 19-17:2x4 SP SS,  
6-16:2x4 SP No.3, 20-2:2x6 SP DSS  
WEBS 2x4 SP No.3  
SLIDER Right 2x4 SP No.3 -- 2-0-0

#### BRACING

TOP CHORD Structural wood sheathing directly applied,  
except  
2-0-0 oc purlins (4-0-4 max.): 5-8.  
BOT CHORD Rigid ceiling directly applied.  
WEBS 1 Row at midpt 7-15, 7-17

#### REACTIONS

(size) 2=0-3-8, 11= Mechanical  
Max Horiz 2=139 (LC 13)  
Max Grav 2=1771 (LC 49), 11=1671 (LC 51)

#### FORCES

(lb) - Maximum Compression/Maximum  
Tension  
TOP CHORD 1-2=0/45, 2-3=-3380/0, 3-5=-3376/77,  
5-6=-1849/55, 6-7=-1845/58, 7-8=-1720/69,  
8-9=-2026/55, 9-11=-2518/0  
BOT CHORD 18-19=0/2709, 17-18=0/1799, 16-17=0/178,  
6-17=-333/54, 15-16=-120/143,  
14-15=0/1575, 12-14=0/2038, 11-12=0/2038,  
2-19=0/2731, 19-20=0/334  
WEBS 5-17=-26/730, 8-14=0/601, 9-14=-637/64,  
9-12=0/366, 18-20=-917/119, 3-20=-533/127,  
5-18=-48/1568, 7-15=-599/75, 7-17=-25/352,  
15-17=0/1742, 8-15=-57/434

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust)  
Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.  
II; Exp B; Enclosed; MWFRS (envelope) and C-C  
Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to  
12-6-0, Exterior(2R) 12-6-0 to 17-6-4, Interior (1) 17-6-4  
to 23-0-0, Exterior(2R) 23-0-0 to 28-0-4, Interior (1)  
28-0-4 to 35-5-4 zone; cantilever left and right exposed ;  
end vertical left and right exposed;C-C for members and  
forces & MWFRS for reactions shown; Lumber  
DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15  
Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL =  
1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially  
Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) Plates checked for a plus or minus 5 degree rotation about its center.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Refer to girder(s) for truss to truss connections.
- 12) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

- 14) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



June 11, 2025

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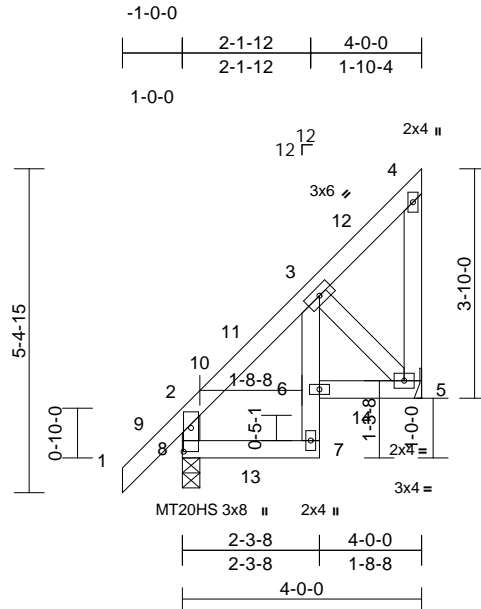
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	M1L	Jack-Closed	3	1	174084214
Job Reference (optional)					

Structural, LLC, Thurmont, MD - 21788,

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:34

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Scale = 1:38.5

Plate Offsets (X, Y): [8:0-4-12,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	-0.01	7	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.39	Vert(CT)	-0.01	7	>999	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	-0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.00	5-6	>999	240		
BCDL	10.0											
Weight: 29 lb											FT = 20%	

#### LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 \*Except\* 7-3:2x4 SP No.3

WEBS 2x4 SP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 5= Mechanical, 8=0-3-8

Max Horiz 8=109 (LC 13)

Max Uplift 5=-46 (LC 13)

Max Grav 5=324 (LC 43), 8=354 (LC 40)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-8=-332/116, 1-2=0/66, 2-3=-242/47,

3-4=-122/81, 4-5=-266/98

BOT CHORD 7-8=-164/119, 6-7=-47/268, 3-6=-1/264,

5-6=-208/157

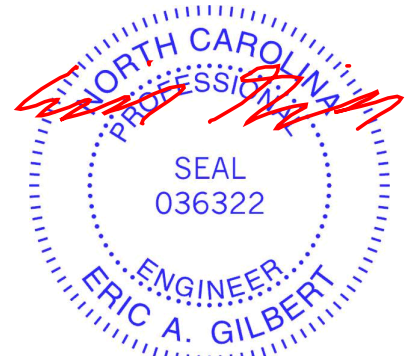
WEBS 3-5=-213/235

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-1-12, Interior (1) 2-1-12 to 3-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) Plates checked for a plus or minus 5 degree rotation about its center.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 5.
- 12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



June 11,2025

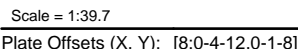
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

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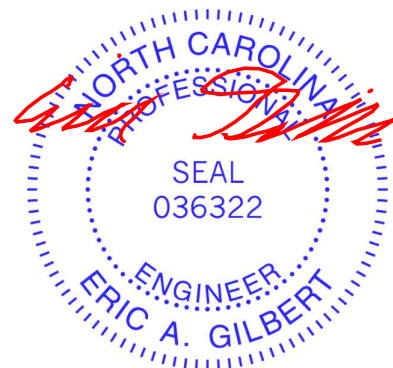


<b>LUMBER</b>	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2 *Except* 7-3:2x4 SP No.3
WEBS	2x4 SP No.3
<b>BRACING</b>	
TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
<b>REACTIONS</b> (size) 5= Mechanical, 8=0-3-8	
	Max Horiz 8=109 (LC 13)
	Max Uplift 5=-46 (LC 13)
	Max Grav 5=324 (LC 43), 8=354 (LC 40)
<b>FORCES</b> (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/66, 2-3=-242/47, 3-4=-122/81, 4-5=-266/84, 2-8=-332/116
BOT CHORD	7-8=-136/119, 6-7=-37/268, 3-6=0/264, 5-6=-172/157
WEBS	3-5=-213/187

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) Plates checked for a plus or minus 5 degree rotation about its center.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 5.
- 12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord. nonconcurrent with any other live loads.

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust)  
Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-1-12, Interior (1) 2-1-12 to 3-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.0; Ct=1.0
- 4) Unbalanced snow loads have been considered for this design.



June 11.2025

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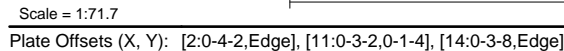
Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Components Association ([www.sbcacomponents.com](http://www.sbcacomponents.com))



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Structural, LLC, Thurmont, MD - 21788, Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 09 19:03:27 Page: 1  
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<b>LUMBER</b>		Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-6-10, Interior (1) 2-6-10 to 9-6-0, Exterior(2R) 9-6-0 to 14-6-4, Interior (1) 14-6-4 to 26-0-0, Exterior(2R) 26-0-0 to 30-9-4, Interior (1) 30-9-4 to 35-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
TOP CHORD	2x4 SP No.2	structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
BOT CHORD	2x4 SP SS *Except* 5-15:2x4 SP No.3, 15-13:2x4 SP No.2, 18-2:2x6 SP DSS	15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
WEBS	2x4 SP No.3	
SLIDER	Right 2x4 SP No.3 -- 1-6-0	
<b>BRACING</b>		<b>LOAD CASE(S)</b> Standard
TOP CHORD	Structural wood sheathing directly applied, except	
	2-0-0 oc purlins (2-2-0 max.): 4-8.	
BOT CHORD	Rigid ceiling directly applied.	
WEBS	1 Row at midpt 7-12	
<b>REACTIONS</b>	(size) 2=0-3-8, 11= Mechanical	
	Max Horiz 2=107 (LC 13)	
	Max Grav 2=1695 (LC 47), 11=1581 (LC 47)	
<b>FORCES</b>	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/45, 2-3=-3109/0, 3-4=-3007/53, 4-5=-2752/1, 5-7=-2737/2, 7-8=-1844/35, 8-9=-2233/15, 9-11=-2304/3	
BOT CHORD	17-19=0/2470, 16-17=0/2162, 15-16=0/194, 5-16=-531/70, 14-15=-42/248, 12-14=0/2464, 11-12=0/1890, 2-19=0/2437, 18-19=-2/309	
WEBS	4-16=-8/972, 14-16=0/2323, 7-16=-15/382, 7-14=-249/162, 7-12=-882/37, 8-12=0/924, 9-12=-364/87, 17-18=-767/122, 3-18=-406/123, 4-17=-48/1191	
<b>NOTES</b>		
1)	Unbalanced roof live loads have been considered for this design.	

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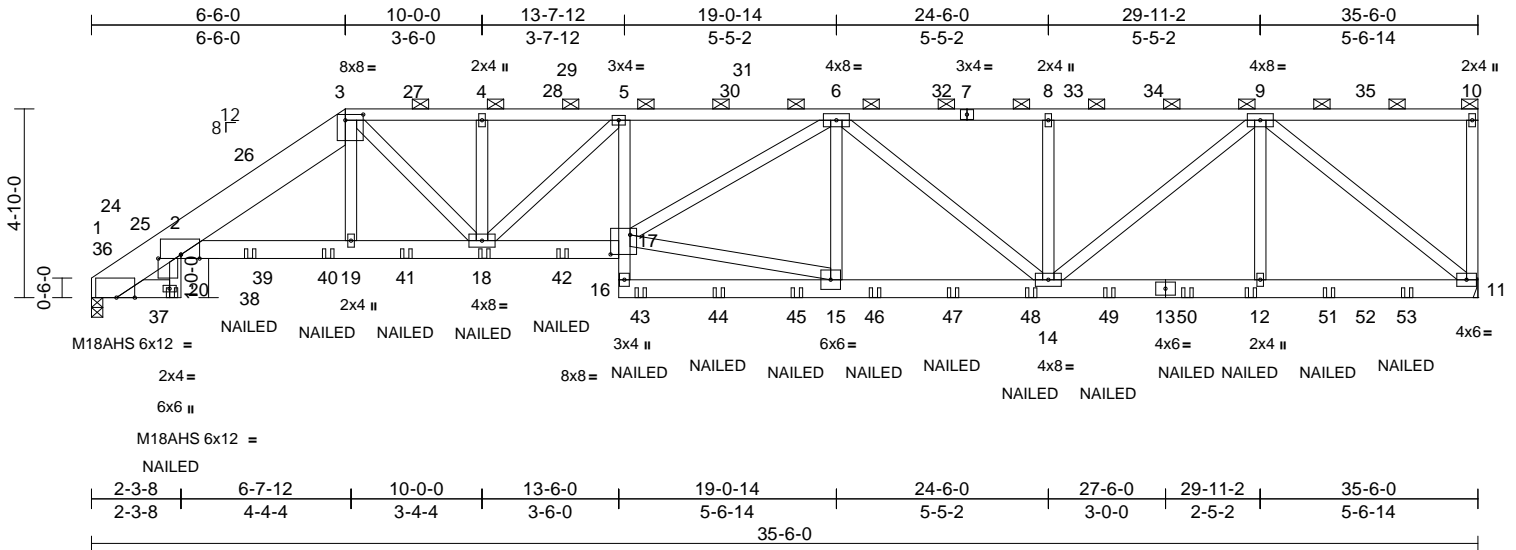
Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof	174084217
	B3GR	Half Hip Girder	1	2	Job Reference (optional)	

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Scale = 1:59

Plate Offsets (X, Y): [1:0-5-10,Edge], [2:0-5-11,Edge], [2:0-1-3,0-7-0], [3:0-5-8,0-1-12], [17:0-6-0,0-6-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.28	16	>999	360	M18AHS 186/179
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.44	16	>947	240	MT20 244/190
TCDL	10.0	Rep Stress Incr	NO	WB	0.93	Horz(CT)	0.20	11	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS		Wind(LL)	0.16	16	>999	240	
BCDL	10.0										
Weight: 512 lb FT = 20%											

#### LUMBER

TOP CHORD 2x4 SP No.2 \*Except\* 1-3:2x10 SP DSS

BOT CHORD 2x6 SP No.2 \*Except\* 20-2:2x4 SP No.3,

5-16:2x4 SP No.2

WEBS 2x4 SP No.3 \*Except\* 15-17:2x4 SP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (3-10-9 max.): 3-10.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(size) 1=0-3-8, 11= Mechanical  
Max Horiz 1=109 (LC 11)  
Max Uplift 1=356 (LC 9), 11=460 (LC 9)  
Max Grav 1=2985 (LC 33), 11=3215 (LC 33)

#### FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1930/248, 2-3=-5134/723,  
3-4=-6300/914, 4-5=-6300/914,  
5-6=-7927/1157, 6-8=-5338/787,  
8-9=-5338/787, 9-10=-59/34, 10-11=-301/25

BOT CHORD 1-20=-98/349, 2-20=-103/730,  
2-19=-700/4533, 18-19=-703/4549,  
17-18=-1211/7997, 16-17=-66/378,  
6-17=-201/1201, 15-16=-95/614,  
14-15=-950/6189, 12-14=-529/3437,  
1-11=-979/3437

WEBS 3-4=-63/485, 3-18=-392/2460,  
4-18=-303/48, 5-18=-2310/355,  
15-17=-877/6690, 6-17=-290/2054,  
6-15=-693/103, 6-14=-1062/162,  
8-14=-353/54, 9-14=-383/2460,  
9-12=-118/716, 9-11=-4403/650

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
Top chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.  
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.  
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.

- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint 11.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.

LOAD CASE(S) Standard

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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Stonefield Rev 3-Elev 4-Roof
	B3GR	Half Hip Girder	1	2	I74084217
Job Reference (optional)					

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- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (lb/ft)  
Vert: 1-2=-51, 2-3=-51, 3-10=-61, 20-21=-20, 2-17=-20, 11-16=-20  
Concentrated Loads (lb)  
Vert: 20=-217 (F), 18=-175 (F), 12=-175 (F), 38=-157 (F), 40=-175 (F), 41=-175 (F), 42=-175 (F), 43=-175 (F), 44=-175 (F), 45=-175 (F), 46=-175 (F), 47=-175 (F), 48=-175 (F), 49=-175 (F), 50=-175 (F), 51=-175 (F), 53=-175 (F)

*Eric Gilbert*



June 11, 2025

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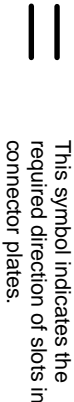
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# Symbols

## PLATE LOCATION AND ORIENTATION



\* Plate location details available in MITek software or upon request.

## PLATE SIZE

**4 X 4**

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING

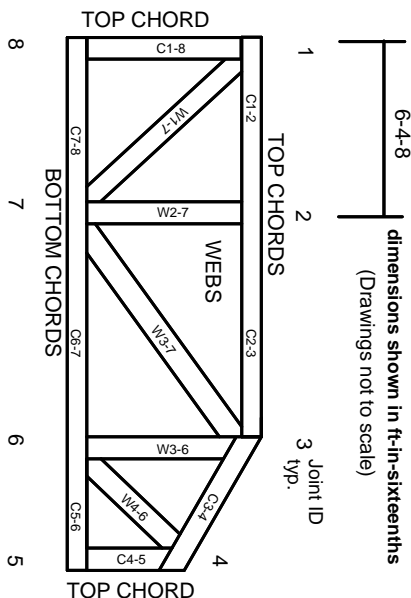


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

## Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.  
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System



**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

# Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282  
ESR-4722, ESL-1388

# Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

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# General Safety Notes

**Failure to Follow Could Cause Property Damage or Personal Injury**

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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