

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 25-1775-A RVF-LOT #48 ROOF

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Riverside Roof Truss.

Pages or sheets covered by this seal: I72314048 thru I72314081

My license renewal date for the state of North Carolina is December 31, 2025.

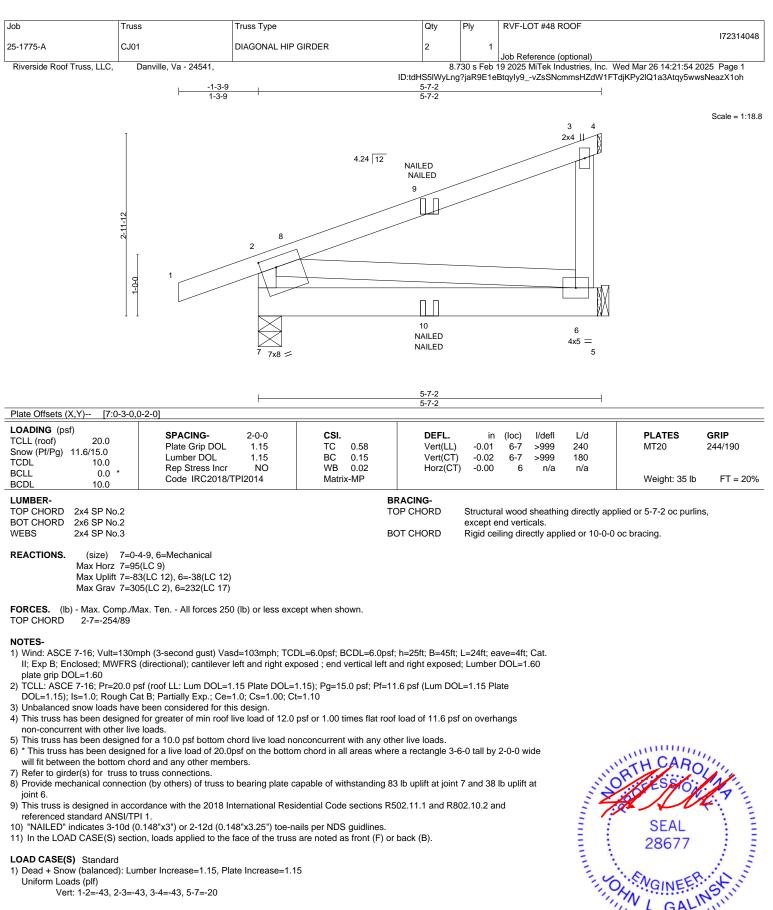
North Carolina COA: C-0844



March 28,2025

Galinski, John

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

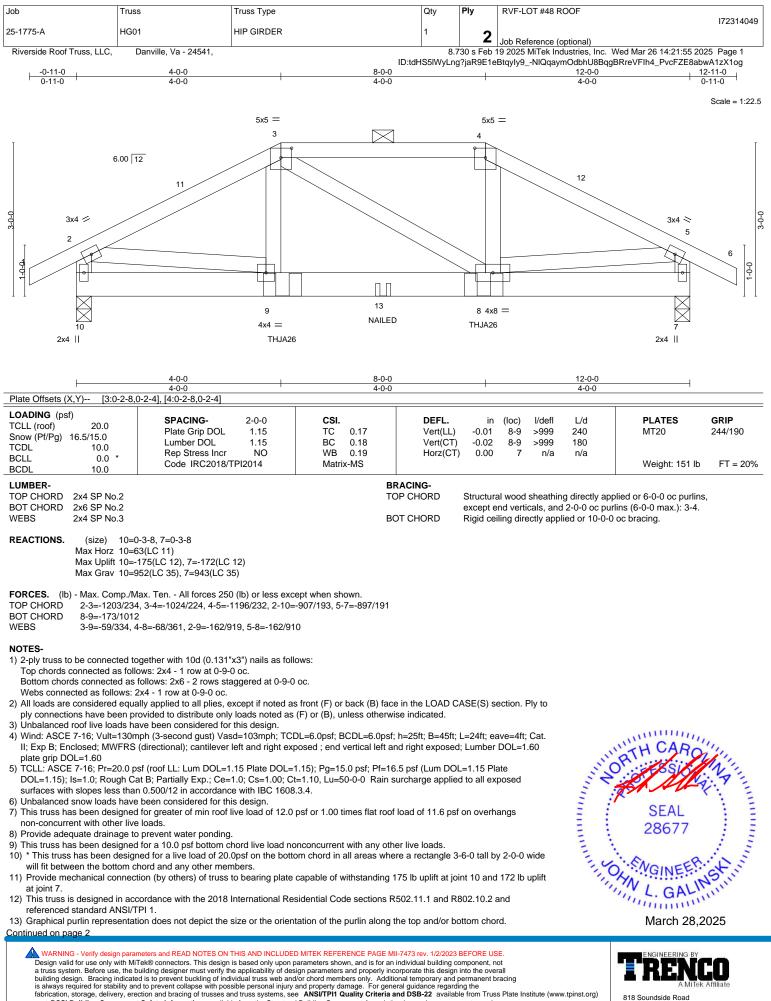
Vert: 1-2=-43, 2-3=-43, 3-4=-43, 5-7=-20



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818 Soundside Road Edenton, NC 27932

OHN



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF
					172314049
25-1775-A	HG01	HIP GIRDER	1	2	
				_	Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	730 s Feb	19 2025 MiTek Industries, Inc. Wed Mar 26 14:21:55 2025 Page 2
		ID:tdH	S5IWyLng	g?jaR9E1e	Btqyly9NIQqaymOdbhU8BqgBRreVFIh4_PvcFZE8abwA1zX1og

14) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Left Hand Hip) or equivalent at 4-0-6 from the left end to connect truss(es) to front face of bottom chord.

15) Use Simpson Strong-Tie THJA26 (THJA26 on 2 ply, Right Hand Hip) or equivalent at 7-11-10 from the left end to connect truss(es) to front face of bottom chord.

16) Fill all nail holes where hanger is in contact with lumber.
17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

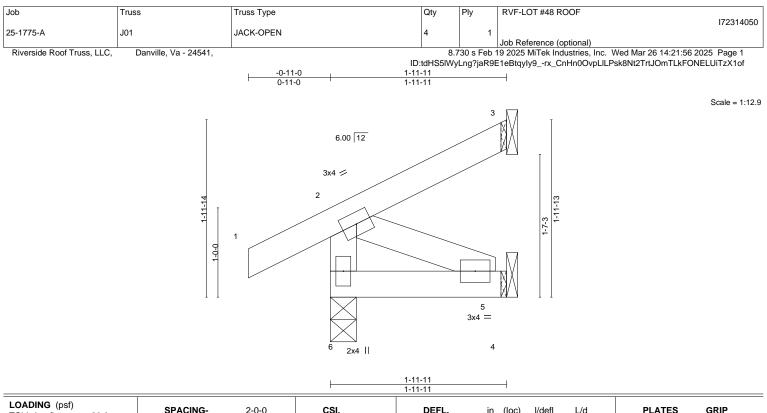
Uniform Loads (plf) Vert: 1-2=-43, 2-3=-43, 3-4=-53, 4-5=-43, 5-6=-43, 7-10=-20

Concentrated Loads (lb)

Vert: 9=-338(F) 8=-338(F) 13=-134(F)

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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.08 BC 0.03 WB 0.03	DEFL. in (loc) I/def Vert(LL) -0.00 6 >999 Vert(CT) -0.00 5-6 >999 Horz(CT) -0.00 3 n/d	180	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-MP	- (-)		Weight: 11 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 1-11-11 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 6=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 6=67(LC 16) Max Uplift 6=-20(LC 16), 3=-10(LC 13), 4=-14(LC 16)

Max Grav 6=159(LC 21), 3=37(LC 21), 4=36(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

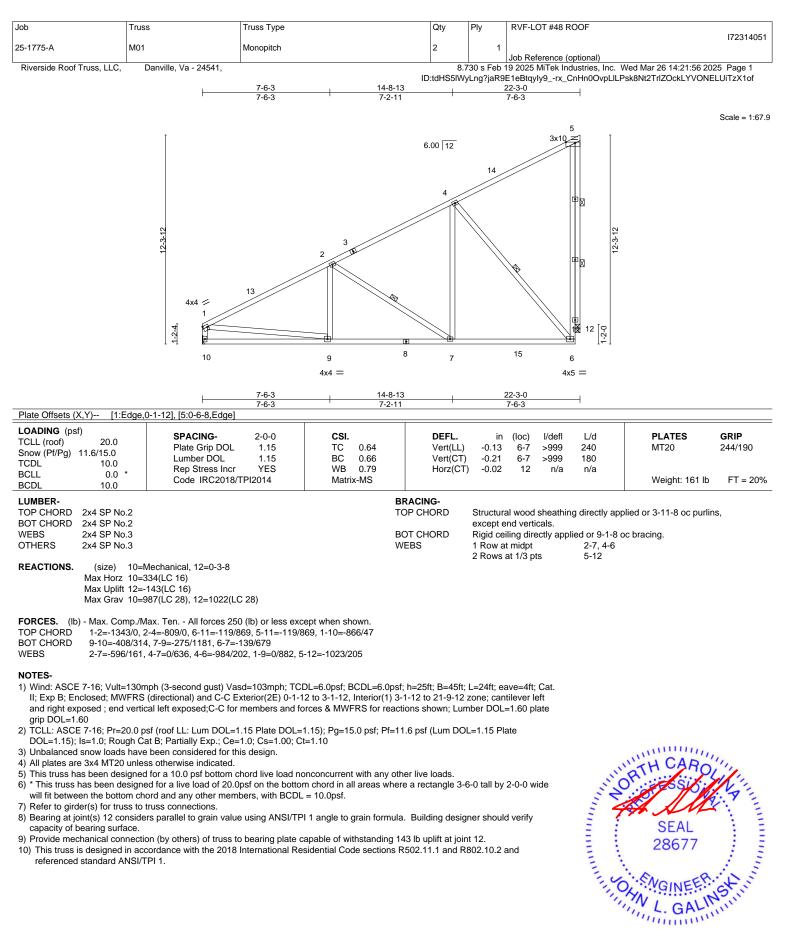
NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 6, 10 lb uplift at joint 3 and 14 lb uplift at joint 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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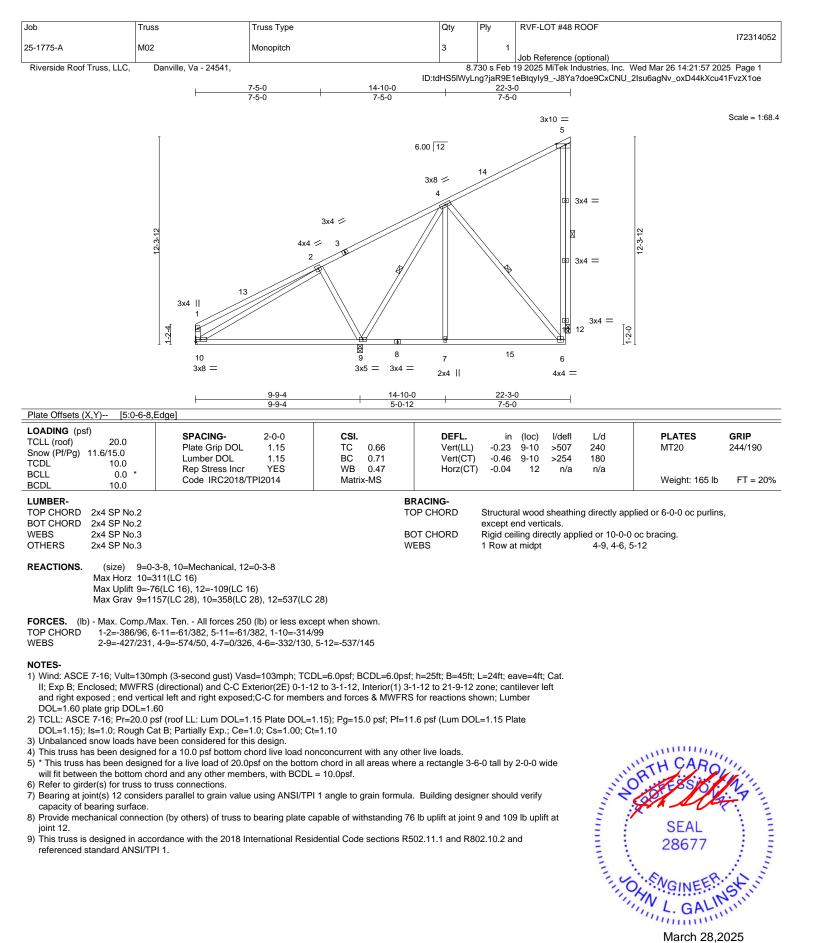
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



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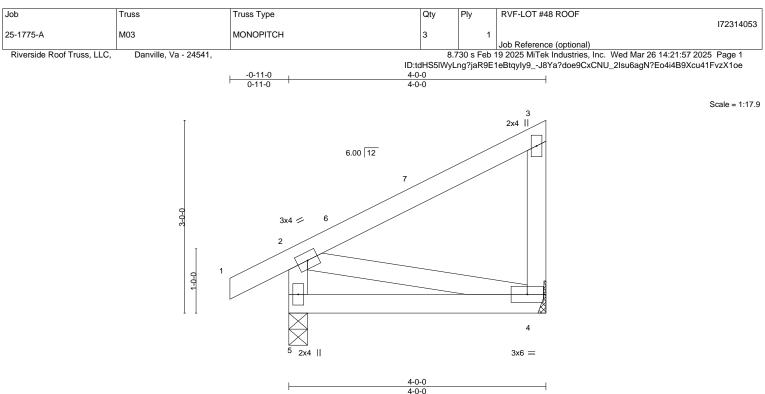


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	1		4-0-0						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.26 BC 0.16 WB 0.06 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 -0.00	(loc) 4-5 4-5 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 4=Mechanical Max Horz 5=97(LC 13) Max Uplift 5=-39(LC 16), 4=-26(LC 13)

Max Grav 5=23(LC 10), 4=25(LC 13)Max Grav 5=221(LC 2), 4=155(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

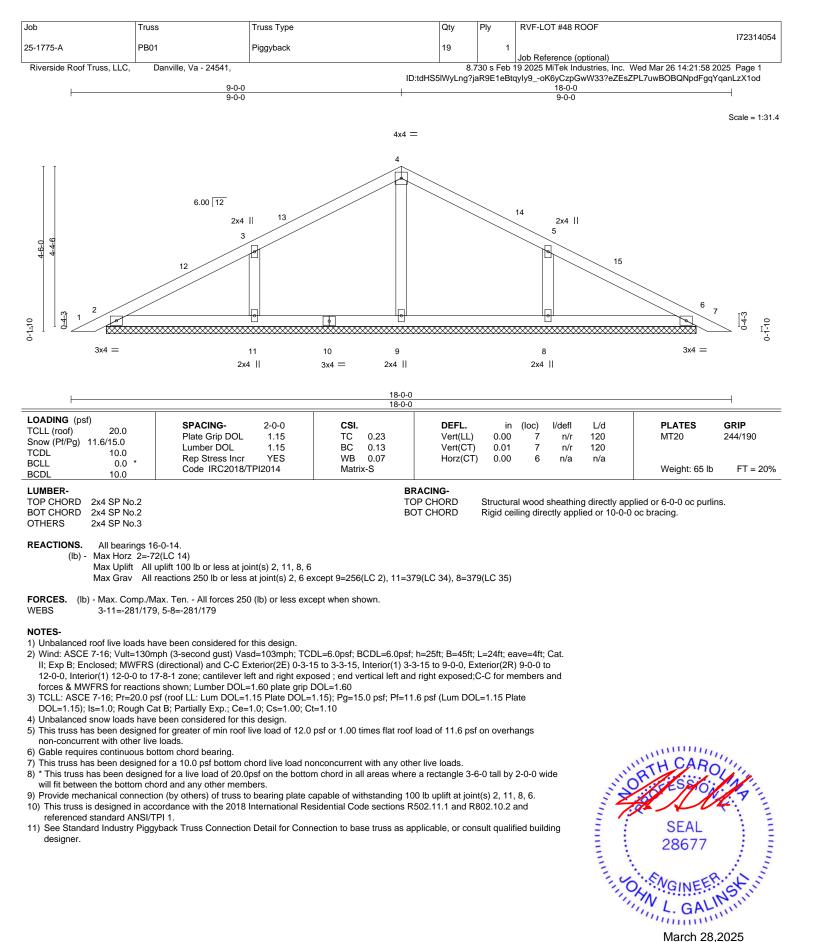
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 5 and 26 lb uplift at joint 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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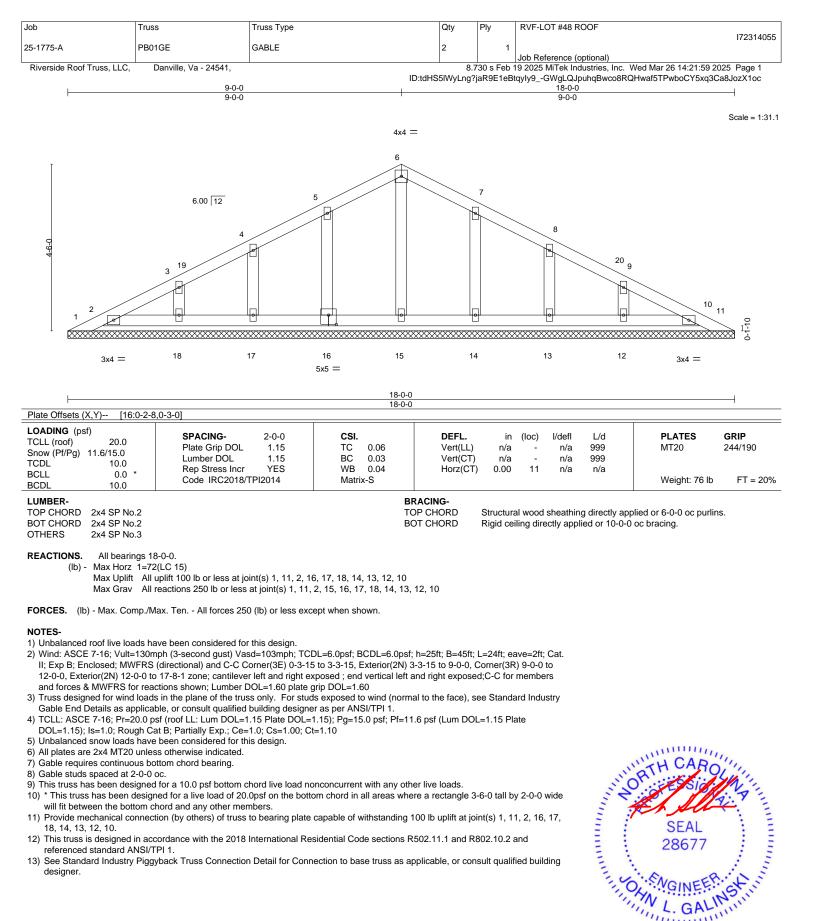
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A MiTek Affilia 818 Soundside Road



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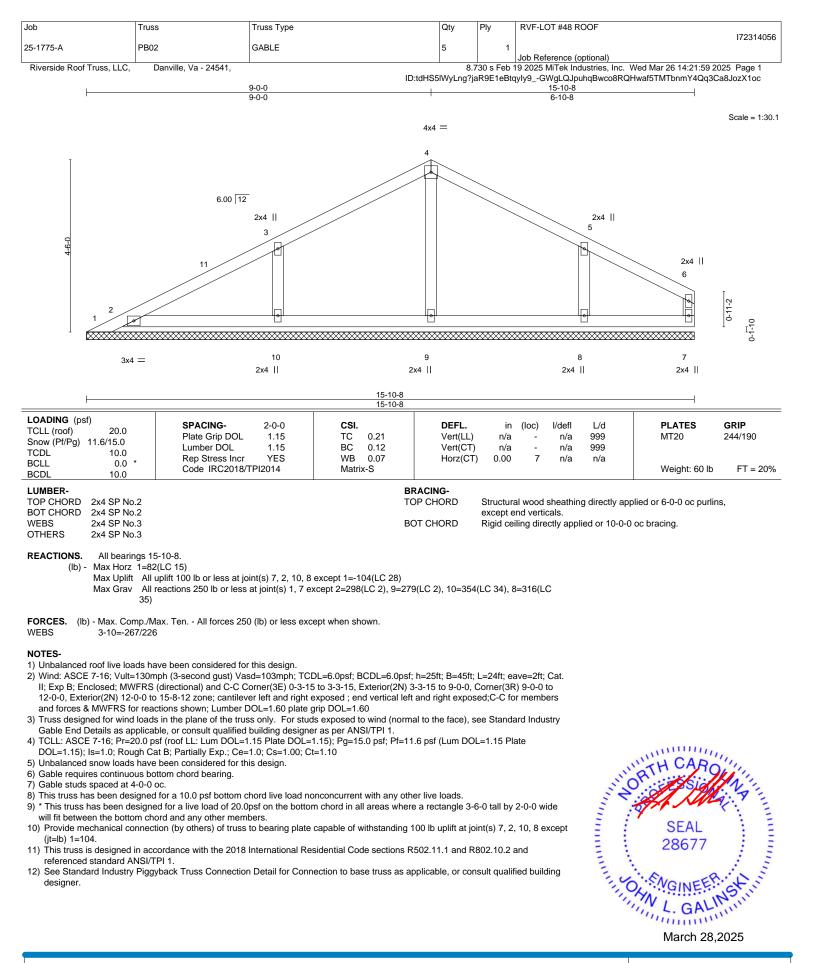




March 28,2025



A Mi Tek Affili 818 Soundside Road

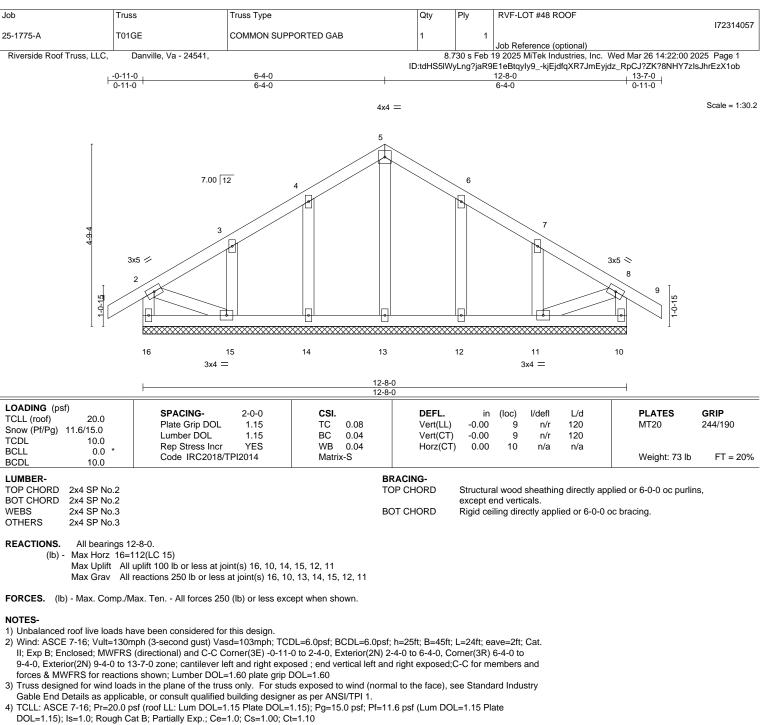


TRENGINEERING BY A MITCH Affiliate

818 Soundside Road

Edenton, NC 27932

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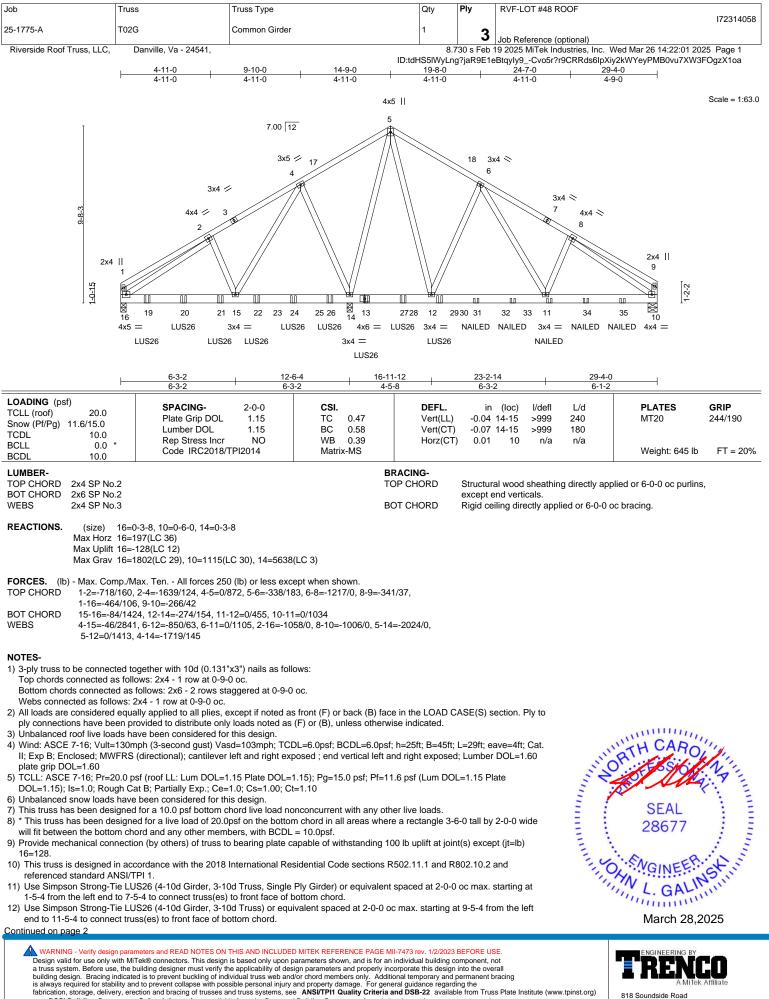
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

SEAL 28677

March 28,2025

TRENCO A MITCH AMITIN

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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

ſ	Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF
						172314058
	25-1775-A	T02G	Common Girder	1	2	
					3	Job Reference (optional)
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.7	'30 s Feb 1	9 2025 MiTek Industries, Inc. Wed Mar 26 14:22:01 2025 Page 2
			ID:tdF	-IS5IWyLng	g?jaR9E1e	Btqyly9Cvo5r?r9CRRds6lpXiy2kWYeyPMB0vu7XW3FOgzX1oa

13) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 13-5-4 from the left end to 17-5-4 to connect truss(es) to front face of bottom chord.

14) Fill all nail holes where hanger is in contact with lumber.
15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

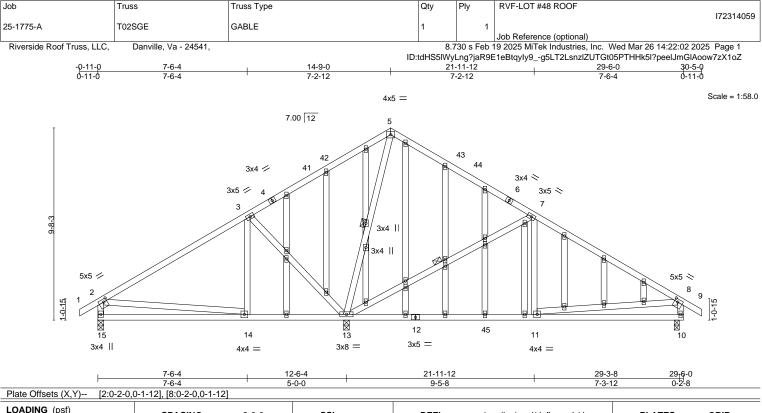
Vert: 1-5=-43, 5-9=-43, 10-16=-20

Concentrated Loads (lb)

Vert: 13=-246(F) 11=-132(F) 19=-501(F) 20=-501(F) 21=-501(F) 22=-499(F) 24=-686(F) 26=-686(F) 27=-246(F) 29=-246(F) 31=-133(F) 33=-132(F) 34=-132(F) 34=-132(F) 34=-132(F) 32=-132(F) 32=-1 35=-132(F)

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LOADING (pst) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	TC 0.71 \ BC 0.75 \	EFL. in (loc) l/defl L/d ert(LL) -0.19 11-13 >999 240 ert(CT) -0.35 11-13 >577 180 orz(CT) 0.02 10 n/a n/a	PLATES GRIP MT20 244/190 Weight: 256 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3		BRACING TOP CHO BOT CHO	RD Structural wood sheathing directly except end verticals.	

2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2x4 SP No.3 WEBS 1 Row at midpt 5-13, 7-13

REACTIONS. (size) 15=0-3-8, 13=0-3-8, 10=0-3-0 Max Horz 15=-210(LC 14) Max Uplift 15=-59(LC 16), 13=-78(LC 16), 10=-71(LC 16) Max Grav 15=466(LC 34), 13=1559(LC 28), 10=719(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-362/68, 3-5=0/372, 7-8=-746/78, 2-15=-394/108, 8-10=-619/111

BOT CHORD 14-15=-104/461, 13-14=-111/271, 11-13=0/544, 10-11=-66/284

WEBS 5-13=-569/37, 7-13=-823/118, 7-11=0/396, 3-13=-629/143, 8-11=0/303

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=30ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 14-9-0, Exterior(2R) 14-9-0 to 17-9-0, Interior(1) 17-9-0 to 30-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

All plates are 2x4 MT20 unless otherwise indicated. 7)

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

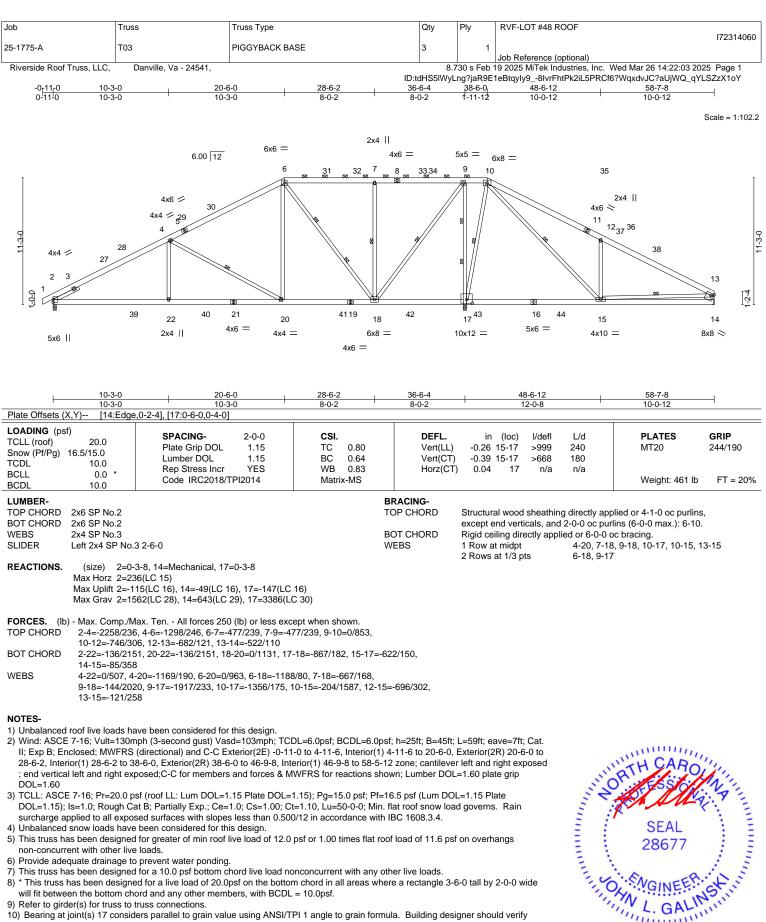
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 13, 10. 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



March 28,2025

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- Refer to girder(s) for truss to truss connections.
- 10) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (it=lb) Continue 10501 7age72

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

March 28,2025

minin



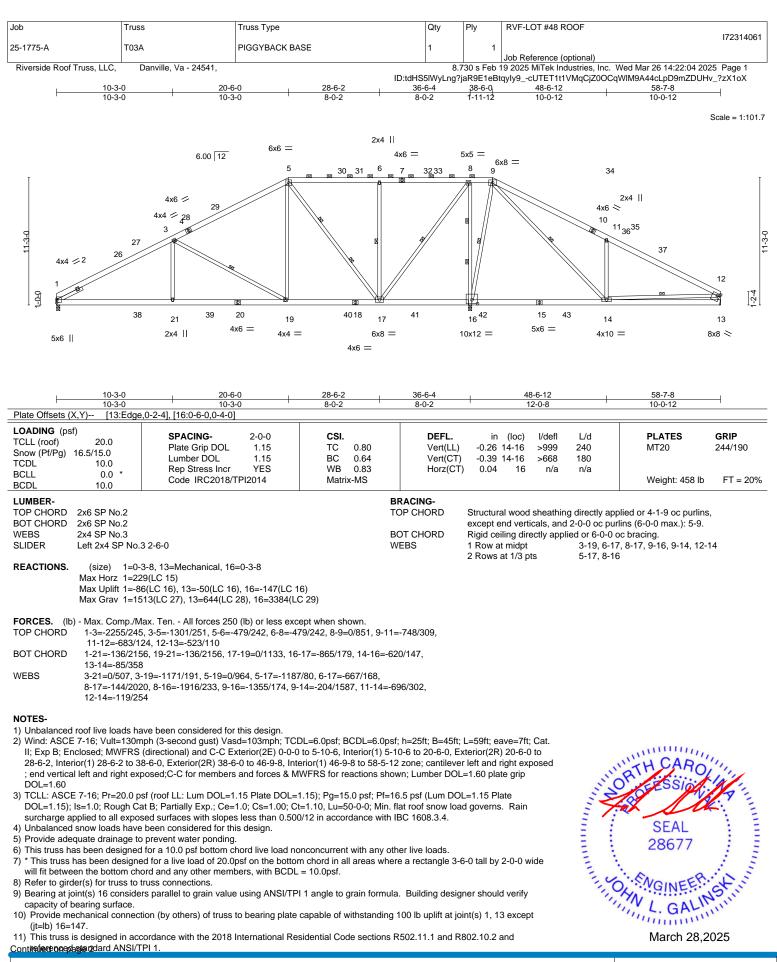
Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF
					172314060
25-1775-A	T03	PIGGYBACK BASE	3	1	
					Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.7	730 s Feb 1	19 2025 MiTek Industries, Inc. Wed Mar 26 14:22:03 2025 Page 2
		ID:t	dHS5lWyL	ng?jaR9E	1eBtqyIy98IvrFhtPk2iL5PRCf6?WqxdvJC?aUjWQ_qYLSZzX1oY

12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





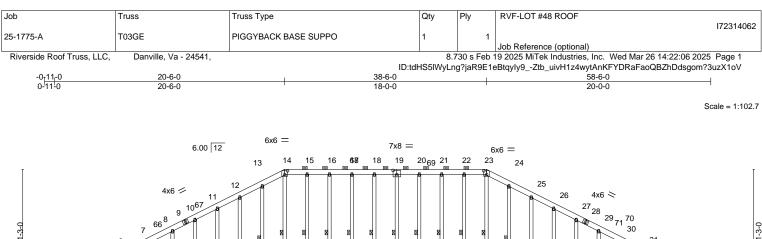
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

[Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF	
						172314061	
	25-1775-A	T03A	PIGGYBACK BASE	1	1		
						Job Reference (optional)	
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.7	'30 s Feb '	19 2025 MiTek Industries, Inc. Wed Mar 26 14:22:04 2025 Page 2	
			ID:tdHS5IWyLng?jaR9E1eBtqyIy9cUTET1t1VMqCjZ0OCqWIM9A44cLpD9mZDUHv_?zX1oX				

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





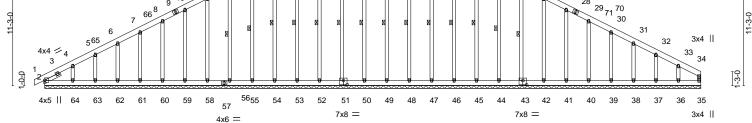


Plate Offsets (X,Y) [14:0-3-0.0-4-0], [19:0-4-0.0-4-8], [23:0-3-0.0-4-0], [43:0-4-0.0-4-8], [51:0-4-0.0-4-8] LOADING (psf) SPACING- 2-0-0 CSL in (loc) l/deft L/d MT20 244/190 Snow (Pt/Pg) 16:5/15.0 Lumber DOL 1.15 EC 0.05 Vert(LL) -0.00 1 n/r 120 MT20 244/190 BCDL 0.0 Rep Stress Incr YES WB 0.17 Horz(CT) 0.01 1 n/r 120 Weight: 588 ib FT = 2C LUMBER Code IRC2018/TPI2014 Matrix-S BRACING- TOP CHORD 2x65 PN 0.2 except end verticals, and 2-0-0 co putins, 6-0-0 max, 114-23. WEBS 2x4 SP No.3 BOT CHORD Ref 2x4 SP No.3 BOT CHORD Reg 2x4 SP No.3 2-45, 22-46, 24-4, 20-48, 19-49, 18-50, 17-51, 16-52, 15-53, 14-54, 13-55, 12-56, 24-42, 25-43 SUDER Left 2x4 SP No.3 BOT CHORD Nu paint Au upint 100 to r less at joint(s) 35, 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36 BOT CHORD 11-0-2-12/3/24, 13-454, 13-55, 12-56, 24-42, 25-43 FORCES. (b) - Max. C			58-6- 58-6-						
TCLL (roof) 20.0 SPACING- 20-0 CSL DEFL in (loc) Udelt L/d PLATES GRIP Snow (Pt/Pg) 16.5/15.0 Lumber DOL 1.15 TC 0.14 Vert(LL) -0.00 1 n/r 120 MT20 244/190 BCLL 0.0* Rep Stress Incr YES WB 0.17 Horz(CT) 0.00 1 n/r 120 MT20 244/190 BCDL 10.0 Rep Stress Incr YES WB 0.17 Horz(CT) 0.01 35 n/a n/a Weight: 588 lb FT = 2C LUMBER- Code IRC2018/TP12014 Matrix-S BRACING- TOP CHORD Structural wood sheathing directly applied or 60-00 co purlins, except end verticals, and 2-0-0 co purlins, 60-00 max.): 14-23. WEBS 1 Row at midpt 23-45, 22-46, 21-47, 20-48, 19-49, 18-50, 17-51, 16-52, 15-53, 14-54, 13-55, 12-56, 24-44, 25-43 24-44, 25-43 24-44, 25-43 REACTIONS. All bearings 58-60. WEBS 1 Row at midpt 23-45, 14-54, 13-55, 12-56, 24-54, 14-54, 13-55, 12-56, 59, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36 24-44, 25-43 24-44, 25-43 FORCES. (lb)	Plate Offsets (X,Y) [14:0-3	-0,0-4-0], [19:0-4-0,0-4-8], [23:0-3-0,0-4-0							
TOP CHORD 2x6 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 cc purlins, except end verticals, and 2-0-0 cc purlins (6-0-0 max.): 14-23. WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing. OTHERS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing. SLIDER Left 2x4 SP No.3 1-6-4 WEBS 1 Row at midpt 23-45, 22-46, 21-47, 20-48, 19-49, 18-50, 17-51, 16-52, 15-53, 14-54, 13-55, 12-56, 24-44, 25-43 REACTIONS. All bearings 58-6-0. 17-51, 16-52, 15-53, 14-54, 13-55, 12-56, 24-44, 25-43 (b) Max Horz 2=234(LC 15) Max Uplift All uplift 100 bor less at joint(s) 35, 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 33, 42, 41, 40, 39, 38, 37, 36 FORCES. (b) Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 11-12=-122/253, 12-13=-125/301, 13-14=-138/339, 14-15=-124/324, 15-16=-124/324, 16-17=-124/324, 16-17=-124/324, 16-17=-124/324, 12-12=-123/324, 20-21=-123/324, 20-21=-123/324, 20-21=-123/324, 21-22=-123/324, 22-23=-124/300, 25-26=-106/252 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=2ft; Cat.	TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0	Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	TC 0.14 BC 0.05 WB 0.17	Vert(LL) Vert(CT)	-0.00 0.00	1 n/r 1 n/r	120 120	MT20	
 (lb) - Max Horz 2=234(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 35, 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 33, 42, 41, 40, 39, 38, 37, 36 Max Grav All reactions 250 lb or less at joint(s) 35, 2, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36 FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 11-12=-122/253, 12-13=-125/301, 13-14=-138/339, 14-15=-124/324, 15-16=-124/324, 15-16=-124/324, 15-16=-124/324, 12-12=-123/324, 18-19=-123/324, 19-20=-123/324, 20-21=-123/324, 20-21=-123/324, 21-22=-123/324, 22-23=-124/323, 23-24=-138/339, 24-25=-124/300, 25-26=-106/252 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=2ft; Cat. 	LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3	0.3 1-6-4	Т	OP CHORD OT CHORD	except end v Rigid ceiling	erticals, and directly app	d 2-0-0 oc pui lied or 10-0-0 23-45, 22 17-51, 16	rlins (6-0-0 max.): 14-23 oc bracing. -46, 21-47, 20-48, 19-4 -52, 15-53, 14-54, 13-5	3. 9, 18-50,
NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=2ft; Cat.	Max Uplift A 63, Max Grav A 59, FORCES. (Ib) - Max. Comp./ TOP CHORD 11-12=-122/ 16-17=-124/3	II uplift 100 lb or less at joint(s) 35, 2, 46, 4, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36 II reactions 250 lb or less at joint(s) 35, 2, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39 Max. Ten All forces 250 (lb) or less exc 253, 12-13=-125/301, 13-14=-138/339, 14 324, 17-18=-123/324, 18-19=-123/324, 15	45, 46, 47, 48, 49, 50, 5 , 38, 37, 36 ept when shown. I-15=-124/324, 15-16=-1 D-20=-123/324, 20-21=-1	1, 52, 53, 54, 55, 5 24/324, 23/324,					
 9) Gable requires continuous bottom chord bearing. 10) Gable studs spaced at 2-0-0 oc. 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 	 NOTES- Unbalanced roof live loads I Wind: ASCE 7-16; Vult=130 II; Exp B; Enclosed; MWFR 26-6-0, Exterior(2N) 26-6-0 exposed; end vertical left a grip DOL=1.60 Truss designed for wind loa Gable End Details as applic TCLL: ASCE 7-16; Pr=20.0 DOL=1.15); Is=1.0; Rough 0 surcharge applied to all exp Unbalanced snow loads hara This truss has been designe non-concurrent with other lii Provide adequate drainage All plates are 2x4 MT20 unh Gable requires continuous to 	have been considered for this design. Imph (3-second gust) Vasd=103mph; TCI S (directional) and C-C Corner(3E) -0-11- to 38-6-0, Corner(3R) 38-6-0 to 44-6-0, E nd right exposed;C-C for members and for ds in the plane of the truss only. For study cable, or consult qualified building designe psf (roof LL: Lum DOL=1.15 Plate DOL=- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Cf osed surfaces with slopes less than 0.500 we been considered for this design. ad for greater of min roof live load of 12.0 we loads. to prevent water ponding. ess otherwise indicated. bottom chord bearing.	DL=6.0psf; BCDL=6.0psi to to 4-11-6, Exterior(2N) xterior(2N) 44-6-0 to 58- orces & MWFRS for reac ls exposed to wind (norm r as per ANSI/TPI 1. 1.15); Pg=15.0 psf; Pf=1 t=1.10, Lu=50-0-0; Min. f)/12 in accordance with I	f; h=25ft; B=45ft; L 4-11-6 to 20-6-0, 4-4 zone; cantilev tions shown; Luml hal to the face), se 6.5 psf (Lum DOL lat roof snow load BC 1608.3.4.	Corner(3R) 2 ver left and rig ber DOL=1.6 ee Standard II =1.15 Plate I governs. Ra	20-6-0 to ght 0 plate ndustry ain	and a superior	SEAL 28677	111 A Contraction of the Contrac

Continued on page 2

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A MiTek Affi

⁸¹⁸ Soundside Road Edenton, NC 27932

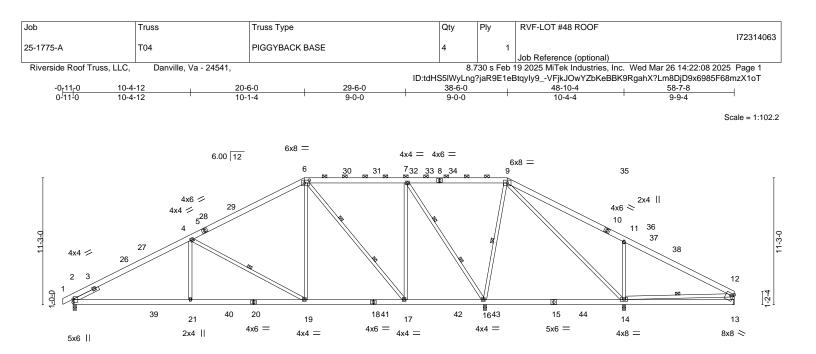
Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF	
					172314062	
25-1775-A	T03GE	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.7	'30 s Feb 1	9 2025 MiTek Industries, Inc. Wed Mar 26 14:22:07 2025 Page 2	
		ID:tdHS5IWyLng?jaR9E1eBtqyly9139M52wwoHCna1lzuy3S_nolYqXoQgt?vRWZbKzX1oU				

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 35, 2, 46, 47, 48, 49, 50, 51, 52, 55, 56, 58, 59, 60, 61, 62, 63, 64, 44, 43, 42, 41, 40, 39, 38, 37, 36.

14) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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I	10-4-12 10-4-12	20-6-0	<u>29-6-0</u> 9-0-0	<u>36-6-4</u> 7-0-4	48-10-4	<u>58-7-8</u> 9-9-4	
Plate Offsets (X,		0], [13:Edge,0-2-4]		704		004	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL BCDL	20.0 6.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.79 BC 0.68 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.21 14-16 >693 240 -0.31 14-16 >477 180 0.06 16 n/a n/a	PLATES MT20 Weight: 445 lb	GRIP 244/190 FT = 20%
LUMBER-			I	BRACING-			
	2x6 SP No.2 2x6 SP No.2			TOP CHORD	Structural wood sheathing directly ap except end verticals, and 2-0-0 oc pu		ns,
	2x4 SP No.3 *Exce 7-16: 2x4 SP No.1	Dt*	I	BOT CHORD	Rigid ceiling directly applied or 10-0- 6-0-0 oc bracing: 14-16.	0 oc bracing, Except:	
SLIDER I	Left 2x4 SP No.3 2-	6-0	1	WEBS	1 Row at midpt 4-19, 9-1	6, 12-14	

2 Rows at 1/3 pts

6-17, 7-16

REACTIONS. All bearings 0-3-8 except (jt=length) 13=Mechanical.

(lb) - Max Horz 2=236(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 16 except 2=-124(LC 16), 14=-135(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 13 except 2=1623(LC 28), 16=2882(LC 28), 14=1062(LC 49)

- TOP CHORD 2-4=-2368/257, 4-6=-1404/268, 6-7=-517/265, 7-9=0/641, 9-11=-42/489, 11-12=-83/412
- BOT CHORD
 2-21=-150/2245, 19-21=-150/2245, 17-19=0/1253, 16-17=0/546, 14-16=-465/137

 WEBS
 4-21=0/508, 4-19=-1168/190, 6-19=0/998, 6-17=-1214/61, 7-17=0/1209, 7-16=-1980/179, 9-16=-769/135, 9-14=-24/274, 11-14=-714/307, 12-14=-496/170

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-11-6, Interior(1) 4-11-6 to 20-6-0, Exterior(2R) 20-6-0 to 28-9-8, Interior(1) 28-9-8 to 38-6-0, Exterior(2R) 38-6-0 to 46-9-8, Interior(1) 46-9-8 to 58-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16 except (jt=lb) 2=124, 14=135.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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March 28,2025



818 Soundside Road

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

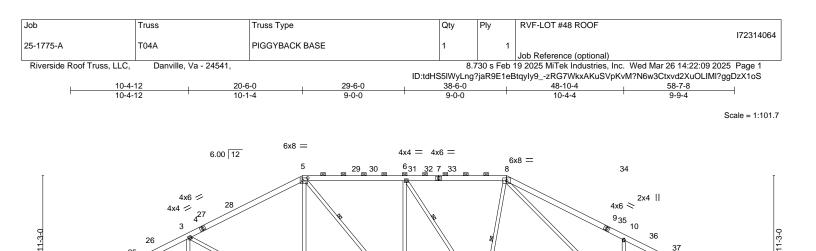


Plate Offse	⊢ 10-4-12 10-4-12 ets (X,Y) [5:0-5-4,0-	20-6-0 10-1-4 3-0], [12:Edge,0-2-4]	29-6-0 9-0-0	36-6-4 7-0-4	48-10-4 12-4-0	58-7-8 9-9-4
LOADING TCLL (roof Snow (Pf/F TCDL BCLL BCDL	f) 20.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.79 BC 0.67 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT		PLATES GRIP MT20 244/190 Weight: 443 lb FT = 20%
LUMBER- TOP CHOI BOT CHOI WEBS	RD 2x6 SP No.2	cept*		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly a except end verticals, and 2-0-0 oc p Rigid ceiling directly applied or 10-0-	urlins (6-0-0 max.): 5-8.

WEBS

1740

 $4x6 \equiv$

16

4x4 =

41

1542

4x4 =

14 43

5x6 =

6-0-0 oc bracing: 13-15.

1 Row at midpt 2 Rows at 1/3 pts

02.02.1	
REACTIONS.	All bearings 0-3-8 except (it=length) 12=Mechanical.
REACTIONS.	All bearings 0-3-6 except (Jt=length) 12=lwechanical.

REACTIONS. (lb) -

SLIDER

4x4 ≠2

5x6 ||

38

39

20

2x4 ||

19

4x6 =

18

4x4 =

9-9-

Max Horz 1=229(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 1, 15 except 13=-135(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 12 except 1=1573(LC 27), 15=2879(LC 27), 13=1063(LC 48)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-2362/267, 3-5=-1407/273, 5-6=-518/269, 6-8=0/638, 8-10=-41/487, 10-11=-83/410

BOT CHORD 1-20=-150/2251, 18-20=-150/2251, 16-18=0/1255, 15-16=0/547, 13-15=-462/133 WEBS 3-20=0/508, 3-18=-1171/190, 5-18=0/999, 5-16=-1214/61, 6-16=0/1209, 6-15=-1980/179, 8-15=-766/134, 8-13=-24/272, 10-13=-714/307, 11-13=-495/170

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 5-10-6, Interior(1) 5-10-6 to 20-6-0, Exterior(2R) 20-6-0 to 28-9-8, Interior(1) 28-9-8 to 38-6-0, Exterior(2R) 38-6-0 to 46-9-8, Interior(1) 46-9-8 to 58-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) Unbalanced snow loads have been considered for this design.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.

6-15: 2x4 SP No.1

Left 2x4 SP No 3 2-6-0

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 15 except (jt=lb) 13=135

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



37

13

4x8 =

3-18, 8-15, 11-13

5-16. 6-15

11

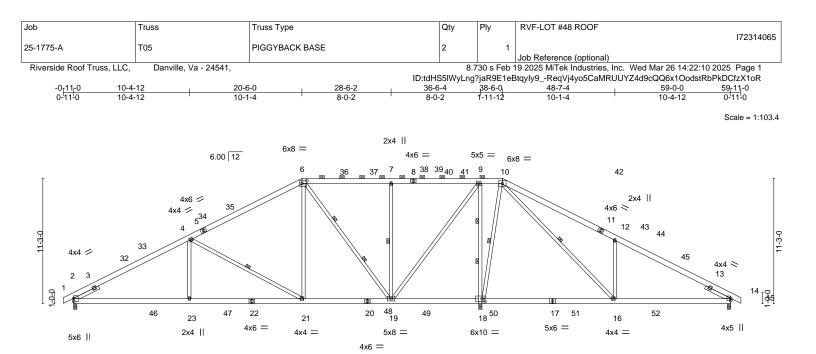
12

8x8 🛸

-2-4-

March 28,2025





10-4-12	20-6-0	28-6-2	36-6-4	48-7-4	1	59-0-0	_
10-4-12 Plate Offsets (X,Y) [6:0-5-4	<u> </u>	8-0-2	8-0-2	12-1-0		10-4-12	·
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.77 BC 0.67 WB 0.86	DEFL. Vert(LL) Vert(CT) Horz(CT		L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-MS				Weight: 454 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No	o.3 2-6-0, Right 2x4 SP No.3 2-6-0	T	BRACING- FOP CHORD BOT CHORD WEBS	Structural wood sheathing 2-0-0 oc purlins (6-0-0 ma Rigid ceiling directly appli 1 Row at midpt 2 Rows at 1/3 pts	ax.): 6-10.	acing.	, except
Max Horz 2=2 Max Uplift 2=-)-3-8, 14=0-3-8, 18=0-3-8 222(LC 15) 130(LC 16), 14=-104(LC 16), 18=-109(L 1598(LC 28), 14=828(LC 29), 18=3321(L						
TOP CHORD 2-4=-2320/2	Max. Ten All forces 250 (lb) or less ex 71, 4-6=-1345/276, 6-7=-532/276, 7-9=-5						
	349, 12-14=-810/170 207, 21-23=-128/2207, 19-21=0/1203, 18 02	3-19=-771/137, 16-18=-5	27/115,				
	4-21=-1184/185, 6-21=0/964, 6-19=-117 978, 9-18=-1897/208, 10-18=-1353/166,	, , ,	=-698/297				
 9-19=-124/1978, 9-18=-1897/208, 10-18=-1353/166, 10-16=-213/1706, 12-16=-698/297 NOTES- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; E=59ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-11-13, Interior(1) 4-11-13 to 20-6-0, Exterior(2R) 20-6-0 to 28-10-2, Interior(1) 28-10-2 to 38-6-0, Exterior(2B) 38-6-0 to 46-10-2, Interior(1) 44-10-2 to 59-11-0 zone; cantilever left and right exposed; cnd vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 S) TCLL: ASCE 7-16; Vult=130mpt (xpc = 1.15): Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15): Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15; IS=1-0; Rough Cat B; Partially Exp; Ce=1.0; CS=-1.00; CL=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed for min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with ther live loads. Provide adequate drainage to prevent water pondig. This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will flt between the bottom chord and any other members, with BCDL = 10.0psf. Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2-13.0 14=104, 18=-109. This truss is designed in a accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Contreleterongelage adraf ANSI/TPI 1. 							
Design valid for use only with I	ameters and READ NOTES ON THIS AND INCLUDI diTek® connectors. This design is based only upon building designer must verify the applicability of de	parameters shown, and is for a	in individual building c	omponent, not			: ∩

TRENCO A MiTek Affiliate

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

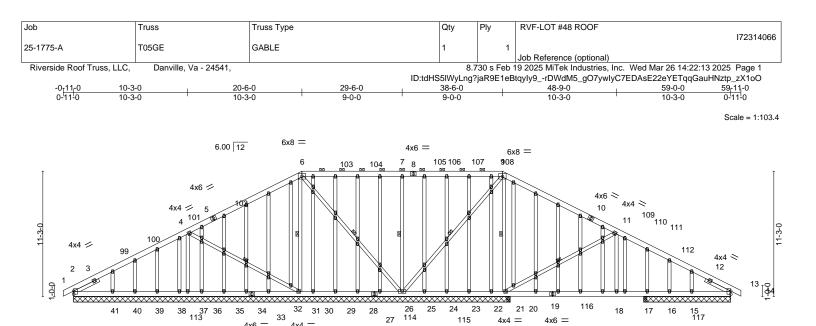


\	ob	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF	
						172314065	
2	5-1775-A	T05	PIGGYBACK BASE	2	1		
						Job Reference (optional)	
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	730 s Feb	9 2025 MiTek Industries, Inc. Wed Mar 26 14:22:10 2025 Page 2	
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9ReqVj4yo5CaMRUUYZ4d9cQQ6x1OodstRbPkDCfzX1oR					

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





 $4x6 \equiv 4x8 \equiv$

 $4x4 \equiv$

4x6 =

4x6 ||

⊢	10-3-0 10-3-0	20-6-0		<u>29-6-0</u> 9-0-0			<u>38-6-0</u> 9-0-0	39-2-4 0-8-4		48-9-0 9-6-12		51-5-12 2-8-12	59-0-0 7-6-4	
		10-3-0		9-0-0			9-0-0	0-0-4		9-0-12		2-0-12	7-0-4	
LOADING (ps TCLL (roof)	1) 20.0	SPACING-	2-0-0	CSI.			DEFL.		(loc)	l/defl	L/d		PLATES	GRIP
Snow (Pf/Pg)		Plate Grip DOL	1.15	тс	0.69		Vert(LL)	-0.07		>999	240		MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.46		Vert(CT)	-0.13		>999	180			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.67		Horz(CT)	0.02	95	n/a	n/a			
BCDL	10.0	Code IRC2018/TF	912014	Matrix	x-MS								Weight: 710 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS SLIDER REACTIONS.	2x6 SP No.2 2x4 SP No.3 2x4 SP No.3 Left 2x4 SP No.	3 2-6-0, Right 2x4 SP No 3-2-4 except (it=length) 1:		7-9-12. 17=7	-9-12. 16	toi BO WE	T CHORD BS	2-0-0 oc Rigid ceil 1 Row at	purlins ling dire midpt	(10-0-0 r ectly appl	nax.): 6 ied or 6	-9. -0-0 oc bra	or 6-0-0 oc purlins Icing. 5, 7-25, 9-25, 9-2	, I
(lb) -	Max Horz 2=-2 Max Uplift All	22(LC 14) uplift 100 lb or less at join	t(s) 2, 31, 13, 4	,	,									
		-376(LC 7), 16=-132(LC	,	20.20.20	24 25 2		20 40 24 22	22 47						
		reactions 250 lb or less a 6 except 2=369(LC 54),) 20-1	022/1 C 1	D.			
		639(LC 29), 41=363(LC 2						55(LC 49), 20=1	032(LC),			
BOT CHORD	36-37=-78/298 24-25=-277/13 18-20=-33/640 4-37=-497/158	40-41=-78/298, 39-40=-7 8, 35-36=-78/298, 34-35= 33, 23-24=-277/133, 22-2 0, 17-18=-33/640, 16-17= 8, 4-31=-283/165, 6-31=-7 11-20=-1014/190, 11-18	-78/298, 32-34 3=-277/133, 21 -33/640, 15-16 312/54, 6-25=-2	=-78/298, 31 -22=-277/13 =-33/640, 13	-32=-78/ 33, 20-21 3-15=-33/	/298, =-277 /640	7/133,							
NOTES-													annun.	
 Wind: ASCE II; Exp B; Er to 28-10-2, I exposed ; ei grip DOL=1. Truss desigi 	7-16; Vult=130m nclosed; MWFRS Interior(1) 28-10-2 nd vertical left and .60 ned for wind loads	ave been considered for t hph (3-second gust) Vaso (directional) and C-C Ext 2 to 38-6-0, Exterior(2R) 3 d right exposed;C-C for m s in the plane of the truss ble, or consult gualified b	=103mph; TCE erior(2E) -0-11 88-6-0 to 46-10 nembers and fo only. For stud	-0 to 4-11-13 -2, Interior(1 prces & MWF Is exposed to	3, Interior) 46-10-2 RS for re o wind (n	r(1) 4 2 to 5 eactio	-11-13 to 20-6-0 9-11-0 zone; car ons shown; Lumb	, Exterior ntilever le per DOL=	(2R) 20 eft and =1.60 p	Cat. D-6-0 right late istry	annun,	NORT	SEAL 28677	NA STREET
		sf (roof LL: Lum DOL=1.1				f=16.	5 psf (Lum DOL:	=1.15 Pla	ate		Ξ		28677	- 1 - E
		at B; Partially Exp.; Ce=1									-	1		1 3
surcharge a	pplied to all expo	sed surfaces with slopes	less than 0.500)/12 in accor	dance wi	ith IB	C 1608.3.4.	0					oa	123
,		e been considered for this	0									0.5	NGINEE	45
,	0	for greater of min roof liv	e load of 12.0	psf or 1.00 t	imes flat	roof l	oad of 11.6 psf o	on overha	angs			1. MA	1	12.11
	ent with other live											111	L. GAL	111
		o prevent water ponding. ss otherwise indicated.											munn	
	spaced at 2-0-0												March 28.2	025
		ed for a 10.0 psf bottom c	hord live load r	onconcurrer	nt with ar	nv oth	er live loads.						101011 20,2	.020
Design val a truss sys building de	id for use only with Mi stem. Before use, the b esign. Bracing indicate	meters and READ NOTES ON TT Tek® connectors. This design is uilding designer must verify the ed is to prevent buckling of indivi d to prevent collapse with possil ction and bracing of trusses and	based only upon p applicability of des dual truss web and	arameters show ign parameters l/or chord memb	wn, and is fo and proper pers only, A	or an ir Iy inco Additior	ndividual building cor rporate this design ir nal temporary and pe	nponent, no to the over ermanent br	ot all acing	stitute (wwv	v.tpinst.or		ENGINEERING BY REENG A MITEK	LO Affiliate

and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

4x6 =

4x6 ||

4x4 =

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF		
					172314066		
25-1775-A	T05GE	GABLE	1	1			
					Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,	8.730 s Feb 19 2025 MiTek Industries, Inc. Wed Mar 26 14:22:14 2025 Page					
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9KP40ZR?J8R4nv6nJowh5mGapIep3Zjq1W1iRLQzX1oN					

11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

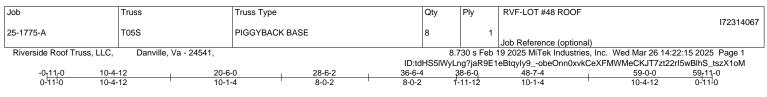
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 13, 40, 41, 17, 15, 2, 13 except (jt=lb) 37=124, 25=227, 21=376, 16=132.

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

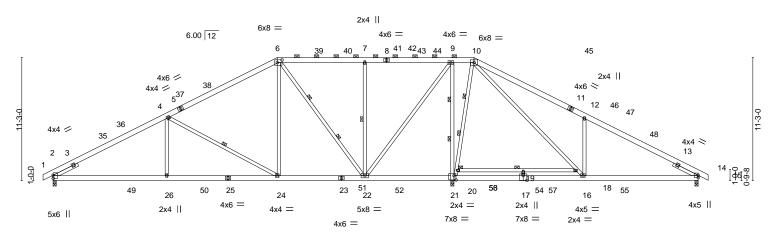
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Scale = 1:105.4



L	10-4-12	20-6-0	28-6-2		36-6-4	48-7-4			59-0-0	
Plate Offsets ()	10-4-12 X X) [6:0-5-4.0	<u> </u>	8-0-2		8-0-2	12-1-0)	1	10-4-12	
LOADING (psi TCLL (roof) Snow (Pf/Pg) TCDL BCLL	20.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC BC WB	0.76 0.87 1.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.37 19-20 -0.58 19-20) 0.03 21		L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2018/TPI2014	Matri	x-MS					Weight: 471 lb	FT = 20%
	10.0				DD A OINIO					
LUMBER- TOP CHORD	2v6 SD No 2				BRACING- TOP CHORD	Structural wood	choothing (directly on	plied or 4-6-8 oc purlins	ovcont
	2x6 SP 2400F 2	0 F *Except*			TOP CHORD	2-0-0 oc purlins (, except
BOT CHORD		6 SP No.2, 18-20: 2x4 SP No.1			BOT CHORD		`	,	oc bracing. Except:	
WEBS	2x4 SP No.3 *E:				BOT ONORD	6-0-0 oc bracing				
WEBG	10-21,10-16: 2x				WEBS	1 Row at midpt	10 20	4-24, 7-22	2	
SLIDER		3 2-6-0, Right 2x4 SP No.3 2-6-0				2 Rows at 1/3 pt	S	6-22, 9-21		
REACTIONS.	Max Horz 2=22 Max Uplift 2=-1	3-8, 14=0-3-8, 21=0-3-8 22(LC 15) 41(LC 16), 14=-87(LC 16) 584(LC 28), 14=923(LC 29), 21=3717(L	C 28)							
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2291/291, 4-6=-1315/299, 6-7=-512/305, 7-9=-512/305, 9-10=0/801, 10-12=-1220/310, 12-14=-1048/121 BOT CHORD 2-26=-146/2182, 24-26=-146/2182, 22-24=0/1176, 21-22=-775/99, 17-21=-257/107, 16-17=-257/107, 14-16=-2/910, 19-20=-353/0, 18-19=-353/0 WEBS 4-26=0/523, 4-24=-1184/183, 6-24=0/970, 6-22=-1193/31, 7-22=-661/169, 9-22=-114/1951, 9-21=-1922/204, 20-21=-1645/66, 10-20=-1418/106, 10-18=-117/2036, 16-18=-142/1738, 12-16=-689/300, 17-19=-292/0										
NOTES-										
		ave been considered for this design.							MILLION MILLION	Gaine
II; Exp B; Er to 28-10-2, I	nclosed; MWFRS nterior(1) 28-10-2 nd vertical left and	nph (3-second gust) Vasd=103mph; TCl (directional) and C-C Exterior(2E) -0-11 2 to 38-6-0, Exterior(2R) 38-6-0 to 46-10 d right exposed;C-C for members and fo	-0 to 4-11-1 -2, Interior(1	3, Interior	(1) 4-11-13 to 20-6- to 59-11-0 zone; ca	0, Exterior(2R) 20 antilever left and ri	Cat. -6-0 ght ate	and a	SEAL 28677	N.S.
DOL=1.15); surcharge a	Is=1.0; Rough Ca pplied to all expo	sf (roof LL: Lum DOL=1.15 Plate DOL= at B; Partially Exp.; Ce=1.0; Cs=1.00; C sed surfaces with slopes less than 0.500 been considered for this design.	=1.10, Lu=5	60-0-0; Mir	n. flat roof snow loa				SEAL 28677	
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.								121		
		prevent water ponding.					1,0	D, VGINEE!	at s	
8) * This truss	has been designe	d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the botton ord and any other members, with BCDL	n chord in a			δ-0 tall by 2-0-0 wi	de	14	TAL. GALIN	11111
9) Bearing at jo		ers parallel to grain value using ANSI/TP		ula. Building desig	ner should verify			March 28.2	0.25	

9) Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Continued on page 2

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818 Soundside Road Edenton, NC 27932

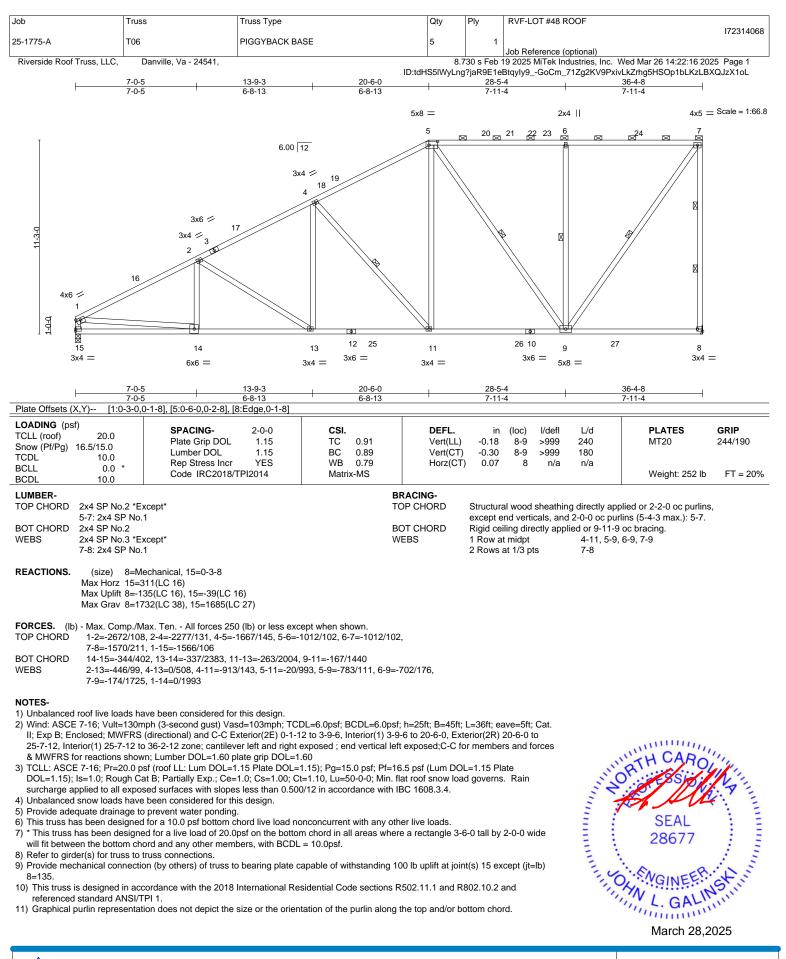
March 28,2025

Job	Truss	Truss Type	Qty	Ply	RVF-LOT #48 ROOF		
					172314067		
25-1775-A	T05S	PIGGYBACK BASE	8	1			
					Job Reference (optional)		
Riverside Roof Truss, LLC,	Danville, Va - 24541,	8.730 s Feb 19 2025 MiTek Industries, Inc. Wed Mar 26 14:22:15 2025 Page 2					
		ID:tdHS5IWyLng?jaR9E1eBtqyIy9obeOnn0xvkCeXFMWMeCKJT7zt22rI5wBlhS_tszX1oM					

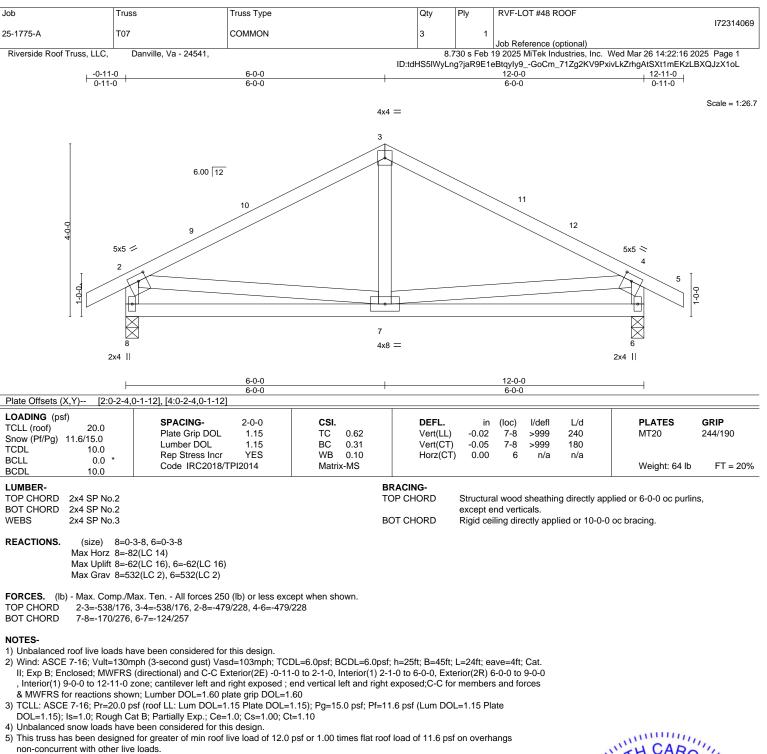
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=141.
11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

a) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

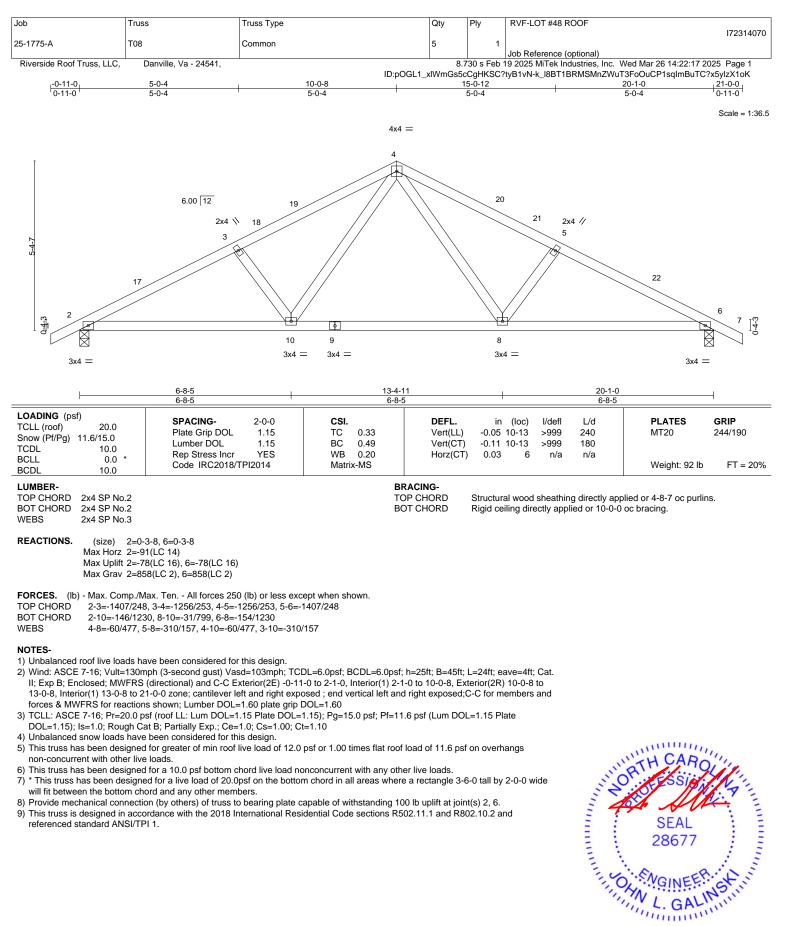
referenced standard ANSI/TPI 1.



March 28,2025

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A MiTek Afr 818 Soundside Road

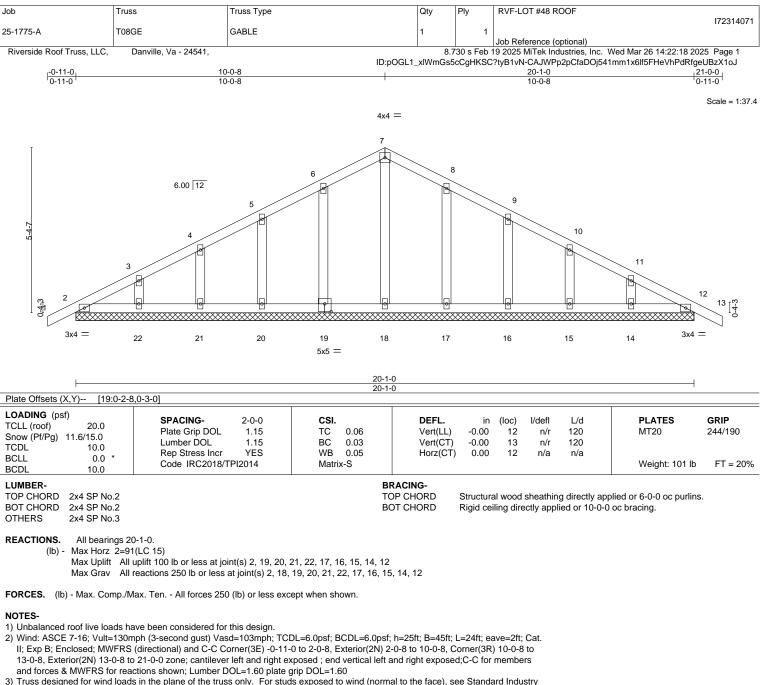


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818 Soundside Road



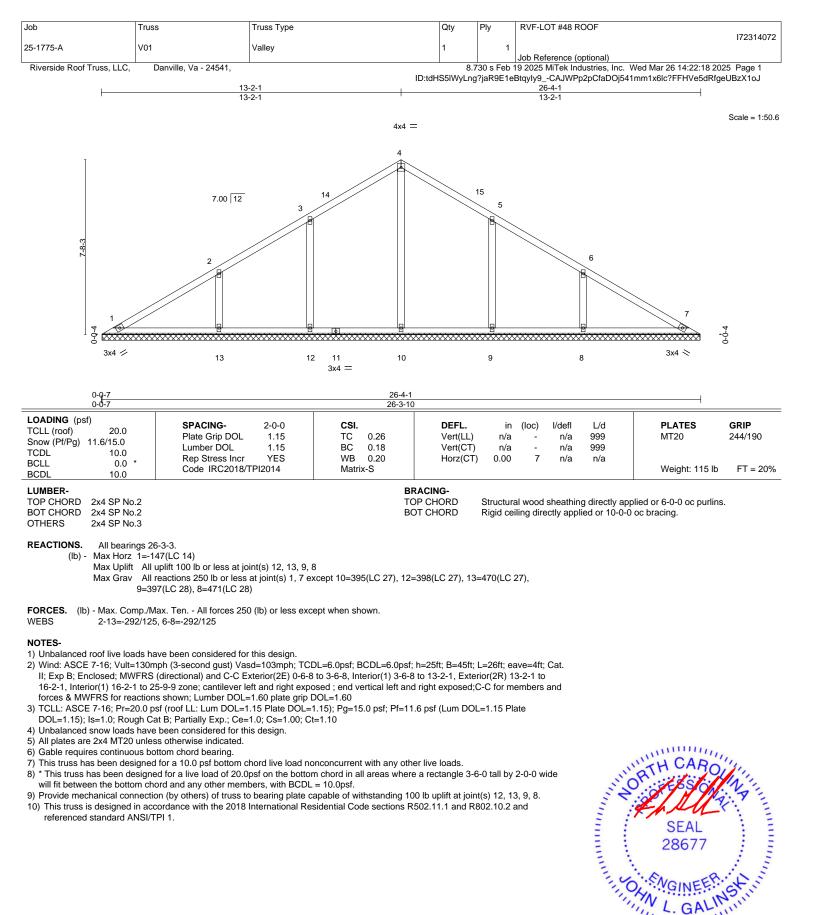
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, 12.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



March 28,2025

TRENCO

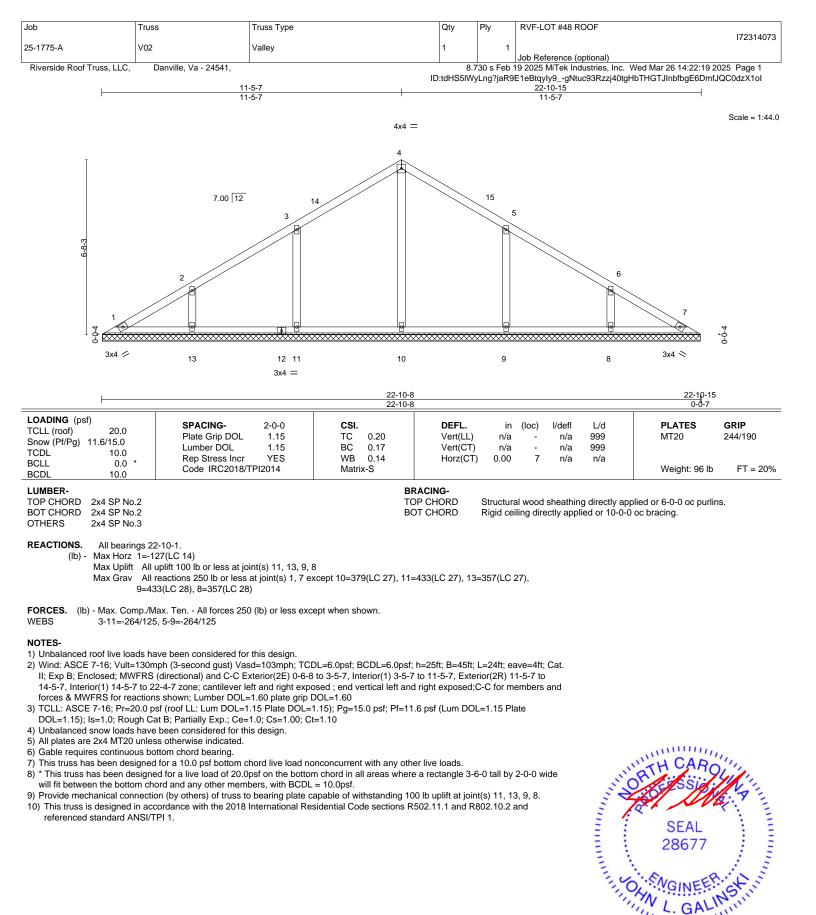
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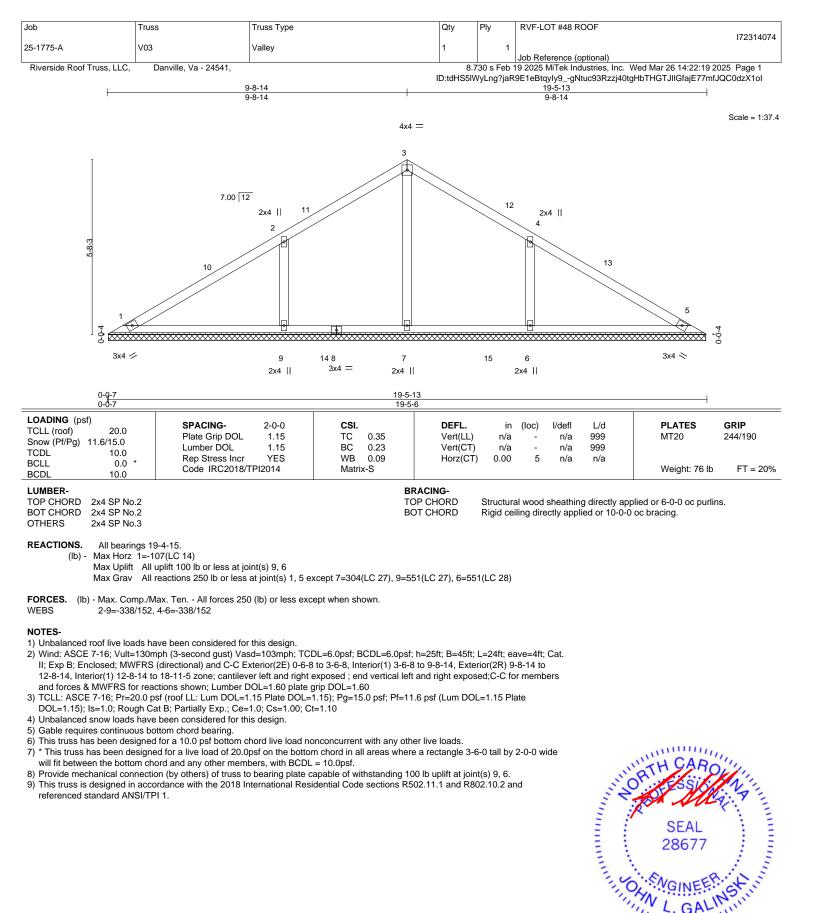


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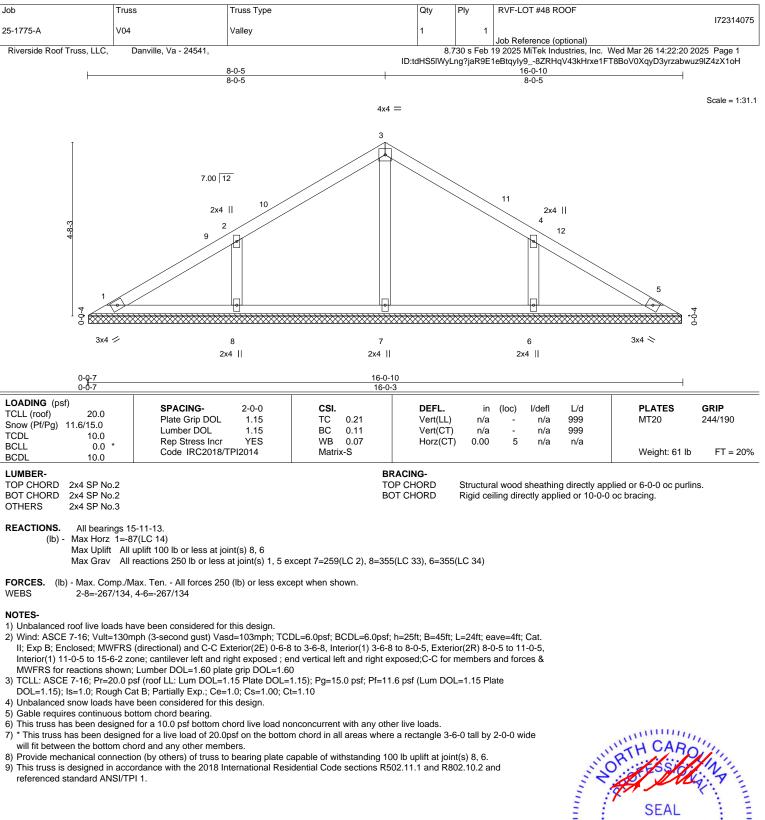




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818 Soundside Road Edenton, NC 27932

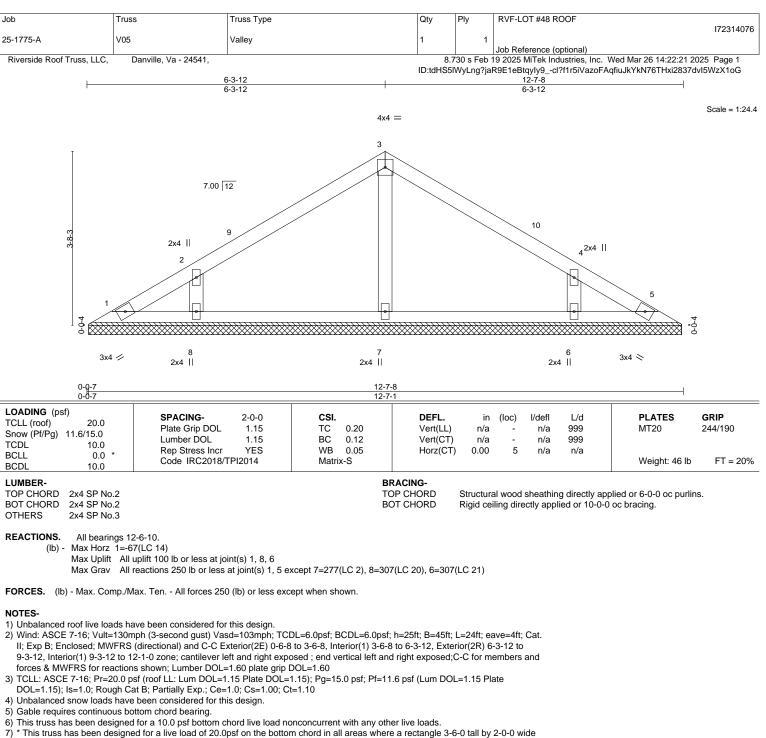
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March 28,2025

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- will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

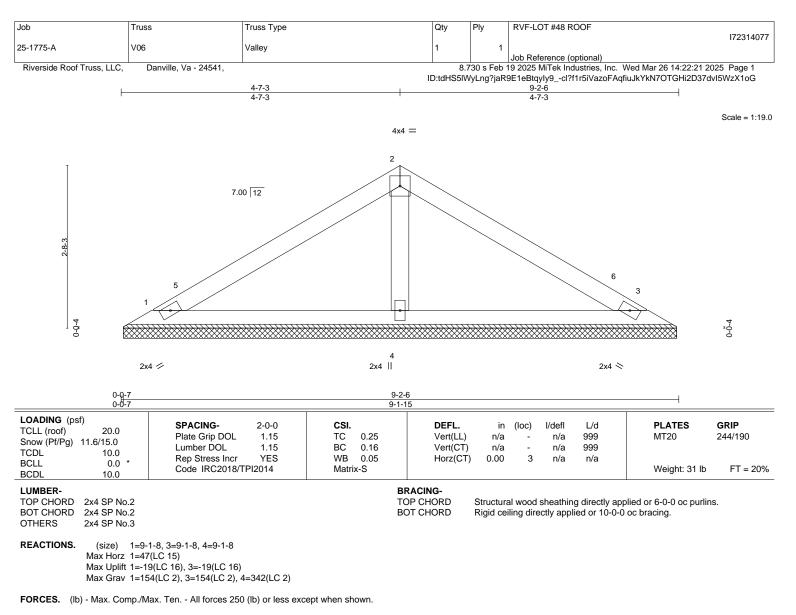


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818 Soundside Road



- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 4-7-3, Exterior(2R) 4-7-3 to 7-7-3, Interior(1) 7-7-3 to 8-7-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
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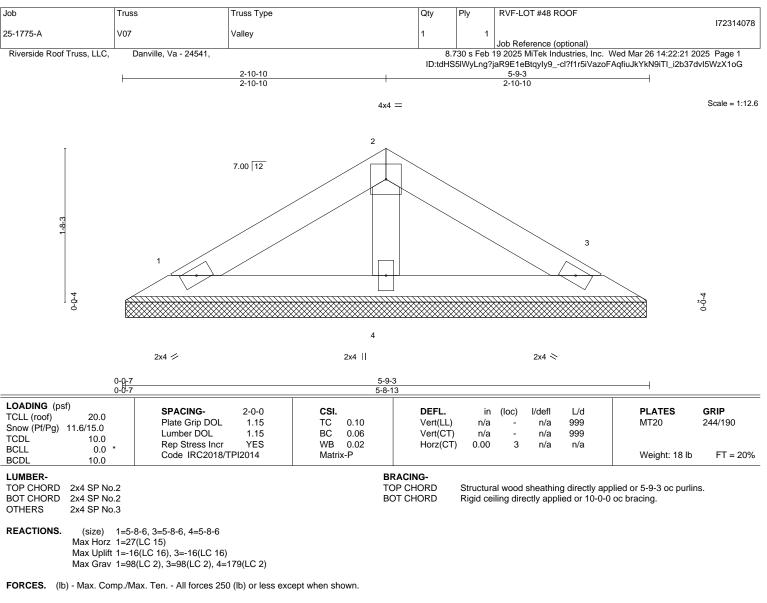


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818 Soundside Road



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- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

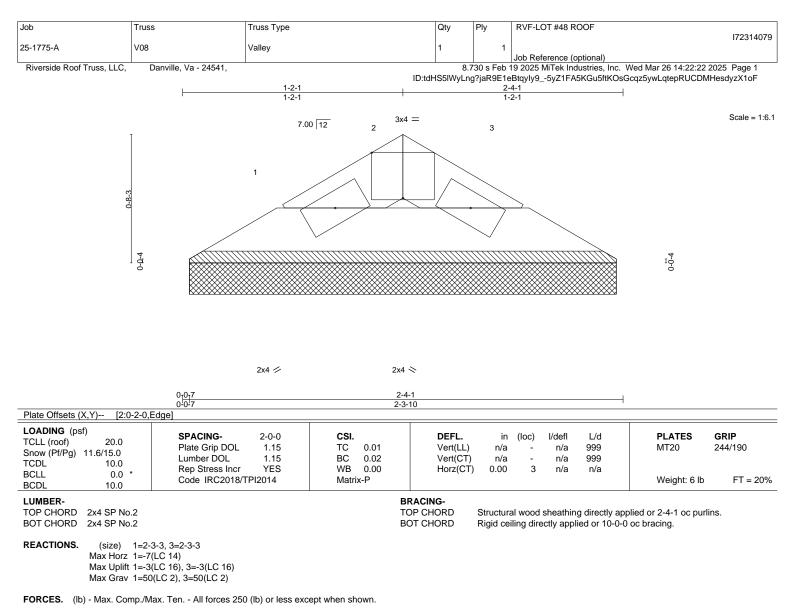
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

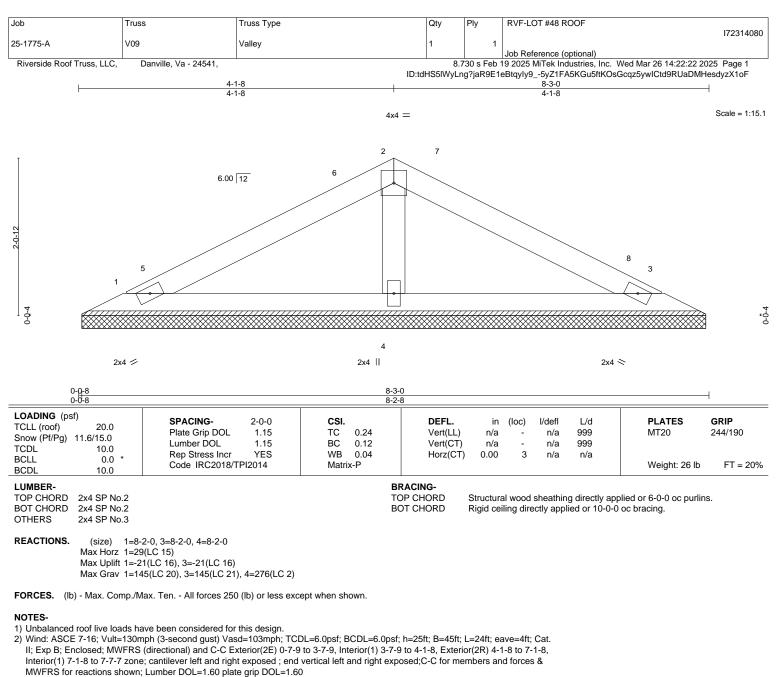


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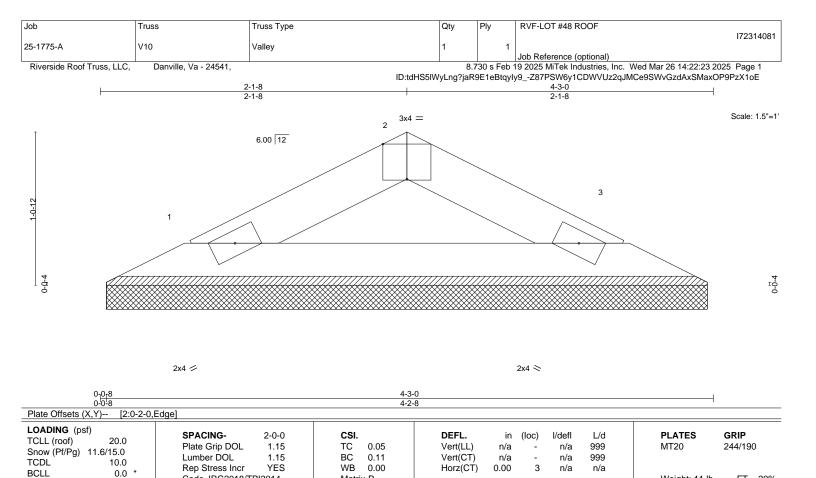


- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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BRACING-

TOP CHORD

BOT CHORD

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2

BCDL

LUMBER-

REACTIONS. 1=4-2-0, 3=4-2-0 (size) Max Horz 1=12(LC 15)

10.0

Max Uplift 1=-7(LC 16), 3=-7(LC 16) Max Grav 1=120(LC 2), 3=120(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2018/TPI2014

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



March 28,2025

FT = 20%

Weight: 11 lb

Structural wood sheathing directly applied or 4-3-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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