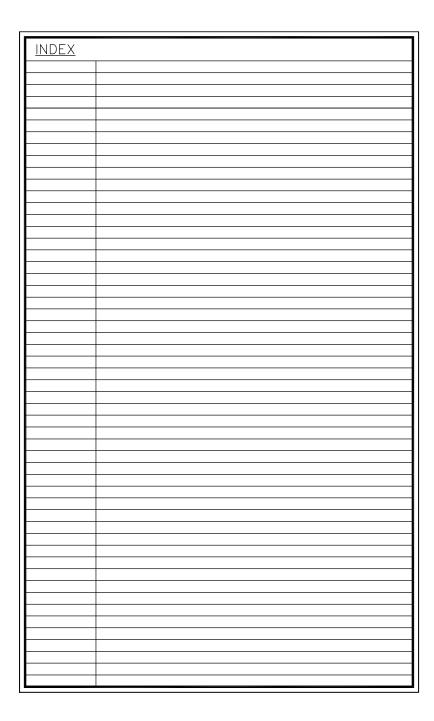
DRAYTON-RALE

RALEIGH- LOT 00.0154 THE FARM AT NEILL'S CREEK

(MODEL# 2695) ELEVATION 3 - GL

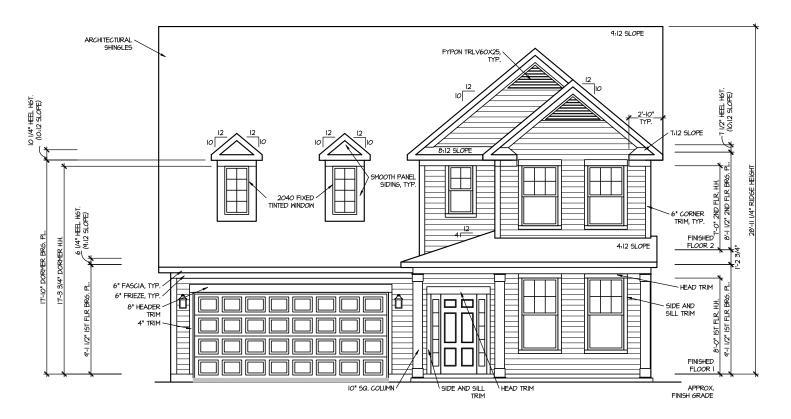




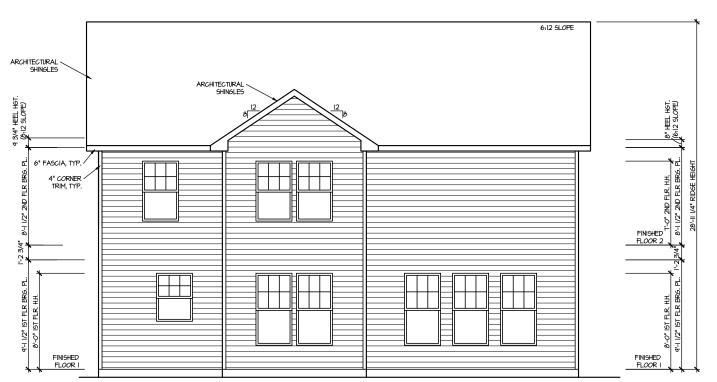
AREA CALCULATIONS ELEVATION 3 FIRST FLOOR GARAGE FRONT PORCH — ELEVATION 3 SECOND FLOOR OPTIONS EXTENDED BREAKFAST SITTING ROOM TOTAL COVERED / UNHEATED UNCOVERED 1266 SF 120 SF 1491 SF 152 SF TOTAL 2997 SF 699 SF
FIRST FLOOR GARAGE FRONT PORCH — ELEVATION 3 SECOND FLOOR OPTIONS EXTENDED BREAKFAST SITTING ROOM 120 SF
GARAGE 547 SF FRONT PORCH — ELEVATION 3 152 SF SECOND FLOOR 1491 SF OPTIONS EXTENDED BREAKFAST 120 SF SITTING ROOM 120 SF
FRONT PORCH — ELEVATION 3 152 SF SECOND FLOOR 1491 SF OPTIONS EXTENDED BREAKFAST 120 SF SITTING ROOM 120 SF
FRONT PORCH — ELEVATION 3 152 SF SECOND FLOOR 1491 SF OPTIONS EXTENDED BREAKFAST 120 SF SITTING ROOM 120 SF
SECOND FLOOR 1491 SF OPTIONS EXTENDED BREAKFAST 120 SF SITTING ROOM 120 SF
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TOTAL 2997 SF 699 SF
TOTAL 2997 SF 699 SF
TOTAL 2997 SF 699 SF

73 Little Branch Drive

LOT	CDEOIEIO								
LOT SPECIFIC									
1	LOT 00.0154	THE FARM AT NEILL'S CREEK DRAYTON REV. RALE 2 ELEVATION 3							
2	ADDRESS	73 LITTLE BRANCH DR LILLINGTON, NC 27546							
-									
—									
Ī	1								



FRONT ELEVATION 3
SCALE. 1/8" = 1'-0"



REAR ELEVATION 3

SCALE: 1/8" = 1'-0"



DRAWN BY: ITS 05/29/2025 PLAN NO. 2695



HOUSE NAME:

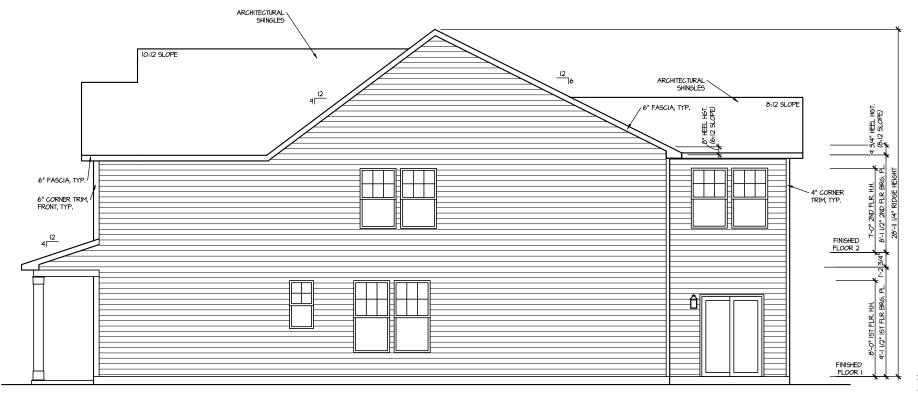
DRAYTON

DRAWING TITLE

FRONT & REAR ELEVATIONS

SHEET No.

Lot 00.0154.dwg D



RIGHT ELEVATION 3

ACCHTECTION
ACCHTE

LEFT ELEVATION 3

SCALE: 1/8" = 1'-0"



DRAWN BY: ITS DATE: 05/29/2025 PLAN NO. 2695



HOUSE NAME:

DRAYTON

DRAWING TITLE

RIGHT & LEFT ELEVATIONS



ROOF VENTILATION CALCULATIONS:

ROOF AREA = 1836 50. FT.

OVERALL REQUIRED VENTILATION:

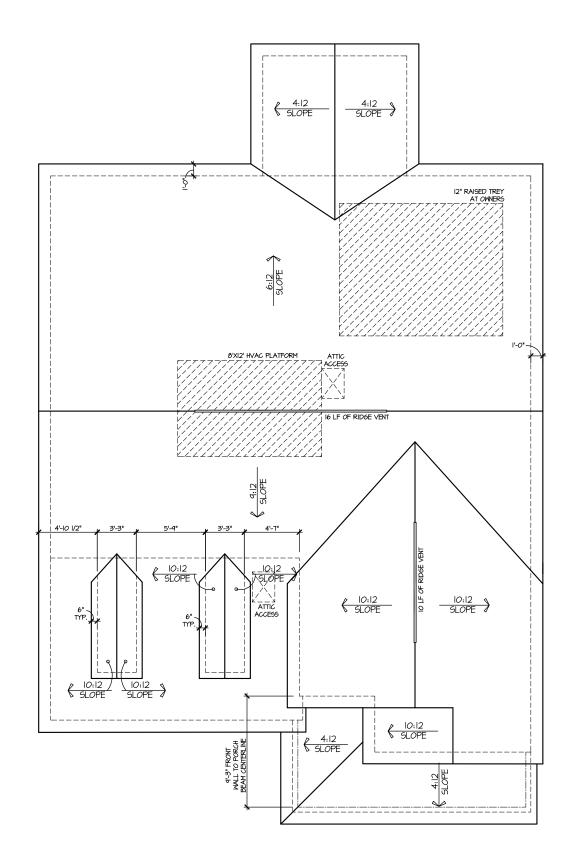
1 TO 150 = 12.24 50. FT.

1 TO 300 = 61.2 50. FT.

50-80% IN TOP THIRD = 3.06-4.40 FT. (1 TO 300)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LONER VENTING. (BOTTOM 2/3 RD9)
TI LINEAR FEET OF SOFFII X 5.1 SQ. IN. = 3.05 SQ. FT.
UPPER VENTING. (TOP 1/3 RD)
26 LINEAR FEET OF RIDGE X I8 SQ. IN = 3.25 SQ. FT.
3.25 SQ. FT. BETWEEN 50% - 80%
(I TO 300 ALLOWED)
TOTAL ROOF VENTILATION: 6.3 SQ. FT. > 6.12 SQ. FT. (RQ'D)



ROOF PLAN ELEV. 3 SCALE: 1/8" = 1'-0"

DATE
03-06-2019

DRAWN BY: DATE: 05/29/2025

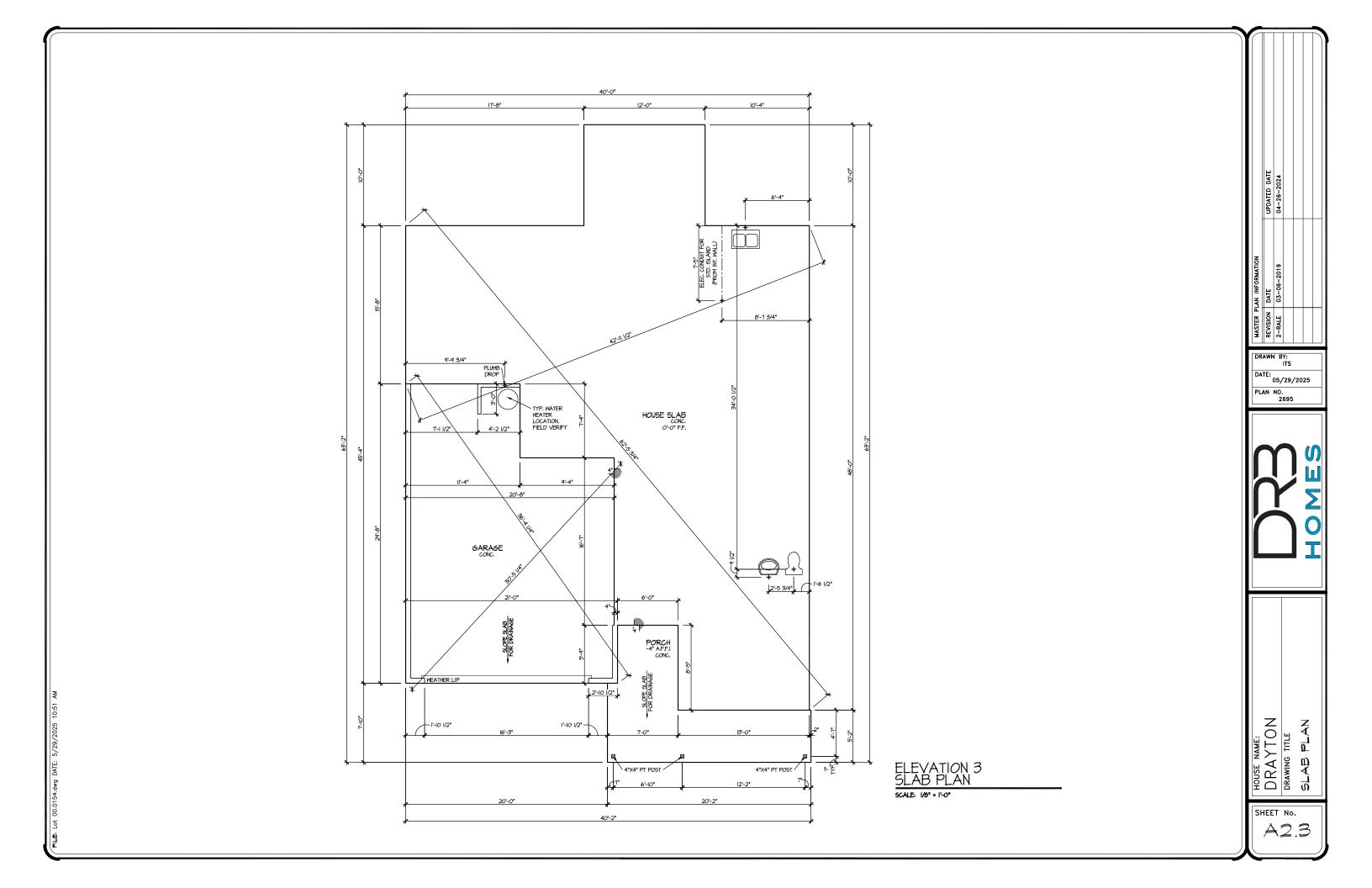
PLAN NO. 2695

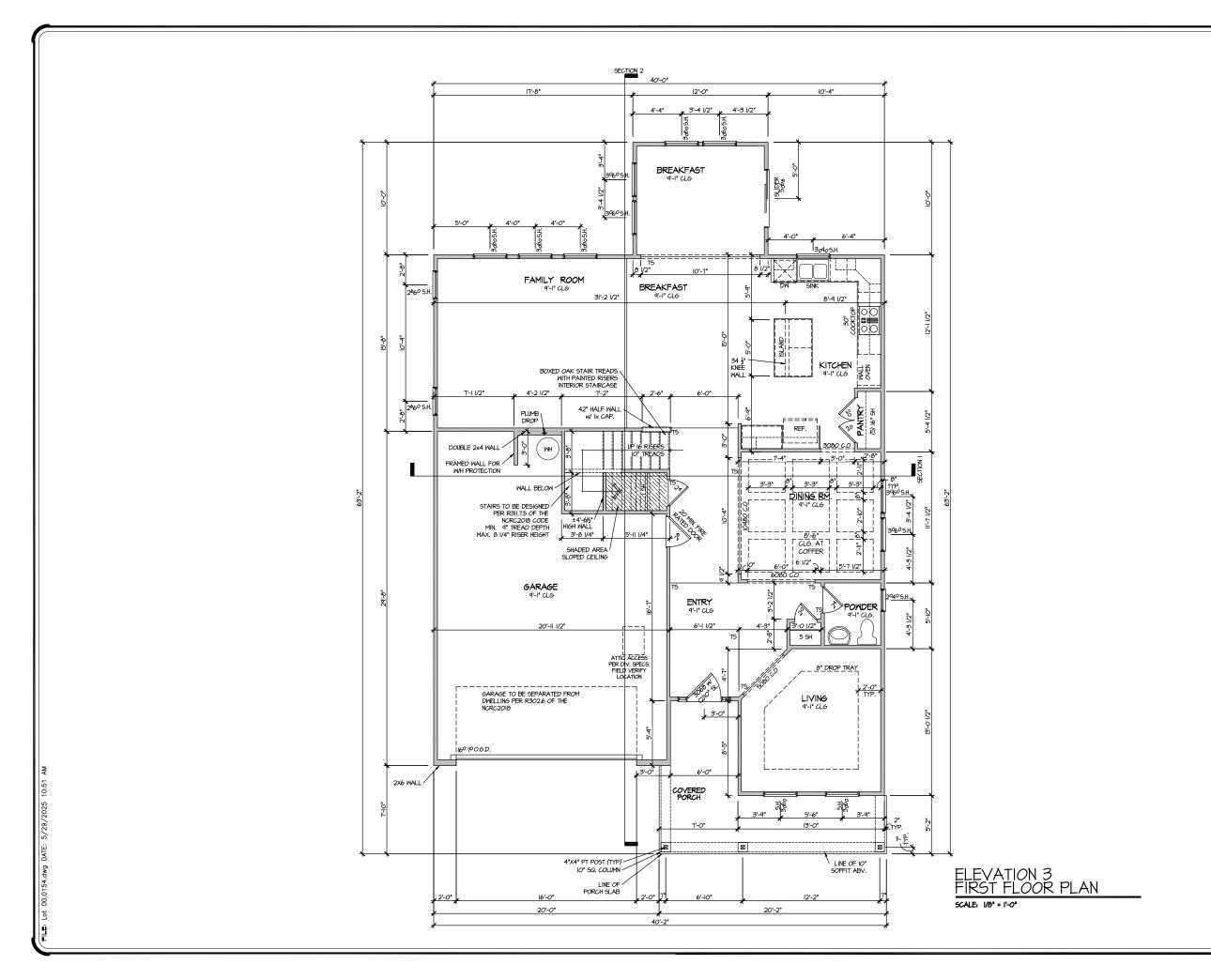


HOUSE NAME:
DRAYTON
DRAWING TITLE
ROOF PLAN

SHEET No.

A1.3





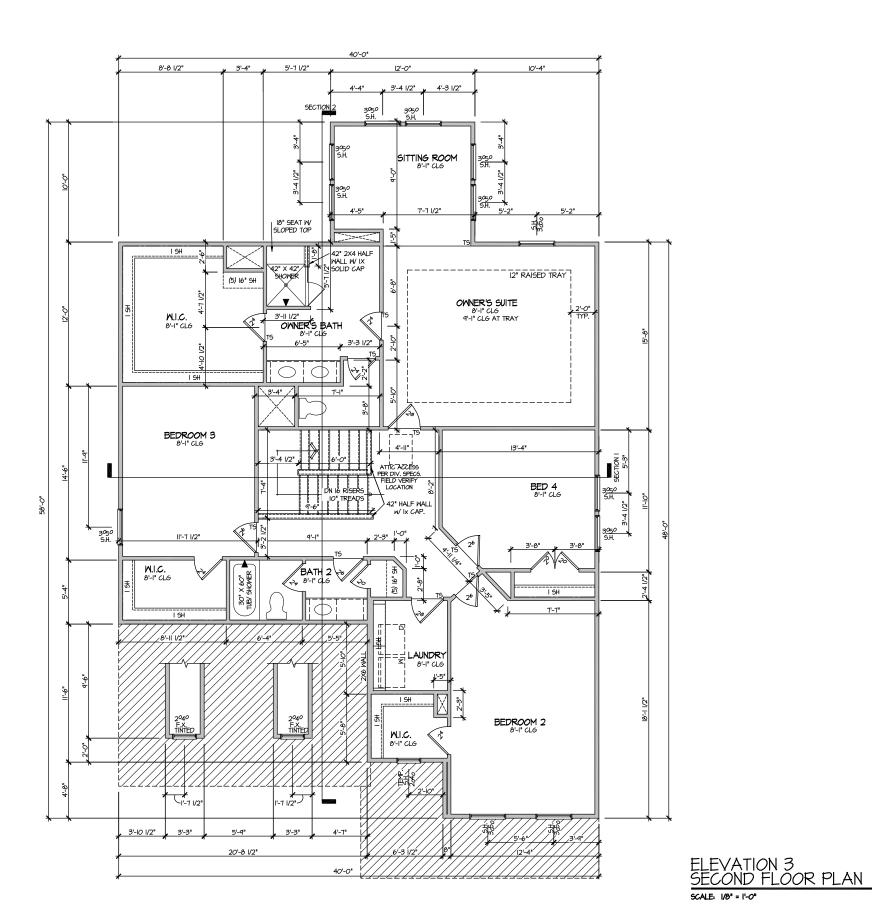
DRAWN BY:

DATE: 05/29/2025 PLAN NO. 2695



HOUSE NAME:
DRAYTON
DRAWING TITLE
FIRST FLOOF

SHEET No. A3.



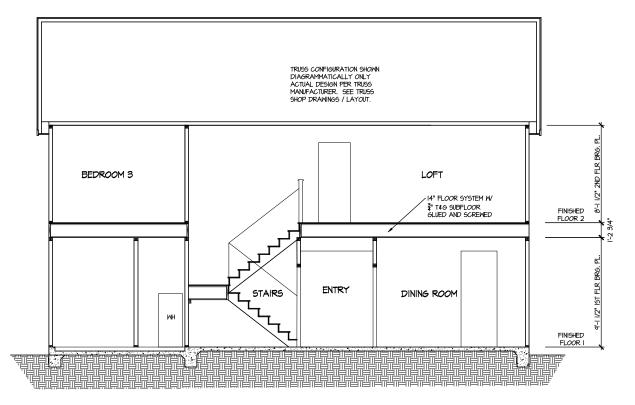
DRAWN BY: DATE: 05/29/2025

PLAN NO. 2695

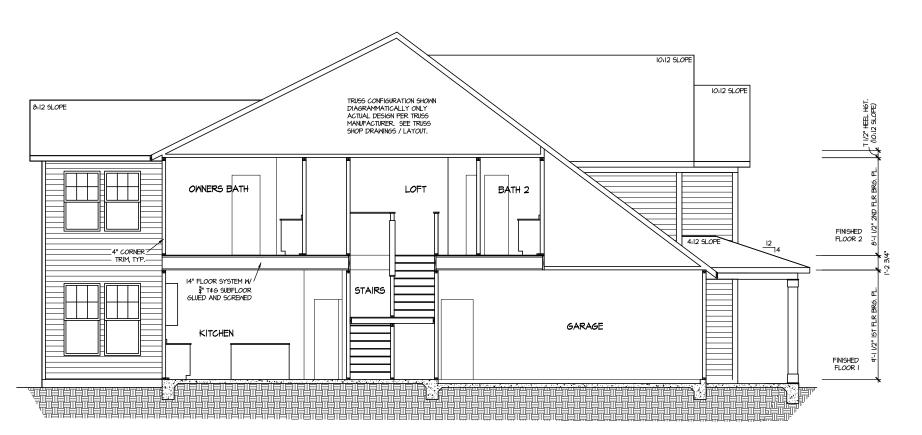


HOUSE NAME:
DRAYTON
DRAWING TITLE
SECOND FLO

SHEET No. A3.2

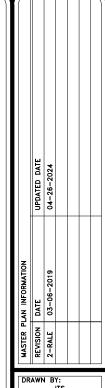


SECTION 1 SCALE: 1/0" = 1'-0"



SECTION 2

SCALE: 1/8" = 1'-0"

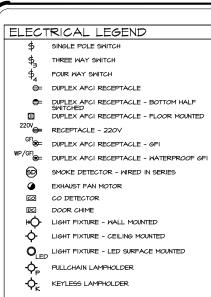


DRAWN BY: DATE: 05/29/2025 PLAN NO. 2695

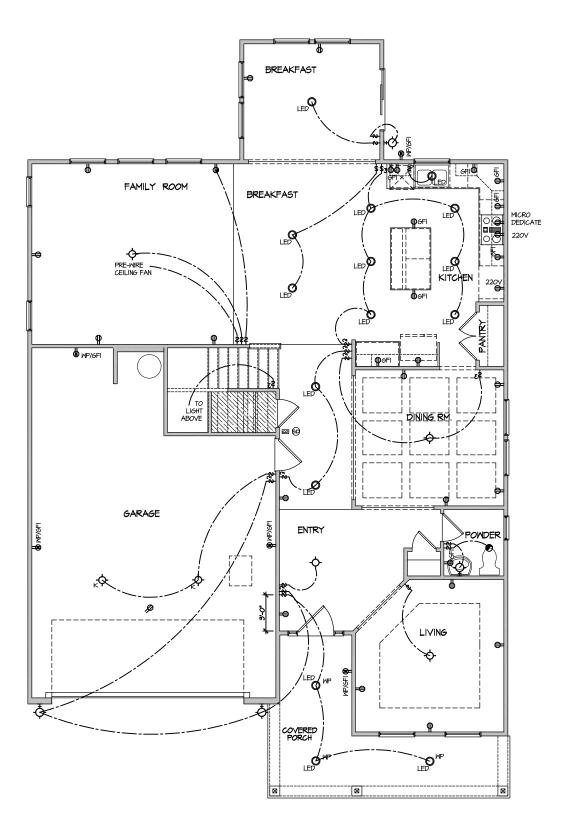


O U U HOUSE NAME:
DRAYTON
DRAWING TITLE
BUILDING SEC

SHEET No. A4.1



MOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



DRAWN BY:

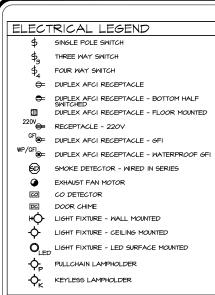
DATE: 05/29/2025

PLAN NO. 2695

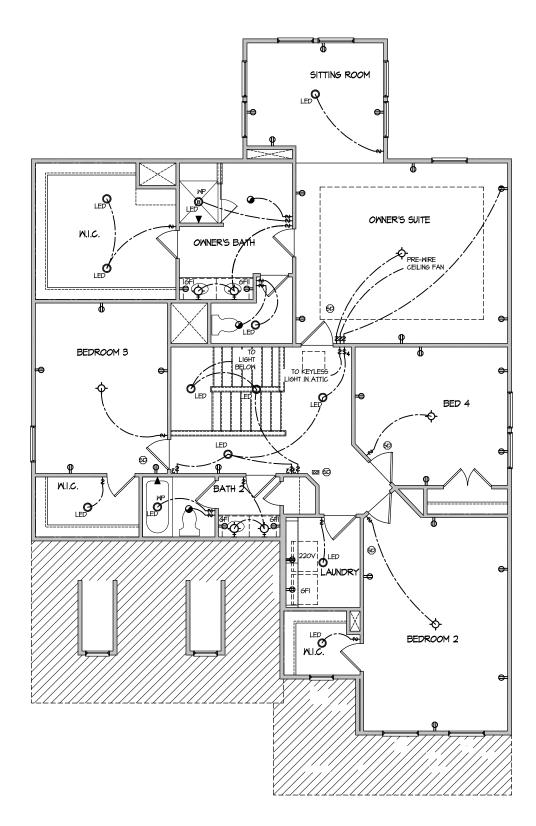


ᇳ

HOUSE NAME:
DRAYTON
DRAWING TITLE
FIRST FLOOF



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL PONER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.

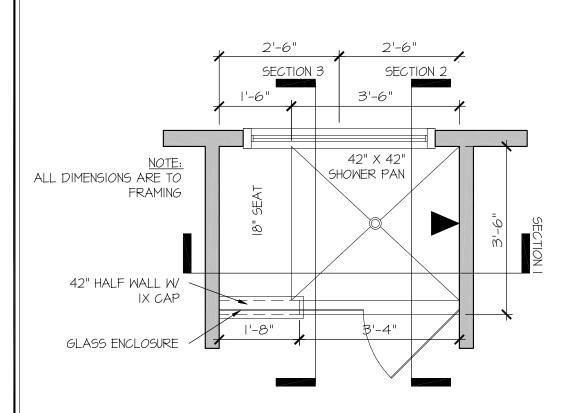


DRAWN BY: ITS DATE: 05/29/2025 PLAN NO. 2695



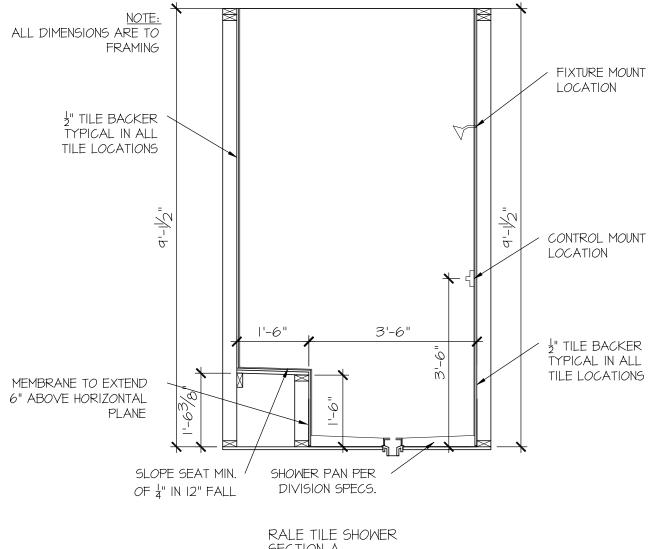
PLOOR ELECTRICAL

HOUSE NAME:
DRAYTON
DRAWING TITLE
SECOND FLO



RALE TILE SHOWER 42" X 42" W 18" SEAT

SCALE: 1/2" = 1'-0"



SECTION A

SCALE: 1/2" = 1'-0"

CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE

24 X 36 SCALE



DETAIL SHOWER RALE



SEAL

DRAWN BY:
L. BEAVERS
DATE: 9/1/22

PLAN NO.

24 X 36 SCALE

~ "

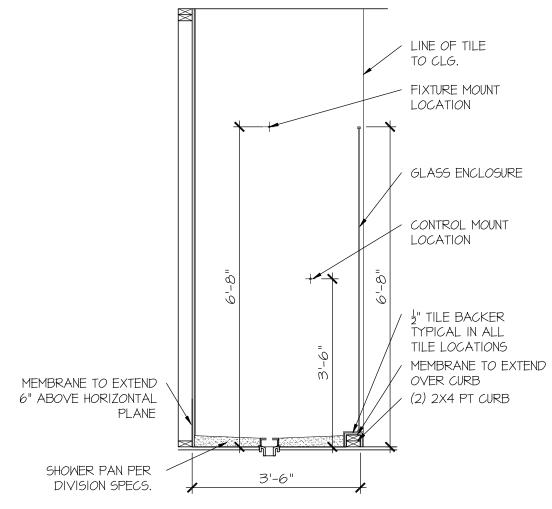


E ILE SHOWER DETAIL

OUSE NAME:

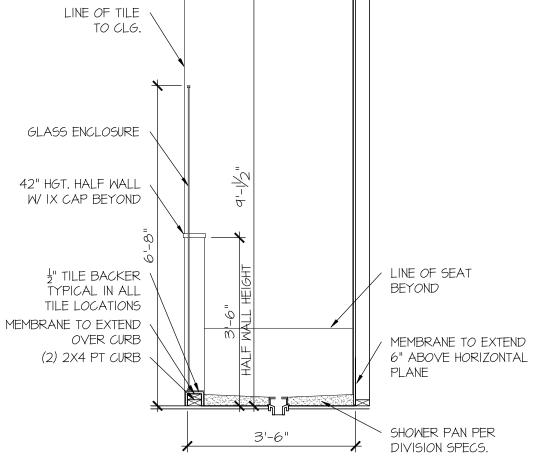
SHEET No.

P||.2



SCALE: 1/2" = 1'-0"





SCALE: 1/2" = 1'-0"

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX. FROM PLATE ENDS - UTILIZING:

 ■ 1/2" DIA. ANCHOR BOLTS ● 6'-0" O.C., 7" MIN. EMBEDMENT
- (CONC), 15" MIN. EMBEDMENT (CMU)
- SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS @ 2'-8" O.C. (CMU)
- (REFER TO DETAILS FOR 10' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 🛭 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACI 318. CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.:
 - 4,000 psi: FOUNDATION WALLS
 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE 60,000 psi
- BASEMENT FOUNDATION WALL DESIGN BASED ON
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
- TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (9 1/2" FOR 10" THICK WALL)
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC. ML-CL. OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL WITH 2" CLEAR, REINFORGEMENT
- SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS • FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE
- DEPTH OVER OPENING OR (3)2x10 w/ (2)2x6 JACK STUDS, U.N.C LARGER OPENINGS SHALL BE PER PLAN.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMEN
- ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP
- JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" O.C. (MAXIMUM)
- · JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I:I RATIO), WITH A MAXIMUM OF I:1.5 RATIO
- · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi), MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 & 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA, MINIMUM @ 16" O.C.
- PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS. FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE.
- DIMENSIONS BY OTHERS, BUILDER TO VERIFY
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.
- ROOF DEAD = 1 PSF T.C., IO PSF B.C.

1 IVF = 16 PSF LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (I-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- ALL TYP. NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS, ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION. ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP. U.N.O.
- FXT & INT BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS @ 16" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL B SPRUCE-PINE-FIR #2 (SPF) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED). ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX., U.N.O.)
- . HEADERS IN NON-LOAD BEARING WALLS SHALL BE (1)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'.
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING LSL' - Fb=2325 psi; Fv=310 psi; E=1.55x10^6 psi
- 'LVL' Fb=2600 psi; Fv=285 psi; E=2.0xl0^6 ps
- 'PSI.' FB=2900 PSI. FV=290 PSI. F=2.0XI0^6 PSI. M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THE OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR
- FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x3/5" SIMPSON SDS SCREWS (OR 3½" TRUSSLOK SCREWS) & 16" O/C, USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATI TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE. SOLID 3 1/5" OR 5 1/4" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF K"X6" SIMPSON SDS SCREWS (OR 6 3/4" TRUSSI OK SCREWS) @ 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTH OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREW 2" FROM EDGE. A SOLID 7" BEAM IS ACCEPTABLE.
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STILD MINIMIM THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE
- NUMBER OF JACK STUDS REQUIRED, U.N.O., ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER W/ 3"X0.I3I"
- NAILS @ 24" O.C. (MIN.), EACH PLY. PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND./BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED. OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

FLOOR FRAMING

- -JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED 1 /480 LIVE LOAD DEELECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT M&K FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN, OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 ½" × 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12"o.c. FIELD. - 2 3" x 0 120" NAIL S @ 4" O C @ PANEL EDGES & @ 8" O C FIELD
- 2 3" × 0 II3" NAII S @ 3" O C @ PANEL EDGES & @ 6" O C IN FIELD 6 x 2" MIN. SCREWS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIF (OR APPROVED EQUAL) @ ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES, (3) H2.5T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO:
 - RIM BOARD W (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS W/ (3) 3"x0.131" NAILS @ 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 1/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS - W/ 2 ½" × 0.131" NAILS • 6"o.c. • PANEL EDGES € • 12" O.C. FIELD.
- w/ 2 (× 0.120" NAILS 4"o.c. PANEL EDGES € 8" O.C. FIELD. - w/ 2 🐉 × 0.113" NAILS @ 3"o.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.

HOLD-DOWN SCHEDULE

SYMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC66B3 ALTERNATE
▶ HD-3	SIMPSON STHDI4/STHDI4RJ

* UTILIZE THE SSTB24 ANCHOR BOLT @ ALL MONOSLAB & INTERIOR RAISED JLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS. MINIMUM 24" MIN. COTING THICKNESS REQUIRED

POXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO EASTEN HREADED ROD INTO CONCRETE FOUNDATION. PROVIDE 10" (FOR 5/8" DIA.) OR 5" (FOR 7/8" DIA.) MIN, EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. INSTRUCTIONS. MINIMUM 16" FOOTING THICKNESS REQ'D. DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE.

LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE • ---- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING

IP TO 3'-0"

7 TO 6'-0"

IP TO 8'-0"

JP TO 12'-0"

NOTES:

* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

NON-BEARING HEADER SCHEDULE

2x6 NON-BEARING

PARTITION WALL

(I)2x6 FLAT

(3)2x4

(3)2x6

(3)2x8

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

PARTITION WALL

(I)2x4 FI AT

(2)2x4

(2)2x6

(2)2x8

SPACED a 24" O.C. (MAX.)

ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS

NALL SHEATHING SPECIFICATIONS

LATERAL BRACING & SHEAR

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RC

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301.2.1.1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NCSBC:RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY CCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8 XO.113" NAILS @ 6" O.C. AT EDGES & @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL, BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1½" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 34" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 34" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C. IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING w/ 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING.

NOTES

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN. IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX, STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING.
- PRE-MANUFACTURED PANELIZED WALLS: FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWALL
OR 3" O.C., OSB SHEARWALL.

INDICATES HOLDOWN BELOW

ALTERNATE F.J MANUFACTURERS

FLOOR JOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED PROPERTIES INCLUDING, BUT NOT LIMITED TO, ALLOWABLE SHEAR. ALLOWABLE MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I-JOISTS. ALL ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST CONNECTIONS ARE PER THE JOIST MANUFACTURER.

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS NOTED OTHERWISE ON PLAN.

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUSI BEAMS DO NOT EXCEED THE FOLLOWING: ROOF TRUSSES:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS: 1/8" DEAD LOAD
- FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAI LOAD, (NOT DIFFERENTIAL DEFLECTION)

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING GLYS AND TIF-DOWNS CONTRACTOR BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENT IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING TO VERIEY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTR'S OR WARRANTY TOLERANCES.

VEHICLE CATEDITE

	VENEER LINIEL SCHEDULE				
SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	STEEL ANGLE SIZE			
3'-0"	20 FT. MAX	L3"x3"x/4"			
6'-0"	3 FT. MAX	L3"x3"x/4"			
	I2 FT. MAX	L4"x3"x/4"			
	20 FT. MAX	L5"x3½"x¾;"			
ð'-0"	3 FT. MAX	L4"x4"x¼" *			
	I2 FT. MAX	L5"x3½"x%;"			
	I6 FT. MAX	L6"x3½"x¾"			
9'-6"	I2 FT. MAX	L6"x3½"x"%"			
16'-0"	2 FT. MAX	L7"x4"x½" **			
	3 FT MAX	8"x4"xK" **			

- ALL LINIELS:
 SHALL SUPPORT 2 3/s' 3 1/s' VENEER W 40 PSF MAXIMUM WEIGHT.

 I 6/5 SHALL HAVE 4* MIN. BEARING

 I 6/5 SHALL HAVE 6* MIN. BEARING

 I 6/5 SHALL NOT BE FASTENED BACK TO HEADER.

 I 6/5 SHALL BOT BE FASTENED BACK TO WOOD HEADER IN WALL 048°0...

 W/S' DIA. x 3/s' LONG LAG SCRENG IN 2" LONG VERTICALLY

 SLOTTED HOLES. AX, VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- O'ENING.
 ALL LINTELS SHALL BE LONG LEG VERTICAL.
 HIEN SUPPORTING VENEER (3" MIDE THE EXTERIOR TOE OF THE
 HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 %," MIDE OVER
 THE BEARING LENGTH ONLY, THIS IS TO ALLON FOR MORTAR JOINT
- EE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT NE SINGCINAL PLANS FOR AN LINIEL CONDITION NOT NCOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL ASTENED BACK TO BEAM, FASTENERS SHALL MAINTAIN A 2%," HINIMM) CLEAR DISTANCE FROM BOTTOM OF BEAM.

HINDROY CLEAK LICHANCE FROM BOTTOM OF BEAM. FOR GUEEN VENEER USE LAXSA'S. FOR 35' VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF ENEER < 35' THICK.

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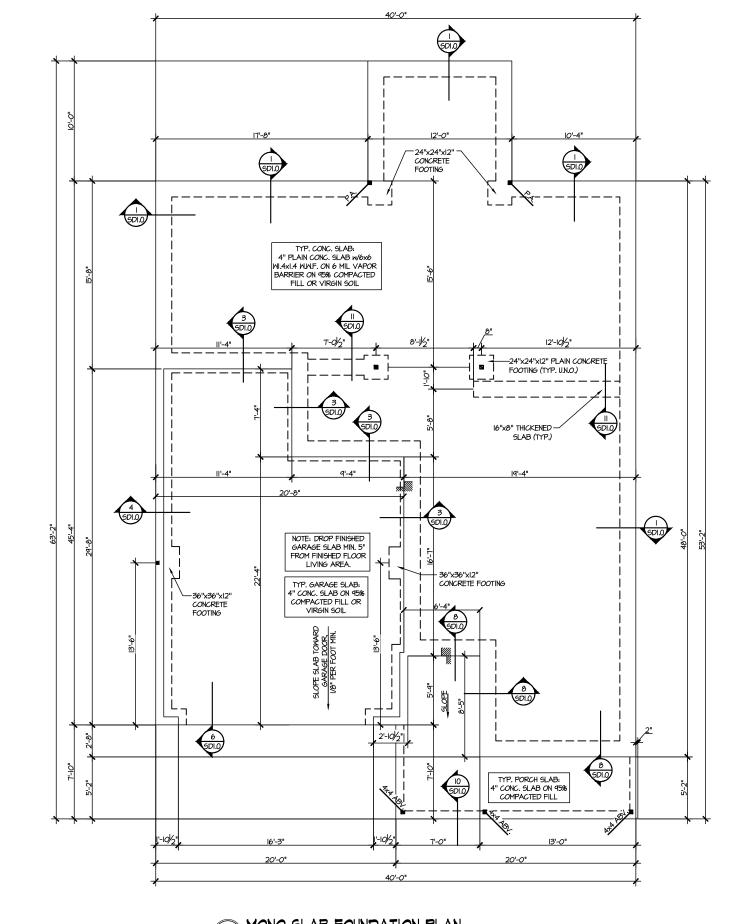
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drawn by: GTK issue date: 06-10-25

REVISIONS:

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NEIL'S CREEK FOUNDATION PLANS

LOT 154 - DRAYTON 3 RALEIGH, NC

FARM AT

S1

LEGEND

- IIIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- --- BEAM / HEADER
- ullet = \blacksquare INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES \$ SCHEDULES

3" O.C. EDGE NAILING (SEE NOTES)

ALTERNATE F.J MANUFACTURERS

FLOOR JOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED I-JOISTS DESIGN AND CONSTRUCTION GUIDE. MINIMUM JOIST PROPERTIES INCLUDING, BUT NOT LIMITED TO, ALLOWABLE SHEAR. ALLOWABLE MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I-JOISTS. ALL ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST CONNECTIONS ARE PER THE JOIST MANUFACTURER.

ENGINEERED BEAM MATERIAL SCHEDULE									
BEAM NUMBER	LYL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION				
001	(2)194"×14" - F	3½"xl4" - F	(2)13/4"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - FB	WI2xI4 - F				
002	(2)i¾"xi4" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - FB	WI2xI4 - F				
003	(3)134"x18" - FB or (2)134"x20" - FB	5¼"xl8" - FB	N/A	(3)2xl2 + (2) %"xll以" STEEL FLITCH PLATES - FB	WI2x26 - F				
004	(2)194"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - FB	WI2xI4 - F				
005	(2)134"×1136" - H cont.	3½"x11%" - H cont.	(2)13/4"×113/6" - H cont.	(3)2xl2 + (2)从"xll%" STEEL FLITCH PLATES - H cont.	N/A				
005A	(3)134"×14" - H cont.	51/4"×14" - H cont.	N/A	(3)2xl2 + (2)从"xll%" STEEL FLITCH PLATES - H cont.	N/A				
006	(I)134"×14" - F	3½"xl4" - F	(2)13/4"x14" - F	(2)2x12 + (1) ¼"x14" STEEL FLITCH PLATES - FB	WI2xI4 - F				
001	(2)134"x11%" - D	3½"×11½" - D	(2)1¾"x11%" - D	(2)2xl2 + (l) 从"xll以" STEEL FLITCH PLATES - D	MIOxi2 - D				
008	(2)194"×16" - H cont.	3½"x16" - H cont.	(3)194"x16" - H cont.	(3)2xl2 + (2) 片"xl片" STEEL FLITCH PLATES - H cont.	N/A				
009	(2)194"×94" - F	3½"x9¼" - F	(2)19/4"×9/4" - F	(2)2x10 + (1) ¼"x9¼" STEEL FLITCH PLATES - F	W8xI0 − F				
010	(2)13/4"×14" - F	3½"xl4" - F	(2)194"x14" - F	(2)2xl2 + (I)从"xll以" STEEL FLITCH PLATES - FB	WI2xI4 - F				
OII	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"x14" - F	(2)2xl2 + (I)从"xll以" STEEL FLITCH PLATES - FB	WI2xI4 - F				
012	(2)134"x11%" - D	3½"×11½" - D	(2)1¾"x11%" - D	(2)2xl2 + (I) 从"xll以" STEEL FLITCH PLATES - D	MIOxi2 - D				

- BEAM NOTATION: "F" INDICATES FLUSH BEAM "FT" INDICATES FLUSH TOP BEAM
- "FB" INDICATES FLUSH BOTTOM BEAM

- 'HE' INDICATES PEOPPED BEAM
 'H' INDICATES DROPPED DEAM
 'H' INDICATES DROPPED OPENING HEADER
 REFER TO DETAIL D/5D2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
 REFER TO DETAIL E/5D2.0 FOR TYPICAL STELL BEAM CONNECTIONS
 FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W (2) 3"X0,120" NAILS @ 8" O.C.
- FOR FLUSH BOTTOM BEAMS PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION w/ (2) 3"XO.120" NAILS 8" O.C.

SD2, REFERS TO SD2, IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC

LEGEND

FOR STEEL BEAMS

- IIIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- --- BEAM / HEADER
- ullet INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

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RESIDENTIAL STRUCTURAL ENGINEERING

6/11/25



1&K project number 126-22076

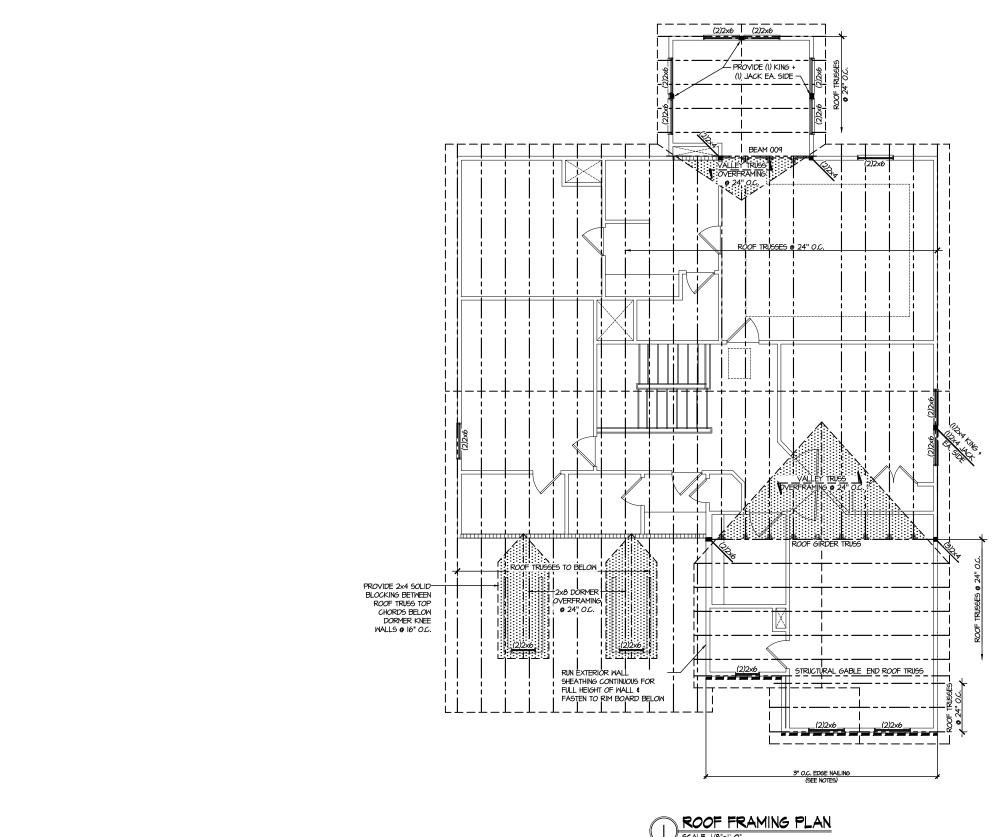
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CREEK PLANS NEIL'S OOR FRAMING **DRAYTON 3** ATFARM

LOT 154 **S2.0**

2ND FLOOR FRAMING PLAN



SCALE: 1/8"=1'-0"

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RESIDENTIAL STRUCTURAL ENGINERING
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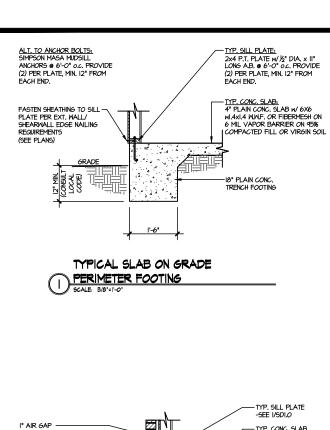
FARM AT NEIL'S CREEK
LOT 154 - DRAYTON 3
RALEIGH, NC

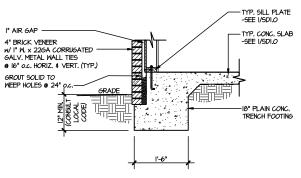
LEGEND

- IIIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- --- BEAM / HEADER
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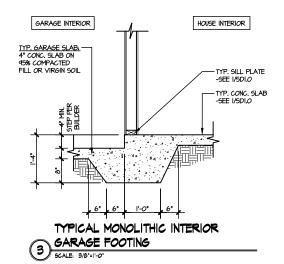
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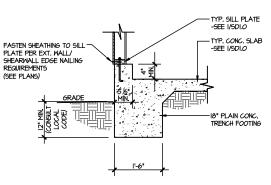
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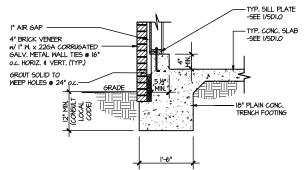


TYPICAL SLAB ON GRADE PERIMETER FOOTING w/ BRICK VENEER

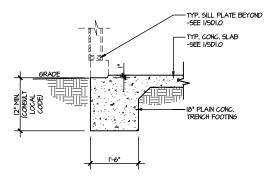




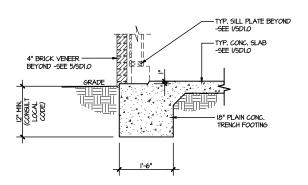




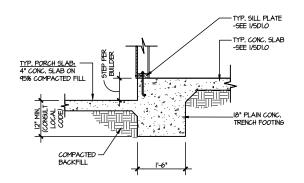




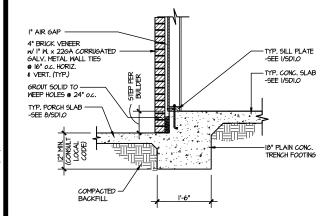
TYPICAL SLAB ON GRADE GARAGE 6 ENTRY @ PERIMETER FOOTING



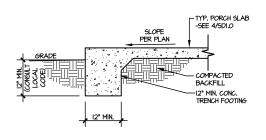
TYPICAL SLAB ON GRADE GARAGE PENTRY @ PERIMETER FOOTING



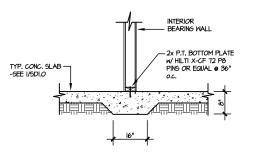
TYPICAL SLAB ON GRADE PERIMETER (8) FOOTING @ PORCH/PATIO



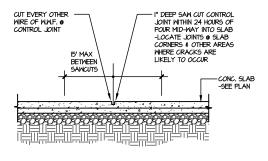
TYPICAL SLAB ON GRADE PERIMETER POOTING @ PORCH/PATIO



TYPICAL FOOTING @ PORCH SLAB



TYPICAL THICKENED SLAB @ INTERIOR BEARING WALL





LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

FARM

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M&K project number 126-22076

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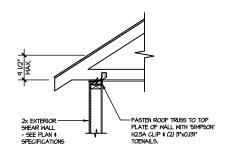
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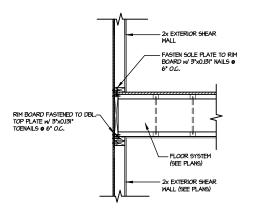
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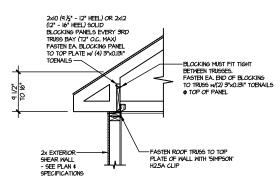
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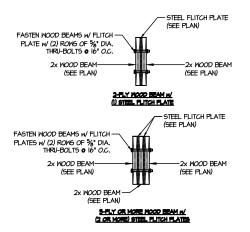
TYPICAL SHEAR TRANSFER DETAIL @ ROOF SCALE: 9/8"=1"-0" HEEL HEIGHT LESS THAN 9 1/2" NO BLOCKING REQT)



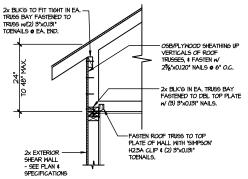








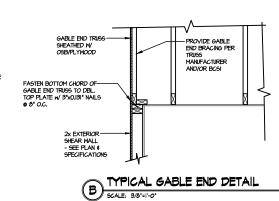
TYPICAL FLITCH BEAM CONNECTION DETAIL SCALE 544-1-0*

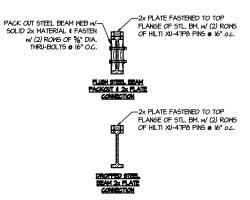


TYPICAL SHEAR TRANSFER

DETAIL @ RAISED HEEL TRUSS

SCALE: 9/6*c1'-0" HEEL HEIGHT UP TO 40" MAX.





TYPICAL STEEL BEAM CONNECTION DETAIL SCALE SUM-11-0"



FRAMING DETAILS
FARM AT NEIL'S CREEK
LOT 154 - DRAYTON 3
RALEIGH, NC

SD2.0

6/11/25

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REVISIONS:

126-22076

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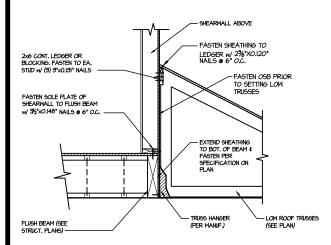
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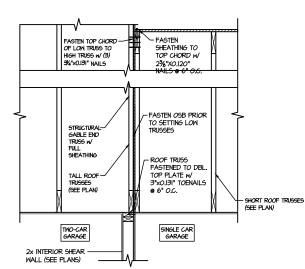
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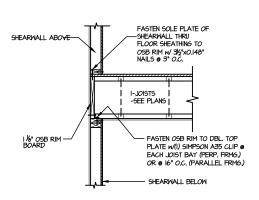
SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL BELOW SCALE: 9/4"=1"-0" PAR



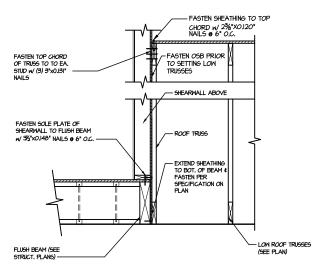
SHEAR TRANSFER DETAIL @ 5 EXTERIOR SHEARWALL ABOVE



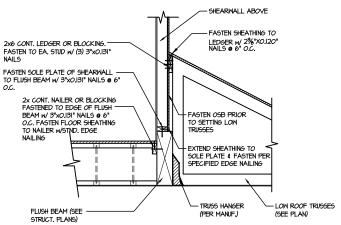
TYPICAL SHEAR TRANSFER DETAIL BETWEEN GARAGE BAYS



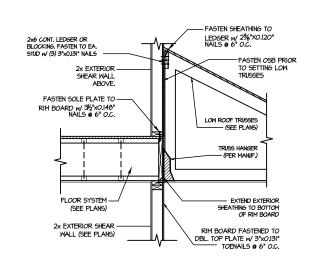
SHEAR TRANSFER DETAIL @ INT. 2 SHEARWALL ABOVE & BELOW SCALE: 8/4"=1"-A"



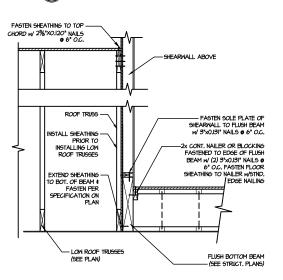
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



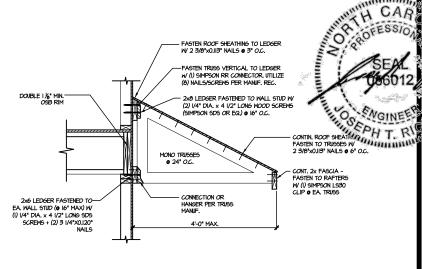
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



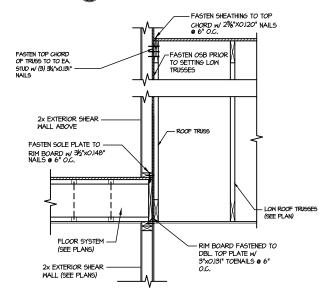
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARMALL ABOVE



DETAIL @ SHED ROOF



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL

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CREEK RAMING DETAIL NEIL'S I AT Drayt FARM LOT

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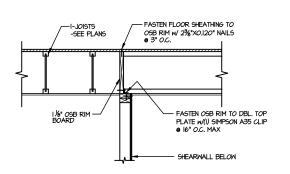
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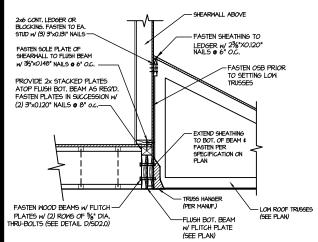
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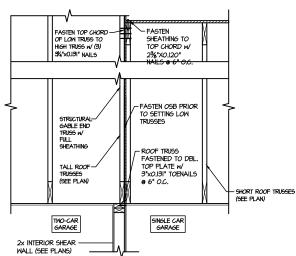
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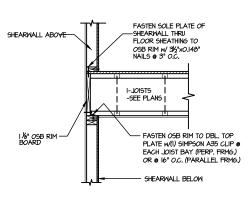
SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL BELOW SCALE: 3/4'=1'-0' PAR



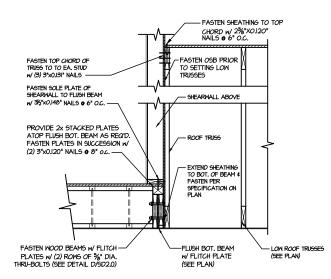
SHEAR TRANSFER DETAIL @ 5 EXTERIOR SHEARWALL ABOVE



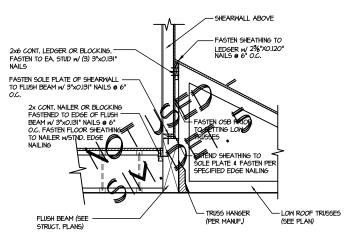
TYPICAL SHEAR TRANSFER DETAIL BETWEEN GARAGE BAYS



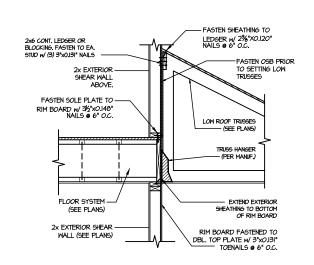
SHEAR TRANSFER DETAIL @ INT. 2 SHEARWALL ABOVE & BELOW SCALE: 3/4'=1'-0'



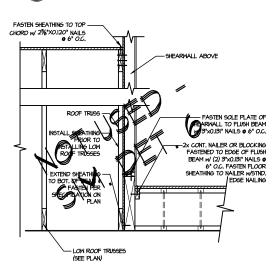
SHEAR TRANSFER DETAIL @ 6 EXTERIOR SHEARWALL ABOVE



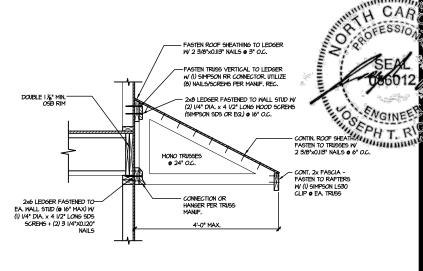
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



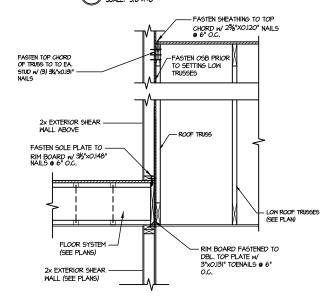
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



DETAIL @ SHED ROOF
SCALE: 3/8"=1"-0"



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL

SCALE: 3/4':1'-0'

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

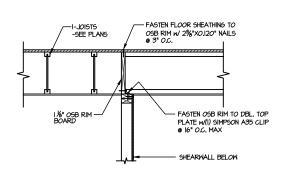
STRUCTURAL ENGINEER $\mathbf{\Sigma}^{\mathbf{g}}$ Y 1&K project number 126-2207

6/11/2

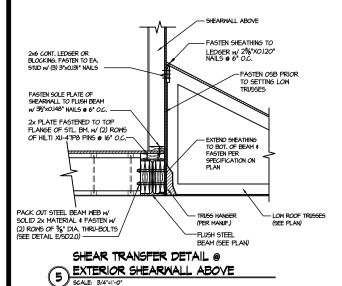
JTF rawn by: GTŁ ssue date: 06-10-2 REVISIONS: initial:

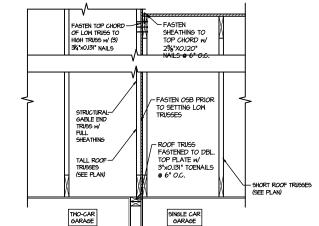
CREEK RAMING DETAIL NEIL'S I AT DRAY FARM LOT

SD2.1B



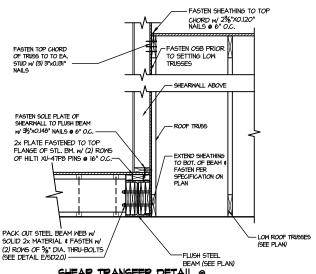
SHEAR TRANSFER DETAIL @ INTERIOR SHEARMALL BELOW



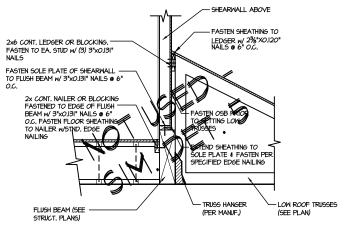


FASTEN SOLE PLATE OF SHEARWALL THRU FLOOR SHEATHING TO SHEARWALL ABOVE OSB RIM w/ 3½"x0.148" NAILS @ 3" O.C. |-Joists | |-See Plans - Fasten *os*b RIM to DBL. Top Plate W/(1) SIMPSON A35 CLIP @ EACH JOIST BAY (PERP. FRMG.)
OR @ 16" O.C. (PARALLEL FRMG.) SHEARWALL BELOW

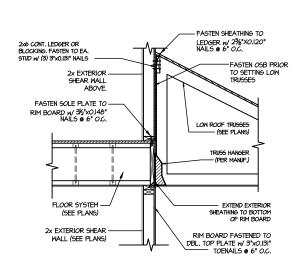
SHEAR TRANSFER DETAIL @ INT. 2 SHEARWALL ABOVE & BELOW



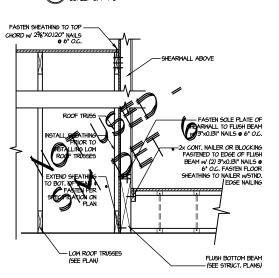
SHEAR TRANSFER DETAIL @ 6 EXTERIOR SHEARWALL ABOVE



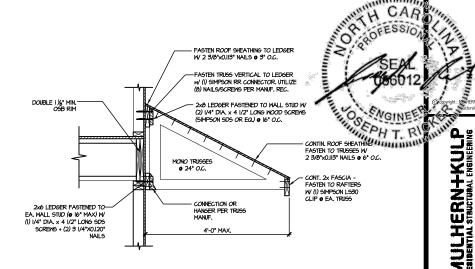
SHEAR TRANSFER DETAIL @ 3 EXTERIOR SHEARWALL ABOVE

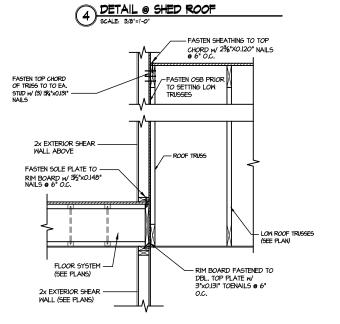


TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE SCALE SATISTO





TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL

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CREEK RAMING DETAIL NEIL'S AT DRAY FARM LOT

SD2.1C

TYPICAL SHEAR TRANSFER DETAIL BETWEEN GARAGE BAYS

2x INTERIOR SHEAR WALL (SEE PLANS)

6/11/2

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1&K project number

REVISIONS

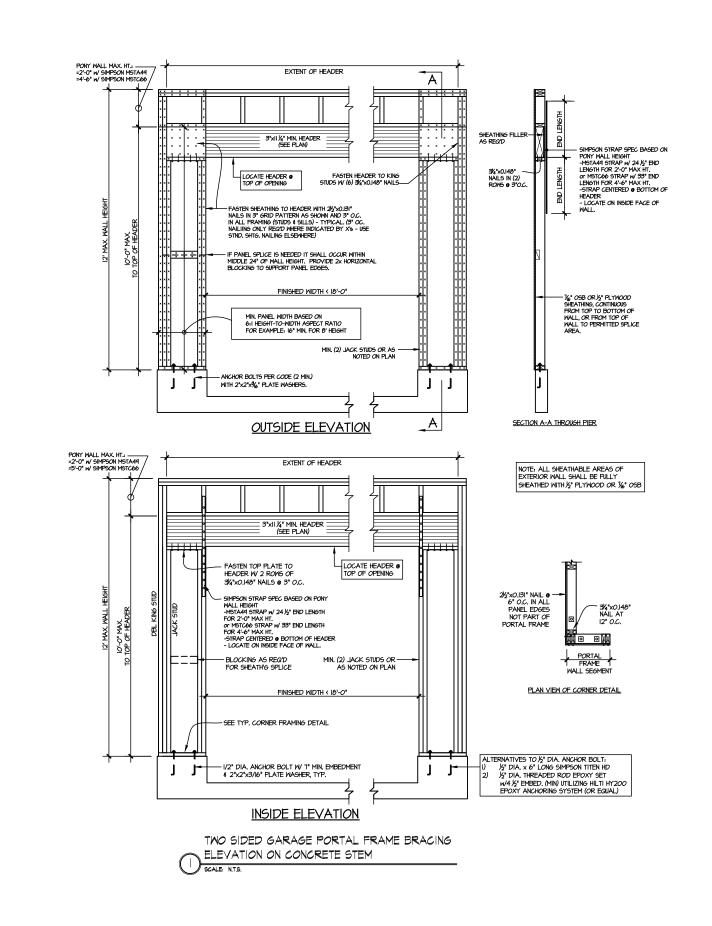
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ssue date: 06-10-2

JTF

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initial:



MUCHERN+KULP

6/11/25

TH CAR

¥ M&K project number:

126-22076 JTR

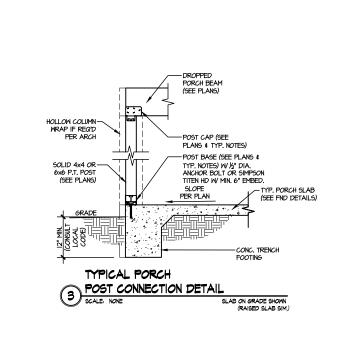
drawn by: GTK issue date: 06-10-25

REVISIONS:

initial:

FRAMING DETAILS
FARM AT NEIL'S CREEK

FARM AT NEIL'S (LOT 154 - DRAYTON 3



6/11/25 H CAR OSEPH T. RI MULHERN+KULP

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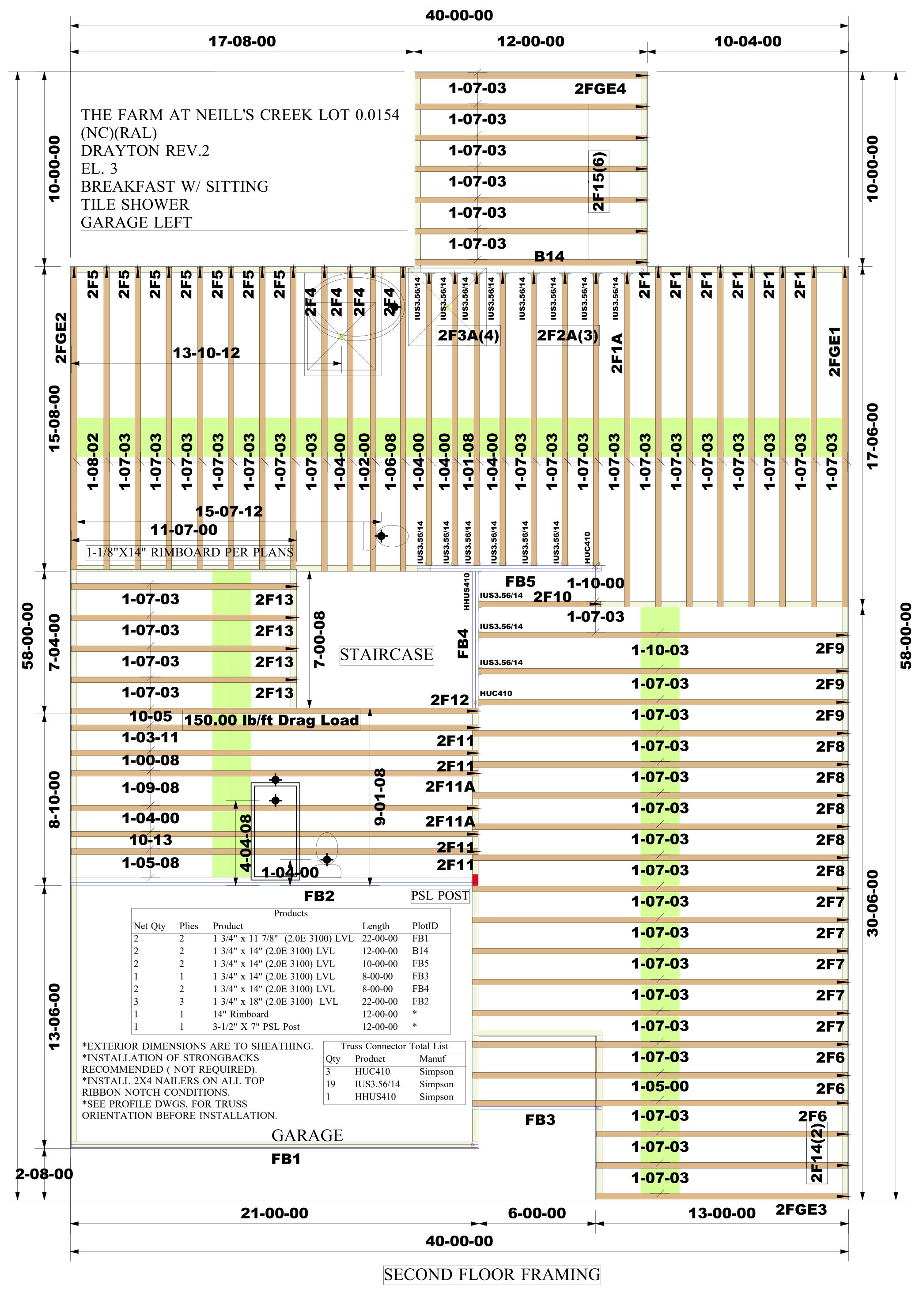
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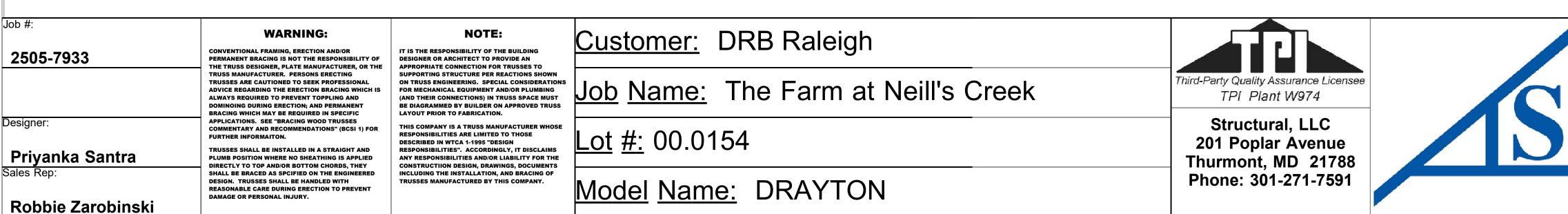
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REVISIONS:

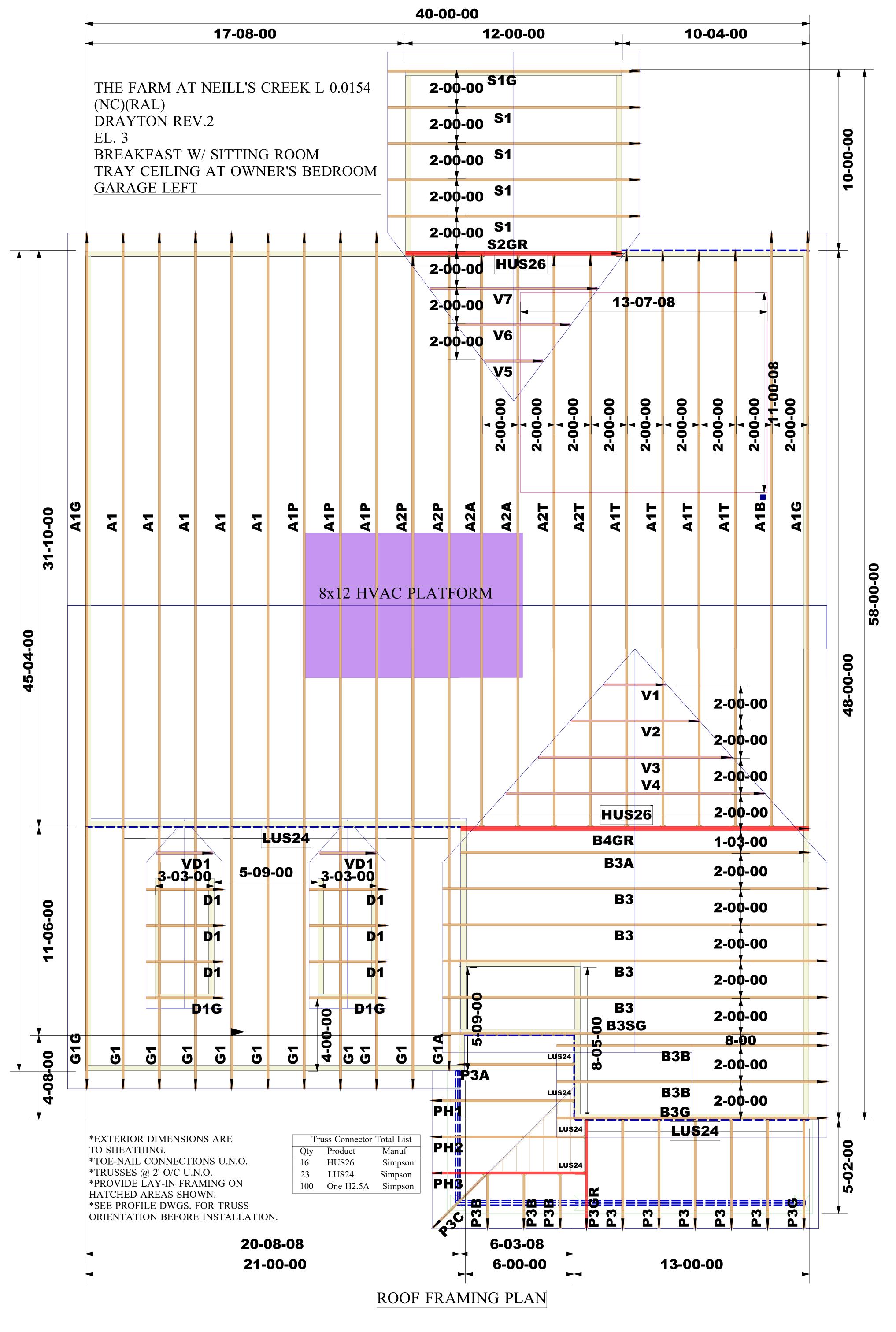
FRAMING DETAILS
FARM AT NEIL'S CREEK
LOT 154 - DRAYTON 3

SD3.0





ROOF TRUSS LAYOUT SCALE: NTS



Job #: **WARNING: NOTE:** Customer: DRB Raleigh IT IS THE RESPONSIBILITY OF THE BUILDING 2505-7934 PERMANENT BRACING IS NOT THE RESPONSIBILITY OF DESIGNER OR ARCHITECT TO PROVIDE AN THE TRUSS DESIGNER. PLATE MANUFACTURER. OR THE APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN TRUSS MANUFACTURER. PERSONS ERECTING Third-Party Quality Assurance Licensee Job Name: The Farm at Neill's Creek TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS ADVICE REGARDING THE ERECTION BRACING WHICH FOR MECHANICAL EQUIPMENT AND/OR PLUMBING TPI Plant W974 ALWAYS REQUIRED TO PREVENT TOPPLING AND (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION. DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES Structural, LLC Designer: THIS COMPANY IS A TRUSS MANUFACTURER WHOSE <u>ot #:</u> 00.0154 RESPONSIBILITIES ARE LIMITED TO THOSE 201 Poplar Avenue DESCRIBED IN WTCA 1-1995 "DESIGN TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS Priyanka Santra
Sales Rep: Thurmont, MD 21788 PLUMB POSITION WHERE NO SHEATHING IS APPLIED ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE DIRECTLY TO TOP AND/OR BOTTOM CHORDS. THEY CONSTRUCTION DESIGN. DRAWINGS. DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED Phone: 301-271-7591 TRUSSES MANUFACTURED BY THIS COMPANY. Model Name: DRAYTON REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY. **Robbie Zarobinski**