# STONEFIELD-RALE

RALEIGH - LOT 00.0020 CAMPBELL RIDGE SF (MODEL# 1635)

ELEVATION 4 - GL

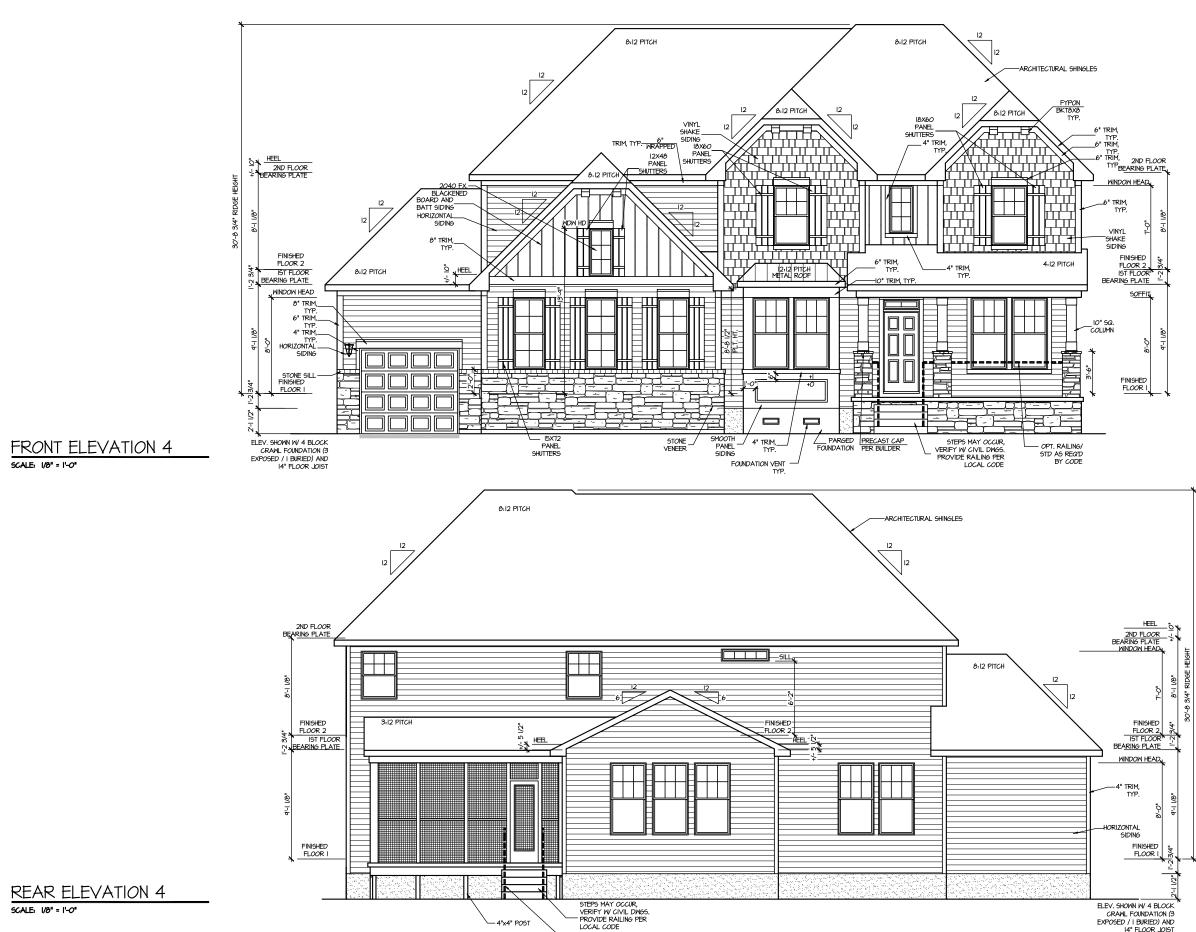
INDEX



AREA CALCULATIONS	115 1 750	COVERED /	
ELEVATION 4	HEATED	UNHEATED	UNCOVERED
FIRST FLOOR	1552 SF		
GARAGE		496 SF	
FRONT PORCH - ELEVATION 4		142 SF	
SECOND FLOOR	1624 SF		
5255115 125511	1021 01		
OPTIONS			
EXT. BRKFST W/ EXT. OWNER'S SUITE/ SCREEN	+186 SF	+160 SF	
ZDD CAD CADAGE W/ CIDEL CAD CADAGE	+100 SF		
3RD CAR GARAGE W/ SIDELOAD GARAGE	+	+265 SF	
BED 5 W/ BATH 3	+55 SF	-55 SF	
TOTAL	3417 SF	1008 SF	
		1	
	1		
	1	1	
	+	+	

### 161 Alden Way

107	CDECIE	
LUI	<b>SPECIFIC</b>	
1		CAMPBELL RIDGE SF
- '	LOT 00.0020	CAMPBELL RIDGE OF
		STONEFIELD REV. RALE—3 ELEVATION 4
2	ADDRESS	161 ALDEN WAY ANGIER, NC 27501
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ELEV. SHOWN W 4 BLOCK CRAWL FOUNDATION (3 EXPOSED / I BURIED) AND I4" FLOOR JOIST

SHEET No.

HOUSE NAME:
STONEFIELE
DRAWING TITLE

DRAWN BY:

DATE: 05/28/2025 PLAN NO. 1635

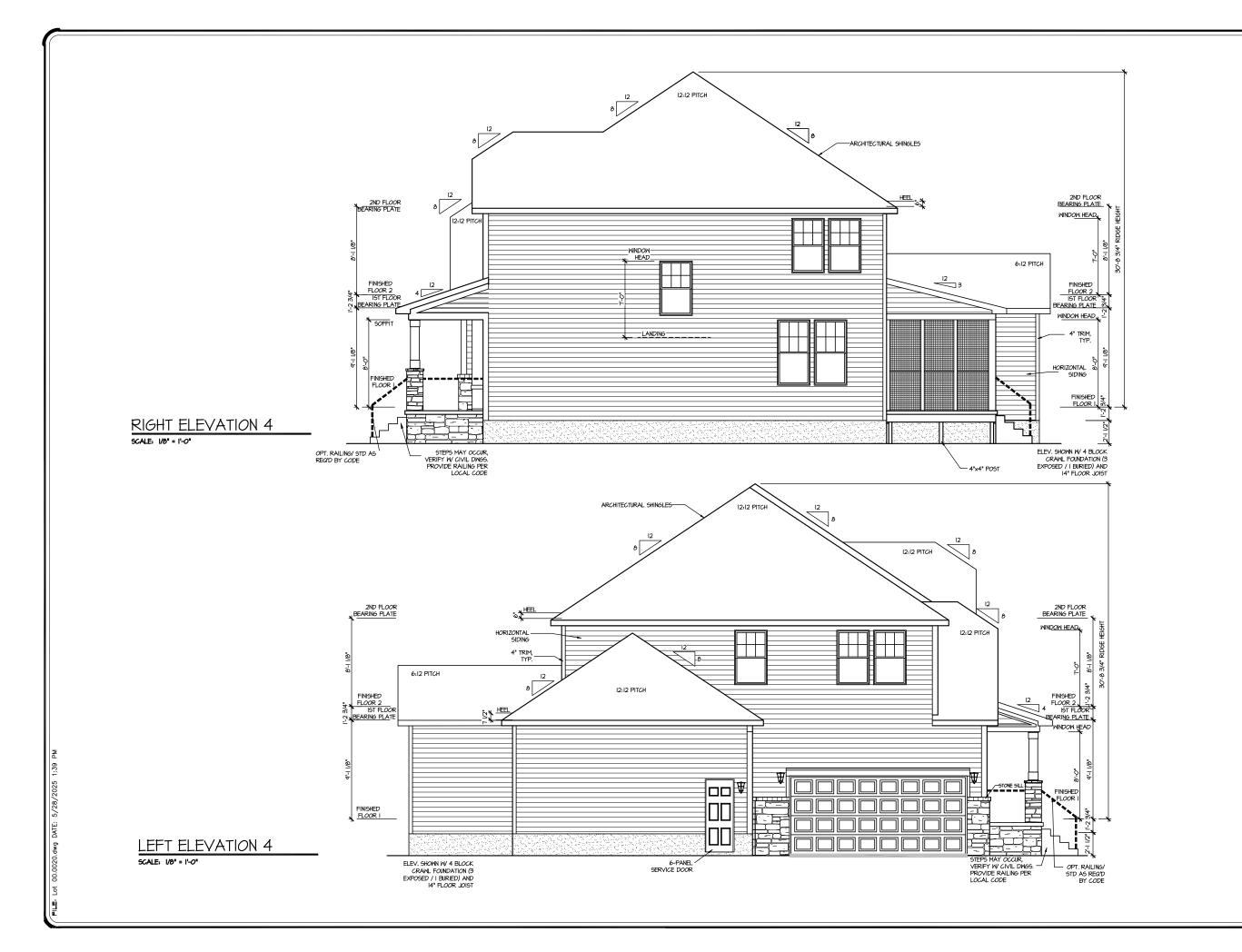
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FRONT

SCALE: 1/8" = 1'-0"



DRAWN BY: ITS DATE: 05/28/2025 PLAN NO. 1635



DRAWING TITLE RIGHT & LEFT ELEVATIONS

HOUSE NAME:
STONEFIELD
DRAWING TITLE

SHEET No.

A1.2

UPPER ROOF VENTILATION CALCULATIONS:

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOWER VENTING. (BOTTON 2/3 RDS)

82 LINEAR FEET OF SOFFIT X 5.7 50. IN. = 3,245 50. FT.

IPPER VENTING. I/OF U/3 BD.

3 4 LINEAR FEET OF RIDGE X I/O 50. IN. = 3 50. FT.

3 50. FT. AT 50%
(1 TO 300 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS:
ROOF AREA 3 = 69 50, FT.
OUFFALL REGULEED STORM TO STORM
I TO SO = 20 T 50, FT.
1 TO SO = 20 T 50, FT. (1 TO SOC)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 10 SQ. IN/ LINEAR FT.

LOWER VENTINS. (BOTTOM 2/3 RDS)

23 LNEAR FEET OF SCHETT X 5.1 SQ. IN. = 0.41 SQ. FT.

UPPER VENTINS. (TOP 1/3 BD)

3 LNEAR FEET OF RIDGE X IB SQ. IN = 1 SQ. FT.

1 SQ. FT. BETWERN SQB.

(1 TO 300 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA 5 = 144 5Q. FT.
OVERALL REQUIRED VENTILATION:
1 TO 150 = 0.76 5Q. FT.
1 TO 300 = 0.48 5Q. FT.
50% IN TOP THIRD = 0.24 5Q. FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 10 SQ. IN/ LINEAR FT.

LONER VENTING: (BOTTOM 2/3 RDS)
7 LINEAR FEET OF SOFFIT X 5.7 SQ. IN. = 0.277 SQ. FT. T LINEAR FIET OF SOFFIT X 5.7 SQL IN. = 0.211 SQL FT.

PUPPER VIBITIES (100 JUS RD)

2 LINEAR FIET OF RIDGE X IB SQ. IN = 0.25 SQ. FT.
15Q. FT. AT 50%

(1 TO 300 ALLOWED)

UPPER ROOF VENTILATION CALCULATIONS: | NOF AREA 2 = 252 SQ FT. |
| OVERALL REQUIRED VENTILATION: | 170 ISO = 1.68 SQ. FT. |
| 1 0 300 = 0.84 SQ. FT. |
| 50% IN TOP THIRD = 0.42 SQ. FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LONER VENTINS. (BOTTOM 2/3 RDS)

II LINEAR FIET OF SOFFIT X 5.1 50. IN. = 0.435 50. FT.

UPTER VENTINS. (CIP. U.B. 2015)

4 LINEAR FIET OF RIDGE X ID 50. IN = 0.5 50. FT.

0.5 50. FT. AT 50%

(1 TO 300 ALLONED)

UPPER ROOF VENTILATION CALCULATIONS:
ROOF AREA 4 = 142 50, FT.
OVERBALL SEGMENT VANILATION.
1170 300 =
50% IN TOP THIRD = 0.231 50, FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LONER VENTING. (BOTTOM 2/3 RDS)
20 LINEAR FIET OF SOFFIT X 5.1 SQ. IN. = 0.74 SQ. FT.

UPPER VENTING. (70P L/3 BD)
6 LINEAR FIET OF RIDGE X IB SQ. IN. = 1 SQ. FT.

1 SQ. FT. AT 50%
(1 TO 300 ALLONED)

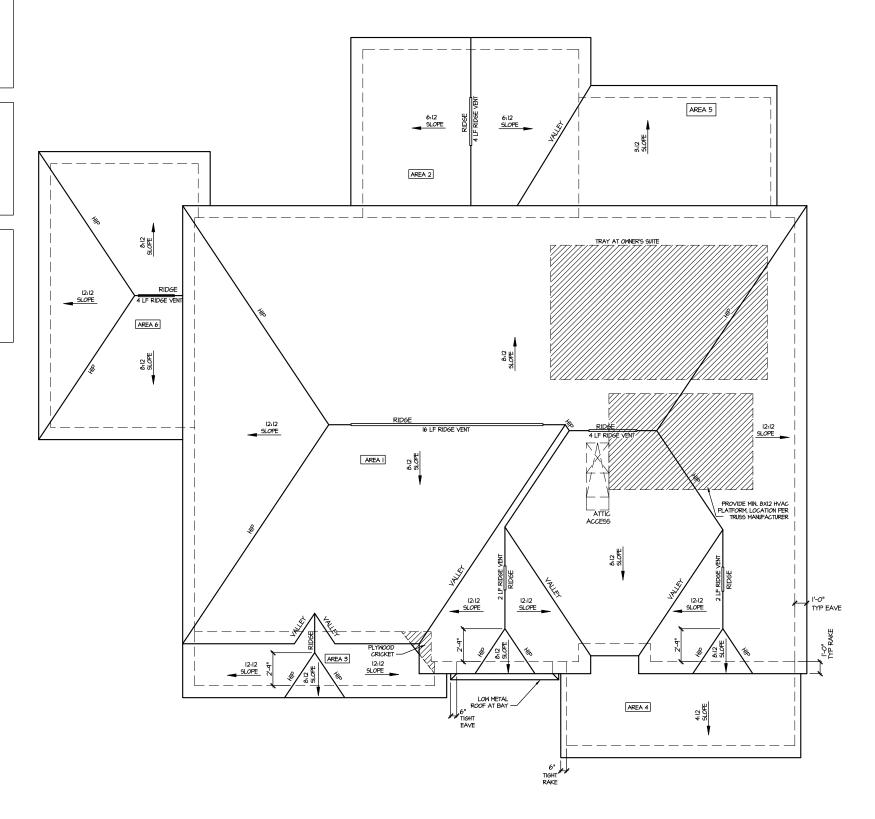
UPPER ROOF VENTILATION CALCULATIONS:

ROOF AREA 6 = 234 Sq. FT.
OVERALL REQUIRED VENTILATION:
1 TO 150 = 156 Sq. FT.
1 TO 300 = 0.716 Sq. FT.
50% IN TOP THIRD = 0.34 Sq. FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN / LINEAR FT.

LONER VENTING: (BOTTOM 2/3 RDS) IO LINEAR FEET OF SOFFIT X 5.7 Sq. IN. = 0.346 Sq. Ft. () LINEAR HELT OF SOFFIT X 5.7 50. IN. = 0.396 50. UPPER VENING. (10P I/3 RD). 4 LINEAR FEET OF RIDGE X IÐ 50. IN = 0.5 50. FT. I 50. FT. AT 50% () 10 300 ALLOWED)

NOTE: ROOF PLANS SHOWN W/ MIN. REQ'D RIDGE VENT LOCATIONS. ACTUAL RIDGE VENT LOCATIONS AND QUANTITY PER BUILDER



ROOF PLAN ELEV. 4

SCALE: 1/8" = 1'-0"

SHEET No. AI.3

HOUSE NAME:
STONEFIELD
DRAWING TITLE

DRAWN BY:

PLAN NO. 1635

DATE: 05/28/2025

CRAWL SPACE VENT CALCULATIONS: ELEV 4
CRAWL AREA = 1552 SQ. FT.
OVERALL REQUIRED VENTILATION:
1 SQ. IN. PER 1 SQ. FT. = 1552 SQ. IN.

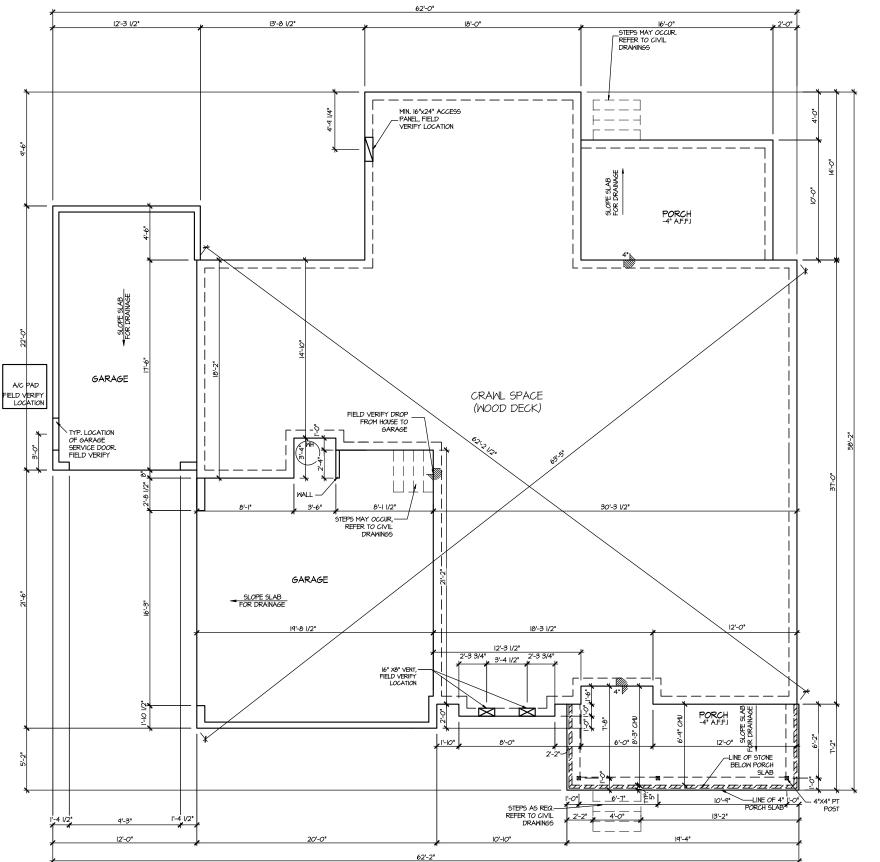
NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

VENTING REQUIREMENT:

1552 SQ. IN. / 72 SQ. IN. = 21.6 VENTS = 22 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED.

VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1



ELEVATION 4 CRAWL SPACE PLAN SCALE: 1/0"

SHEET No.

HOUSE NAME:
STONEFIELE
DRAWING TITLE

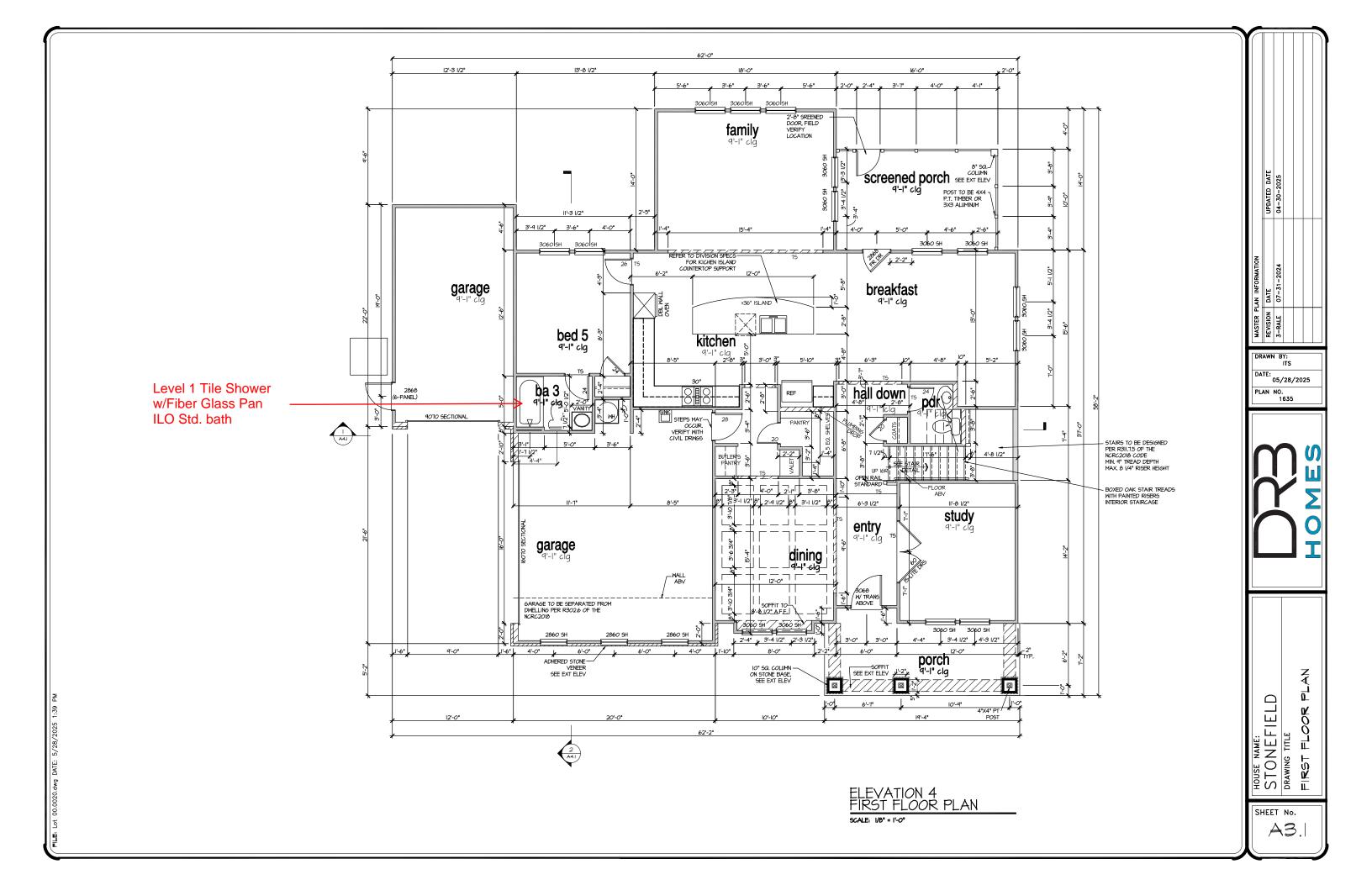
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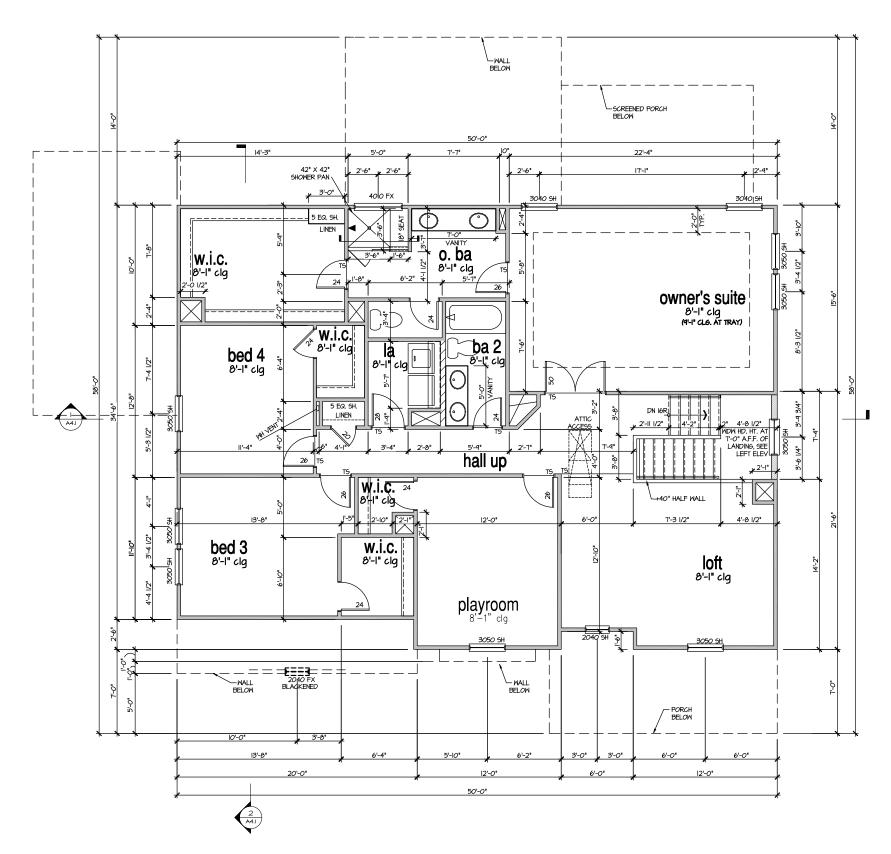
DATE: 05/28/2025 PLAN NO. 1635

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ELEVATION 4 SECOND FLOOR PLAN SCALE: 1/8" = 1'-0"

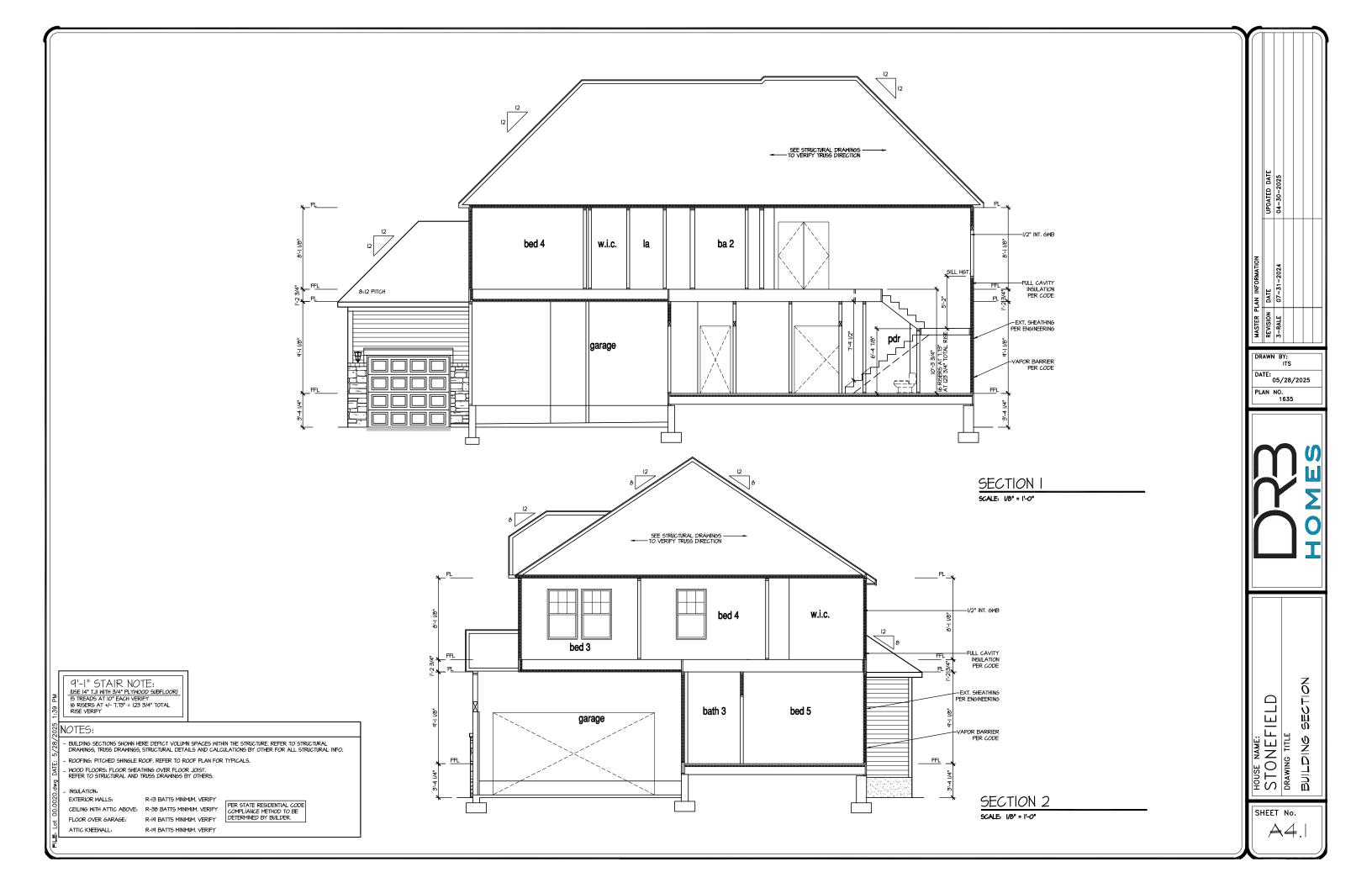
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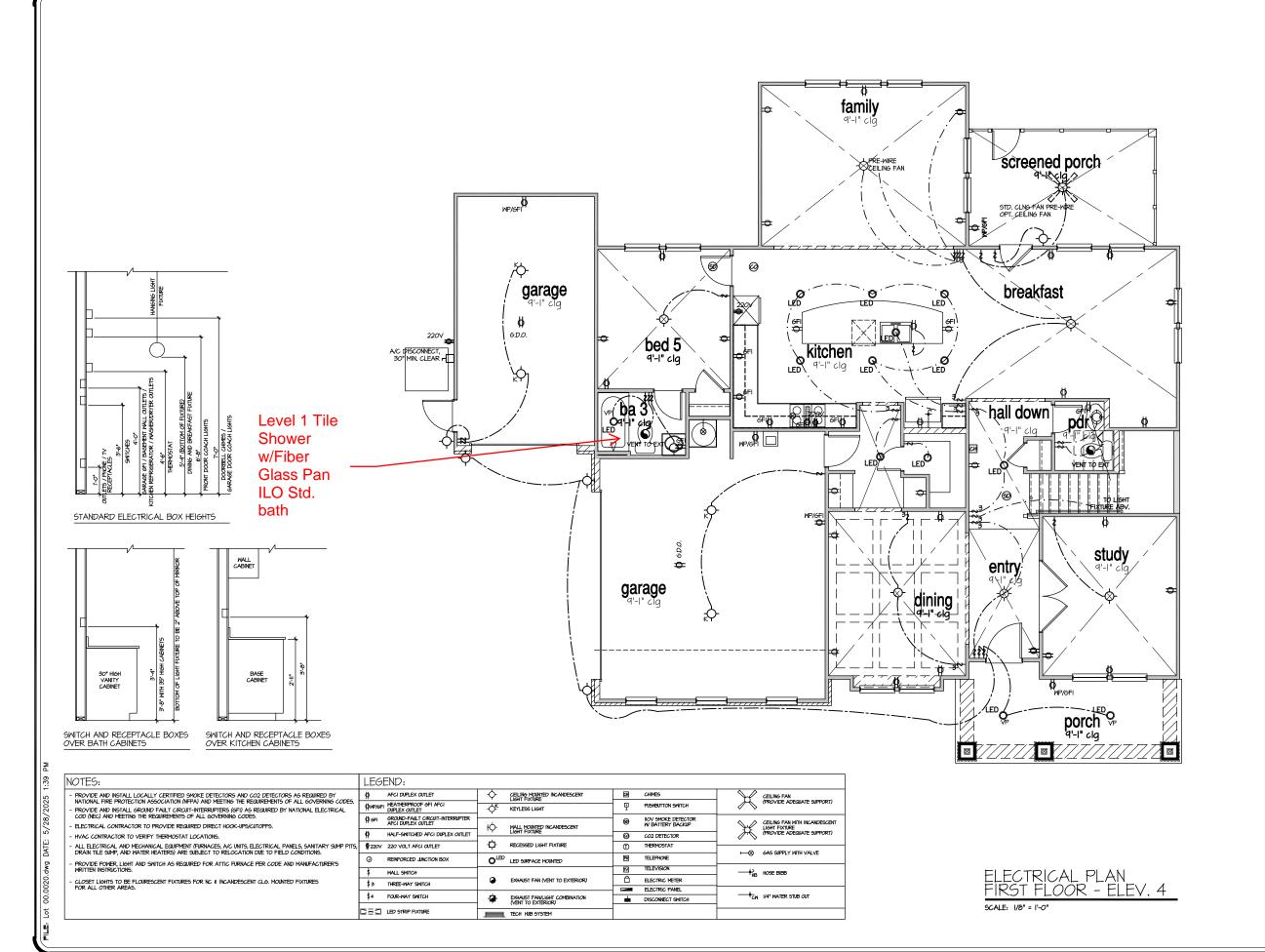
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HOUSE NAME:
STONEFIELD
DRAWING TITLE
SECOND FLOOR

SHEET No.

A3.2





MASTER PLAN INFORMATION

REVISION DATE
3-RALE 07-31-2024 04-30-2025

DRAWN BY: ITS

DATE: 05/28/2025

PLAN NO. 1635

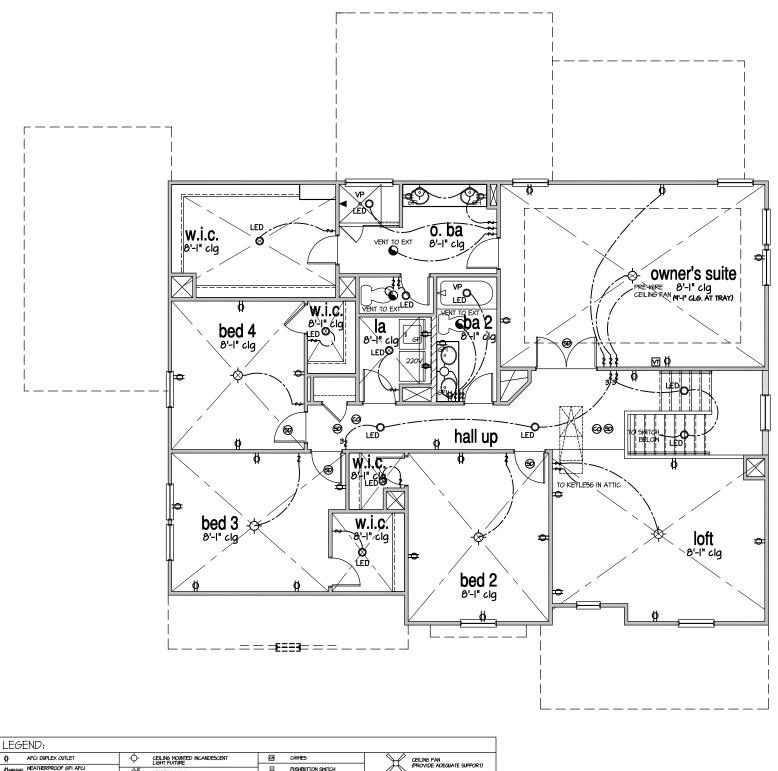


TLE LOOR ELECTRICAL

HOUSE NAME:
STONEFIELE
DRAWING TITLE

SHEET No.

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STANDARD ELECTRICAL BOX HEIGHTS

SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

NOTES: PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES AFGI DUPLEX OUTLET PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL COD (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. - ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS. HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS. ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FIRNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUM DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS. \$220V 220 VOLT AFCI OUTLET PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

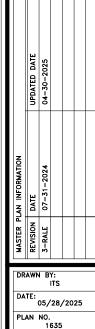
- CLOSET LIGHTS TO BE FLOURESCENT FIXTURES FOR NC & INCANDESCENT CLG. MOUNTED FIXTURES FOR ALL OTHER AREAS.

SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

WALL CABINET

CEILING FAN (PROVIDE ADEQUATE SUPPORT) MP/GFI WEATHERPROOF GFI AFCI DUPLEX OUTLET PUSHBUTTON SMITCH GFI GROUND-FAULT CIRCUIT-INTERRUIT
AFCI DUPLEX OUTLET IIOV SMOKE DETECTOR
W BATTERY BACKUP CEILING FAN WITH INCANDESCENT LIGHT FIXTURE (PROVIDE ADEQUATE SUPPORT) HO- WALL MOUNTED INCANDESCENT LIGHT FIXTURE HALF-SWITCHED AFCI DUPLEX OUTLET RECESSED LIGHT FIXTURE ① THERMOSTAT PH TELEPHONE REINFORCED JUNCTION BOX O<sup>LED</sup> LED SURFACE MOUNTED TELEVISION WALL SMITCH EXHAUST FAN (VENT TO EXTERIOR) ELECTRIC METER \$ 3 THREE-WAY SWITCH ELECTRIC PANEL CM 1/4" WATER STUB OUT FOUR-WAY SWITCH EXHAUST FAN/LIGHT COMBINATION (VENT TO EXTERIOR) DISCONNECT SWITCH □□□ LED STRIP FIXTURE TECH HUB SYSTEM

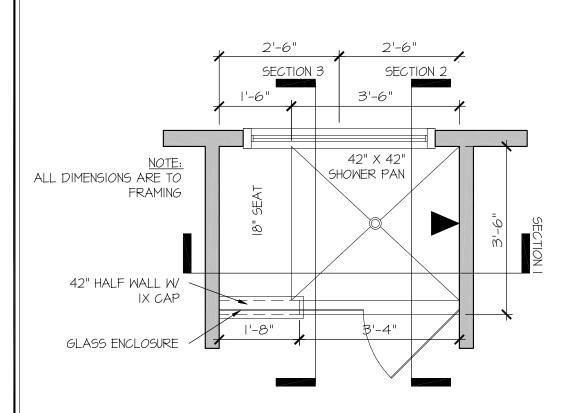
ELECTRICAL PLAN SECOND FLOOR - ELEV. 4 SCALE: 1/8" = 1'-0"





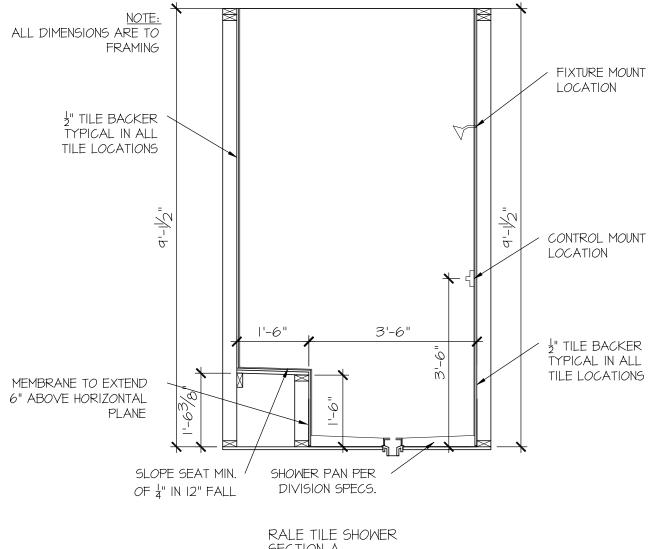
畆 HOUSE NAME:
STONEFIELE
DRAWING TITLE

SHEET No.



RALE TILE SHOWER 42" X 42" W 18" SEAT

SCALE: 1/2" = 1'-0"



SECTION A

SCALE: 1/2" = 1'-0"

CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE

24 X 36 SCALE



DETAIL SHOWER RALE

SHEET No.



SEAL

DRAWN BY:
L. BEAVERS
DATE: 9/1/22

PLAN NO.

24 X 36 SCALE

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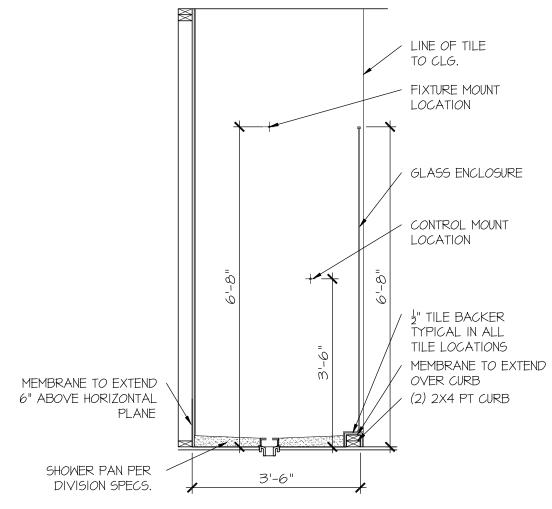


E ILE SHOWER DETAIL

OUSE NAME:

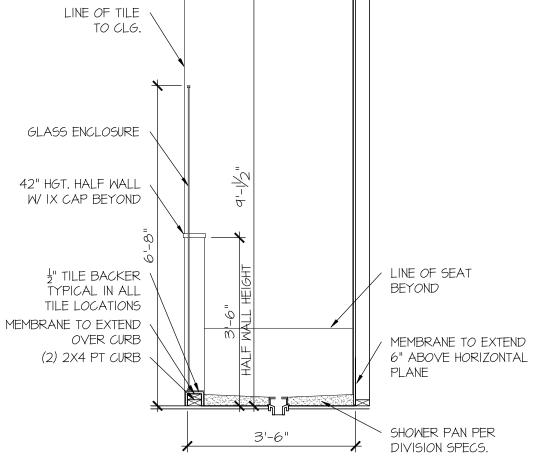
SHEET No.

P||.2



SCALE: 1/2" = 1'-0"





SCALE: 1/2" = 1'-0"

DESCRIPTION OF BLDG. ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS @ 4" o.c.	(3) NAILS @ 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS ● 8" O.C.	TOENAILS @ 6" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS @ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS <b>@</b> 24" O.C.	NAILS @ 16" o.c.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP @ CORNERS \$	(2) NAILS	(2) NAILS
INTERSECTING WALLS		
and the second s		

\* 2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE \* ARE SHOWN)

#### MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES

#### ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS IOTED OTHERWISE ON PLAN.

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO

FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

#### GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

ROOF DEAD = 7 PSF T.C., 10 PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

#### GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION, ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP. U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
   I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (I)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8' ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
  - 'LSL' Fb=2325 psi; Fv=3i0 psi; E=1.55xi0^6 psi
  - 'LVL' Fb=2600 psi; Fv=265 psi; E=2.0x10^6 psi
- 'PSL' FB=2400 PSI; FV=240 PSI; E=2.0XIO^6 PSI M+K SHALL BE FILLY INDEMNIFIED FOR ANY AND ALL ISSUES
- RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THE OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
- FOR 2 \$ 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x31/2" SIMPSON SDS SCREWS (OR 3½" TRUSSLOK SCREWS) & 16" O/C. USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM FDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF  $\frac{1}{4}$ "x6" SIMPSON SDS SCREMS (OR 6  $\frac{3}{4}$ " TRUSSLOK SCREMS) • 16" O/C. USE A MINIMUM OF 4 ROMS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE. A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

#### FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MEK FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I 1/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131" NAILS @ 6"o.c. @ PANEL EDGES & @ 12"o.c. FIELD. - 2 🖣 × 0.120" NAILS • 4" O.C. • PANEL EDGES € • 8" O.C. FIELD.
- 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN, SCREMS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

#### ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO:
  - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS w/ (3) 3"x0.131" NAILS • 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS @ 6"O.C. @ PANEL EDGES & @ 12" O.C. FIELD. - w/ 2 3 × 0.120" NAILS • 4"o.c. • PANEL EDGES € • 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"0.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.

#### VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size	
3'-0"	20 FT. MAX	L3"x3"x/4"	
	3 FT. MAX	L3"x3"x/4"	
6'-0 <b>"</b>	I2 FT. MAX	L4"x3"x/4"	
	20 FT. MAX	L5"x31½"x7%"	
	3 FT. MAX	L4"x4"x/4" *	
8'-0"	I2 FT. MAX	L5"x31½"x7%"	
	I6 FT. MAX	L6"x3½"x¾"	
9'-6"	I2 FT. MAX	L6"x31½"x5%"	
16'-0"	2 FT. MAX	L7"x4"x½" **	
10 0	3 FT. MAX	L8"x4"x½" **	

#### SHALL SUPPORT 2 %" - 3 ½" VENEER w/ 40 psf MAXIMUM WEIGHT ( 16' SHALL HAVE 4" MIN. BEARING

- : 16' SHALL HAVE 8" MIN, BEARING : 16' SHALL NOT BE FASTENED BACK TO HEADER.
- 6' SHALL BE FASTENED BACK TO WOOD HEADER IN WALL #48"0. W/S" DIA. x 3 S" LONG LAG SCRENS IN 2" LONG VERTICALLY SLOTTED HO! FS
- MAX, VENEER HT, APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
  WHEN SUPPORTING VENEER < 3° WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 1/2" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT
- FINISHING:

  FINISH
- FOR QUEEN VENEER USE L4x3x FOR 31/2" VENEER ONLY. SEE PLAN FOR VENEER SUPPORT IF

SD2. | REFERS TO SD2. | A FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RO

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301211) EXP. B. RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NOSEC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

#### EXT. WALL SHEATHING SPECIFICATION

- 1/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1/5" 16 GA STAPLES (1/6" CROWN) • 3" O.C. AT EDGES \$ • 6" O.C IN FIELD.

#### BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 3/8" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 3/4" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C IN FIELD, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

#### 3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN. IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- <u>PRE-MANUFACTURED PANELIZED WALLS:</u> FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWALL
OR 3" O.C. OSB SHEARWALL.

► INDICATES HOLDOWN BELOW

#### GENERAL STRUCTURAL NOTES

#### **FOUNDATION**

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING
  - 1/2" DIA. ANCHOR BOLTS @ 6'-0" O.C, 7" MIN. EMBEDMENT
  - (CONC), 15" MIN. EMBEDMENT (CMU)
    SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS @ 2'-8" O.C. (CMU) (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2. • BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF
- HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD. BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT
- BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER. • CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.:
- 4,000 psi: ...... FOUNDATION WALLS 2,500 psi: ...... FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: ...... GARAGE & EXTERIOR SLABS ON GRADE leq 000,00
- BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS) - TALLER WALLS MUST BE ENGINEERED.
- NOMINAL WIDTH (91/5" FOR 10" THICK WALL).
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC. ML-CL, OR CL). BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL WITH 2" CLEAR, REINFORCEMENT
- SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS. \* FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE
- DEPTH OVER OPENING OR (3)2x10 W (2)2x6 JACK STUDS, U.N.O. LARGER OPENINGS SHALL BE PER PLAN.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS
- THAN 5% OR MORE THAN 7% AIR ENTRAINMENT • ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP
- . JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR
- 15'-0" O.C. (MAXIMUM) JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS
- POSSIBLE (IJ RATIO) WITH A MAXIMUM OF IJ 5 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (F/m=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 € 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORGEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS. FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE
- \* DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

#### HOLD-DOWN SCHEDULE

5YMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC6683 ALTERNATE
► HD-3	SIMPSON STHDI4/STHDI4RJ

**\*** UTILIZE THE SSTB24 ANCHOR BOLT **②** ALL MONOSLAB **\$** INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN
THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 15" (FOR 1/8" DIA.) MIN. EMBEDMENT INTO CONCRETE, INSTALL PER MANUF, INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D.

DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE

"H CAR  $\circ$ 

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SHAKE 

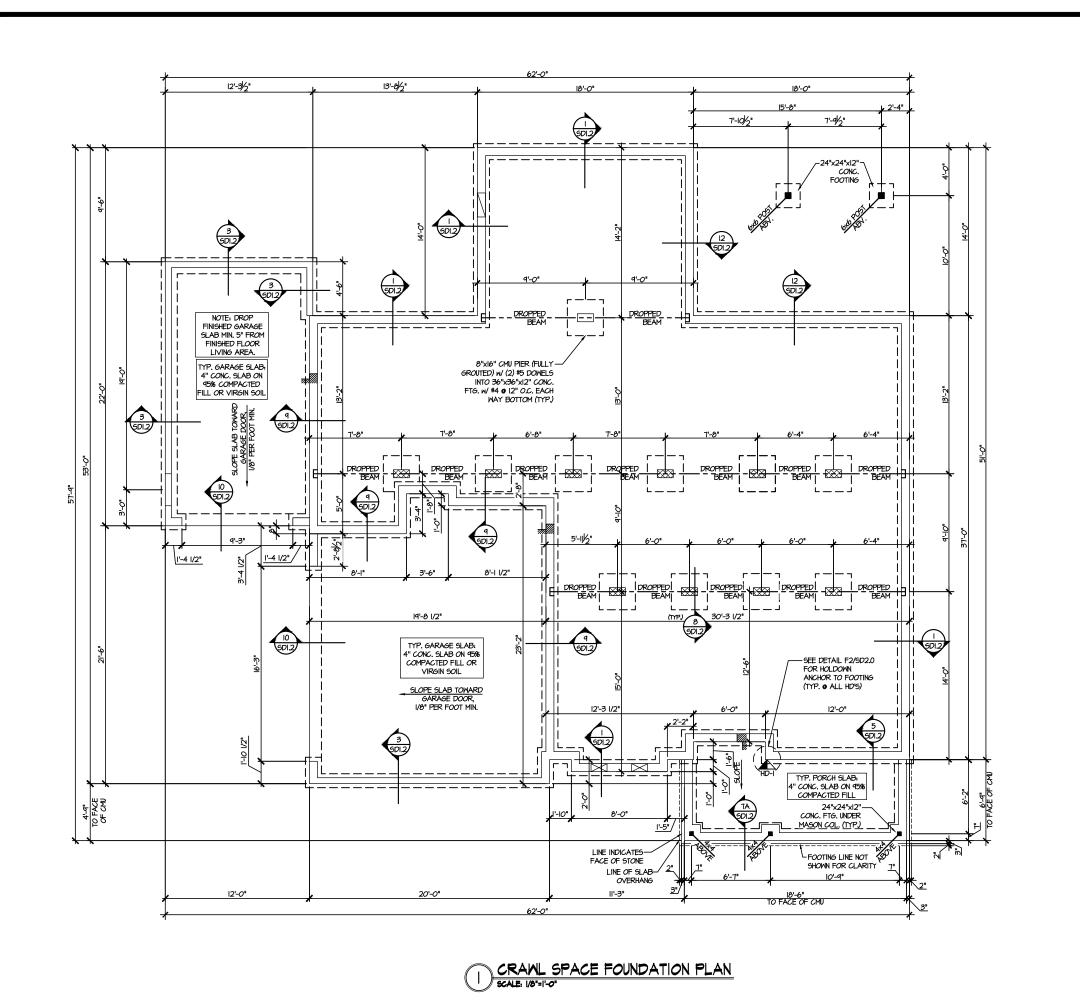


l&K project numbe 126-24045

JTR rawn by: ssue date: 06-13-2

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 $\widehat{\bigcap}$ RID AMPBEL



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MULHERN+KUL RESIDENTIAL STREETURAL ENGINEERI

M&K project number:

126-24045

JTR drawn by: issue date: 06-13-2

REVISIONS:

CAMPBELL RIDGE LOT 20 - STONEFIELD 4 RALEIGH, NC

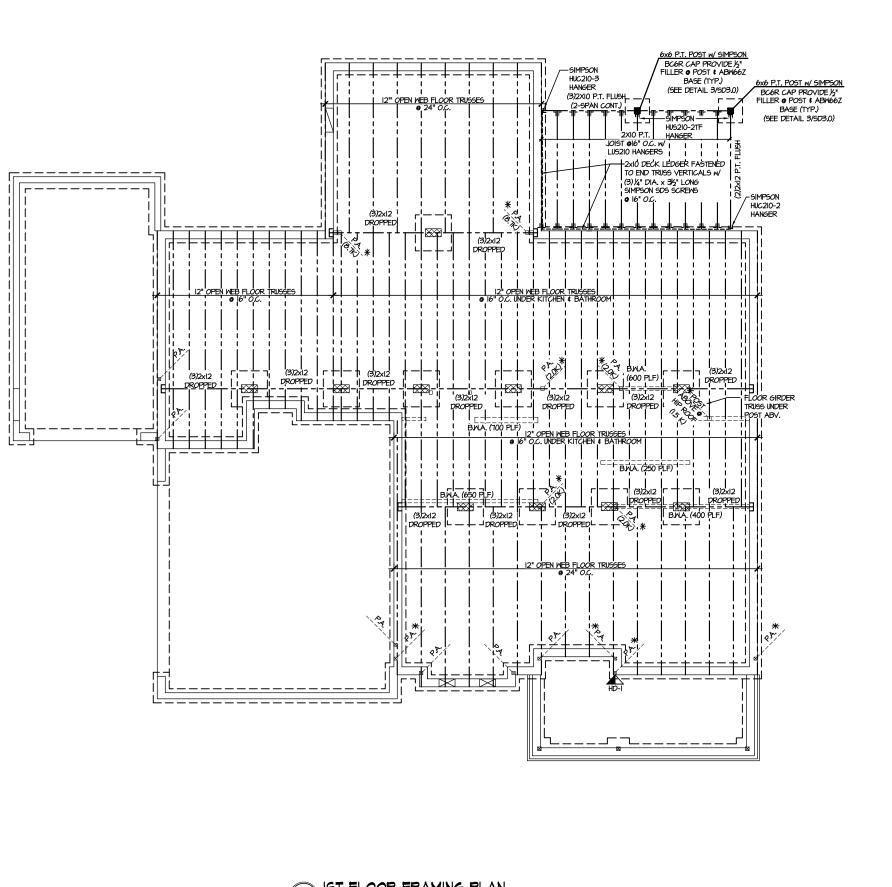
#### LEGEND

- IIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- --- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JIL METAL HANGER
- \* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE,
- INDICATES HOLD-DOWN OR STRAP.
  REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES

& SCHEDULES

OUNDATION



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MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEERI SUBMISSIERA BLIBER! - MARKER FORTER



M&K project number: 126-24045

project mgr: JTR drawn by: GTK issue date: O6-13-25

REVISIONS:

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LEGEND

- INTERIOR BEARING WALL
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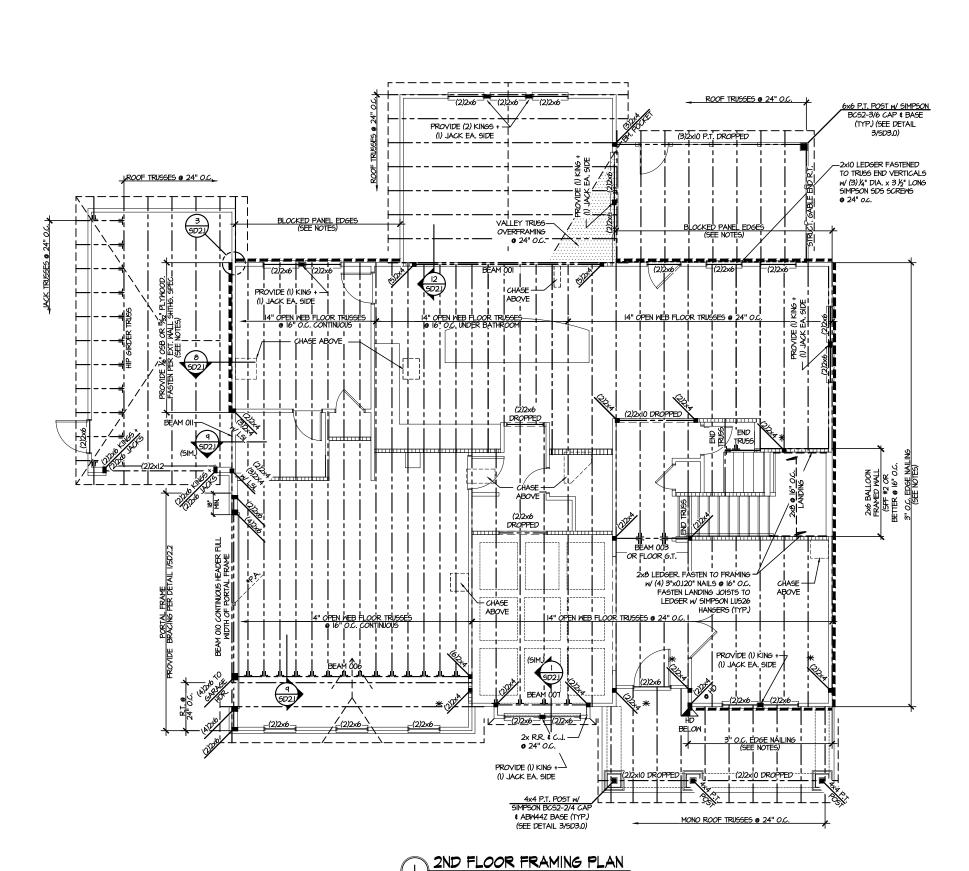
\$ SCHEDULES

**S2.0** 

OR

CAMPBELL RIDGE LOT 20 - STONEFIELD 4 RALEIGH, NC

. Lot 20 - Structurals DAIE: 6/13/2025 11:39 AM



ENGINEERED BEAM MATERIAL SCHEDULE LVL OPTION STEEL OPTION (2)2x12 + (1) 3/2x114 5TEE FLITCH PLATES - D 001 (2)134"x16" - D WI2xI4 - D (2)2xl2 + (I) ¼ xll¼ 5TEE FLITCH PLATES - D 002 (2)13/4"×113/4" - D 3½"x11%" - D (3)13/4"×113/4" - D WI2xI4 - 1 (2)2xl2 + (1) ¼"xll¼" 5TEEL FLITCH PLATES - F 003 (2)13/4"x14" - F (2)13/4"x14" - F 36"xl4" - F WI2xI4 - F (2)2xl2 + (I) %"xll" STEEL FLITCH PLATES - F (2)13/4"×14" - F 3½"xl4" - F (3)13/4"×14" - F WI2xI4 - F (3)2xl2 + (2) 5/8"xll/4" STEEL FLITCH PLATES - 1 005 (3)1¾"x18" - FT 5¼"xl8" - FT (3)2x12 + (2) %"x114" STEEL FLITCH PLATES - 1 006 (3)13/4"x18" - FT WI2x26 - F 5¼"xl8" - FT N/A (2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F 007 (2)13/4"x14" - F 3½"xl4" - F (2)13/4"x14" - F WI2xI4 - F (2)2xl2 + (I) 以"xll以" 5TEEL FLITCH PLATES - F 000 (2)13/4"x14" - F (2)13/4"x14" - F 36"x|4" - F WI2xI4 - F (2)2x10 + (1) ¼"x4¼" STEE FLITCH PLATES - D 009 (2)13/4"×9/4" - D 3½"x9¼" - D (2)134"x94" - D WI0x12 - D (3)2xl2 + (2) ½"xll½" STEEL FLITCH PLATES - H 010 (2)13/4"×16" - H NΑ 3½"x16" - H (3)13/"x16" - + (2)2xi2 + (i) /¼"xi/¼" 5TEEL FLITCH PLATES - D (2)13/4"x113/4" - D 35"x11%" - D (3)13/4"x113/4" - D WI2xI4 - D (3)2x12 + (2) ½"xII¼" STEEL FLITCH PLATES - D 012 (3)134"x16" - D 5¼"x16" - D (4)1¾"x16" - D W12x19 - D (2)2xl2 + (I) 从"xli从" STEE FLITCH PLATES - D 013 (2)134"x1136" - D 3½"x11%" - D (3)134"x1136" - D (3)2x12 + (2) %"x114" 5TEEL FLITCH PLATES - D 014 (2)13/4"x16" - D 3½"x16" - D (3)13/4"x16" - D WI2xI4 - D (3)2x12 + (2) ¼"x1¼" 5TEEL FLITCH PLATES - D (3)13/4"x113/4" - D N/A (3)15/4"×117/4" - I WI2xI4 - D (2)2xl2 + (I) 以"xll以" STEEL FLITCH PLATES - F (2)134"×1136" - F 3½"xII%" - F (3)134"×1176" - F WI2xI4 - F

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM

- "FB" INDICATES FLUSH BOTTOM BEAM

- "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER

REFER TO DETAIL D/502.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL E/502.0 FOR TYPICAL STEEL BEAM CONNECTIONS
FOR FLUSH TOP DEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN PLATES IN SUCCESSION W/ (2) 3"XO.120" NAILS @ 8" O.C.

FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN

PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C.

\*\*\* - SEE PLAN FOR EXTENT OF 3-PLY BEAM

WORTH .

1&K project number 126-24045

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EVISIONS:

SD2, I REFERS TO SD2, IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- ■ INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

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MULHERN+KULP
RESIDENTIAL STRECTURAL ENGINEERING

M&K project number: 126-24045

JTR drawn by: issue date: 06-13-25

REVISIONS:

ROOF FRAMING

LEGEND

• == INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

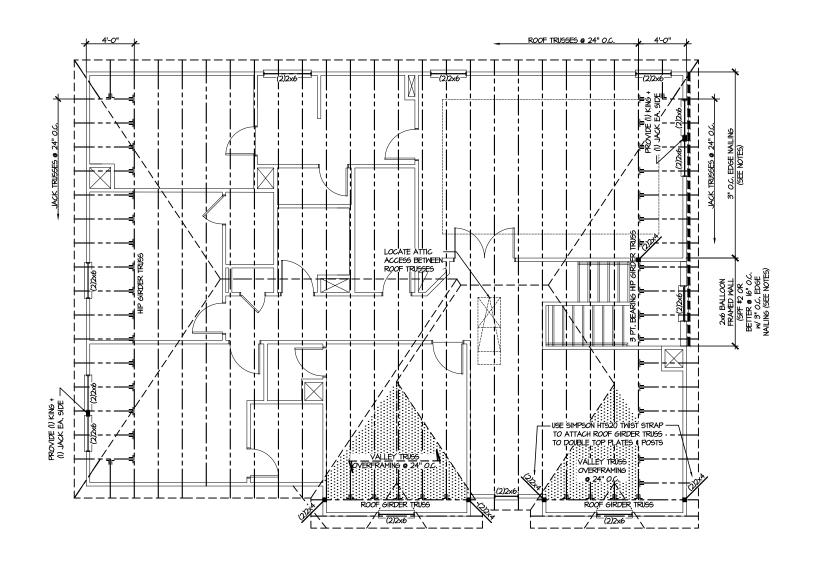
INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

INTERIOR BEARING WALL

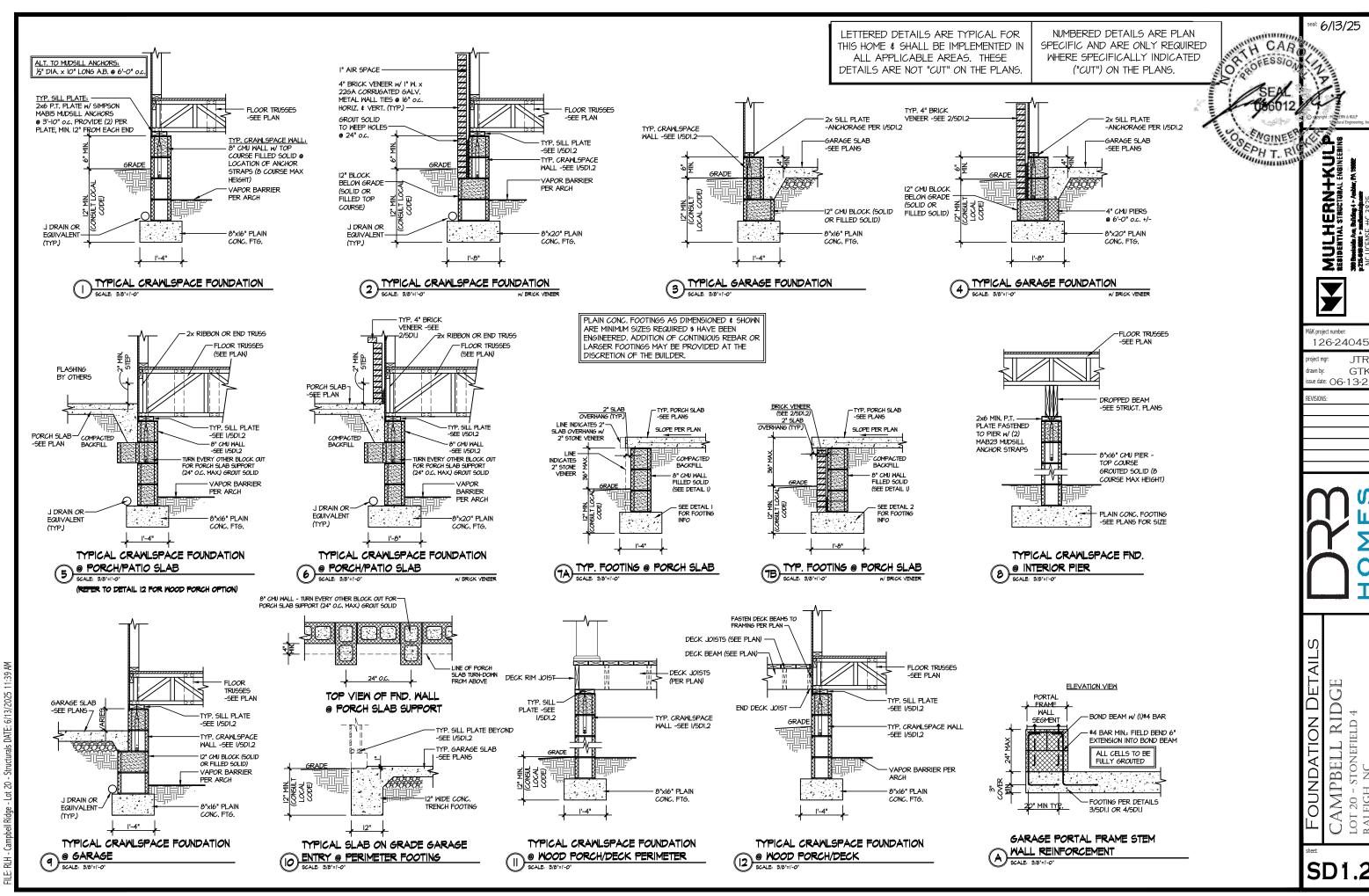
● □===□ BEARING WALL ABOVE BEAM / HEADER

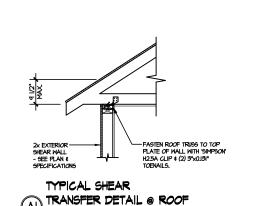
JL METAL HANGER

CAMPBELL RIDGE LOT 20 - STONEFIELD 4 RALEIGH, NC

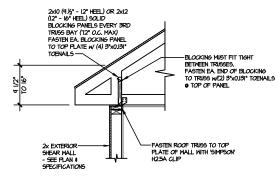


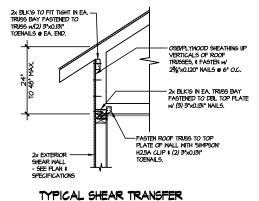




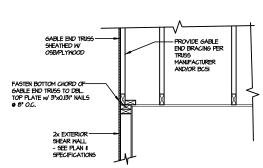


HEEL HEIGHT LESS THAN 9½" NO BLOCKING REQD





DETAIL @ RAISED HEEL TRUSS



MAR CAR W ROFESSION OSEPH T. RI MULHERN+KUL Residential Structural Engineerin

6/13/25

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issue date: 06-13-2

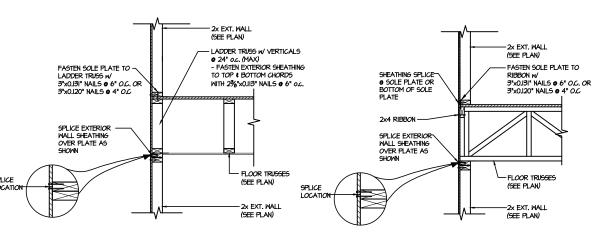
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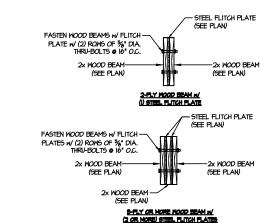
TYPICAL GABLE END DETAIL SOLLE SIDE STATE

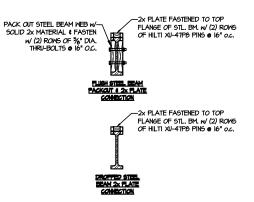
#### TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

SCALE: 9/8"=1"-0" HEEL HEIGHT BETWEEN 9. HEEL HEIGHT BETWEEN 9½" - 16" BLOCKING REQで





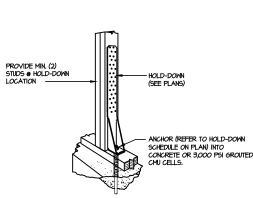


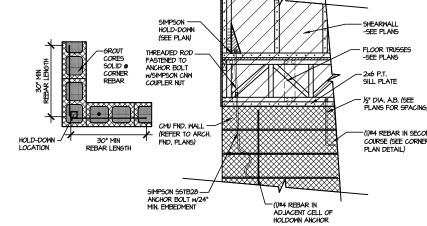
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL

TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL

TYPICAL FLITCH BEAM CONNECTION DETAIL SCALE 944-1-07

TYPICAL STEEL BEAM CONNECTION DETAIL





NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

TYPICAL HOLD DOWN INSTALLATION SCALE: N.T.S.

HOLD-DOWN INSTALLATION
SCALE: N.T.S.

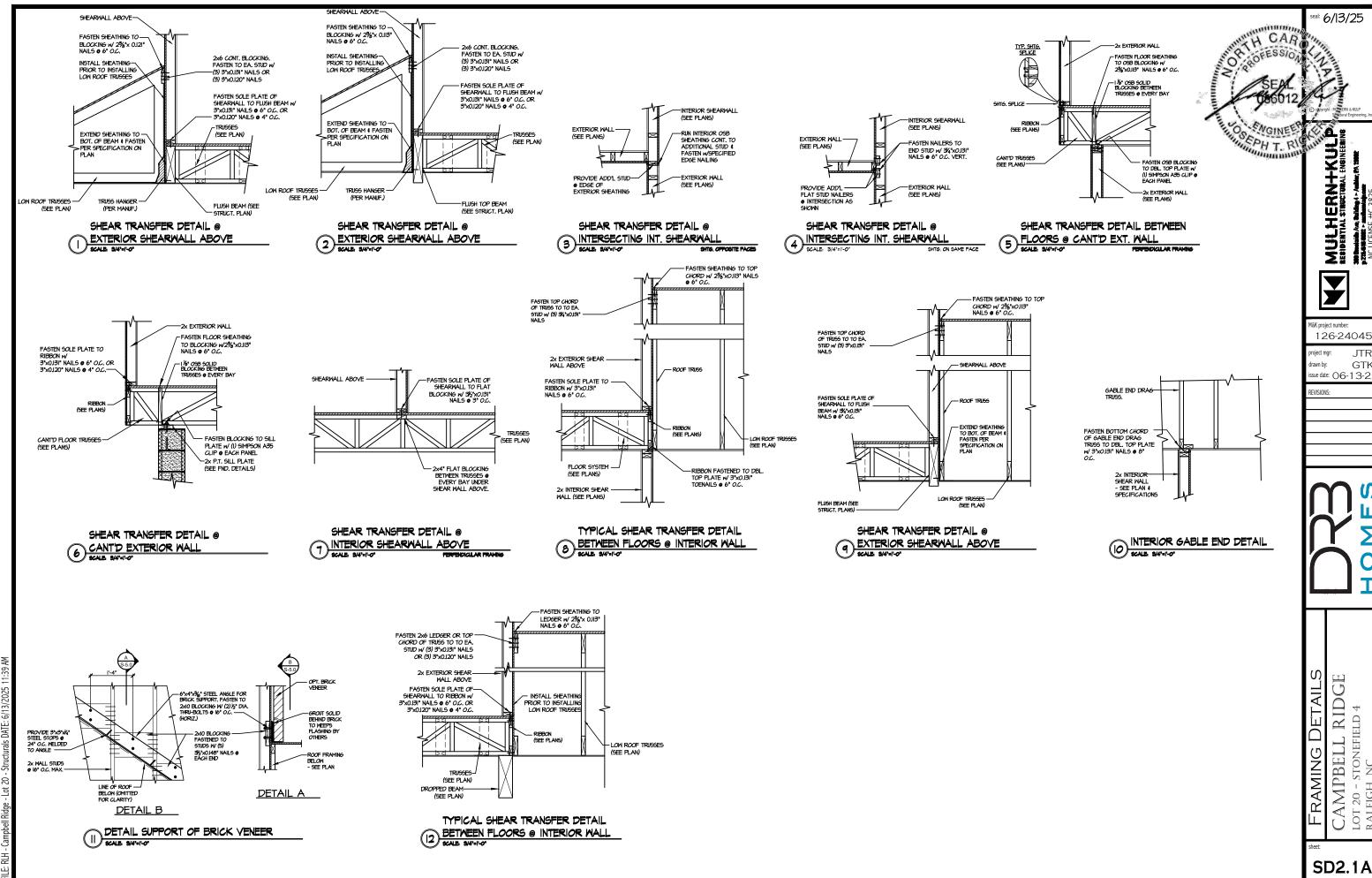
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

DETAILS

RIDGE

AMPBEL

が DIA, A.B. (SEE PLANS FOR SPACING) (I)#4 REBAR IN SECOND - ANCHOR (REFER TO HOLD-DOWN SCHEDULE ON PLAN) INTO CONCRETE OR 3,000 PSI GROUTED CMU CELLS. TYPICAL CORNER FOUNDATION

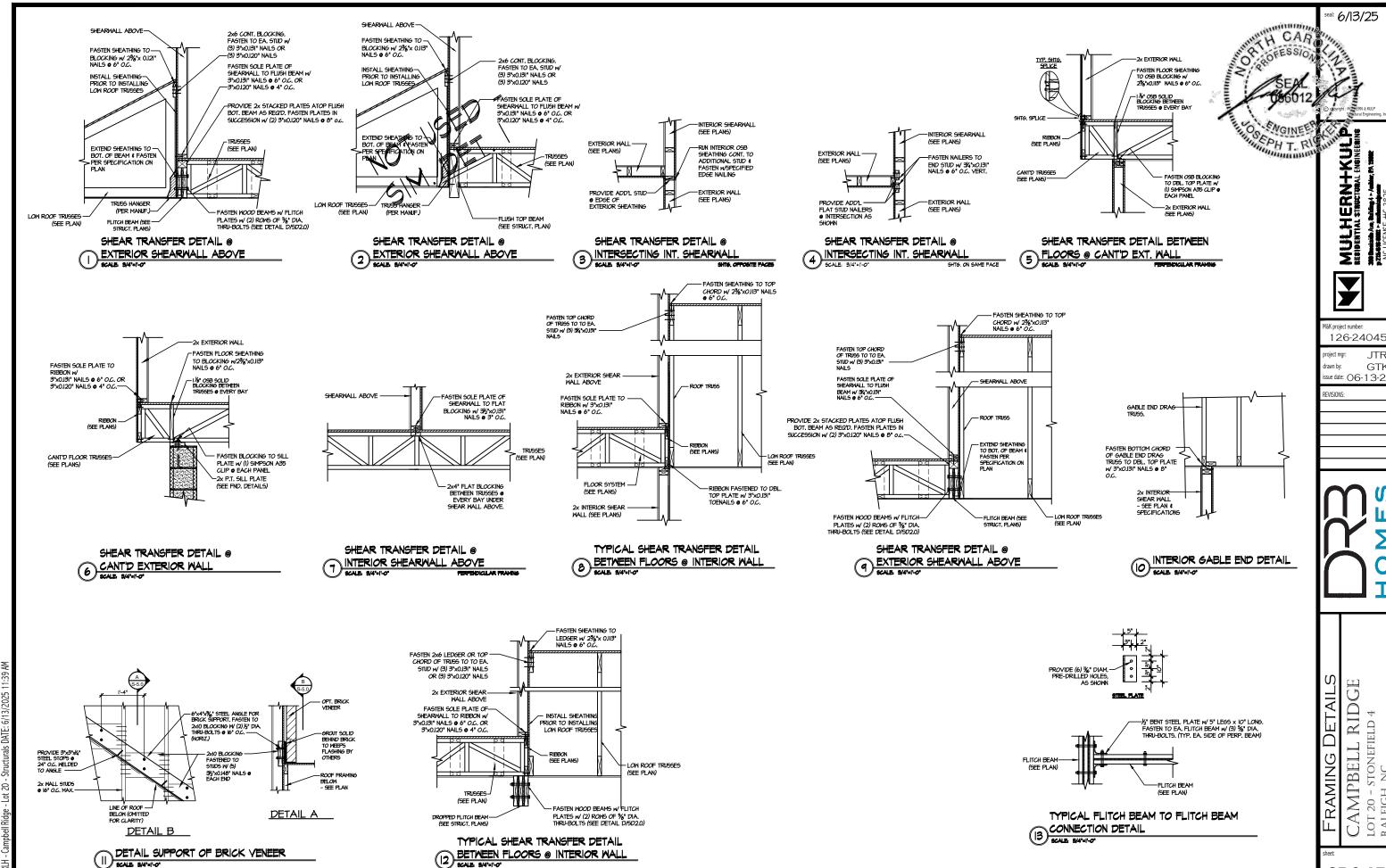


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GTK

AMPBEL

SD2.1A



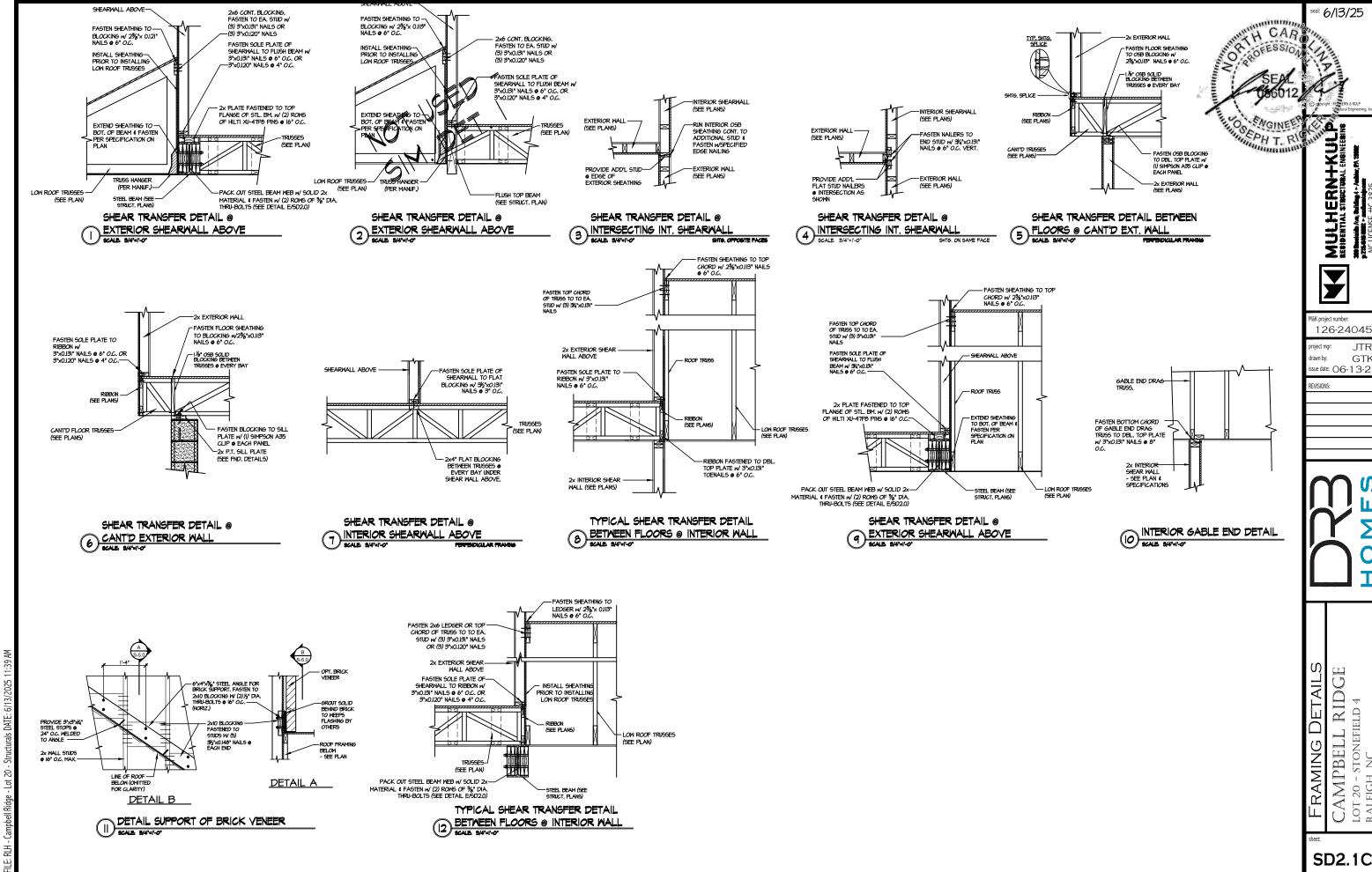
SD2.1B

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AMPBEL

JTR

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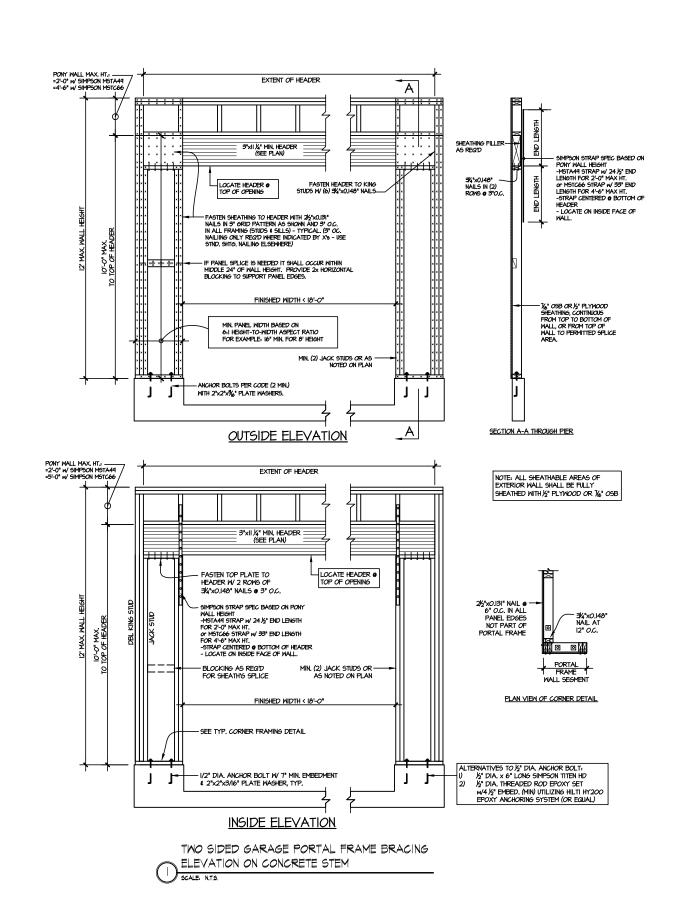
RID AMPBEL

SD2.1C

**JTR** 

GTK





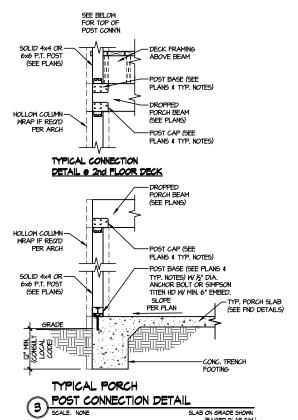
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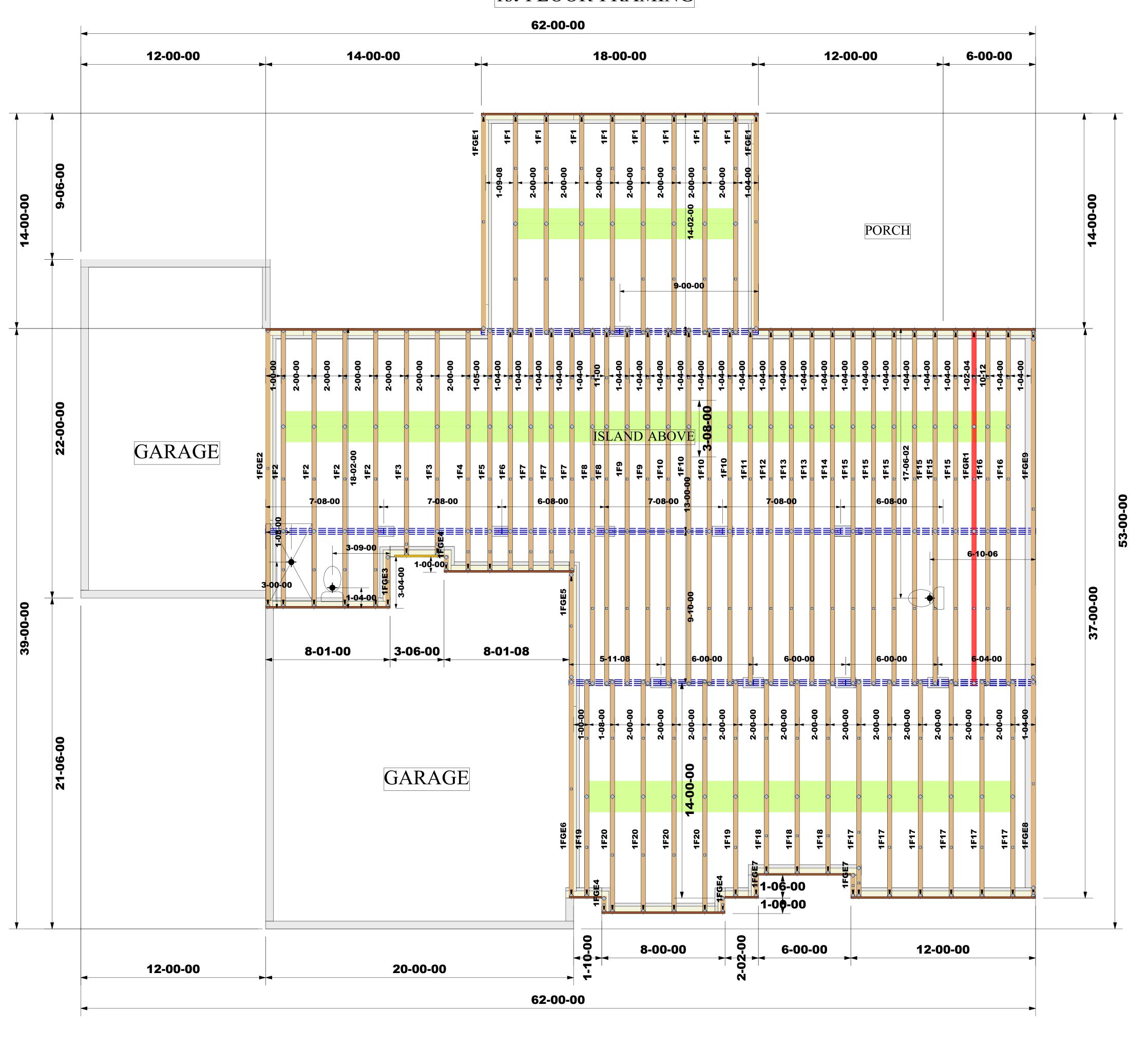
JTR

GTK



CAMPBELL RIDGE LOT 20 - STONEFIELD 4 RALEIGH, NC

# 1st FLOOR FRAMING



CAMPBELL RIDGE LOT 00.0020
(NC)(RAL)
STONEFIELD REV.3
EL. 4
OPT.CRAWLSPACE
OPT.EXTENDED BREAKFAST/ MASTER
BEDROOM
OPT.GARAGE SIDE LOADED W/ 3RD CAR
GARAGE LEFT

\*EXTERIOR DIMENSIONS ARE
TO STUD.

\*TRUSSES @ 16" O/C U.N.O.

\*INSTALLATION OF STRONGBACKS
RECOMMENDED ( NOT REQUIRED).

\*INSTALL 2X4 NAILERS ON ALL TOP RIBBON
NOTCH CONDITIONS.

Job #:	WARNING:	NOTE:
2505-7923	CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT	IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIO FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS
Designer:	BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON.	LAYOUT PRIOR TO FABRICATION.  THIS COMPANY IS A TRUSS MANUFACTURER WHOS RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN
Savvy Nath Sales Rep:	TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT	RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.

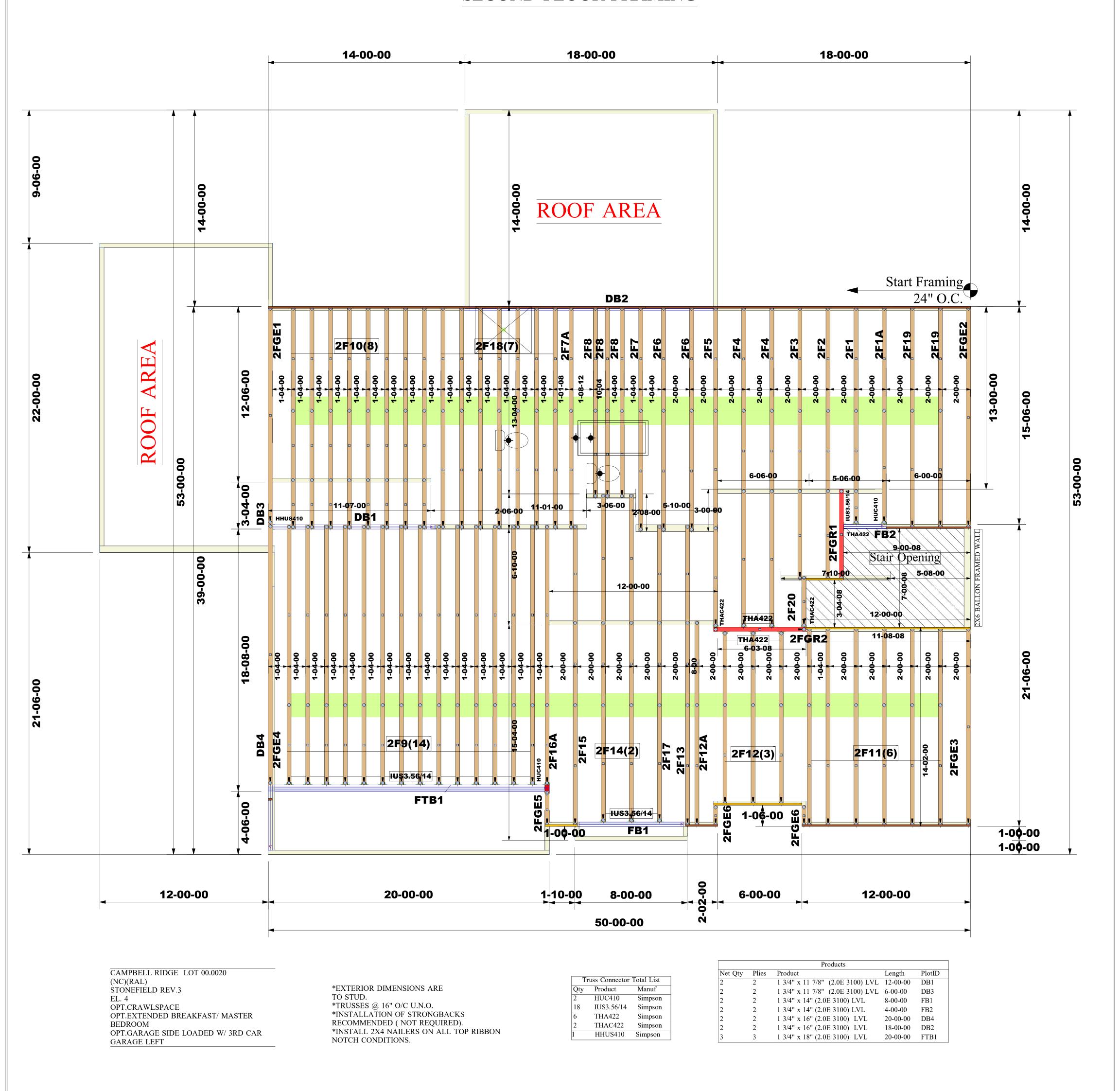
Robbie Zarobinski

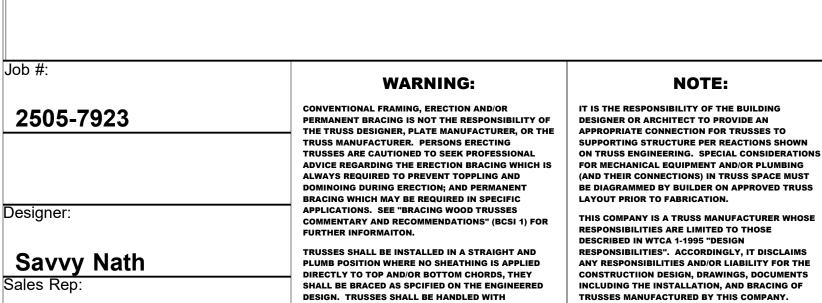
	Customer: DRB Raleigh
N DNS T BS	Job Name: Campbell Ridge Lot 00.0020 OWF
SE	Lot #: 00.0020

Model Name: Stonefield Rev 3



## SECOND FLOOR FRAMING





REASONABLE CARE DURING ERECTION TO PREVENT

DAMAGE OR PERSONAL INJURY.

Robbie Zarobinski

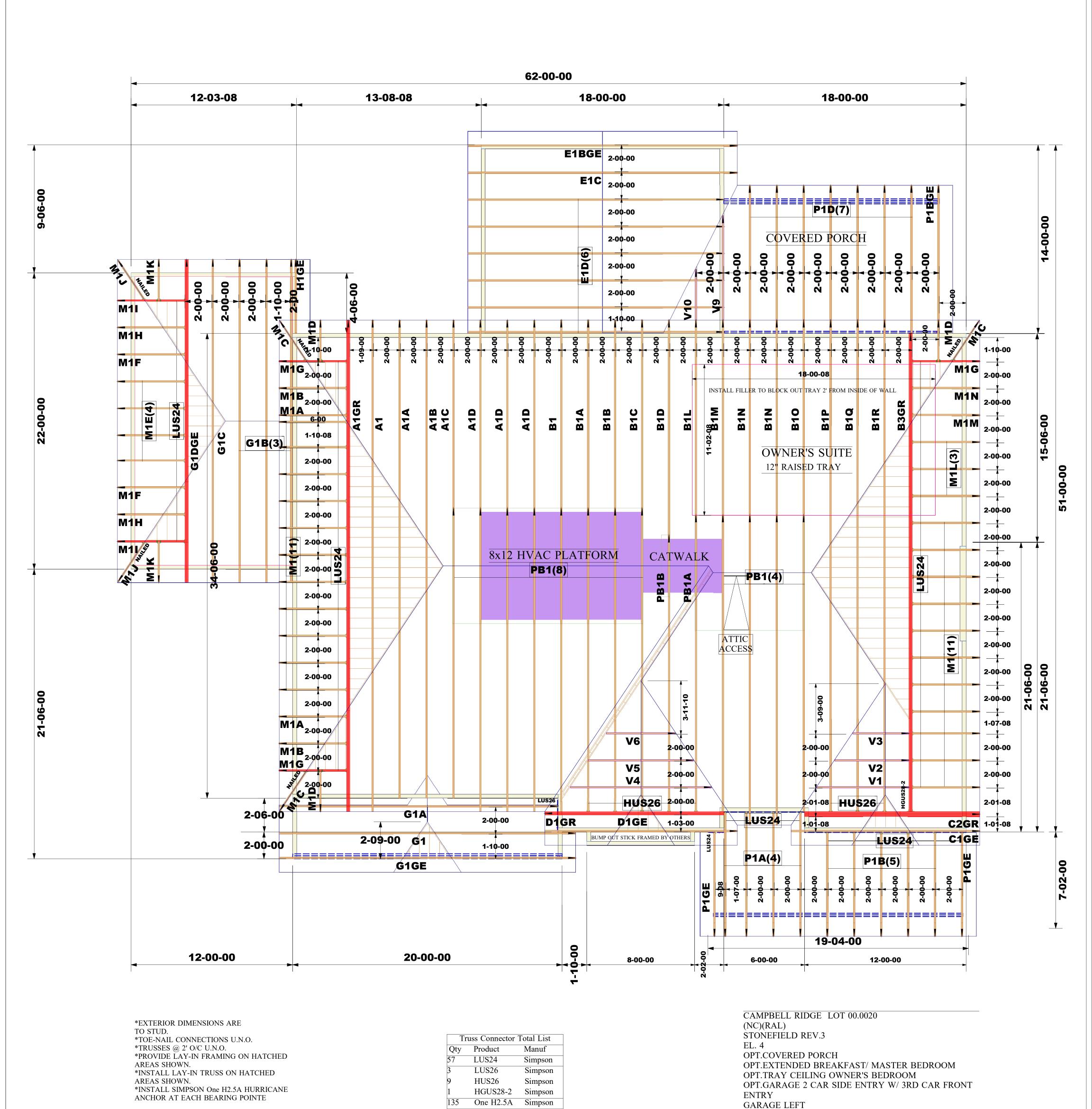
Customer: DRB Raleigh Job Name: Campbell Ridge Lot 00.0020 OWF <u>ot #:</u> 00.0020

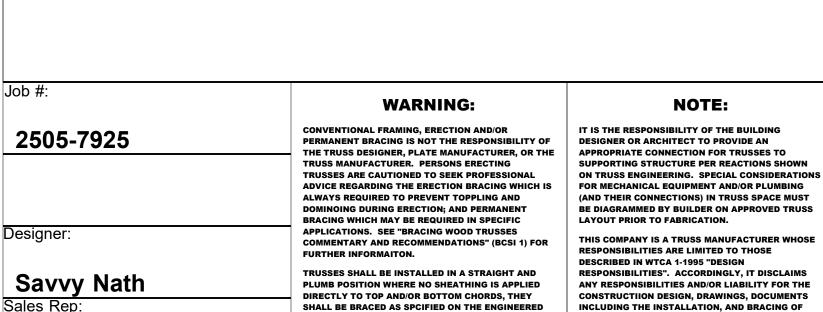
Model Name: Stonefield Rev 3

Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

TPI Plant W974







DESIGN. TRUSSES SHALL BE HANDLED WITH

DAMAGE OR PERSONAL INJURY.

Robbie Zarobinski

REASONABLE CARE DURING ERECTION TO PREVENT

TRUSSES MANUFACTURED BY THIS COMPANY.

	Customer: DRB Raleigh
S	Job Name: Campbell Ridge Roof
	<u>Lot #:</u> 00.0020
	Model Name: Stonefield Rev. 3

