

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 25-0326-A RVF-LOT #44 ROOF

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Riverside Roof Truss.

Pages or sheets covered by this seal: I70996792 thru I70996833

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



January 27,2025

# Tony Miller

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



A MiTek 3 818 Soundside Road Edenton, NC 27932



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF	
						170996793
25-0326-A	HG01	HIP GIRDER	1	2		
				-	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s D	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:34:48 2025	Page 2
		ID:tdl	HS5IWyLn	q?jaR9E1	eBtqyly93eJAnFzZKx3YHqK12p?Y?qdhFPMB61AA8nLiV	/3zs6kL

NOTES-

14) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Left Hand Hip) or equivalent at 4-0-6 from the left end to connect truss(es) to front face of bottom chord.

15) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Right Hand Hip) or equivalent at 7-11-10 from the left end to connect truss(es) to front face of bottom chord.

(a) Fill all nail holes where hanger is in contact with lumber.
(17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-43, 2-3=-43, 3-4=-53, 4-5=-43, 5-6=-43, 7-10=-20

Concentrated Loads (lb)

Vert: 9=-339(F) 8=-339(F) 13=-135(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





TCDL BCLL BCDL	10.0 0.0 * 10.0	Rep Stress Incr YES Code IRC2018/TPI2014
DODL	10.0	

# LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

Horz(CT)

-0.00

3

n/a

WB

Matrix-MF

0.03

 TOP CHORD
 Structural wood sheathing directly applied or 1-11-11 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0 oc bracing.

n/a

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=67(LC 16) Max Uplift 5=-20(LC 16), 3=-10(LC 13), 4=-14(LC 16)

Max Grav 5=158(LC 21), 3=37(LC 21), 4=35(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Weight: 11 lb

FT = 20%

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TRENCO A MITOK AMITINA



LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.26 BC 0.16 WB 0.06 Matrix-MP	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.01         4-5         >999         240           Vert(CT)         -0.02         4-5         >999         180           Horz(CT)         -0.00         4         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 24 lb         FT = 20%
BCDL 10.0		Matrix Mi		

# LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 4=Mechanical Max Horz 5=97(LC 13) Max Uplift 5=-39(LC 16), 4=-26(LC 13)

Max Grav 5=221(LC 2), 4=155(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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A MiTel 818 Soundside Road Edenton, NC 27932



TRENCO A MITEK Affiliate



 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Job	Truss	Truss Type		Qty	Ply	RVF-LOT #44 F	ROOF		170006901
25-0326-A	T01G	COMMON GIRDE	R	1	2				170990001
Riverside Roof Truss, LLC,	Danville, Va - 24541,				8.730 s D	Job Reference ( ec 5 2024 MiTek	optional) Industries, Inc. F	- Fri Jan 24 14:34:53 202	5 Page 1
	3-3-12		ID: 6-4-0	tdHS5IWyL 9-4-4	.ng?jaR9E I	1eBtqyly9Qc73	3qy1i9ThqOcC?rN 12-8-0	//ajitKTNQ0SnESvI33T	AHzs6kG
	3-3-12		3-0-4	3-0-4	1		3-3-12		
			4x4 =						Scale = 1:29.4
			3						
I									
	7	00 12							
		245 4				0.5 \			
		3x5			$\overline{\ }$	3x5 🔨			
		2							
9-4	1	1				$\sim$			
4					///		$\searrow$		
	3x5 💋							3x5 ≫ 5	
	1							× .	
	12							12	
				ΠΠ		╷╦╸╴╴			
1			14	15			16		
	10 LUS26	9	LUS26 200 - L	_US26		73x6 =	LUS26	6	
	4x4	3x6 =	3x8 —		I	-0526	4x4	+	
		20020							
	3-3-12 3-3-12		<u> </u>	9-4-4	1 1		12-8-0 3-3-12		
Plate Offsets (X,Y) [6:E	dge,0-3-8]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc) l/defl	L/d	PLATES	GRIP
Snow (Pf/Pg) 11.6/15.0	Plate Grip DOL	1.15	TC 0.47 BC 0.37	Vert(LL)	-0.02	8-9 >999 8-9 >999	240 180	MT20	244/190
TCDL 10.0 BCU 0.0 *	Rep Stress Incr	NO	WB 0.44	Horz(CT)	0.04	6 n/a	n/a		
BCDL 10.0	Code IRC2018/	FPI2014	Matrix-MS					Weight: 171 lb	FT = 20%
LUMBER-	0		BRACIN	IG-	0	- Louis and the standard			
BOT CHORD 2x4 SP No.	2		TOP CF	IORD	except e	al wood sheathir end verticals.	ng directly applie	a or 6-0-0 oc puriins,	
WEBS 2x4 SP No.	3		BOT CH	IORD	Rigid ce	iling directly app	lied or 10-0-0 oc	c bracing.	
REACTIONS. (size)	10=0-3-8, 6=0-3-8								
Max Horz	10=-97(LC 10)								
Max Opint Max Grav	10=2634(LC 3), 6=2351(LC	3)							
FORCES. (lb) - Max. Con	np./Max. Ten All forces 250	) (lb) or less exce	ot when shown.						
TOP CHORD 1-2=-2874	4/0, 2-3=-2148/96, 3-4=-214	8/95, 4-5=-2694/1	43, 1-10=-2025/0,						
5-6=-1900 BOT CHORD 9-10=-13/	6/104 /399, 8-9=0/2439, 7-8=-73/22	285, 6-7=-19/317							
WEBS 3-8=-45/1	961, 4-8=-615/129, 4-7=-95/	/560, 2-8=-815/0,	2-9=0/786, 1-9=0/2141,						
5-7=-50/2	.024								
1) 2-ply trues to be connect	ed together with 10d (0 131	'v3") naile as follo	NC.						
Top chords connected a	s follows: 2x4 - 1 row at 0-9-	0 oc.	wo.						
Bottom chords connecte Webs connected as follo	d as follows: 2x6 - 2 rows sta ws: 2x4 - 1 row at 0-9-0 oc	aggered at 0-9-0 o	DC.						
2) All loads are considered	equally applied to all plies, e	except if noted as	front (F) or back (B) face in th	e LOAD C	ASE(S) s	ection. Ply to			
<ol> <li>a) Unbalanced roof live load</li> </ol>	en provided to distribute only ds have been considered for	/ loads noted as ( this design.	<ul> <li>F) or (B), unless otherwise inc</li> </ul>	licated.				CAD"	h.
4) Wind: ASCE 7-16; Vult=	130mph (3-second gust) Vas	sd=103mph; TCD	L=6.0psf; BCDL=6.0psf; h=25	ft; B=45ft;	L=24ft; e	ave=4ft; Cat.	unit.	THUNHO	111
plate grip DOL=1.60	rks (directional), cantilever	ien and right expo	osed, end vertical left and figr	it exposed	, Lumber	DOL=1.00	is a	OFFORT	Mar.
5) TCLL: ASCE 7-16; Pr=20	0.0 psf (roof LL: Lum DOL=1	.15 Plate DOL=1.	15); Pg=15.0 psf; Pf=11.6 psf	(Lum DOI	_=1.15 Pl	ate	2	a l may	the second second
6) Unbalanced snow loads	have been considered for th	is design.	1.10				E E	SEAL	E E
<ul> <li>7) This truss has been desi</li> <li>8) * This truss has been desi</li> </ul>	gned for a 10.0 psf bottom c	hord live load nor	concurrent with any other live	loads.	S-0 tall by	2-0-0 wide	E 1	023594	÷ Ξ
will fit between the bottom chord and any other members.									
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=127.									
10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and									
11) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at									
1-2-12 from the left end to 11-2-12 to connect truss(es) to back face of bottom chord.					125				
Continued on page 2		IDEI.						January 27,20	520
LOAD CASE(S) Standard				rev 1/2/2022		SE .			
Design valid for use only w	vith MiTek® connectors. This design	is based only upon pa	rameters shown, and is for an individu	al building co	omponent, r	ot rall	1000		'n
building design. Bracing ir is always required for stabi	ility and to prevent collapse with pos	ividual truss web and/o sible personal injury a	or chord members only. Additional ter	nporary and plance regardi	permanent b	racing		A MiTek /	Affiliate

billioning design. Stacking indicated is to prevent oblasing of individual russ web and/or of members of the second members of the s

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF	
					1709	996801
25-0326-A	T01G	COMMON GIRDER	1	2		
				<b>_</b>	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s D	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:34:53 2025 Pag	je 2

8.730 s Dec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:34:53 2025 Page 2 ID:tdHS5IWyLng?jaR9E1eBtqyly9\_-Qc73qy1i9ThqOcC?rMajitKTNQ0SnESvI33TAHzs6kG

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-5=-43, 6-10=-20

Concentrated Loads (lb)

Vert: 7=-474(B) 9=-595(B) 13=-595(B) 14=-474(B) 15=-474(B) 16=-474(B)

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- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



A MiTek Affiliat 818 Soundside Road



referenced standard ANSI/TPI 1.

# January 27,2025

A MILLIN R. MI

3594

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Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF	
						170996804
25-0326-A	T02G	COMMON GIRDER	1	2		
				<b>J</b>	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s D	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:34:55 2025	Page 2
		ID:tdF	-IS5IWyLn	a?jaR9E1e	Btgyly9 -M EgFe3yh5xYdvMNyndBnIPn EgdF7lCmNYaF	9zs6kE

# NOTES-

13) Fill all nail holes where hanger is in contact with lumber.

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-43, 3-5=-43, 5-6=-43, 7-12=-20 Concentrated Loads (lb)

Vert: 9=-469(B) 15=-474(B) 16=-469(B) 17=-469(B) 18=-469(B) 19=-469(B) 20=-469(B) 21=-469(B) 22=-469(B) 23=-480(B)

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6-0-0 oc bracing: 23-24,14-15.

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. All bearings 20-8-0.

Max Horz 24=-158(LC 14) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 24, 20, 21, 22, 23, 18, 17, 16, 15 All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15 Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-4-0, Exterior(2N) 2-4-0 to 10-4-0, Corner(3R) 10-4-0 to 13-4-0, Exterior(2N) 13-4-0 to 21-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate 4) DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 12) will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 20, 21, 22, 23, 18, 17, 16, 15.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road



9-9-0	19-3-6	27-10-0	36-4-8	367p-4	46-0-10	55-9-0	1
9-9-8	9-6-0	8-6-8	8-6-8	0-1 <sup>!!</sup> 12	9-6-6	9-8-6	1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.70 BC 0.69 WB 0.85 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.12 18-20 -0.23 18-20 0.05 15	l/defl L/ >999 24 >999 18 n/a n/	/d PLATES .0 MT20 .0 /a Weight: 406 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 WEDGE Right: 2x4 SP No.3 SLIDER Left 2x4 SP No.3	3 2-6-0	BI TC BC W	RACING- DP CHORD DT CHORD EBS	Structural wood except 2-0-0 oc purlins Rigid ceiling dir 6-0-0 oc bracing 1 Row at midpt 2 Rows at 1/3 p	sheathing dire (6-0-0 max.): ectly applied o g: 15-16. 4- ts 9-	ectly applied or 4-4-13 oc purlir 6-9. ır 10-0-0 oc bracing, Except: -18, 6-16, 7-16, 11-15 -15	ns,

REACTIONS. (size) 2=0-3-8, 15=0-3-8, 12=Mechanical Max Horz 2=205(LC 15) Max Uplift 2=-121(LC 16), 15=-124(LC 16), 12=-54(LC 16)

Max Grav 2=1664(LC 28), 15=2954(LC 29), 12=688(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2541/258, 4-6=-1637/263, 6-7=-858/260, 7-9=-858/260, 9-11=0/718,

11-12=-726/143

BOT CHORD	2-20=-144/2330, 18-20=-144/2330, 16-18=0/1436, 15-16=-511/137, 13-15=-25/578,
	12-13=-25/578
WEBS	4-20=0/448, 4-18=-1055/175, 6-18=0/920, 6-16=-984/46, 7-16=-741/173,
	9-16=-137/1927, 9-15=-2081/217, 11-15=-1209/187, 11-13=0/493

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=56ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-7-14, Interior(1) 4-7-14 to 19-3-8, Exterior(2R) 19-3-8 to 27-2-2, Interior(1) 27-2-2 to 36-4-8, Exterior(2R) 36-4-8 to 44-3-2, Interior(1) 44-3-2 to 55-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Refer to girder(s) for truss to truss connections.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=121, 15=124.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Continuiere one state 2 dard ANSI/TPI 1

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF		
					l'	70996806	
25-0326-A	T03	PIGGYBACK BASE	6	1			
					Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s De	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:34:57 2025 F	Page 2	
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9INMagK4CDiBGsDWm4CffsjU6k2IPjw3UDh1hJ2zs6kC					

# NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Rep Stress Incr YES WB 0.19 Horz(CT) 0.01 34 n/a n/a BCLL 0.0 \* Code IRC2018/TPI2014 Weight: 542 lb Matrix-S BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2x6 SP No.2 2-0-0 oc purlins (6-0-0 max.): 13-23 OTHERS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEDGE WEBS 23-45, 22-46, 21-47, 20-48, 18-49, 17-50, 1 Row at midpt Right: 2x4 SP No.3 16-52, 15-53, 14-54, 12-55, 11-56, 24-44 Left 2x4 SP No.3 1-6-4 SLIDER REACTIONS. All bearings 55-9-0. Max Horz 2=-208(LC 14) (lb) -Max Uplift All uplift 100 lb or less at joint(s) 2, 46, 47, 48, 49, 50, 52, 53, 56,

58, 59, 60, 61, 62, 63, 64, 44, 42, 41, 40, 39, 38, 37, 36, 35, 34 Max Grav All reactions 250 lb or less at joint(s) 2, 45, 46, 47, 48, 49, 50, 52, 53, 54, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 42, 41, 40, 39, 38, 37, 36, 35, 34

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 11-12=-113/285, 12-13=-115/286, 13-14=-111/287, 14-15=-111/287, 15-16=-111/287, 16-17=-111/287, 17-18=-111/287, 18-20=-111/287, 20-21=-111/287, 21-22=-111/287, 22-23=-111/287, 23-24=-118/299, 24-25=-102/259

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=56ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 4-4-8, Exterior(2N) 4-4-8 to 19-3-8, Corner(3R) 19-3-8 to 24-10-6, Exterior(2N) 24-10-6 to 36-4-8, Corner(3R) 36-4-8 to 41-11-6, Exterior(2N) 41-11-6 to 55-7-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Continued on page 2

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



FT = 20%



Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF	
						170996807
25-0326-A	103GE	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s De	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:35:00 2025	Page 2
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9iy2jIM65WdarjgELIKCMUM6muFUOwS9xvfFLwNzs6k9				

# NOTES-

12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 46, 47, 48, 49, 50, 52, 53, 56, 58, 59, 60, 61, 62, 63, 64, 44, 42, 41, 40, 39, 38, 37, 36, 35, 34.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)





Code IRC2018/TPI2014 Matrix-MS Weight: 431 lb FT = 20% BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-7-3 oc purlins, except BOT CHORD 2x6 SP No.2 \*Except\* 2-0-0 oc purlins (6-0-0 max.): 6-9. 16-19: 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 6-0-0 oc bracing: 16-19 2x4 SP No.3 WEBS WEBS 4-23, 6-21, 7-21, 10-19 WEDGE 1 Row at midpt Right: 2x4 SP No.3 2 Rows at 1/3 pts 9-20

SLIDER Left 2x4 SP No.3 2-6-0

#### REACTIONS. (size) 2=0-3-8, 13=Mechanical, 20=0-3-8 Max Horz 2=206(LC 15) Max Uplift 2=-136(LC 16), 13=-39(LC 16), 20=-2(LC 16) Max Grav 2=1591(LC 28), 13=637(LC 29), 20=3493(LC 29)

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-4=-2386/277, 4-6=-1473/284, 6-7=-670/287, 7-9=-670/287, 9-10=0/945, 10-12=-852/187, 12-13=-848/68

 BOT CHORD
 2-25=-161/2193, 23-25=-161/2193, 21-23=-111/1289, 20-21=-770/95, 13-14=0/665

 WEBS
 4-25=0/453, 4-23=-1066/174, 6-23=0/928, 6-21=-1070/31, 7-21=-744/181,

 
 VVEDS
 4-23=0/403, 4-23=-1060/174, 6-23=0/926, 6-21=-1070/31, 7-21=-744/181, 9-21=-134/2019, 9-20=-2224/162, 19-20=-1117/104, 10-19=-975/148, 10-16=-6/1357, 14-16=-50/1213, 12-14=-390/195

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=56ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-0-0 to 4-6-14, Interior(1) 4-6-14 to 19-3-8, Exterior(2R) 19-3-8 to 27-2-2, Interior(1) 27-2-2 to 36-4-8, Exterior(2R) 36-4-8 to 44-3-2, Interior(1) 44-3-2 to 55-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Refer to girder(s) for truss to truss connections.

10) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify Continueation page and surface.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF		
					170996808		
25-0326-A	T03S	Piggyback Base	8	1			
					Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s De	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:35:01 2025 Page 2		
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9B8c5Vh7jHxiiLqpXJ2jb1Zfnsfb0fju48J?uSpzs6k8					

# NOTES-

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 20 except (jt=lb) 2=136.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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	9-7-4	19-3-8	27-10-0	36-4-8	39-2-4	46-0-12	51-5-12	55-9-0	-
Plate Offse	ets (X,Y) [17:0-4-0	9-8-4 1,0-4-8], [25:0-4-0,0-4-8], [30:0-3-0,0-1-4	4], [57:0-1-3,0-1-0]	0-0-0	2-9-12	6-10-8	5-5-0	4-3-4	
LOADING TCLL (roof Snow (Pf/P TCDL BCLL BCDL	(psf) ) 20.0 'g) 16.5/15.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.61 BC 0.27 WB 0.57 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.03 16-18 -0.05 16-18 0.01 85	l/defl L/d >999 240 >999 180 n/a n/a	PLA MT2 Weig	<b>TES</b> 0 Jht: 634 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHOF BOT CHOF WEBS OTHERS WEDGE Right: 2x4 SLIDER	RD 2x6 SP No.2 RD 2x6 SP No.2 2x4 SP No.3 2x4 SP No.3 SP No.3 Left 2x4 SP No.	3 2-6-0	BI TC B( W	R <b>ACING-</b> DP CHORD DT CHORD EBS	Structural wood 2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt	sheathing directly (10-0-0 max.): 6-5 ectly applied or 6-6 4-29,	v applied or 6-0-0 9. 0-0 oc bracing. 6-29, 6-24, 7-24	) oc purlins , 9-24, 9-19	, except 9, 11-19
REACTION (	<ul> <li>All bearings 33 15=0-3-8, 12=</li> <li>15=0-3-8, 12=</li> <li>Max Horz 2=20 Max Uplift All 24=- 16)</li> <li>Max Grav All 37, 38), 23),</li> </ul>	9-2-4 except (jt=length) 12=4-6-12, 14= 4-6-12. 15(LC 15) uplift 100 lb or less at joint(s) 2, 29, 37 e 198(LC 16), 19=-167(LC 16), 38=-130(l reactions 250 lb or less at joint(s) 2, 26, 23, 22, 21, 20, 14, 2 except 34=577(LC 19=820(LC 49), 12=457(LC 49), 38=43 15=532(LC 29), 12=292(LC 1)	4-6-12, 13=4-6-12, 18=0-3 except 34=-104(LC 16), LC 16), 14=-415(LC 29), 7 , 27, 28, 31, 32, 33, 35, 36 ; 47), 29=488(LC 28), 24= ;8(LC 28), 13=297(LC 29),	3-8, 13=-117(LC 5, 1043(LC 18=306(LC					
FORCES. TOP CHOP BOT CHOP	(lb) - Max. Comp.// RD 2-4=-187/364, RD 23-24=-287/15 18-19=-29/552 12-13=-29/552	lax. Ten All forces 250 (lb) or less exc 6-7=0/262, 7-9=0/262, 9-11=0/388, 11- 54, 22-23=-287/154, 21-22=-287/154, 21 2, 16-18=-29/552, 15-16=-29/552, 14-15 2	cept when shown. -12=-730/138 0-21=-287/154, 19-20=-2{ 5=-29/552, 13-14=-29/552	37/154, ,			Number of C	AP	11,
WEBS	4-34=-497/154 11-19=-969/21	4, 6-29=-317/58, 6-24=-274/83, 7-24=-7  0, 11-16=0/313	48/173, 9-19=-508/88,				OR	Sici	DK II
NOTES- 1) Unbalan 2) Wind: A: II; Exp E 27-2-2, 1 end vert DOL=1.1 3) Truss de Gable E 4) TCLL: A DOL=1. surcharg 5) Unbalan Continued of	aced roof live loads ha SCE 7-16; Vult=130n b; Enclosed; MWFRS interior(1) 27-2-2 to 3 iccal left and right exp 60 asigned for wind load nd Details as applica SCE 7-16; Pr=20.0 p 15); Is=1.0; Rough C; ge applied to all expo icced snow loads have an page 2	ave been considered for this design. http://directional/and/C-C Exterior(2E) -0-11 6-4-8, Exterior(2R) 36-4-8 to 44-3-2, Int bosed;C-C for members and forces & MV is in the plane of the truss only. For study ble, or consult qualified building designed sf (roof LL: Lum DOL=1.15 Plate DOL= at B; Partially Exp.; Ce=1.0; Cs=1.00; C sed surfaces with slopes less than 0.50 been considered for this design.	DL=6.0psf; BCDL=6.0psf; 1-0 to 4-7-14, Interior(1) 4 terior(1) 44-3-2 to 55-9-0 z WFRS for reactions showr ds exposed to wind (norm er as per ANSI/TPI 1. $\epsilon_1.15$ ); Pg=15.0 psf; Pf=16 $\epsilon_2$ =1.10, Lu=50-0-0; Min. fl 00/12 in accordance with If	h=25ft; B=45ft; -7-14 to 19-3-8, l cone; cantilever l h; Lumber DOL= al to the face), su 6.5 psf (Lum DOL at roof snow load 3C 1608.3.4.	L=56ft; eave=7ft Exterior(2R) 19-3 eft and right exp 1.60 plate grip ee Standard Indu L=1.15 Plate d governs. Rain	; Cat. 3-8 to osed ; ustry	SI 023	EAL 3594	025
WA Desig a trus buildir is alw. fabrica and E	RNING - Verify design paran n valid for use only with Mi s system. Before use, the b g design. Bracing indicate ays required for stability an ation, storage, delivery, ere BCSI Building Componen	neters and READ NOTES ON THIS AND INCLUDE Tek® connectors. This design is based only upon uilding designer must verify the applicability of de d is to prevent buckling of individual truss web an d to prevent collapse with possible personal injury ction and bracing of trusses and truss systems, se t Safety Information available from the Structure	D MITEK REFERENCE PAGE M parameters shown, and is for an sign parameters and properly inc d/or chord members only. Additi v and property damage. For gene e ANSI/TP1 Quality Criteria a al Building Component Association	II-7473 rev. 1/2/2023 individual building cc corporate this design onal temporary and p aral guidance regardi and DSB-22 availabi on (www.sbcacompoli-	BEFORE USE. omponent, not into the overall permanent bracing ng the le from Truss Plate In nents.com)	stitute (www.tpinst.org)	818 Sounds Edenton, NG	A MiTek	<b>10</b> Affiliate

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #44 ROOF		
					17	/0996809	
25-0326-A	T03SGE	GABLE COMMON	1	1			
					Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s De	ec 5 2024 MiTek Industries, Inc. Fri Jan 24 14:35:04 2025 Pa	age 2	
		ID:tdHS5IWyLng?jaR9E1eBtgyly9 -bjHD8j9bZs4GCIY6 AHIfCHJNsnWsACWgHDY38zs6k5					

NOTES-

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

7) Provide adequate drainage to prevent water ponding.

- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 29, 37, 2 except (jt=lb) 34=104, 24=198, 19=167, 38=130, 14=415, 13=117.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building component Association (www.sbcacomponents.com)





l	9-9-8	19-3-8	27-10-0	36-4-8	36-4 <sub>1</sub> 6-4	48-10-4	55-5-8 5	55 <sub>1</sub> 8-0
1	9-9-8	9-6-0	8-6-8	8-6-8	0-1"-12	12-4-0	6-7-4	0-2-8
LOADING ( TCLL (roof) Snow (Pf/Pg TCDL BCLL	(psf) 20.0 1) 16.5/15.0 10.0 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.71 BC 0.70 WB 1.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 16-18 -0.29 16-18 0.05 18	l/defl L/d >784 240 >505 180 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2018/TPI2014	Matrix-MS				Weight: 420 lb	FT = 20%
LUMBER- TOP CHOR BOT CHOR WEBS SLIDER	D 2x6 SP No.2 D 2x6 SP No.2 2x4 SP No.3 Left 2x4 SP No.3	.3 2-6-0, Right 2x4 SP No.3 2-6-0	B Tr B W	RACING- OP CHORD OT CHORD /EBS	Structural wood 2-0-0 oc purlins Rigid ceiling dirr 1 Row at midpt	sheathing directly app (6-0-0 max.): 6-9. ectly applied or 6-0-0 o 4-21, 6-19	lied or 4-5-1 oc purlins c bracing. , 7-19, 9-19, 9-18, 10-	s, except -18, 10-16
REACTION: (It	<ul> <li>All bearings 0</li> <li>Max Horz 2=20</li> <li>Max Uplift All</li> <li>Max Grav All</li> <li>14=3</li> </ul>	-3-8 except (jt=length) 14=0-3-0. )7(LC 15) uplift 100 lb or less at joint(s) 16, 14 ex reactions 250 lb or less at joint(s) exce 341(LC 55)	cept 2=-126(LC 16), 18=- pt 2=1664(LC 28), 18=274	105(LC 16) 47(LC 28), 16=684	ł(LC 29),			
FORCES. TOP CHOR BOT CHOR WEBS	(lb) - Max. Comp./N D 2-4=-2542/26 D 2-23=-124/23 4-23=0/445, 4 9-19=-148/18	1ax. Ten All forces 250 (lb) or less ex 1, 4-6=-1639/269, 6-7=-842/268, 7-9=- 36, 21-23=-124/2336, 19-21=0/1443, 1 -21=-1052/175, 6-21=0/928, 6-19=-975 95, 9-18=-1992/191, 10-18=-506/187, 1	cept when shown. 342/268, 9-10=0/623 3-19=-435/149, 16-18=-25 5/41, 7-19=-743/180, 0-16=-53/263, 12-16=-38	54/73 5/167				
NOTES- 1) Unbalanc 2) Wind: AS II; Exp B; 27-2-0, Ir end vertio DOL=1.6 3) TCLL: AS DOL=1.1: surcharge 4) Unbalanc 5) This truss non-conc 6) Provide a 7) This truss 8) * This truss 9) Provide n (jt=lb) 2= 10) This trus reference 11) Graphic	ed roof live loads h: CE 7-16; Vult=130r Enclosed; MWFRS tterior(1) 27-2-0 to 3 cal left and right exp 0 GCE 7-16; Pr=20.0 p 5); Is=1.0; Rough C e applied to all expo ted snow loads have s has been designed urrent with other live dequate drainage tt a has been designed sen ab solution chinection table and the solution constrained and the solution sis is designed in ac- ced standard ANSI/7 al purlin representai	ave been considered for this design. nph (3-second gust) Vasd=103mph; TC (directional) and C-C Exterior(2E) -0-1 i6-4-8, Exterior(2R) 36-4-8 to 44-3-0, In osed;C-C for members and forces & M usf (roof LL: Lum DOL=1.15 Plate DOL= at B; Partially Exp.; Ce=1.0; Cs=1.00; C sed surfaces with slopes less than 0.50 been considered for this design. d for greater of min roof live load of 12.0 e loads. o prevent water ponding. d for a 10.0 psf bottom chord live load r ed for a live load of 20.0psf on the botts iord and any other members, with BCD on (by others) of truss to bearing plate cordance with the 2018 International R TPI 1. tion does not depict the size or the orie	CDL=6.0psf; BCDL=6.0psf 1-0 to 4-7-13, Interior(1) 4 terior(1) 44-3-0 to 56-7-0 WFRS for reactions show =1.15); Pg=15.0 psf; Pf=10 2t=1.10, Lu=50-0-0; Min. f =0/12 in accordance with I =0 psf or 1.00 times flat roo nonconcurrent with any oth pm chord in all areas where L = 10.0psf. capable of withstanding 1 esidential Code sections F intation of the purlin along	f; h=25ft; B=45ft; L I-7-13 to 19-3-8, E zone; cantilever le n; Lumber DOL=1 6.5 psf (Lum DOL= lat roof snow load BC 1608.3.4. f load of 11.6 psf of her live loads. re a rectangle 3-6- 00 lb uplift at joint( R502.11.1 and R80 the top and/or bot	=56ft; eave=7ft; xterior(2R) 19-3 ff and right exp .60 plate grip =1.15 Plate governs. Rain on overhangs 0 tall by 2-0-0 w (s) 16, 14 excep 02.10.2 and tom chord.	r Cat. I-8 to osed ; ride	SEAL 023594	Harrison Charles and the second se

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oullapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCB Building Component Scietu Information available from the Structural Building Component Section Component Scietul Information available from the Structural Building Component Scietul Information** and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road Edenton, NC 27932

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9-9-8 9-9-8	<u> </u>	27-10-0	<u>36-4-8</u> 8-6-8	36-6-4 0-1-12	48-10-4 12-4-0	<u> </u>	55 <sub>1</sub> 8-0 0-2-8
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         16.5/15.0           TCDL         10.0           BCLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2018/TPI2014	CSI. TC 0.71 BC 0.69 WB 1.00 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 15-17 -0.29 15-17 ) 0.05 17	l/defl L/d >784 240 >505 180 n/a n/a	PLATES MT20 Weight: 418 I	<b>GRIP</b> 244/190
BCDL     10.0       LUMBER- TOP CHORD     2x6 SP No.2       BOT CHORD     2x6 SP No.2       WEBS     2x4 SP No.3       SLIDER     Left 2x4 SP No.       REACTIONS.     All bearings 0- (lb) - Max Horz 1=-2 Max Uplift All Max Grav All 13=3	3 2-6-0, Right 2x4 SP No.3 2-6-0 -3-8 except (jt=length) 13=0-3-0. :05(LC 14) uplift 100 lb or less at joint(s) 1, 1 reactions 250 lb or less at joint(s) 341(LC 55)	5, 13 except 17=-104(LC 16 except 1=1615(LC 28), 17=	BRACING- TOP CHORD BOT CHORD WEBS 6) =2744(LC 28), 15=68	Structural wood 2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt 36(LC 29),	I sheathing directly ap (6-0-0 max.): 5-8. ectly applied or 6-0-0 3-20, 5-1	oplied or 4-5-2 oc purlin oc bracing. 18, 6-18, 8-18, 8-17, 9-	ns, except
FORCES.         (lb) - Max. Comp./N           TOP CHORD         1-3=-2547/266           BOT CHORD         1-22=-124/232           WEBS         3-22=0/446, 3           8-18=-148/185	lax. Ten All forces 250 (lb) or le: 5, 3-5=-1643/273, 5-6=-843/271, 6 41, 20-22=-124/2341, 18-20=0/14 -20=-1054/176, 5-20=0/929, 5-18 95, 8-17=-1990/191, 9-17=-505/18	ss except when shown. -8=-843/271, 8-9=0/620 15, 17-18=-433/149, 15-17: -975/41, 6-18=-744/181, 6, 9-15=-53/261, 11-15=-3	=-251/73 85/167				
<ol> <li>NOTES-         <ol> <li>Unbalanced roof live loads ha</li> <li>Wind: ASCE 7-16; Vult=130n II; Exp B; Enclosed; MWFRS 27-2-0, Interior(1) 27-2-0 to 3 end vertical left and right exp DOL=1.60</li> <li>TCLL: ASCE 7-16; Pr=20.0 p DOL=1.15); Is=1.0; Rough C; surcharge applied to all expod</li> <li>Unbalanced snow loads have</li> <li>This truss has been designed non-concurrent with other live</li> <li>Provide adequate drainage to</li> <li>This truss has been designed will fit between the bottom ch</li> <li>Provide mechanical connectit (jt=lb) 17=104.</li> <li>This truss is designed in act referenced standard ANSI/T</li> <li>Graphical purlin representat</li> </ol> </li> </ol>	ave been considered for this design (3-second gust) Vasd=103mp (directional) and C-C Exterior(2E, 6-4-8, Exterior(2R) 36-4-8 to 44-3 osed; C-C for members and forces of (roof LL: Lum DOL=1.15 Plate at B; Partially Exp.; Ce=1.0; Cs=1 sed surfaces with slopes less than been considered for this design. If for greater of min roof live load of be loads. The prevent water ponding. If or a live load of 20.0psf on the ord and any other members, with on (by others) of truss to bearing provance with the 2018 Internation (PI 1. tion does not depict the size or th	n. h; TCDL=6.0psf; BCDL=6.0 0-0-0 to 5-6-13, Interior(1) -0, Interior(1) 44-3-0 to 56- & MWFRS for reactions sl DOL=1.15); Pg=15.0 psf; P 00; Ct=1.10, Lu=50-0-0; M 0.500/12 in accordance w f 12.0 psf or 1.00 times flat pad nonconcurrent with any bottom chord in all areas w BCDL = 10.0psf. klate capable of withstandir nal Residential Code sectio	Opsf; h=25ft; B=45ft; 15-6-13 to 19-3-8, E2 7-0 zone; cantilever hown; Lumber DOL= If=16.5 psf (Lum DOI in. flat roof snow loa ith IBC 1608.3.4. roof load of 11.6 psf y other live loads. where a rectangle 3-f ag 100 lb uplift at joir ns R502.11.1 and R ong the top and/or bo	L=56ft; eave=7ft tterior(2R) 19-3-4 left and right exp 1.60 plate grip L=1.15 Plate d governs. Rain con overhangs 6-0 tall by 2-0-0 v at(s) 1, 15, 13 exc 802.10.2 and bttom chord.	; Cat. 3 to osed ; vide sept	SEAL 023594	2025

A MiTek Af 818 Soundside Road Edenton, NC 27932



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- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



A MiTek At 818 Soundside Road



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REACTIONS. All bearings 20-1-0.

(lb) - Max Horz 24=115(LC 15)

Max Uplit All uplit 100 lb or less at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-0-8, Exterior(2N) 2-0-8 to 10-0-8, Corner(3R) 10-0-8 to 13-0-8, Exterior(2N) 13-0-8 to 21-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



**RENCO** 

January 27,2025



REACTIONS. All bearings 14-9-8. (Ib) - Max Horz 1=-80(LC 1

Max Horz 1=-80(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=270(LC 2), 8=327(LC 33), 6=327(LC 34)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-5-3, Interior(1) 3-5-3 to 7-5-3, Exterior(2R) 7-5-3 to 10-5-3, Interior(1) 10-5-3 to 14-3-14 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)









- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-0-1, Exterior(2R) 4-0-1 to 7-0-1, Interior(1) 7-0-1 to 7-5-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



A Mi Tek Affiliat 818 Soundside Road



REACTIONS. 1=4-6-1, 3=4-6-1 (size) Max Horz 1=-20(LC 14) Max Uplift 1=-8(LC 16), 3=-8(LC 16) Max Grav 1=140(LC 2), 3=140(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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818 Soundside Road



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 5-0-7, Exterior(2R) 5-0-7 to 8-0-7, Interior(1) 8-0-7 to 9-6-7 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

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818 Soundside Road



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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818 Soundside Road



2x4 💋

2x4 📚

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-2-10 0-0-7 0-0-7 3-2-3 Plate Offsets (X,Y)-- [2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 тс 0.02 Vert(LL) 999 MT20 244/190 n/a n/a Snow (Pf/Pg) 11.6/15.0 Lumber DOL 1.15 BC 0.05 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 \* Code IRC2018/TPI2014 FT = 20% Matrix-P Weight: 8 lb BCDL 10.0 LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 3-2-10 oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=3-1-13, 3=3-1-13 Max Horz 1=-12(LC 14) Max Uplift 1=-5(LC 16), 3=-5(LC 16) Max Grav 1=86(LC 2), 3=86(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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818 Soundside Road





- & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 8, 11, 9.
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- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 5-6-0, Exterior(2R) 5-6-0 to 8-6-0, Interior(1) 8-6-0 to 11-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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A MiTek Af 818 Soundside Road Edenton, NC 27932



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- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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ENGINEERING BY A MiTek Affiliate

818 Soundside Road



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

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- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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	0-0-7	2-3-	D						
Plate Offsets (X,Y) [1:0-2-1	,Edge]								
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	<b>CSI.</b> TC 0.01 BC 0.02 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 5 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- BRACING-									

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. 1=2-2-9, 3=2-2-9 (size) Max Horz 1=-7(LC 14) Max Uplift 1=-3(LC 16), 3=-3(LC 16) Max Grav 1=48(LC 2), 3=48(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

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- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 2-3-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
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818 Soundside Road



- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
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