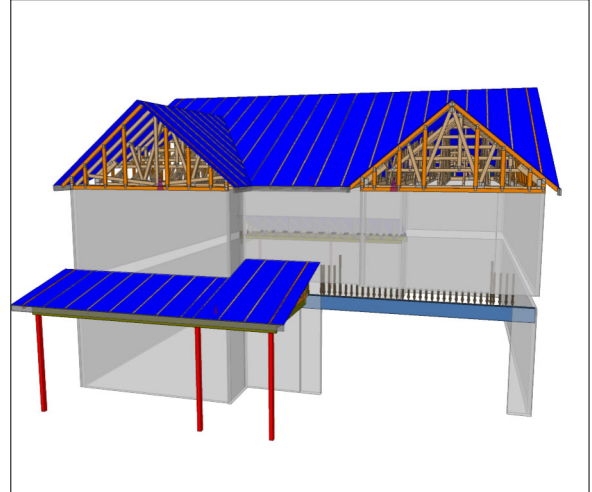




Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450

Builder: DR Horton Inc
Model: 27 Eagle Creek -
Lawson - E



THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

Date: _____

General Notes:

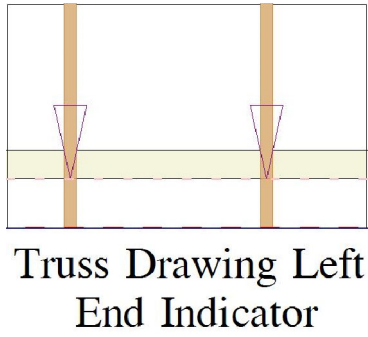
** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.

** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.

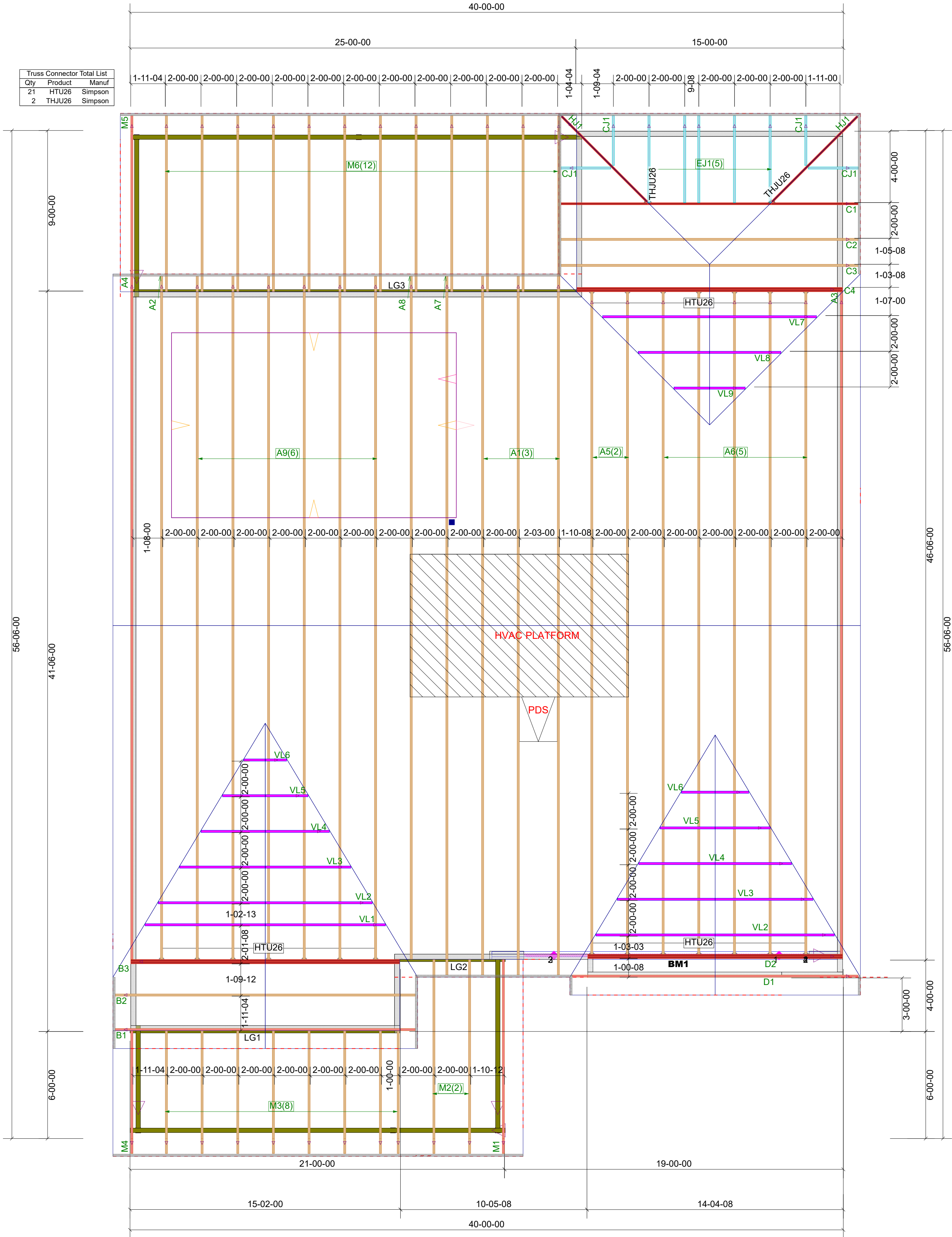


Truss Drawing Left
End Indicator

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.



ROOF PLACEMENT PLAN

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

** All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53179



DR Horton Inc
27 Eagle Creek - Lawson - E
Roof Truss Layout

Scale:	NTS
Date:	5/29/2025
Designer:	Nate Donaldson
Project Number:	25050207-B
Sheet Number:	

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: 25050207-B
27 Eagle Creek-Lawson E

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I73831351 thru I73831386

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844

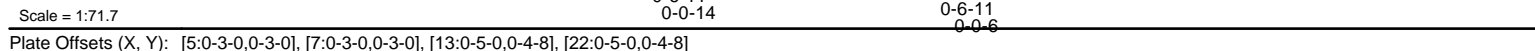


May 30, 2025

Gilbert, Eric

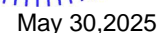
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:27 Page: 1
ID:dtULuPuUMCrLTxvpVp5HPazNtOu-RfC?PsB70Hg3NSqPanL8w3uTXbGKWRCdoi7J4zJC?f



LUMBER		3) Wind: ASCE 7-16; Vult=130mph (3-second gust)
TOP CHORD	2x4 SP No.2	Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
BOT CHORD	2x6 SP 2400F 2.0E *Except* 21-15:2x4 SP 2400F 2.0E	II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-10-13, Interior (1) 2-10-13 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-6, Interior (1) 22-5-6 to 38-3-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
WEBS	2x4 SP No.3 *Except* 22-6,13-6:2x4 SP No.2	3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
SLIDER	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0	4) Unbalanced snow loads have been considered for this design.
BRACING		5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
TOP CHORD	Structural wood sheathing directly applied.	6) 200.0lb AC unit load placed on the bottom chord, 18-8-8 from left end, supported at two points, 5-0-0 apart.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	7) All plates are 2x4 MT20 unless otherwise indicated.
REACTIONS		8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
	(size) 2=0-3-8, 10=0-3-8	9) All bearings are assumed to be SP 2400F 2.0E .
	Max Horiz 2=100 (LC 14)	
	Max Grav 2=1997 (LC 3), 10=1997 (LC 3)	
FORCES		LOAD CASE(S) Standard
	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/28, 2-4=-3641/0, 4-6=-3590/17, 6-8=-3590/17, 8-10=-3641/0, 10-11=0/28	
BOT CHORD	2-23=0/3182, 20-23=0/2948, 18-20=0/2133, 14-18=0/2133, 12-14=0/2948, 10-12=0/3183, 19-21=-12/81, 17-19=-12/81, 16-17=-12/81, 15-16=-12/81	
WEBS	4-23=-257/130, 5-23=-132/281, 5-22=-530/221, 21-22=0/1398, 6-21=0/1425, 6-15=0/1425, 13-15=0/1398, 7-13=-530/221, 7-12=-132/281, 8-12=-257/130, 17-18=-120/0, 19-20=-76/1, 14-16=-76/1	

1) Unbalanced roof live loads have been considered for this design.



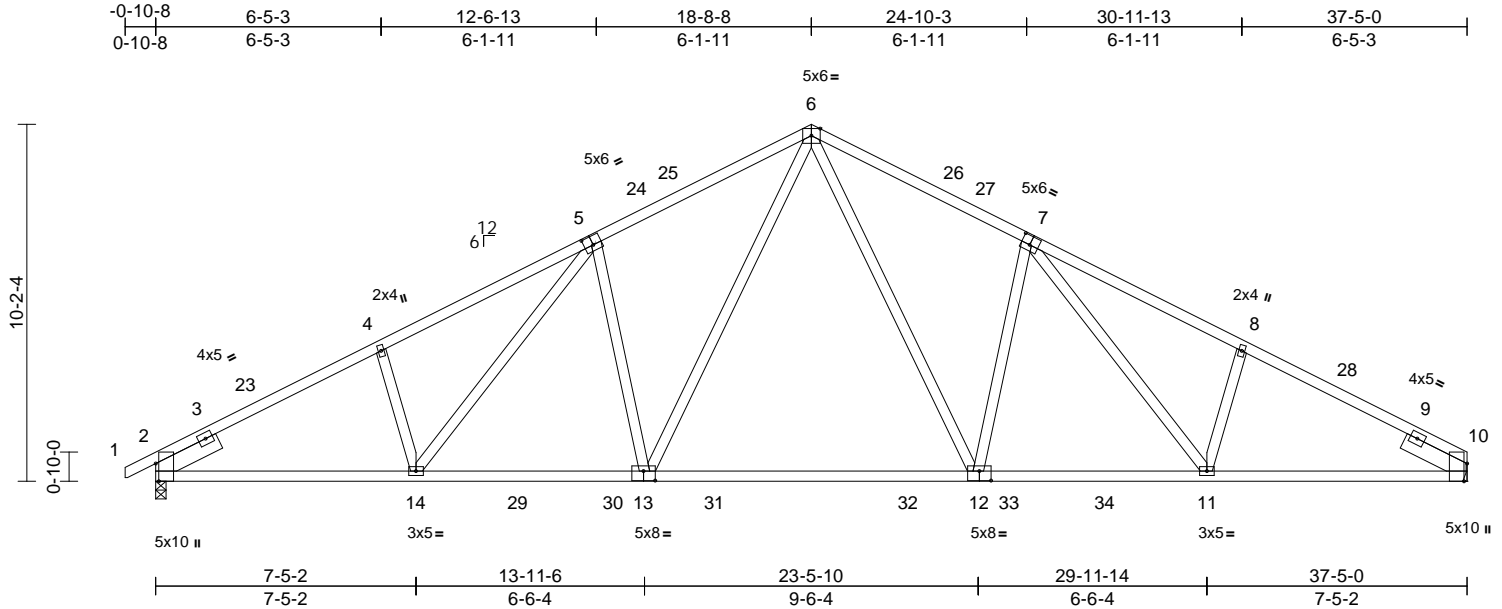
Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831352
25050207-B	A2	Common	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:28

Page: 1

ID:y7z5FdprXYkARFqqtKjy0BzNt6w-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?i



Scale = 1:65.7

Plate Offsets (X, Y): [2:0-6-1,Edge], [5:0-3-0,0-3-0], [7:0-3-0,0-3-0], [10:0-6-1,Edge], [12:0-4-0,0-3-4], [13:0-4-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.92	Vert(LL)	-0.32	12-13	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.56	Vert(CT)	-0.55	12-13	>813	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(CT)	0.11	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 216 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP 2400F 2.0E
 WEBS 2x4 SP No.3 *Except* 13-6,12-6:2x4 SP No.2
 SLIDER Left 2x6 SP 2400F 2.0E -- 2-0-0, Right 2x6 SP 2400F 2.0E -- 2-0-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-0-2 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 10= Mechanical
 Max Horiz 2=101 (LC 12)
 Max Grav 2=1715 (LC 3), 10=1672 (LC 3)

FORCES

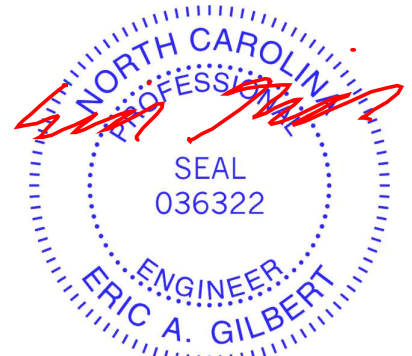
(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/28, 2-4=-3082/243, 4-6=-2998/323, 6-8=-3002/325, 8-10=-3085/250
 BOT CHORD 2-14=-146/2654, 11-14=-88/2367, 10-11=-151/2658
 WEBS 4-14=-228/122, 5-13=-602/188, 5-14=-57/392, 8-11=-230/127, 6-13=-90/1102, 7-12=-603/188, 6-12=-91/1103, 7-11=-64/397

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-10-13, Interior (1) 2-10-13 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-6, Interior (1) 22-5-6 to 37-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP 2400F 2.0E .
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard



May 30,2025

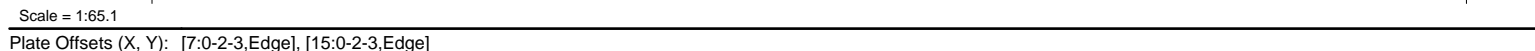
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
 A MiTek Affiliate

818 Soundside Road
 Edenton, NC 27932

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:29 Page: 1



LUMBER		FORCES	(lb) - Maximum Compression/Maximum Tension	3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
TOP CHORD	2x4 SP No.2	TOP CHORD	1-42=90/25, 1-2=102/68, 2-3=89/65, 3-4=80/86, 4-5=76/132, 5-6=87/177, 6-8=99/222, 8-9=111/267, 9-10=128/315, 10-11=143/354, 11-12=143/354, 12-13=128/315, 13-14=111/267, 14-16=99/222, 16-17=87/177, 17-18=76/132, 18-19=63/86, 19-20=63/44, 20-21=79/32, 21-22=72/11	4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
BOT CHORD	2x4 SP No.2			5) Unbalanced snow loads have been considered for this design.
WEBS	2x4 SP No.3			6) All plates are 2x4 MT20 unless otherwise indicated.
OTHERS	2x4 SP No.3			7) Gable requires continuous bottom chord bearing.
BRACING		BOT CHORD	41-42=33/69, 40-41=33/69, 39-40=33/69, 38-39=33/69, 37-38=33/69, 36-37=33/69, 34-36=33/69, 33-34=33/69, 32-33=33/69, 31-32=33/69, 30-31=33/69, 28-30=33/69, 27-28=33/69, 26-27=33/69, 25-26=33/69, 24-25=33/69, 23-24=33/69, 22-23=33/69, 11-32=242/64, 10-33=165/64, 9-34=140/84, 8-36=126/76, 6-37=127/78, 5-38=126/77, 4-39=129/80, 3-40=115/68, 2-41=168/168, 12-31=165/64, 13-30=140/84, 14-28=126/76, 16-27=127/78, 17-26=126/77, 18-25=128/79, 19-24=120/77, 20-23=152/162	8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.			9) Gable studs spaced at 2-0-0 oc.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.			10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
WEBS	1 Row at midpt 11-32, 10-33, 12-31	WEBS		11) All bearings are assumed to be SP No.2 .
REACTIONS (size)	22=37-1-8, 23=37-1-8, 24=37-1-8, 25=37-1-8, 26=37-1-8, 27=37-1-8, 28=37-1-8, 30=37-1-8, 31=37-1-8, 32=37-1-8, 33=37-1-8, 34=37-1-8, 36=37-1-8, 37=37-1-8, 38=37-1-8, 39=37-1-8, 40=37-1-8, 41=37-1-8, 42=37-1-8			
	Max Horiz 42=114 (LC 14)			
Max Uplift	23=52 (LC 16), 24=5 (LC 16), 25=18 (LC 16), 26=15 (LC 16), 27=16 (LC 16), 28=15 (LC 16), 30=19 (LC 16), 31=9 (LC 16), 33=10 (LC 15), 34=19 (LC 15), 36=15 (LC 15), 37=16 (LC 15), 38=15 (LC 15), 39=19 (LC 15), 40=3 (LC 15), 41=55 (LC 15), 42=19 (LC 11)			
	Max Grav 22=99 (LC 32), 23=189 (LC 39), 24=153 (LC 2), 25=162 (LC 39), 26=160 (LC 2), 27=160 (LC 39), 28=160 (LC 2), 30=180 (LC 22), 31=205 (LC 22), 32=160 (LC 35), 33=205 (LC 21), 34=180 (LC 21), 36=160 (LC 2), 37=160 (LC 38), 38=159 (LC 2), 39=164 (LC 38), 40=145 (LC 2), 41=210 (LC 38), 42=121 (LC 33)			

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

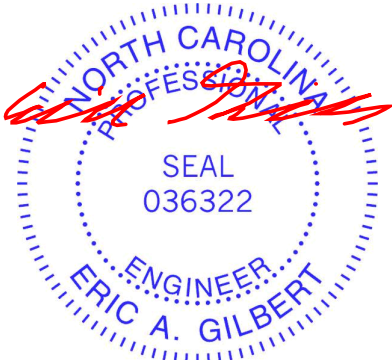
Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbccomponents.com).

ENGINEERING BY
TRENCO
A Mitek Affiliat

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E
25050207-B	A3	Common Supported Gable	1	1	173831353
					Job Reference (optional)

- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 42, 10 lb uplift at joint 33, 19 lb uplift at joint 34, 15 lb uplift at joint 36, 16 lb uplift at joint 37, 15 lb uplift at joint 38, 19 lb uplift at joint 39, 3 lb uplift at joint 40, 55 lb uplift at joint 41, 9 lb uplift at joint 31, 19 lb uplift at joint 30, 15 lb uplift at joint 28, 16 lb uplift at joint 27, 15 lb uplift at joint 26, 18 lb uplift at joint 25, 5 lb uplift at joint 24 and 52 lb uplift at joint 23.
- LOAD CASE(S)** Standard



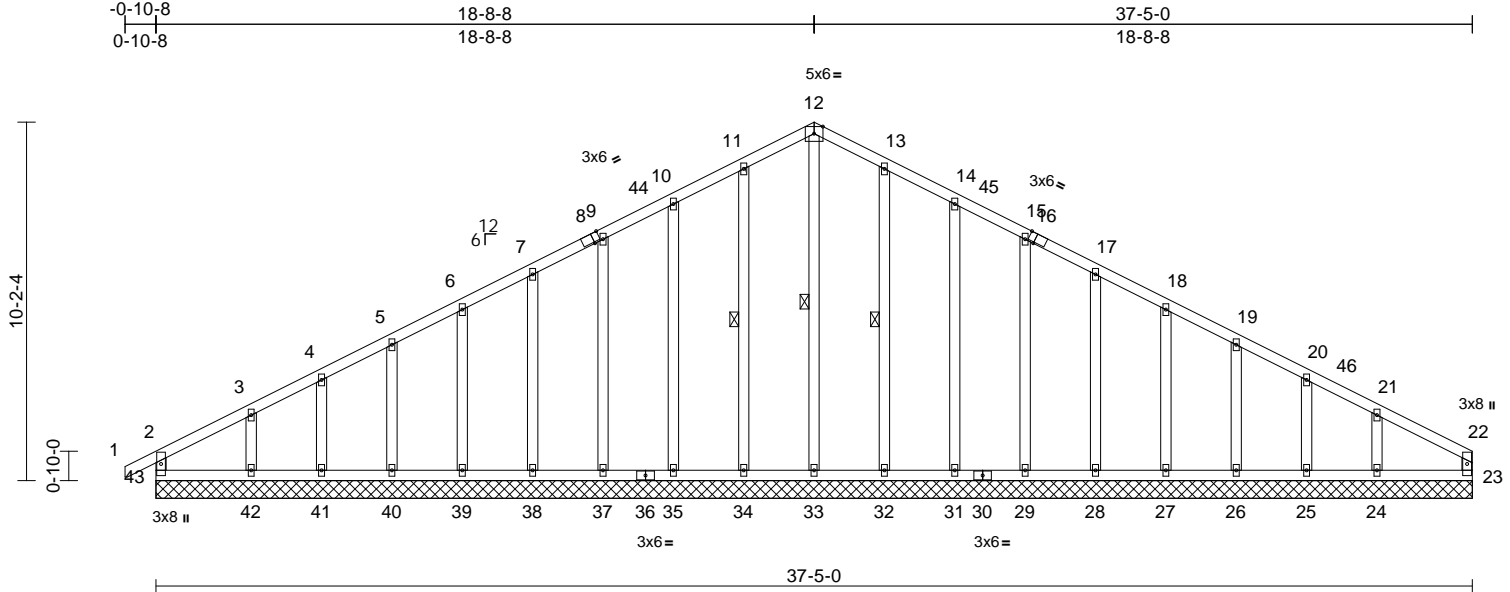
May 30,2025

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831354
25050207-B	A4	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:29
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Page: 1



Scale = 1:65.5

Plate Offsets (X, Y): [8:0-2-3,Edge], [16:0-2-3,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.11	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.06	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(CT)	0.00	23	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
Weight: 257 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS 1 Row at midpt 12-33, 11-34, 13-32

REACTIONS (size) 23=37-5-0, 24=37-5-0, 25=37-5-0, 26=37-5-0, 27=37-5-0, 28=37-5-0, 29=37-5-0, 31=37-5-0, 32=37-5-0, 33=37-5-0, 34=37-5-0, 35=37-5-0, 37=37-5-0, 38=37-5-0, 39=37-5-0, 40=37-5-0, 41=37-5-0, 42=37-5-0, 43=37-5-0

Max Horiz 43=117 (LC 14)

Max Uplift 24=48 (LC 16), 25=5 (LC 16), 26=18 (LC 16), 27=15 (LC 16), 28=16 (LC 16), 29=15 (LC 16), 31=19 (LC 16), 32=9 (LC 16), 34=11 (LC 15), 35=19 (LC 15), 37=15 (LC 15), 38=16 (LC 15), 39=15 (LC 15), 40=19 (LC 15), 41=3 (LC 15), 42=56 (LC 15), 43=25 (LC 11)

Max Grav 23=102 (LC 33), 24=210 (LC 40), 25=145 (LC 2), 26=164 (LC 40), 27=159 (LC 2), 28=160 (LC 40), 29=160 (LC 2), 31=180 (LC 23), 32=205 (LC 23), 33=160 (LC 36), 34=205 (LC 22), 35=180 (LC 22), 37=160 (LC 2), 38=160 (LC 39), 39=160 (LC 2), 40=162 (LC 39), 41=151 (LC 2), 42=191 (LC 33), 43=172 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-43=-149/84, 1-2=0/33, 2-3=-105/67, 3-4=-86/62, 4-5=-80/77, 5-6=-73/122, 6-7=-84/167, 7-9=-96/212, 9-10=-108/257, 10-11=-126/305, 11-12=-143/345, 12-13=-143/345, 13-14=-126/305, 14-15=-108/257, 15-17=-96/212, 17-18=-84/167, 18-19=-72/122, 19-20=-59/76, 20-21=-64/36, 21-22=-81/36, 22-23=-77/16
BOT CHORD 42-43=-30/79, 41-42=-30/79, 40-41=-30/79, 39-40=-30/79, 38-39=-30/79, 37-38=-30/79, 35-37=-30/79, 34-35=-30/79, 33-34=-30/79, 32-33=-30/79, 31-32=-30/79, 29-31=-30/79, 28-29=-30/79, 27-28=-30/79, 26-27=-30/79, 25-26=-30/79, 24-25=-30/79, 23-24=-30/79
WEBS 12-33=-234/64, 11-34=-165/64, 10-35=-140/84, 9-37=-126/76, 7-38=-127/78, 6-39=-126/77, 5-40=-129/81, 4-41=-120/67, 3-42=-157/141, 13-32=-165/64, 14-31=-140/84, 15-29=-126/76, 17-28=-127/78, 18-27=-126/77, 19-26=-129/80, 20-25=-115/68, 21-24=-167/170

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-1 to 2-8-8, Exterior(2N) 2-8-8 to 18-8-8, Corner(3R) 18-8-8 to 22-8-8, Exterior(2N) 22-8-8 to 37-3-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.



May 30, 2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E
25050207-B	A4	Common Supported Gable	1	1	Job Reference (optional)

I73831354

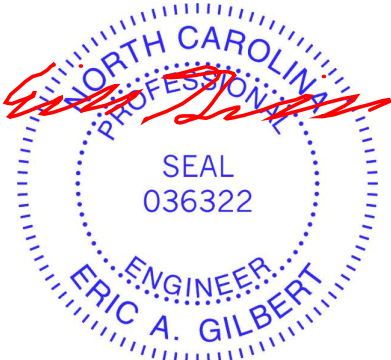
Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:29
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Page: 2

- 12) All bearings are assumed to be SP No.2 .
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 43, 11 lb uplift at joint 34, 19 lb uplift at joint 35, 15 lb uplift at joint 37, 16 lb uplift at joint 38, 15 lb uplift at joint 39, 19 lb uplift at joint 40, 3 lb uplift at joint 41, 56 lb uplift at joint 42, 9 lb uplift at joint 32, 19 lb uplift at joint 31, 15 lb uplift at joint 29, 16 lb uplift at joint 28, 15 lb uplift at joint 27, 18 lb uplift at joint 26, 5 lb uplift at joint 25 and 48 lb uplift at joint 24.

LOAD CASE(S) Standard



May 30,2025

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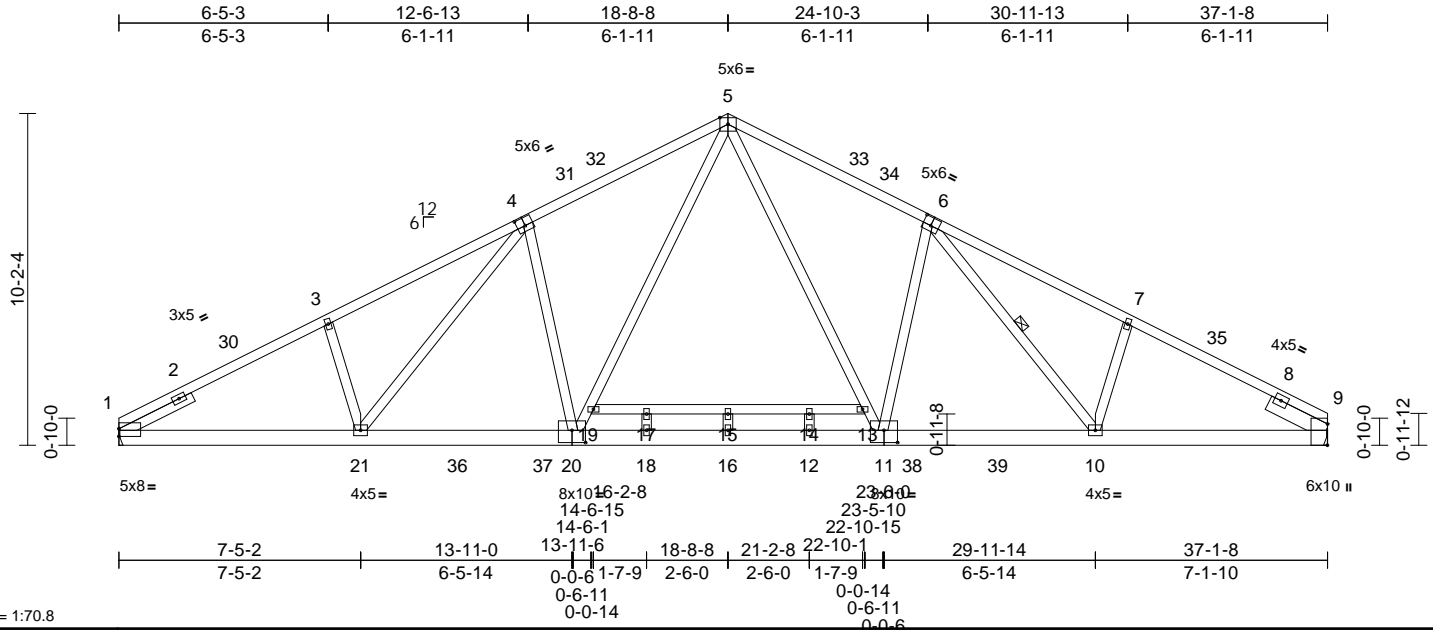
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831355
25050207-B	A5	Common	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:70.8

Plate Offsets (X, Y): [1:Edge,0-2-13], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [9:0-7-13,Edge], [11:0-5-0,0-4-8], [20:0-5-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.98	Vert(LL)	-0.24	15	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.49	15	>918	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(CT)	0.08	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 257 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E *Except* 19-13:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 20-5,11-5:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x6 SP No.2 -- 2-0-0

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-10

REACTIONS

(size) 1= Mechanical, 9= Mechanical
Max Horiz 1=97 (LC 12)
Max Grav 1=1940 (LC 3), 9=1945 (LC 3)

FORCES

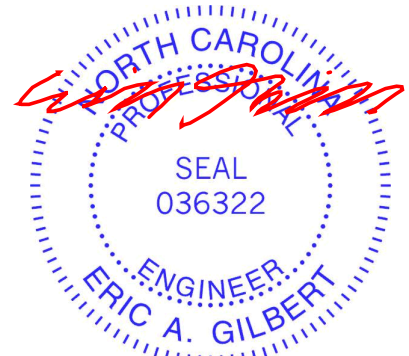
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-3613/0, 3-5=-3562/27, 5-7=-3418/29, 7-9=-3479/0
BOT CHORD 1-21=-11/3158, 18-21=0/2920, 16-18=0/2098, 12-16=0/2098, 10-12=0/2889, 9-10=0/3024, 17-19=-8/87, 15-17=-8/87, 14-15=-8/87, 13-14=-8/87
WEBS 3-21=-259/133, 4-20=-532/222, 4-21=-142/286, 7-10=-238/130, 19-20=0/1413, 5-19=0/1425, 6-11=-472/224, 5-13=0/1387, 11-13=0/1378, 15-16=-131/0, 17-18=-87/0, 6-10=-144/177, 12-14=-91/0

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-8-9, Interior (1) 3-8-9 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-1, Interior (1) 22-5-1 to 37-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 200.0lb AC unit load placed on the bottom chord, 18-8-8 from left end, supported at two points, 5-0-0 apart.
- All plates are 2x4 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard



May 30,2025

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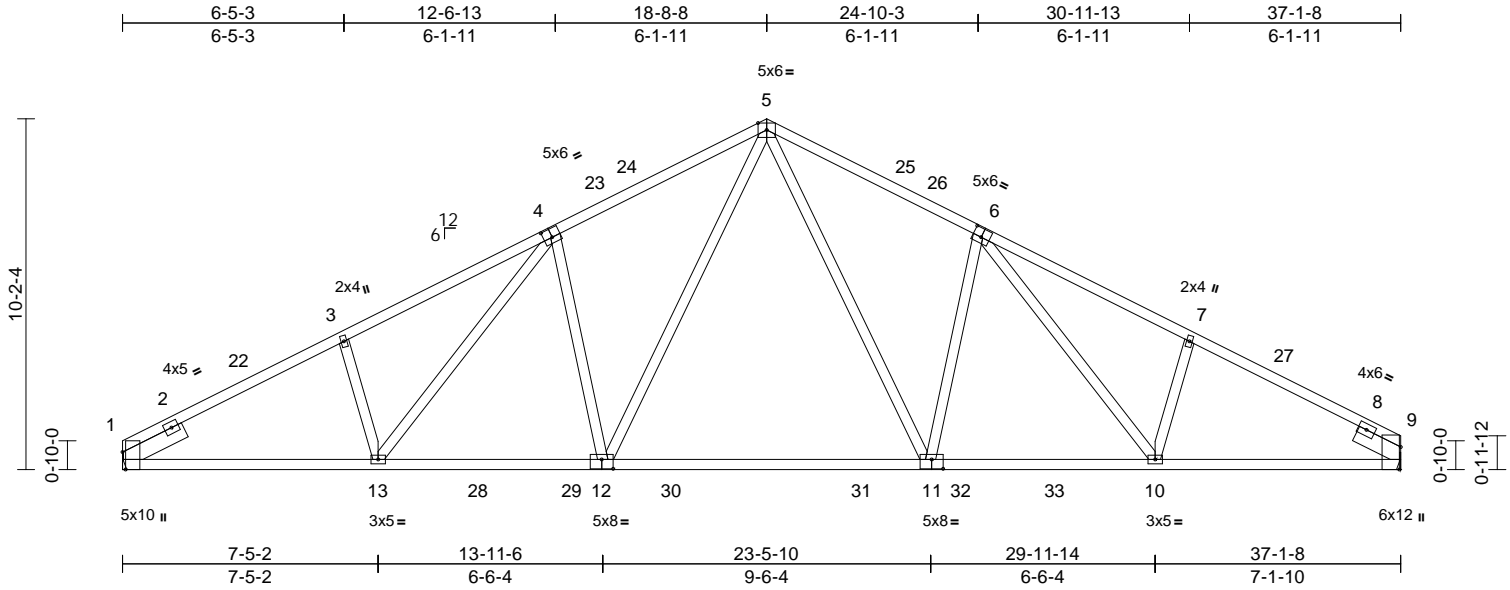
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831356
25050207-B	A6	Common	5	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:29
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Page: 1



Scale = 1:66.9

Plate Offsets (X, Y): [1:0-6-1,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [9:0-7-13,Edge], [11:0-4-0,0-3-4], [12:0-4-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.80	Vert(LL)	-0.31	11-12	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.69	Vert(CT)	-0.54	11-12	>825	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(CT)	0.14	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 212 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 12-5,11-5:2x4 SP No.2
SLIDER Left 2x6 SP 2400F 2.0E -- 2-0-0, Right 2x6 SP 2400F 2.0E -- 1-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1= Mechanical, 9= Mechanical
Max Horiz 1=97 (LC 12)
Max Grav 1=1659 (LC 3), 9=1661 (LC 3)

FORCES

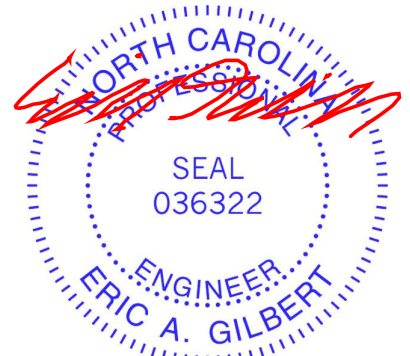
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-3061/248, 3-5=-2975/323,
5-7=-2846/322, 7-9=-2948/240
BOT CHORD 1-13=-149/2635, 10-13=-91/2343,
9-10=-143/2508
WEBS 3-13=-226/127, 4-12=-608/188,
4-13=-64/402, 7-10=-182/123,
5-12=-90/1102, 6-11=-560/185,
5-11=-88/1070, 6-10=-55/264

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-8-9, Interior (1) 3-8-9 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-1, Interior (1) 22-5-1 to 37-1-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard



May 30, 2025

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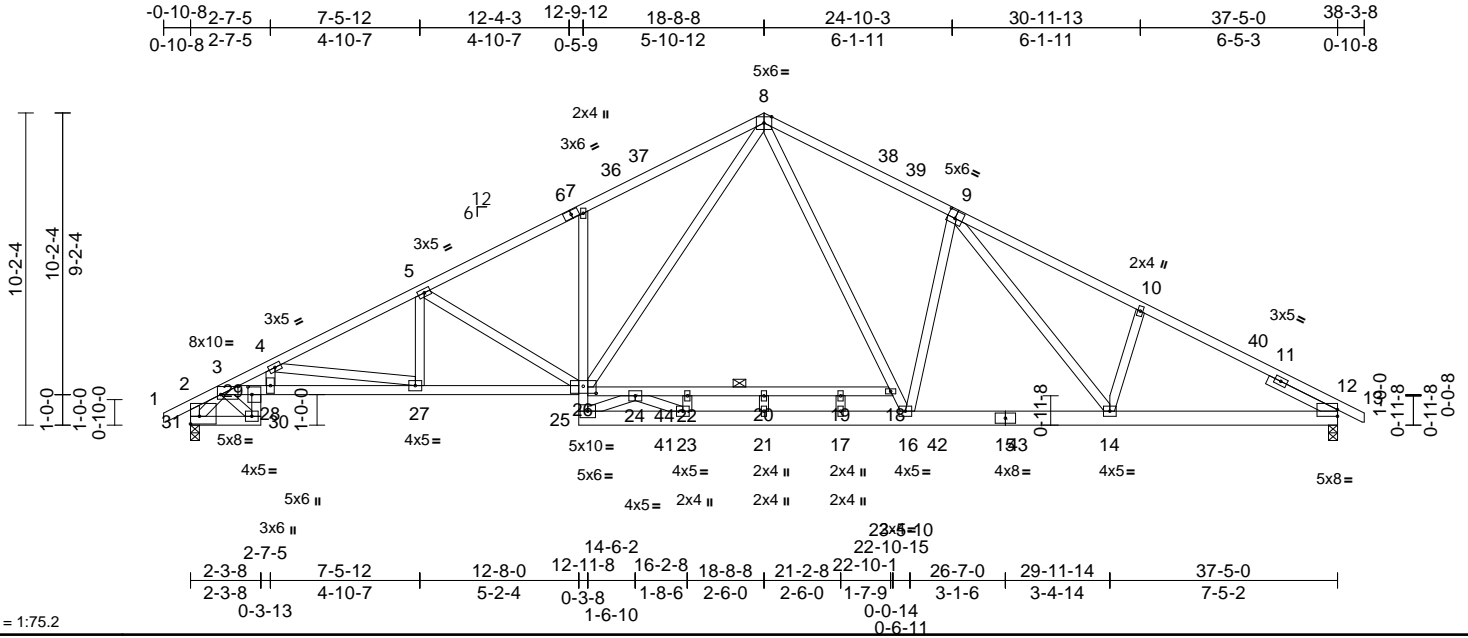
Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831357
25050207-B	A7	Roof Special	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Nov 16 2023 Print: 8.730 S Nov 16 2023 MiTek Industries, Inc. Fri May 30 08:03:26

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Scale = 1:75.2

Plate Offsets (X, Y): [2:Edge,0-3-0], [3:0-6-12,0-3-3], [9:0-3-0,0-3-0], [12:Edge,0-2-9], [26:0-5-0,0-2-12], [29:0-3-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.42	Vert(LL)	-0.23	20-22	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.83	Vert(CT)	-0.47	20-22	>942	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.22	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 263 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
 BOT CHORD 2x4 SP 2400F 2.0E *Except* 30-29,26-18:2x4 SP No.2, 7-25:2x4 SP No.3, 25-15,15-12:2x6 SP 2400F 2.0E
 WEBS 2x4 SP No.3 *Except* 26-8,16-8:2x4 SP No.2
 SLIDER Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
 6-0-0 oc bracing: 22-24,20-22,19-20,18-19.

REACTIONS (lb/size) 12=1499/0-3-8, 31=1516/0-3-8
 Max Horiz 31=112 (LC 13)
 Max Grav 12=2007 (LC 3), 31=2035 (LC 3)

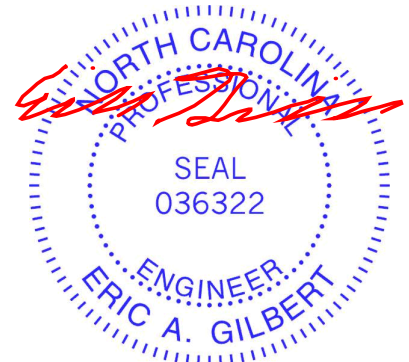
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-496/14, 3-4=-6451/0, 4-5=-4683/0, 5-6=-3833/0, 6-7=-3715/0, 7-36=-3871/0, 36-37=-3821/0, 8-37=-3802/0, 8-38=-3248/0, 38-39=-3268/0, 9-39=-3325/0, 9-10=-3622/7, 10-40=-3655/0, 11-40=-3675/0, 11-12=-1684/0, 2-31=-624/65
 BOT CHORD 30-31=0/1768, 29-30=0/1799, 3-29=0/5536, 28-29=0/5774, 27-28=0/5774, 26-27=0/4147, 25-26=0/609, 7-26=-355/164, 25-41=0/1134, 23-41=0/1134, 21-23=0/2273, 17-21=0/2273, 16-17=0/2273, 16-42=0/2951, 15-42=0/2951, 15-43=0/2951, 14-43=0/2951, 12-14=0/3210, 24-26=0/2145
 WEBS 5-26=-967/69, 8-26=0/1935, 8-18=0/1322, 16-18=0/1269, 9-16=-550/228, 9-14=-147/321, 10-14=-252/131, 5-27=0/540, 4-27=-1655/32, 3-30=-2270/0, 3-31=-2190/0, 24-25=-1155/0, 23-24=0/1282, 4-28=0/875

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-7-5, Interior (1) 2-7-5 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-6, Interior (1) 22-5-6 to 38-3-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 18-8-8 from left end, supported at two points, 5-0-0 apart.
- All plates are 2x4 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

LOAD CASE(S) Standard



May 30,2025

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818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831358
25050207-B	A8	Roof Special	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Nov 16 2023 Print: 8.730 S Nov 16 2023 MiTek Industries, Inc. Fri May 30 08:06:27

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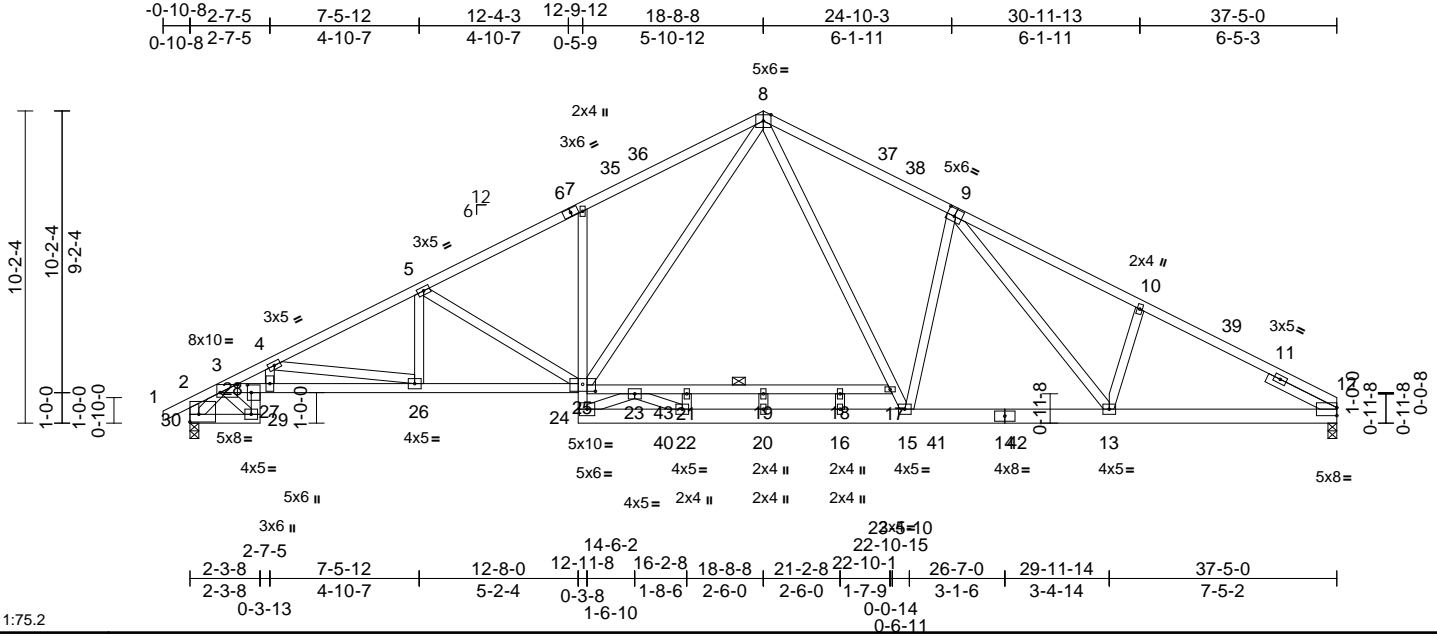


Plate Offsets (X, Y): [2:Edge,0-3-0], [3:0-6-12,0-3-3], [9:0-3-0,0-3-0], [12:Edge,0-3-1], [25:0-5-0,0-2-12], [28:0-3-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.42	Vert(LL)	-0.23	19-21	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.83	Vert(CT)	-0.47	19-21	>942	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.22	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 262 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
 BOT CHORD 2x4 SP 2400F 2.0E *Except* 29-28,25-17:2x4
 SP No.2, 7-24:2x4 SP No.3, 14-12,14-24:2x6
 SP 2400F 2.0E
 WEBS 2x4 SP No.3 *Except* 25-8,15-8:2x4 SP No.2
 SLIDER Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or
 3-0-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
 bracing, Except:
 6-0-0 oc bracing: 21-23,19-21,18-19,17-18.

REACTIONS (lb/size) 12=1458/0-3-8, 30=1517/0-3-8
 Max Horiz 30=110 (LC 12)
 Max Grav 12=1965 (LC 3), 30=2035 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250
 (lb) or less except when shown.

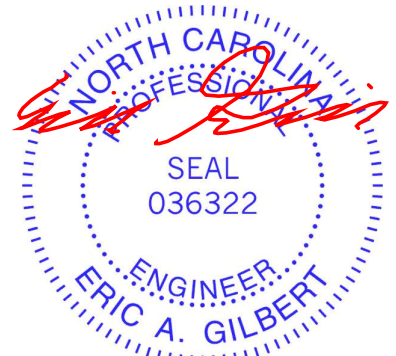
TOP CHORD 2-3=-497/15, 3-4=-6456/0, 4-5=-4685/0,
 5-6=-3835/0, 6-7=-3717/0, 7-35=-3872/0,
 35-36=-3823/0, 8-36=-3804/0, 8-37=-3250/0,
 37-38=-3270/0, 9-38=-3327/0,
 9-10=-3627/16, 10-39=-3619/0,
 11-39=-3680/0, 11-12=-1707/0, 2-30=-624/66
 BOT CHORD 29-30=0/1771, 28-29=0/1802, 3-28=0/5543,
 27-28=0/5781, 26-27=0/5781, 25-26=0/4147,
 24-25=0/609, 7-25=-355/164, 24-40=0/1133,
 22-40=0/1133, 20-22=0/2272, 16-20=0/2272,
 15-16=0/2272, 15-41=0/2951, 14-41=0/2951,
 14-42=0/2951, 13-42=0/2951, 12-13=0/3212,
 23-25=0/2144

WEBS 5-26=0/541, 9-15=-551/228, 10-13=-253/134,
 5-25=-968/71, 4-26=-1658/49, 3-29=-2274/0,
 3-30=-2190/0, 23-24=-1154/0, 22-23=0/1281,
 8-25=0/1935, 8-17=0/1323, 15-17=0/1270,
 9-13=-155/325, 4-27=0/876

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-7-5, Interior (1) 2-7-5 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-6, Interior (1) 22-5-6 to 37-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 18-8-8 from left end, supported at two points, 5-0-0 apart.
- All plates are 2x4 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

LOAD CASE(S) Standard



May 30,2025

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818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831359
25050207-B	A9	Roof Special	6	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Nov 16 2023 Print: 8.730 S Nov 16 2023 MiTek Industries, Inc. Fri May 30 08:07:46

Page: 1

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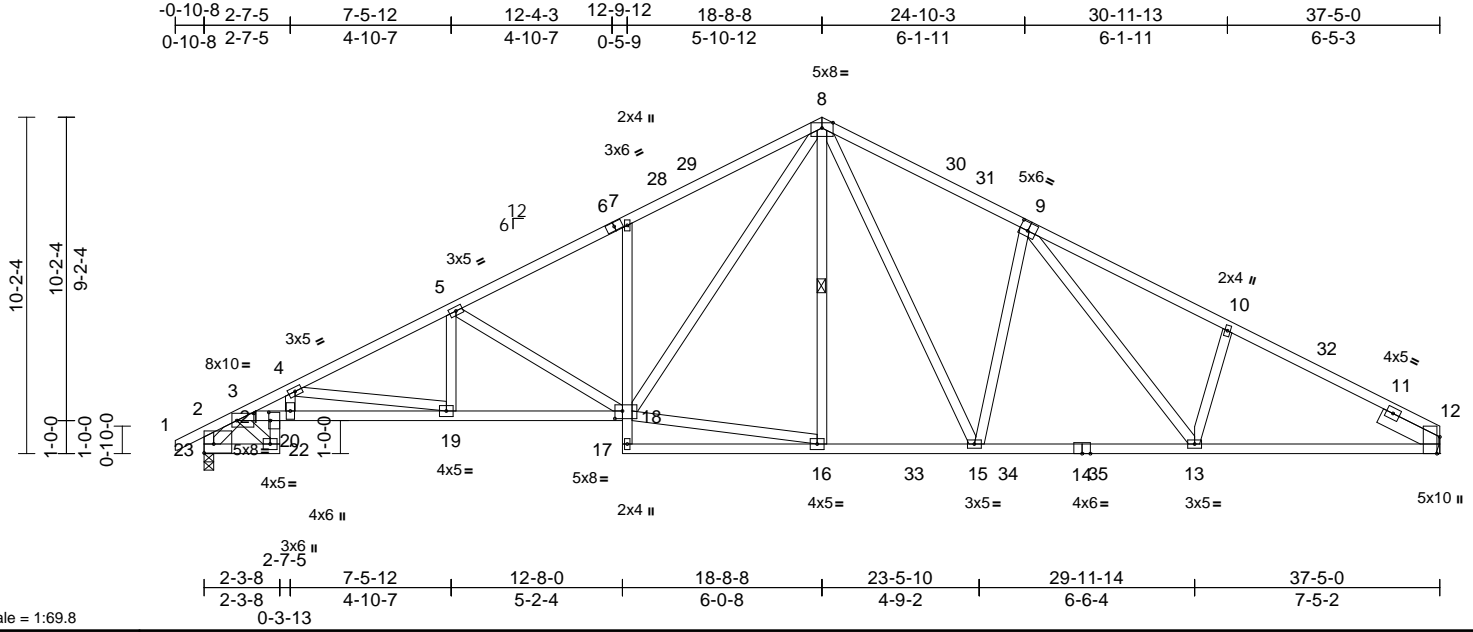


Plate Offsets (X, Y): [2:Edge,0-3-4], [3:0-6-4,0-2-11], [9:0-3-0,0-3-0], [12:0-6-1,Edge], [18:0-2-12,0-2-12], [21:0-3-0,0-0-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.54	Vert(LL)	-0.20	13-15	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.67	Vert(CT)	-0.37	13-15	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(CT)	0.23	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 243 lb FT = 20%											

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E *Except* 22-21:2x4 SP
No.2, 7-17:2x4 SP No.3
WEBS 2x4 SP No.3 *Except* 18-8,15-8:2x4 SP No.2
SLIDER Right 2x6 SP 2400F 2.0E -- 2-0-0

WEBS
5-19=0/501, 5-18=-914/85, 4-19=-1466/141,
3-22=-1886/132, 3-23=-1818/112,
8-18=-176/1519, 16-18=-3/1592,
8-15=-124/972, 9-15=-631/182,
9-13=-51/456, 4-20=0/740

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-7-5, Interior (1) 2-7-5 to 18-8-8, Exterior(2R) 18-8-8 to 22-5-6, Interior (1) 22-5-6 to 37-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 8-16

REACTIONS (lb/size) 12=1261/ Mechanical, 23=1310/0-3-8
Max Horiz 23=110 (LC 12)
Max Grav 12=1634 (LC 3), 23=1680 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-418/52, 3-4=-5353/347, 4-5=-3798/268, 5-6=-2992/269, 6-7=-2874/271, 7-28=-3020/345, 28-29=-2971/354, 8-29=-2951/374, 8-30=-2482/330, 30-31=-2503/310, 9-31=-2560/300, 9-10=-2940/304, 10-32=-2976/247, 11-32=-3026/231, 11-12=-632/0, 2-23=-537/106
BOT CHORD 22-23=-101/1470, 21-22=-91/1501, 3-21=-295/4597, 20-21=-305/4795, 19-20=-305/4795, 18-19=-166/3350, 7-18=-356/165, 16-33=0/1716, 15-33=0/1716, 15-34=-81/2278, 14-34=-81/2278, 14-35=-81/2278, 13-35=-81/2278, 12-13=-146/2601



May 30,2025

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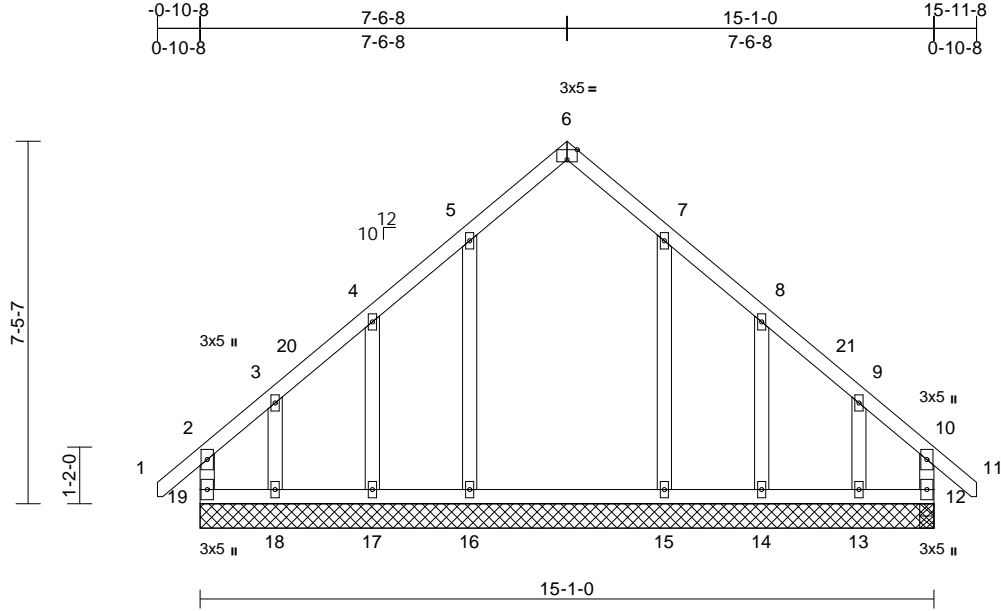
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831360
25050207-B	B1	Common Structural Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:30
ID:EecNkO4bMOFDvs4XnKFX?sy7GRi-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:47.4

Plate Offsets (X, Y): [6:0-2-8, Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.29	Vert(LL)	-0.01	15-16	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.17	Vert(CT)	-0.02	15-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
Weight: 90 lb											FT = 20%	

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)	12=15-1-0, 13=15-1-0, 14=15-1-0, 15=15-1-0, 16=15-1-0, 17=15-1-0, 18=15-1-0, 19=15-1-0
Max Horiz	19=157 (LC 11)
Max Uplift	12=24 (LC 12), 13=127 (LC 14), 14=49 (LC 14), 17=48 (LC 13), 18=129 (LC 13), 19=30 (LC 11)
Max Grav	12=221 (LC 29), 13=191 (LC 30), 14=159 (LC 30), 15=316 (LC 30), 16=319 (LC 29), 17=157 (LC 29), 18=194 (LC 29), 19=226 (LC 30)

FORCES

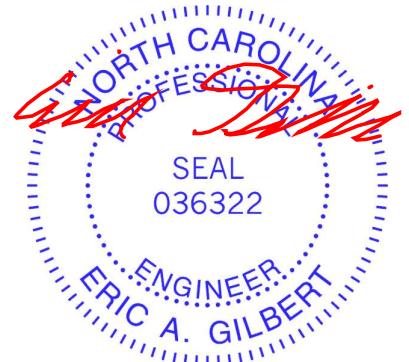
	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/47, 2-3=-204/83, 3-4=-110/47, 4-5=-105/55, 5-6=-131/102, 6-7=-131/102, 7-8=-101/54, 8-9=-107/42, 9-10=-205/82, 10-11=0/47, 2-19=-174/39, 10-12=-171/34
BOT CHORD	18-19=-72/215, 17-18=-72/215, 16-17=-72/215, 15-16=-72/215, 14-15=-72/215, 13-14=-72/215, 12-13=-72/215
WEBS	5-16=-159/49, 4-17=-160/157, 3-18=-139/185, 7-15=-159/49, 8-14=-160/157, 9-13=-139/185

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-9-14 to 2-2-2, Exterior(2N) 2-2-2 to 7-6-8, Corner (3R) 7-6-8 to 10-6-8, Exterior(2N) 10-6-8 to 15-10-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 19, 24 lb uplift at joint 12, 48 lb uplift at joint 17, 129 lb uplift at joint 18, 49 lb uplift at joint 14 and 127 lb uplift at joint 13.

LOAD CASE(S) Standard



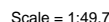
May 30, 2025

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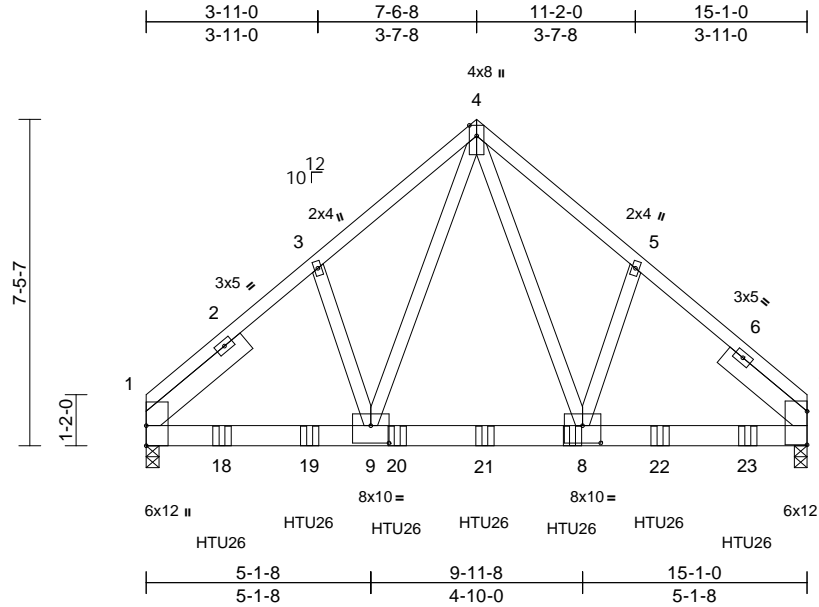
818 Soundside Road
Edenton, NC 27932

Job 25050207-B	Truss B3	Truss Type Common Girder	Qty 1	Ply 2	27 Eagle Creek-Lawson E Job Reference (optional)	I73831362
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:31
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Page: 1



Scale = 1:52.6

Plate Offsets (X, Y): [8:0-5-0,0-4-12], [9:0-5-0,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.79	Vert(LL)	-0.08	8-9	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.48	Vert(CT)	-0.15	8-9	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.76	Horz(CT)	0.03	7	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 221 lb FT = 20%											

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP 2400F 2.0E
WEBS	2x4 SP No.3
SLIDER	Left 2x6 SP 2400F 2.0E -- 3-0-0, Right 2x6 SP 2400F 2.0E -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-7-13 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size) 1=0-3-8, 7=0-3-8
	Max Horiz 1=118 (LC 31)
	Max Grav 1=6046 (LC 3), 7=6347 (LC 3)

FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-3=-6213/0, 3-4=-6061/0, 4-5=-6096/0, 5-7=-6242/0
BOT CHORD	1-9=0/4639, 8-9=0/3424, 7-8=0/4630
WEBS	4-8=0/3984, 5-8=0/279, 4-9=0/3907, 3-9=0/326

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.

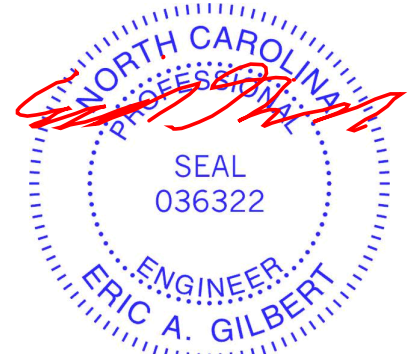
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP 2400F 2.0E .
- Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-8-12 from the left end to 13-8-12 to connect truss(es) to front face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-48, 4-7=-48, 10-14=-20

Concentrated Loads (lb)

Vert: 8=-1241 (F), 18=-1246 (F), 19=-1241 (F), 20=-1241 (F), 21=-1241 (F), 22=-1241 (F), 23=-1241 (F)



May 30, 2025

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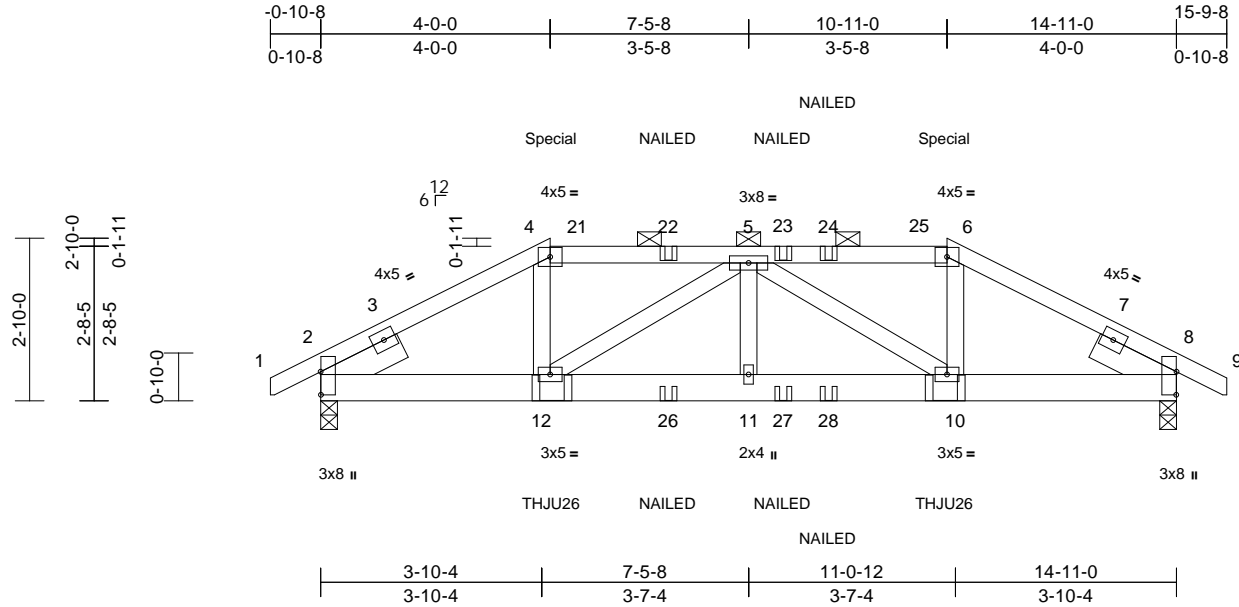
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831363
25050207-B	C1	Hip Girder	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:31
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Page: 1



Scale = 1:40.2

Plate Offsets (X, Y): [2:0-4-13,0-0-1], [8:0-4-13,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	-0.04	11	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.25	BC	0.20	Vert(CT)	-0.07	10-11	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(CT)	0.01	8	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 89 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP 2400F 2.0E -- 1-6-0, Right 2x6 SP 2400F 2.0E -- 1-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-6-11 oc purlins, except 2-0-0 oc purlins (4-11-13 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 8=0-3-8
Max Horiz 2=-24 (LC 9)
Max Uplift 2=-39 (LC 11), 8=-41 (LC 12)
Max Grav 2=1058 (LC 36), 8=1068 (LC 36)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-4=-1455/72, 4-5=-1239/78, 5-6=-1260/82, 6-8=-1474/75, 8-9=0/32
BOT CHORD 2-12=-41/1255, 11-12=-72/1779, 10-11=-72/1779, 8-10=-26/1274
WEBS 4-12=0/304, 5-12=-665/54, 5-11=0/226, 5-10=-651/52, 6-10=0/299

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0

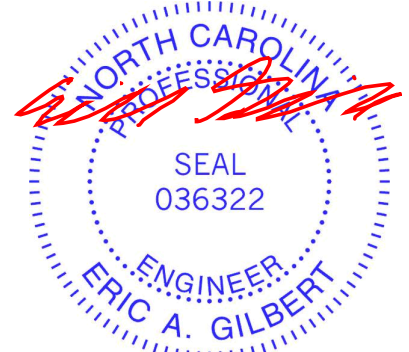
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP 2400F 2.0E .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 2 and 41 lb uplift at joint 8.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Use Simpson Strong-Tie THJU26 (SGL & SGL SHORT RC 1-PLY) or equivalent at 4-0-6 from the left end to connect truss(es) to back face of bottom chord.
- Use Simpson Strong-Tie THJU26 (SGL & SGL SHORT LC 1-PLY) or equivalent at 10-10-10 from the left end to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 226 lb down and 78 lb up at 4-0-0, and 226 lb down and 78 lb up at 10-11-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)

- Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-48, 4-6=-58, 6-9=-48, 13-17=-20

Concentrated Loads (lb)

Vert: 4=-168 (B), 6=-168 (B), 12=-61 (B), 10=-61 (B), 22=-69 (B), 23=-69 (B), 24=-69 (B), 26=-25 (B), 27=-25 (B), 28=-25 (B)



May 30, 2025

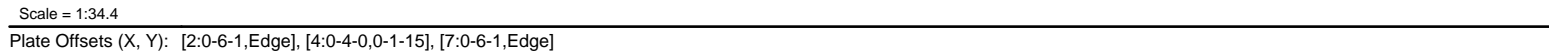
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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818 Soundside Road
Edenton, NC 27932

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:31 Page: 1
ID:irAlxk5D7hO4W0fjK1mmY4y7GRh-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWCrD0i7J4zJC?f



LUMBER		3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
WEBS	2x4 SP No.3	
SLIDER	Left 2x6 SP No.2 -- 1-6-14, Right 2x6 SP No.2 -- 1-6-14	4) Unbalanced snow loads have been considered for this design.
BRACING		5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
TOP CHORD	Structural wood sheathing directly applied or 5-6-9 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.	6) Provide adequate drainage to prevent water ponding.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
REACTIONS		8) All bearings are assumed to be SP No.2 .
	(size) 2=0-3-8, 7=0-3-8	9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
	Max Horiz 2=-34 (LC 13)	
	Max Grav 2=768 (LC 44), 7=768 (LC 44)	
FORCES		
	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/32, 2-4=-868/185, 4-5=-717/210, 5-7=-869/185, 7-8=0/32	LOAD CASE(S) Standard
BOT CHORD	2-10=-144/697, 9-10=-76/693, 7-9=-147/697	
WEBS	4-10=0/113, 4-9=-103/104, 5-9=0/143	

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C
 Exterior(2E) 0-10-1 to 2-1-15, Interior (1) 2-1-15 to 6-0-0, Exterior(2E) 6-0-0 to 8-11-0, Exterior(2R) 8-11-0 to 13-1-15, Interior (1) 13-1-15 to 15-9-1 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
 DOL=1.33

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 .
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 30, 2025

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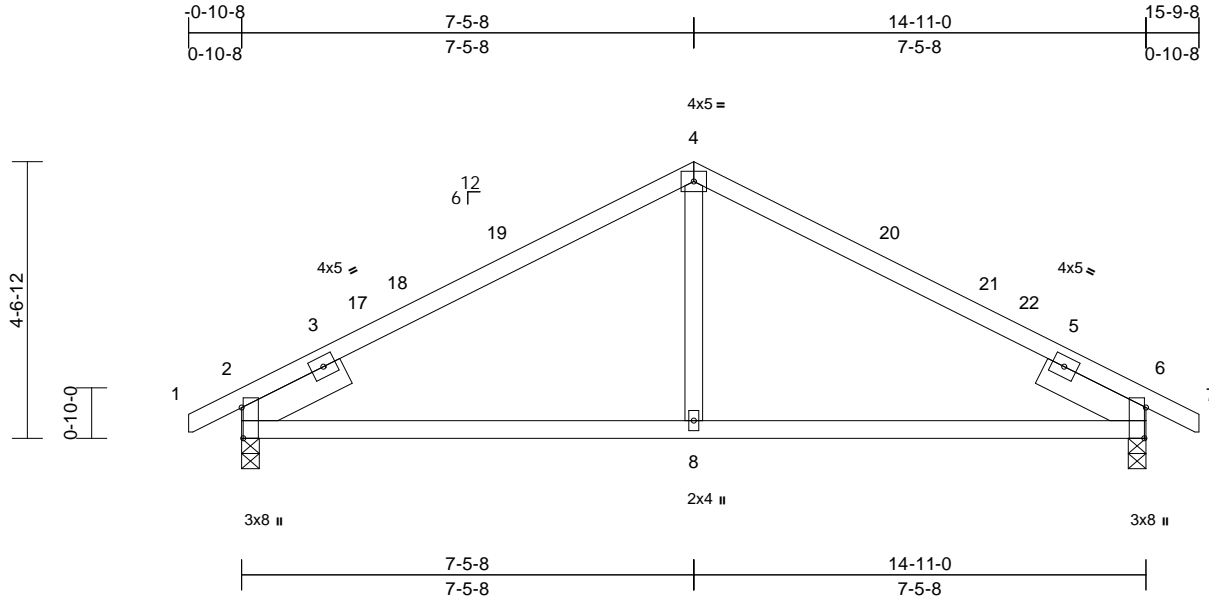
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831365
25050207-B	C3	Common	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:31
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Page: 1



Scale = 1:38

Plate Offsets (X, Y): [2:0-6-1,Edge], [6:0-6-1,Edge]

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.53	Vert(LL)	-0.08	8-11	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.34	Vert(CT)	-0.12	8-11	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.03	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 65 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP No.2 -- 1-10-14, Right 2x6 SP No.2 -- 1-10-14

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-8, 6=0-3-8
Max Horiz 2=-41 (LC 13)
Max Grav 2=627 (LC 2), 6=627 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/27, 2-4=-724/207, 4-6=-725/207, 6-7=0/27
BOT CHORD 2-8=-201/574, 6-8=-202/574
WEBS 4-8=0/203

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-1-15, Interior (1) 2-1-15 to 7-5-8, Exterior(2R) 7-5-8 to 10-5-8, Interior (1) 10-5-8 to 15-9-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.1.

LOAD CASE(S) Standard



May 30, 2025

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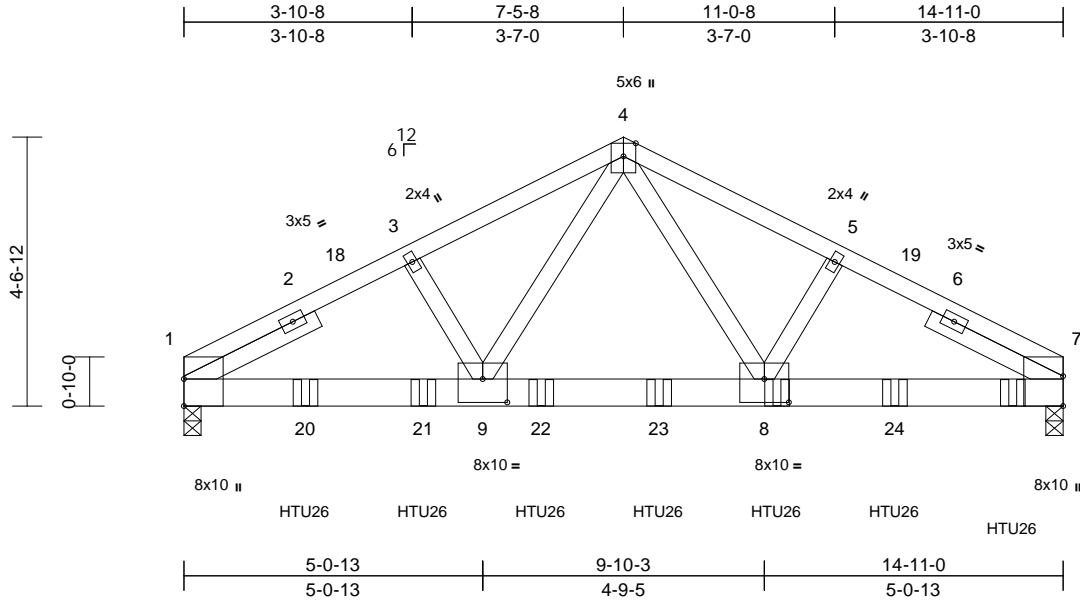
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831366
25050207-B	C4	Common Girder	1	2	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:31
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Page: 1



Scale = 1:39.1									
Plate Offsets (X, Y): [8:0-5-0,0-4-12], [9:0-5-0,0-4-12]									
Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.72	Vert(LL)	-0.12	8-9	>999
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.54	Vert(CT)	-0.22	8-9	>824
TCDL	10.0	Rep Stress Incr	NO	WB	0.89	Horz(CT)	0.03	7	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH					
BCDL	10.0								
Weight: 175 lb FT = 20%									

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=0-3-8, 7=0-3-8
Max Horiz 1=37 (LC 35)
Max Grav 1=5836 (LC 3), 7=7218 (LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-3=-8747/0, 3-4=-8717/0, 4-5=-8861/0, 5-7=-8892/0
BOT CHORD 1-9=0/7687, 8-9=0/5719, 7-8=0/7830
WEBS 4-8=0/4458, 5-8=0/298, 4-9=0/4204, 3-9=0/331

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 3 rows staggered at 0-8-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed;
Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP 2400F 2.0E .
- Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 10-0-12 to connect truss(es) to front face of bottom chord.
- Use Simpson Strong-Tie HTU26 (20-10d Girder, 14-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 12-0-12 from the left end to 14-0-12 to connect truss(es) to front face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-46, 4-7=-46, 10-14=-19
Concentrated Loads (lb)
Vert: 8=-1238 (F), 16=-1424 (F), 20=-1238 (F), 21=-1238 (F), 22=-1238 (F), 23=-1238 (F), 24=-1422 (F)



May 30, 2025

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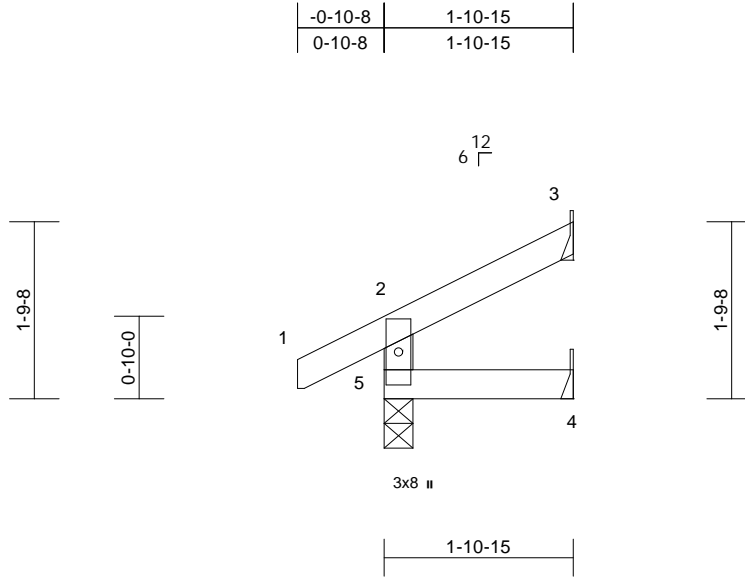
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	
25050207-B	CJ1	Jack-Open	4	1	Job Reference (optional)	I73831367

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:31
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Page: 1



Scale = 1:23.3

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.10	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.05	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 8 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 1-10-15 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size)	3= Mechanical, 4= Mechanical, 5=0-3-8
	Max Horiz	5=32 (LC 12)
	Max Uplift	3=18 (LC 15)
	Max Grav	3=44 (LC 22), 4=18 (LC 13), 5=165 (LC 22)

FORCES

(lb) - Maximum Compression/Maximum Tension

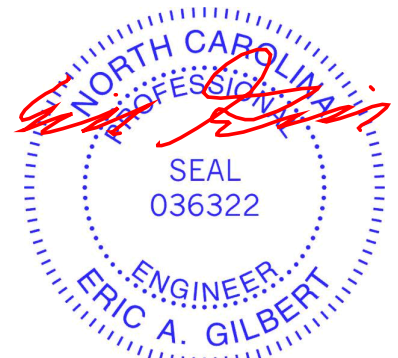
TOP CHORD	2-5=145/98, 1-2=0/33, 2-3=34/22
BOT CHORD	4-5=0/0

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: , Joint 5 SP No.2 .
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 3.

LOAD CASE(S) Standard



May 30, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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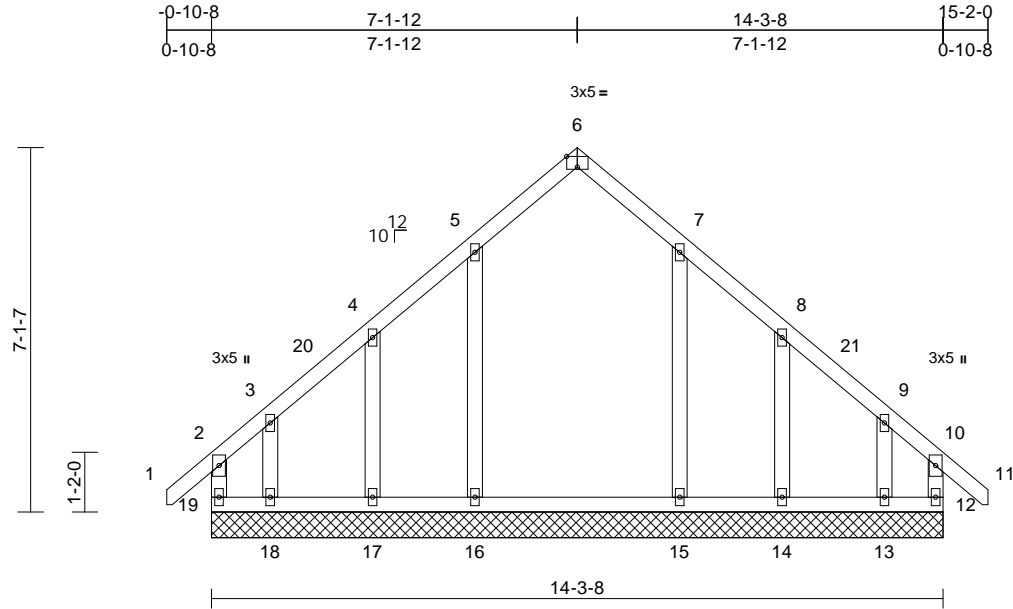
Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831368
25050207-B	D1	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32

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Scale = 1:45

Plate Offsets (X, Y): [6:0-2-8,Edge]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.27	Vert(LL)	n/a	-	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.16	Vert(CT)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	12	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
										Weight: 84 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size)	12=14-3-8, 13=14-3-8, 14=14-3-8, 15=14-3-8, 16=14-3-8, 17=14-3-8, 18=14-3-8, 19=14-3-8
	Max Horiz	19=146 (LC 11)
	Max Uplift	12=44 (LC 12), 13=141 (LC 14), 14=49 (LC 14), 17=49 (LC 13), 18=142 (LC 13), 19=50 (LC 11)
	Max Grav	12=220 (LC 29), 13=165 (LC 30), 14=157 (LC 30), 15=305 (LC 30), 16=308 (LC 29), 17=155 (LC 29), 18=168 (LC 29), 19=225 (LC 30)

FORCES

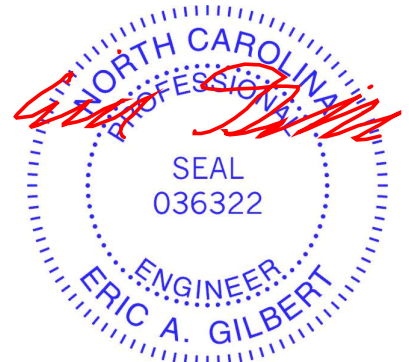
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-19=-172/54, 1-2=0/45, 2-3=-216/87, 3-4=-114/46, 4-5=-103/39, 5-6=-126/94, 6-7=-126/94, 7-8=-99/39, 8-9=-111/42, 9-10=-217/85, 10-11=0/45, 10-12=-169/53
BOT CHORD	18-19=-67/207, 17-18=-67/207, 16-17=-67/207, 15-16=-67/207, 14-15=-67/207, 13-14=-67/207, 12-13=-67/207
WEBS	5-16=-158/61, 4-17=-157/159, 3-18=-125/184, 7-15=-157/61, 8-14=-156/158, 9-13=-124/185

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-9-14 to 2-2-2, Exterior(2N) 2-2-2 to 7-1-12, Corner(3R) 7-1-12 to 10-1-12, Exterior(2N) 10-1-12 to 15-1-6 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 19, 44 lb uplift at joint 12, 49 lb uplift at joint 17, 142 lb uplift at joint 18, 49 lb uplift at joint 14 and 141 lb uplift at joint 13.

LOAD CASE(S) Standard



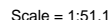
May 30,2025

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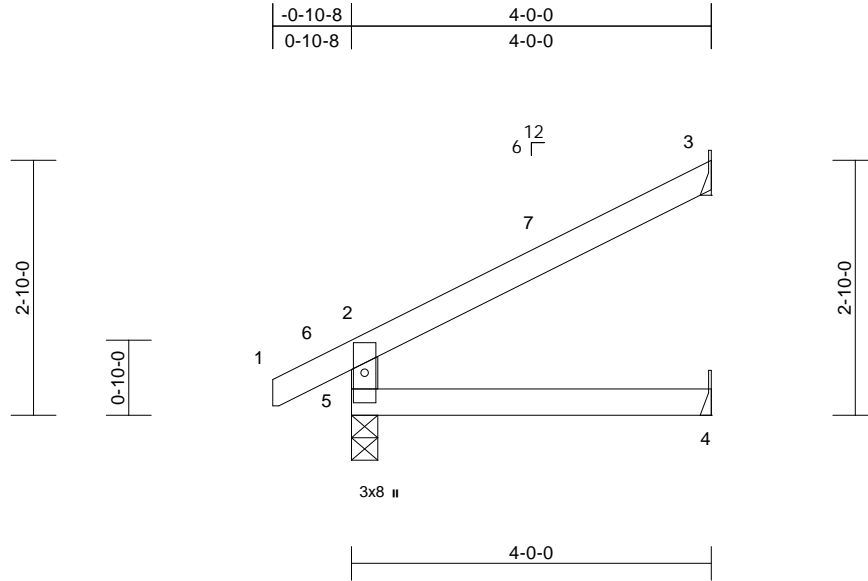
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831370
25050207-B	EJ1	Jack-Open	5	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32
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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.29	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.19	Vert(CT)	-0.02	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 15 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 3= Mechanical, 4= Mechanical,
5=0-3-8
Max Horiz 5=55 (LC 15)
Max Uplift 3=35 (LC 15)
Max Grav 3=127 (LC 22), 4=45 (LC 22),
5=266 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-5=-236/137, 1-2=0/33, 2-3=-75/50
BOT CHORD 4-5=0/0

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
II; Exp B; Enclosed; MWFRS (envelope) and C-C
Exterior(2E) -0-10-1 to 2-1-15, Interior (1) 2-1-15 to
3-11-4 zone; cantilever left and right exposed ; end
vertical left and right exposed;C-C for members and
forces & MWFRS for reactions shown; Lumber
DOL=1.60 plate grip DOL=1.33
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25
Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum
DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully
Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this
design.
4) This truss has been designed for greater of min roof live
load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on
overhangs non-concurrent with other live loads.

- 5) * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
6) Bearings are assumed to be: , Joint 5 SP No.2 .
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 35 lb uplift at joint
3.

LOAD CASE(S) Standard



May 30,2025

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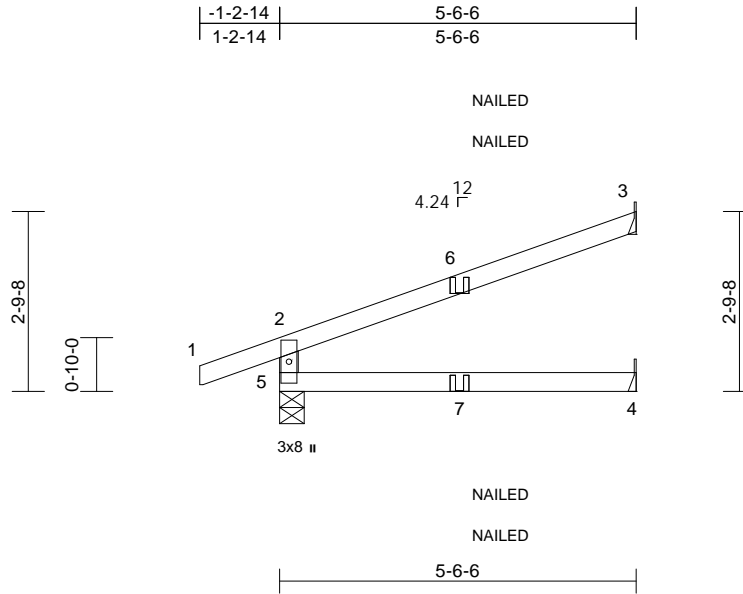
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831371
25050207-B	HJ1	Diagonal Hip Girder	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32
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Page: 1



Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.60	Vert(LL)	-0.03	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.31	Vert(CT)	-0.07	4-5	>891	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.03	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 20 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 3= Mechanical, 4= Mechanical,
5=0-4-9
Max Horiz 5=58 (LC 7)
Max Uplift 3=-37 (LC 11), 5=-28 (LC 7)
Max Grav 3=166 (LC 18), 4=63 (LC 18),
5=308 (LC 18)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-5=-268/68, 1-2=0/33, 2-3=-68/43
BOT CHORD 4-5=0/0

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- Bearings are assumed to be: , Joint 5 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 5 and 37 lb uplift at joint 3.
- "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidelines.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-46, 2-3=-46, 4-5=-19
Concentrated Loads (lb)
Vert: 7=0 (F=0, B=0)



May 30, 2025

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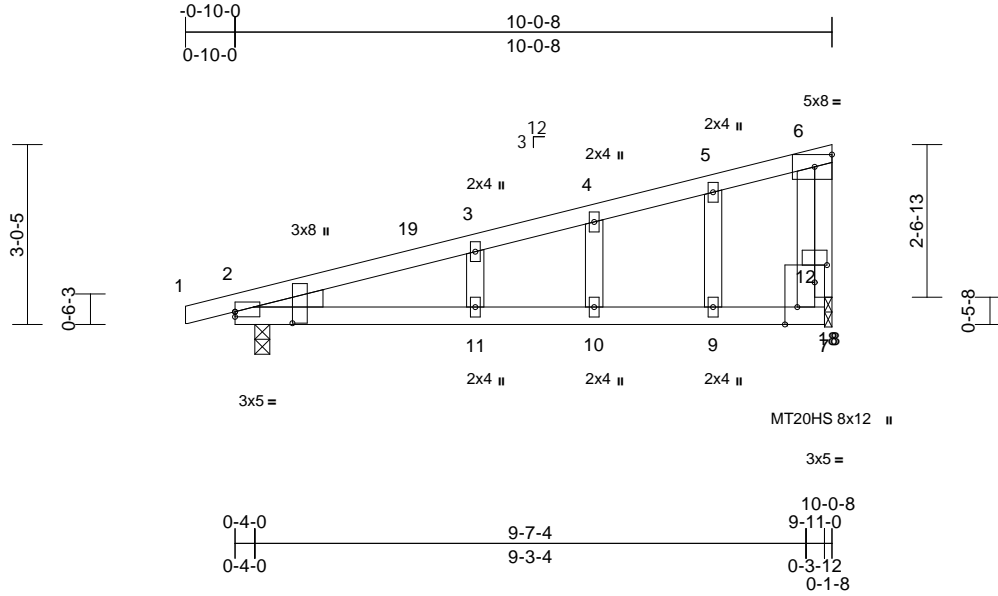
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831372
25050207-B	M1	Monopitch Structural Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32
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Page: 1



Scale = 1:38.8

Plate Offsets (X, Y): [2:Edge,0-1-0], [2:0-2-5,0-11-9], [8:0-3-8,Edge], [12:0-2-8,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.82	Vert(LL)	0.20	10-11	>589	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.39	Vert(CT)	-0.31	10-11	>385	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 47 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-0, 18=0-1-8
Max Horiz 2=64 (LC 11)
Max Uplift 2=-21 (LC 11), 18=-12 (LC 15)
Max Grav 2=465 (LC 2), 18=388 (LC 22)

FORCES

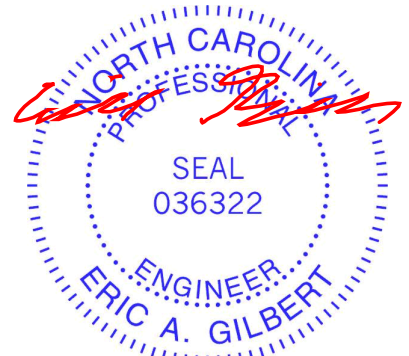
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/15, 2-3=-351/120, 3-4=-329/115, 4-5=-311/139, 5-6=-289/164, 8-12=-122/236, 6-12=-122/236
BOT CHORD 2-11=-224/325, 10-11=-203/325, 9-10=-203/325, 8-9=-203/325, 7-8=0/0
WEBS 5-9=-60/82, 4-10=-48/73, 3-11=-55/89, 6-18=-401/237

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-9-13 to 2-2-3, Exterior(2N) 2-2-3 to 9-7-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are MT20 plates unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP 2400F 2.0E, Joint 18 SP No.3.
- Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 18.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 2 and 12 lb uplift at joint 18.

LOAD CASE(S) Standard



May 30, 2025

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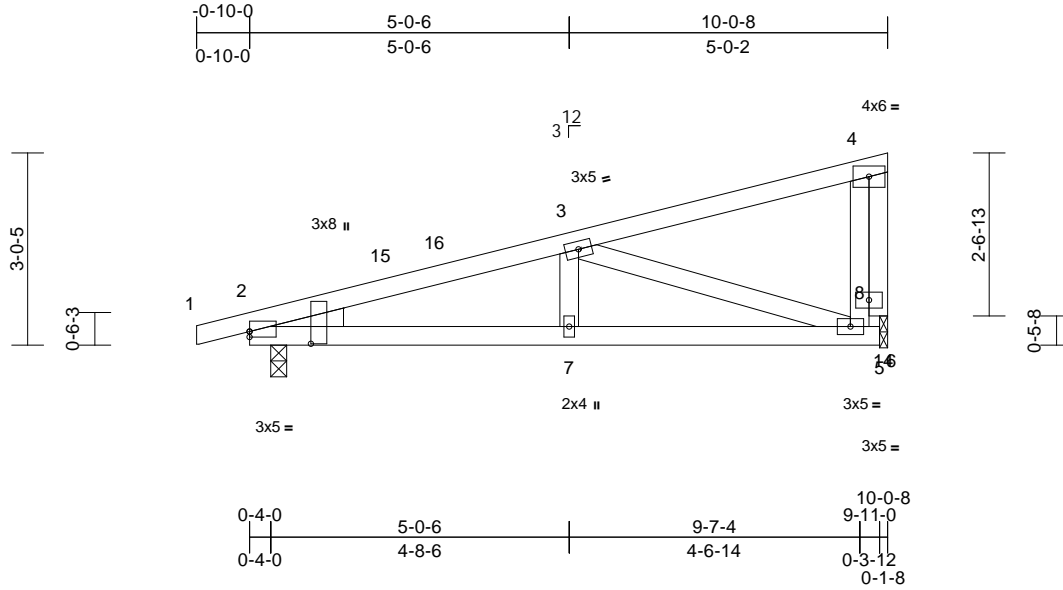
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831373
25050207-B	M2	Monopitch	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32
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Page: 1



Scale = 1:36.3

Plate Offsets (X, Y): [2:Edge,0-1-0], [2:0-2-5,0-11-9]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.27	Vert(LL)	-0.02	7	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.26	Vert(CT)	-0.04	6-7	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.30	Horz(CT)	0.01	14	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 49 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size) 2=0-3-0, 14=0-1-8
	Max Horiz 2=64 (LC 11)
	Max Uplift 2=-21 (LC 11), 14=-12 (LC 15)
	Max Grav 2=465 (LC 2), 14=388 (LC 22)

FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/15, 2-3=-763/190, 3-4=-140/10, 6-8=-49/269, 4-8=-49/269
BOT CHORD	2-7=-269/748, 6-7=-269/748, 5-6=0/0
WEBS	3-7=0/85, 3-6=-680/245, 4-14=-395/130

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-13 to 2-2-3, Interior (1) 2-2-3 to 9-7-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2, Joint 14 SP No.3.
- Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 14.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 2 and 12 lb uplift at joint 14.

LOAD CASE(S) Standard



May 30,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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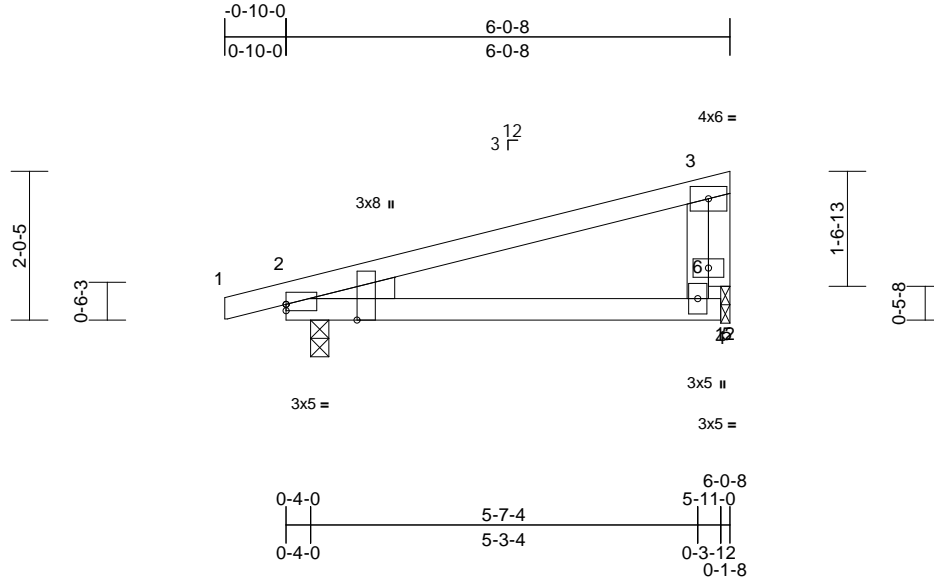
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831374
T25050207-B	M3	Monopitch	8	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32
ID:qLSfgAFN3h0Ea?8DbGVqZpy7GRU-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?fi

Page: 1



Scale = 1:31.4

Plate Offsets (X, Y): [2:Edge,0-1-0], [2:0-2-9,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	0.02	5-11	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.24	Vert(CT)	-0.03	5-11	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 25 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)

2=0-3-0, 12=0-1-8
Max Horiz 2=38 (LC 11)
Max Uplift 2=-23 (LC 11), 12=-5 (LC 15)
Max Grav 2=341 (LC 22), 12=215 (LC 22)

FORCES

(lb) - Maximum Compression/Maximum Tension

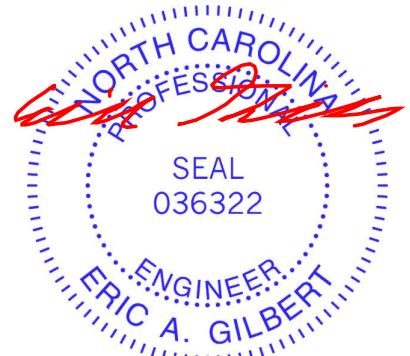
TOP CHORD	1-2=0/15, 2-3=-219/89, 5-6=0/90, 3-6=-51/116
BOT CHORD	2-5=-111/213, 4-5=0/0
WEBS	3-12=-109/9

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-13 to 2-2-3, Interior (1) 2-2-3 to 5-7-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: Joint 2 SP No.2, Joint 12 SP No.3.
- 7) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 12.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 2 and 5 lb uplift at joint 12.

LOAD CASE(S) Standard



May 30,2025

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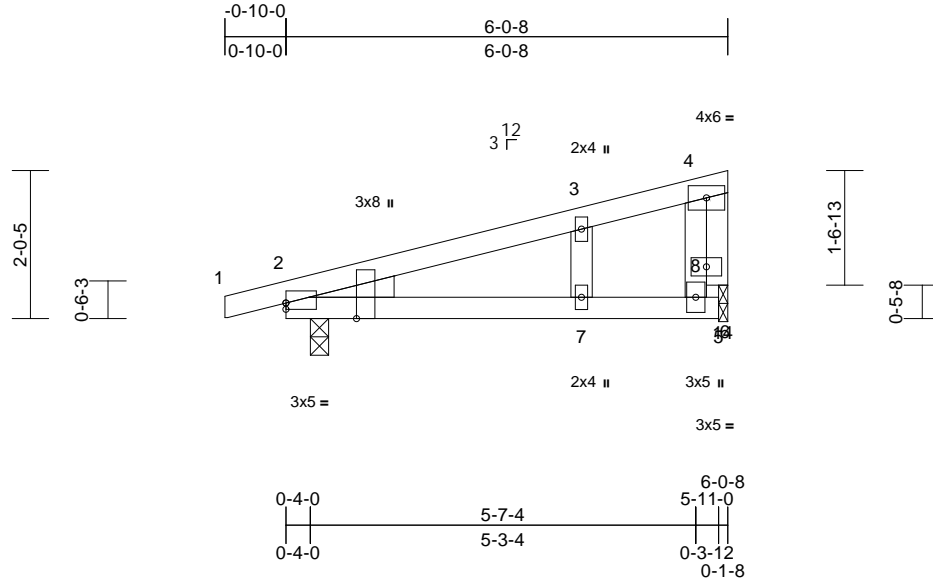
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831375
25050207-B	M4	Monopitch Structural Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32
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Page: 1



Scale = 1:31.5

Plate Offsets (X, Y): [2:Edge,0-1-0], [2:0-2-9,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.43	Vert(LL)	0.02	7-13	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.28	Vert(CT)	-0.03	7-13	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-0, 14=0-1-8
Max Horiz 2=38 (LC 11)
Max Uplift 2=-23 (LC 11), 14=-5 (LC 15)
Max Grav 2=341 (LC 22), 14=215 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension

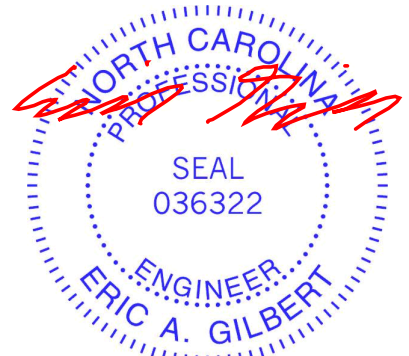
TOP CHORD 1-2=0/15, 2-3=-217/89, 3-4=-193/83,
6-8=-41/119, 4-8=0/21
BOT CHORD 2-7=-109/212, 6-7=-109/212, 5-6=0/0
WEBS 3-7=-46/72, 4-14=-150/72

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-13 to 2-2-3, Interior (1) 2-2-3 to 5-7-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2, Joint 14 SP No.3.
- Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 14.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 2 and 5 lb uplift at joint 14.

LOAD CASE(S) Standard



May 30,2025

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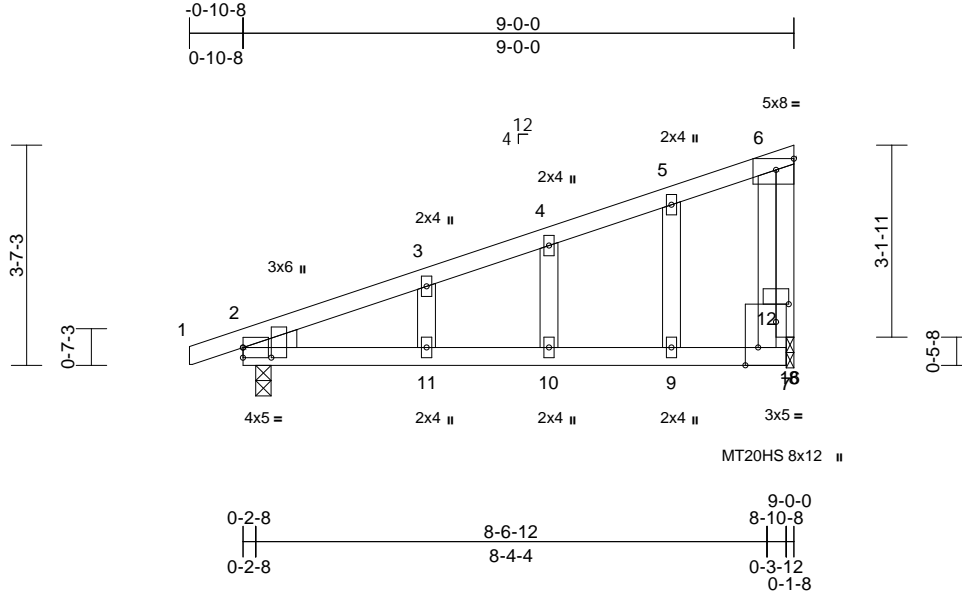
Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831376
25050207-B	M5	Monopitch Structural Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:32

Page: 1

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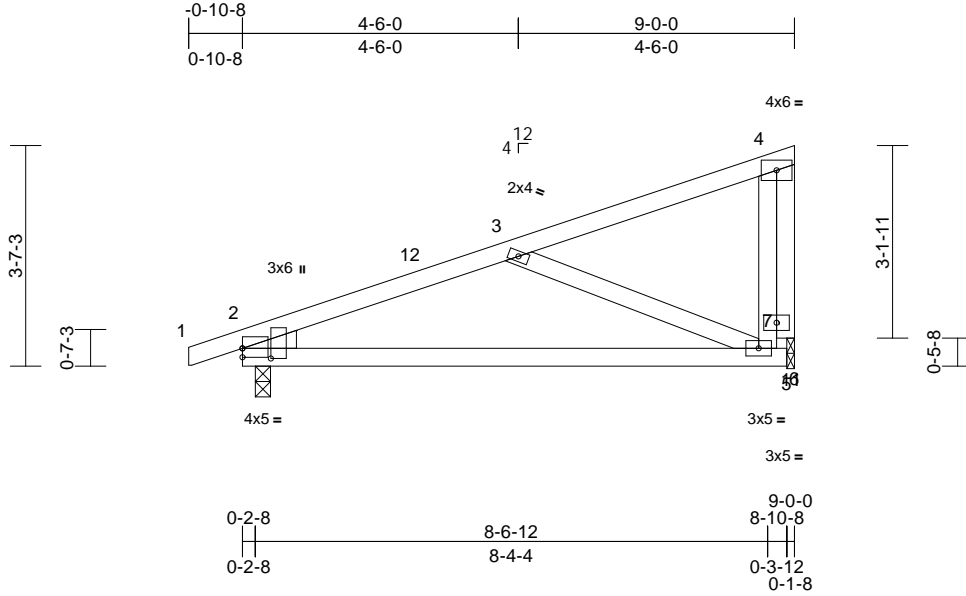


Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831377
25050207-B	M6	Monopitch	12	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33
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Page: 1



Scale = 1:37.6

Plate Offsets (X, Y): [2:Edge,0-1-12], [2:0-2-0,0-5-9]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.33	Vert(LL)	0.01	6-10	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.44	Vert(CT)	-0.14	6-10	>755	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(CT)	0.01	11	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 45 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(size) 2=0-3-0, 11=0-1-8
	Max Horiz 2=79 (LC 11)
	Max Uplift 2=-16 (LC 11), 11=-16 (LC 15)
	Max Grav 2=411 (LC 2), 11=368 (LC 22)

FORCES	(lb) - Maximum Compression/Maximum Tension
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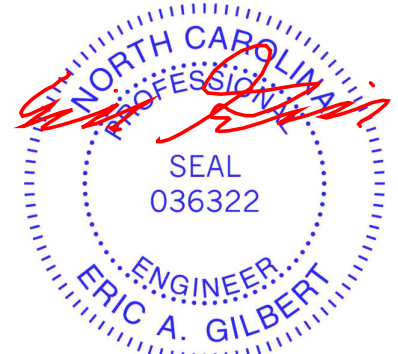
TOP CHORD	1-2=0/20, 2-3=-544/190, 3-4=-123/7, 6-7=-58/258, 4-7=-58/258
BOT CHORD	2-6=-290/532, 5-6=0/0
WEBS	3-6=-499/263, 4-11=-373/159

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-3 to 2-1-13, Interior (1) 2-1-13 to 8-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2, Joint 11 SP No.3.
- Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 2 and 16 lb uplift at joint 11.

LOAD CASE(S) Standard



May 30,2025

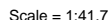
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33 Page: 1
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May 30, 2025

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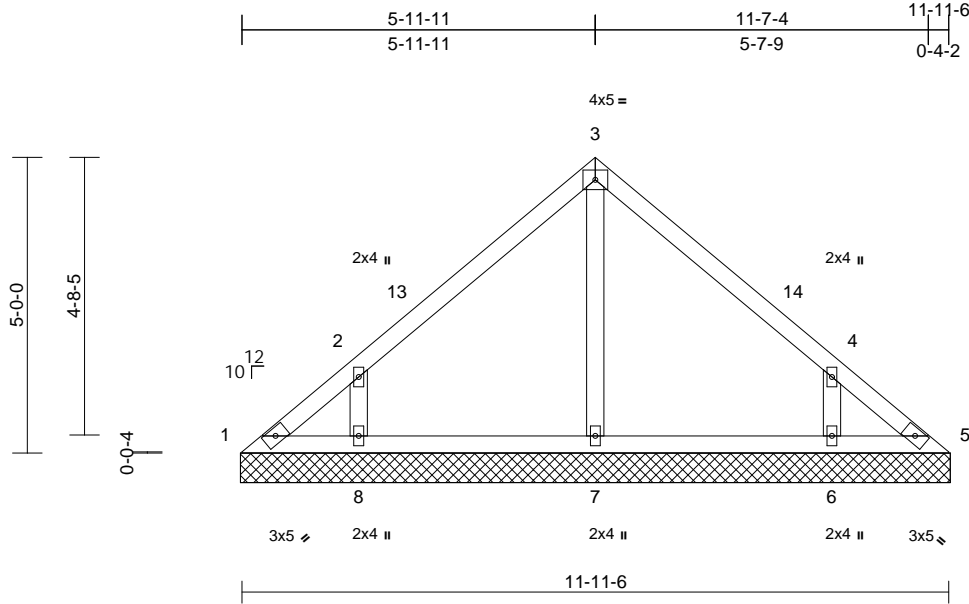
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831379
25050207-B	VL2	Valley	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33
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Page: 1



Scale = 1:39

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.17	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.09	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	5	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 49 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	1=12-0-0, 5=12-0-0, 6=12-0-0, 7=12-0-0, 8=12-0-0
Max Horiz	1=90 (LC 11)
Max Uplift	1=-23 (LC 9), 5=-1 (LC 10), 6=-67 (LC 14), 8=-69 (LC 13)
Max Grav	1=82 (LC 29), 5=65 (LC 28), 6=314 (LC 29), 7=241 (LC 2), 8=317 (LC 28)

FORCES

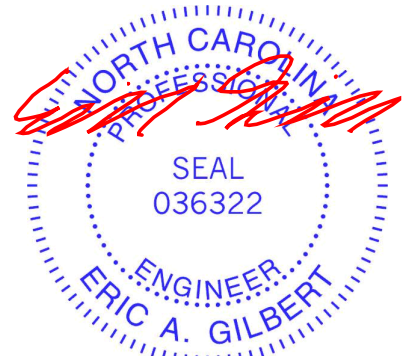
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-102/83, 2-3=-161/118, 3-4=-161/114, 4-5=-80/53
BOT CHORD	1-8=-22/66, 7-8=-22/59, 6-7=-22/59, 5-6=-22/60
WEBS	3-7=-155/0, 2-8=-315/224, 4-6=-315/224

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 12-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1, 1 lb uplift at joint 5, 69 lb uplift at joint 8 and 67 lb uplift at joint 6.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 5.

LOAD CASE(S) Standard



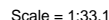
May 30, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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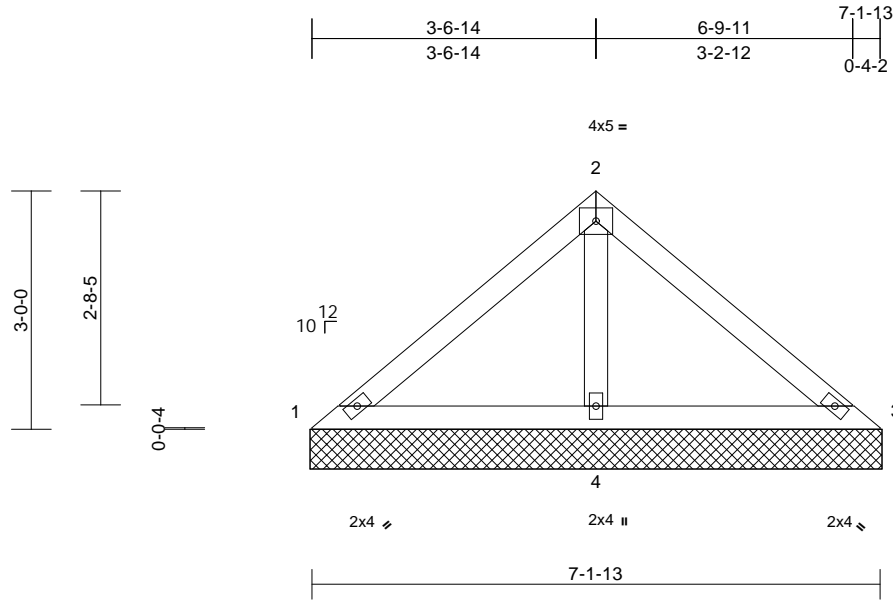
Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831381
25050207-B	VL4	Valley	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33

Page: 1

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Scale = 1:29

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.15	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 7-1-13 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=7-2-6, 3=7-2-6, 4=7-2-6
Max Horiz	1=-53 (LC 11)
Max Uplift	1=-7 (LC 35), 3=-7 (LC 34), 4=-10 (LC 13)
Max Grav	1=65 (LC 34), 3=65 (LC 35), 4=499 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

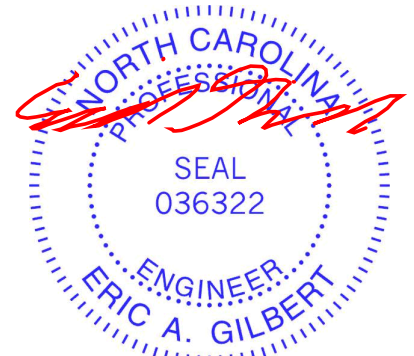
TOP CHORD	1-2=-103/221, 2-3=-98/221
BOT CHORD	1-4=-183/165, 3-4=-183/165
WEBS	2-4=-425/213

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 3-7-3, Exterior(2R) 3-7-3 to 6-4-12, Interior (1) 6-4-12 to 7-2-6 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1, 7 lb uplift at joint 3 and 10 lb uplift at joint 4.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard



May 30, 2025

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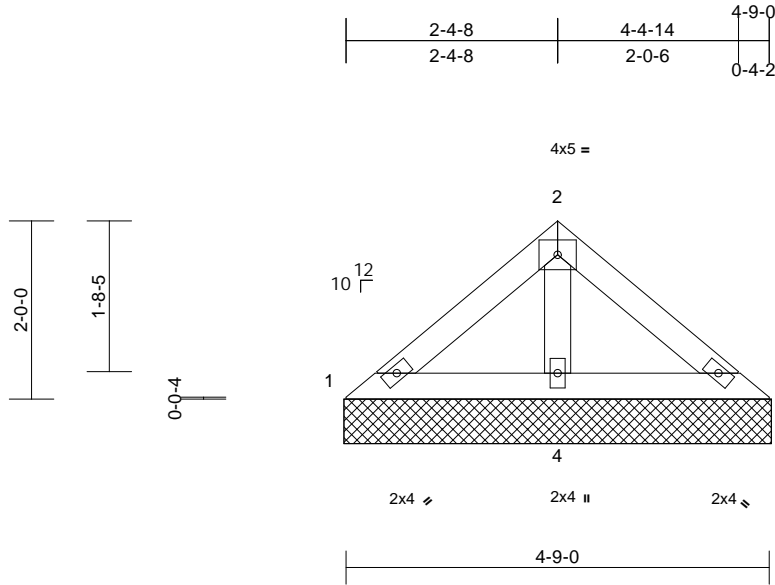
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	
25050207-B	VL5	Valley	2	1	Job Reference (optional)	I73831382

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33
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Page: 1



Scale = 1:25.9

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.07	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horiz(TL)	0.00	3	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										
											Weight: 17 lb FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-9-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=4-9-10, 3=4-9-10, 4=4-9-10
Max Horiz	1=-34 (LC 9)
Max Grav	1=61 (LC 34), 3=61 (LC 35), 4=285 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

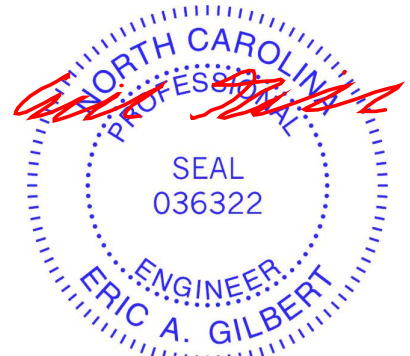
TOP CHORD	1-2=-54/93, 2-3=-54/93
BOT CHORD	1-4=-84/88, 3-4=-84/88
WEBS	2-4=-208/104

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard



May 30,2025

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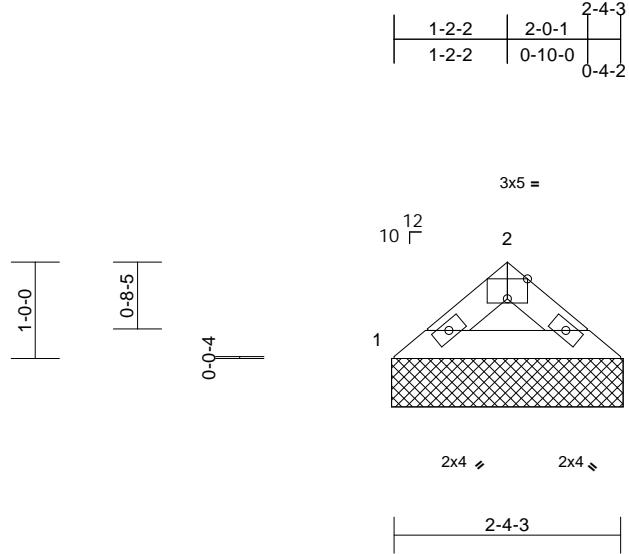
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831383
25050207-B	VL6	Valley	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33
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Page: 1



Scale = 1:23.9

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.04	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 7 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-4-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=2-4-13, 3=2-4-13
Max Horiz 1=15 (LC 11)
Max Grav 1=96 (LC 2), 3=96 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-138/64, 2-3=-138/64
BOT CHORD 1-3=-39/99

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard



May 30,2025

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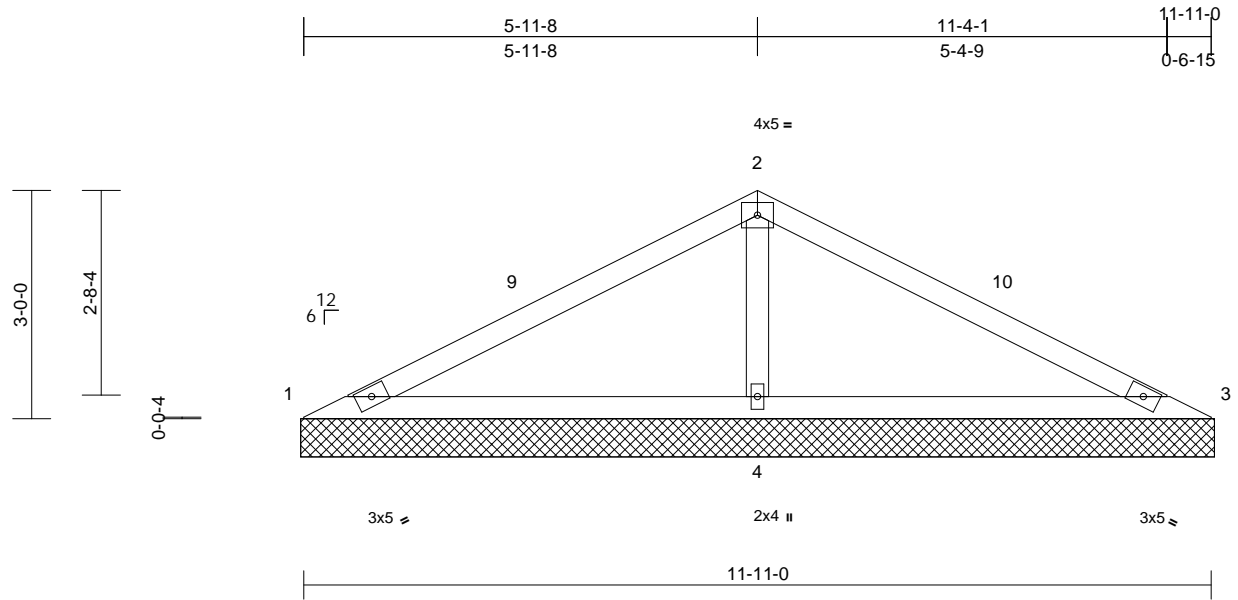
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	173831384
25050207-B	VL7	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33
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Page: 1



Scale = 1:30.3

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.51	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.44	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 39 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=12-0-0, 3=12-0-0, 4=12-0-0
Max Horiz	1=29 (LC 14)
Max Uplift	1=47 (LC 22), 3=47 (LC 21)
Max Grav	1=76 (LC 38), 3=76 (LC 39), 4=917 (LC 2)

FORCES

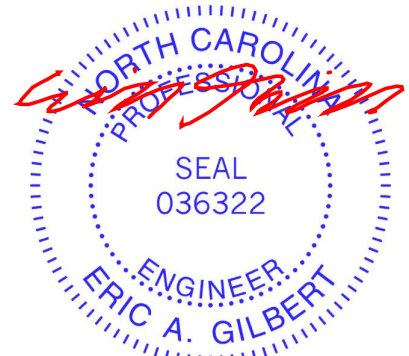
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-178/532, 2-3=-172/532
BOT CHORD	1-4=-444/219, 3-4=-444/219
WEBS	2-4=-757/323

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 12-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 1 and 47 lb uplift at joint 3.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard



May 30, 2025

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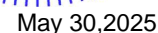
Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33 Page: 1
ID: BZCIWr5VTq4rV2MbmbEYv6Gcc-RfC?PsB70Hg3NSaPqnL8w3uITxbGKWrcDoI7J4zJC?f



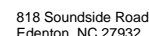
LUMBER		4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
OTHERS	2x4 SP No.3	
BRACING		5) Unbalanced snow loads have been considered for this design.
TOP CHORD	Structural wood sheathing directly applied or 7-11-0 oc purlins.	6) Gable requires continuous bottom chord bearing.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	7) Gable studs spaced at 4-0-0 oc.
REACTIONS		8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
	(size) 1=8-0-0, 3=8-0-0, 4=8-0-0	9) All bearings are assumed to be SP No.2 .
	Max Horiz 1=19 (LC 13)	10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 1 and 5 lb uplift at joint 3.
	Max Uplift 1=1 (LC 15), 3=5 (LC 16)	11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.
	Max Grav 1=91 (LC 21), 3=91 (LC 22), 4=536 (LC 2)	
FORCES		
	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-128/280, 2-3=-120/280	
BOT CHORD	1-4=-249/174, 3-4=-249/174	
WEBS	2-4=-409/222	
LOAD CASE(S)		Standard

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 4-0-0, Exterior(2R) 4-0-0 to 6-9-3, Interior (1) 6-9-3 to 8-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.



Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Components Association (www.sbcacomponents.com)

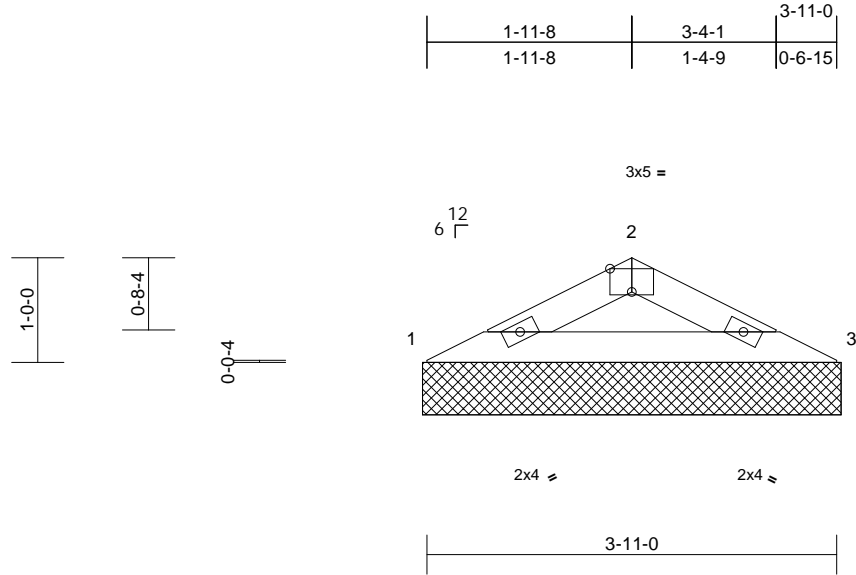


Job	Truss	Truss Type	Qty	Ply	27 Eagle Creek-Lawson E	I73831386
25050207-B	VL9	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Thu May 29 11:27:33
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Page: 1



Scale = 1:22

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.25	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 10 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=4-0-0, 3=4-0-0
Max Horiz 1=9 (LC 13)
Max Grav 1=160 (LC 2), 3=160 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

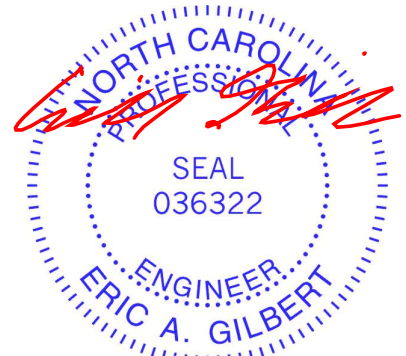
TOP CHORD 1-2=-314/170, 2-3=-314/159
BOT CHORD 1-3=-149/274

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard



May 30,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

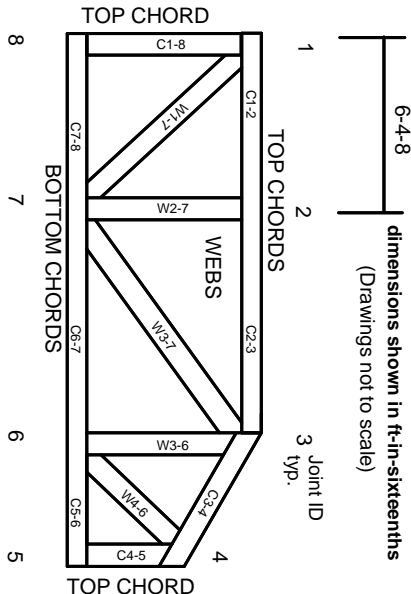
BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:
ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:
ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.
Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023