SE-22203

SE-22205

SE-22206

SE-22209

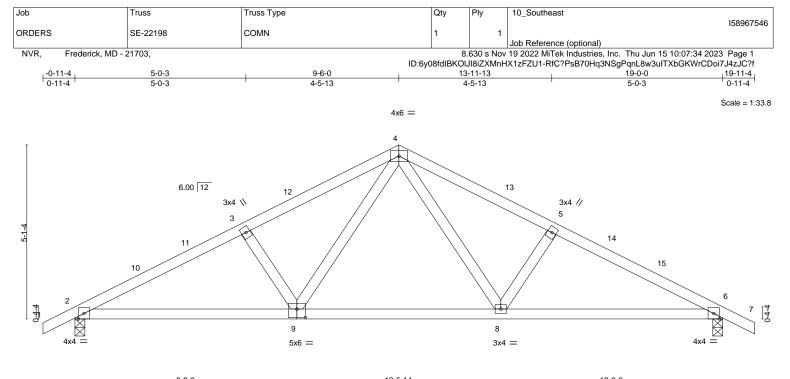


Stell NO. MOBL Set NO. MODL ARUBA BAY VERSION 02 VERSION 02 VERSION 02 RELEASE NO NEMUNG TITLE RELEASE NO NEMUNG TITLE REVENDA BAY NEMUNG TITLE REVENDA BAY NEMUNG TITLE REVENDA BAY NEMUNG TITLE DRAWING TITLE NAMING TITLE							
DRAWING IIILE Interse plans, these plans, there pl	SHEET NO.		SET NO. ABYOO VERSION 02		© NVR, Inc., The owner, expressly reserves its copyright and other property rights	DIV-COMM-LOT-UNIT	K-0 2
OPTION DESCRIPTION 0PTION DESCRIPTION 5285 Westview Drive, Suite 100 assigned to dry, without first obtaining the expressed written consent of NVR, Inc.	ທ ທ	DRAWING TITLE TRUSS BRACING DETAILS	RELEASE NU DRAWN BY DATE:		in mese plans, mese plans are not to be reproduced, changed, or copied in any form or manner whatsoever, nor are they to be	COMM-LOT KIPLING VILLAGE - OI12	
		OPTION DESCRIPTION	OPTION	NVR, Inc. 5285 Westview Drive, Suite 100 Frederick, MD 21703	designed to dry mind party, winder first obtaining the expressed written consent of NVR, Inc.	STREET ADDRESS 42 SAINTSBURY DRIVE	APT. NO.
	<u>и</u>					NAY VARINA	STATE ZIP NC 27526

SE-22210



- IF TRUSS DOES NOT APPEAR ON THIS TRUSS BRACING SHEET, NO ADDITIONAL LATERAL BRACING IS REQUIRED.
- 2. 2X4 SPF#2 LATERAL BRACES SHALL BE NAILED TO MINIMUM (3) TRUSS MEMBERS WITH MINIMUM (2) IOD NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR
- LATERAL BRACING. 3. WEB "T" BRACE, DETAIL **3/RF-IC**, IS REQUIRED WHERE LATERAL BRACING IS NOT CONTINUOUS ACROSS THREE (3) OR MORE TRUSSES AND MAY BE USED IN
- LIEU OF 2X4 LATERAL BRACING.
 4. DIAGONAL BRACING REQUIRED WHEN LATERAL BRACING IS REQUIRED (4/RF-IC)
- 5. STUDDED GABLE BRACING DETAIL (I/RF-Ic) TO BE UTILIZED FOR TRUSSES 6'-9" IN HEIGHT OR GREATER.
 5. PARTIALLY SHEATHED GABLES, SEE (5/RF-Ic) FOR "L"
- BRACING WHEN REQUIRED.
- LATERAL BRACING CAN BE APPLIED TO EITHER SIDE OF THE WEB MEMBER IDENTIFIED IN THE DRAWING.
- . SHEATHING (OSB OR GYPSUM) REPLACES LATERAL AND DIAGONAL TRUSS BRACING.



	6-6-2	1	12-5-14		19-0-0
1	6-6-2	I	5-11-13		6-6-2
Plate Offsets (X,Y) [9:	0-3-0,0-3-0]				
LOADING (psf) TCLL 30.0 (Roof Snow=30.0) TCDL TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIBC2021/TPI2014	CSI. TC 0.52 BC 0.55 WB 0.28 Matrix-S	DEFL. in Vert(LL) -0.06 Vert(CT) -0.12 Horz(CT) 0.04 Wind(LL) 0.03	(loc) l/defl L/d 9 >999 360 2-9 >999 240 6 n/a n/a 9 >999 240	PLATES GRIP MT20 197/144 Weight: 87 lb FT = 20%
BOT CHORD 2x4 SP N	o.2 or 2x4 SPF No.2 o.2 or 2x4 SPF No.2 o.3 or 2x4 SPF Stud			Structural wood sheathing dire Rigid ceiling directly applied or	ctly applied or 4-0-6 oc purlins. 10-0-0 oc bracing.
Max Uplif	2=0-3-8, 6=0-3-8 : 2=90(LC 16) it 2=-121(LC 12), 6=-121(LC 13) / 2=1094(LC 19), 6=1094(LC 20)				

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1781/312, 3-4=-1530/322, 4-5=-1530/322, 5-6=-1781/312

BOT CHORD 2-9=-186/1510, 8-9=-54/913, 6-8=-186/1510

WEBS 3-9=-503/174, 4-9=-85/658, 4-8=-86/658, 5-8=-503/174

NOTES- (8-11)

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 6-6-0, Exterior(2R) 6-6-0 to 12-6-0, Interior(1) 12-6-0 to 16-11-4, Exterior(2E) 16-11-4 to 19-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 2 and 121 lb uplift at joint 6.

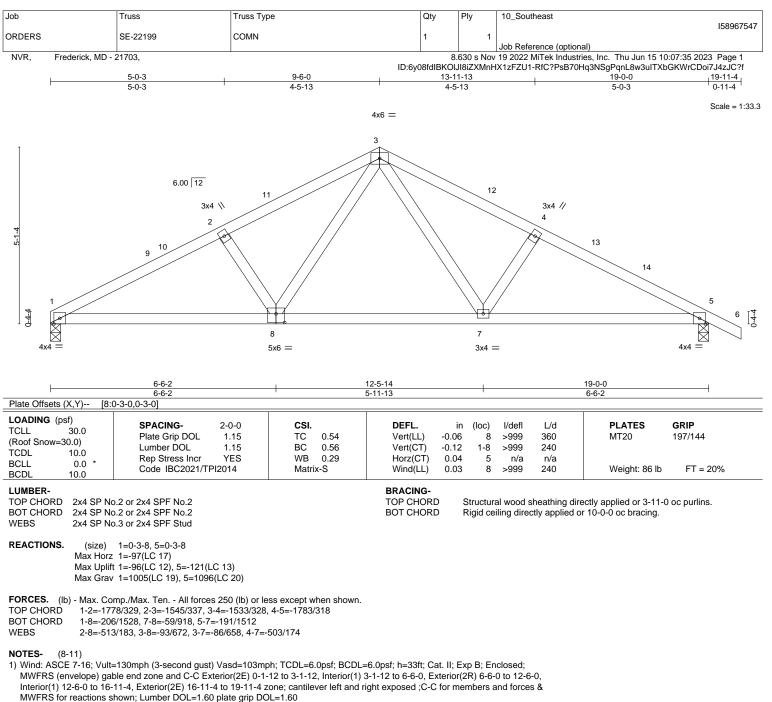
- 8) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 9) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 10) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
- 11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



June 16,2023



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 18.0 psf or 1.00 times flat roof load of 30.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 1 and 121 lb uplift at joint 5.

8) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.

9) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

10) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.

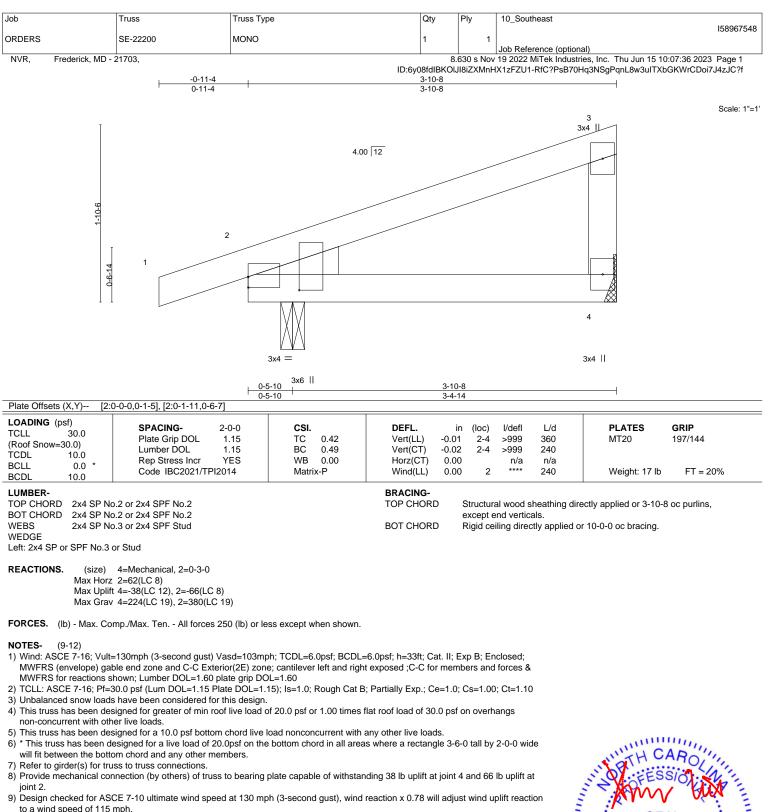
11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



June 16,2023



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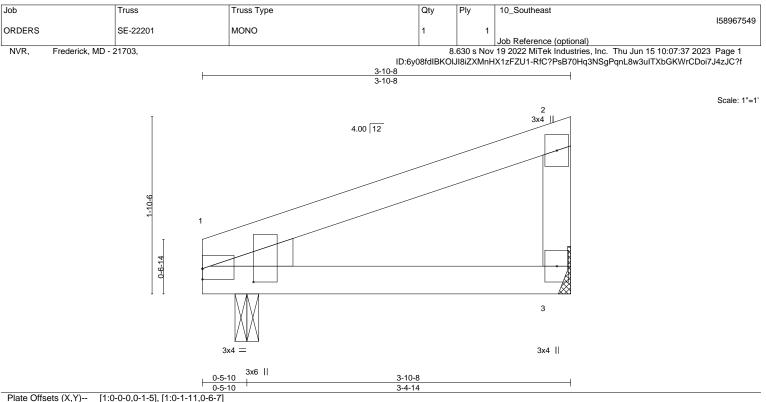
- 10) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 11) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
- 12) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



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A MiTek 818 Soundside Road

Edenton, NC 27932



LOADING (psf) TCLL 30.0 (Roof Snow=30.0) TCDL TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IBC2021/TPI2014	CSI. TC 0.44 BC 0.38 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.02 0.00 0.00	(loc) 1-3 1-3 1	l/defl >999 >999 n/a ****	L/d 360 240 n/a 240	PLATES MT20 Weight: 15 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No	.2 or 2x4 SPF No.2		BRACING- TOP CHORE	o s	structur	al wood s	sheathing dir	ectly applied or 3-10-8	3 oc purlins,

TOP CHC	IRD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 3-10-
BOT CHO	RD 2x4 SP No.2 or 2x4 SPF No.2		except end verticals.
WEBS	2x4 SP No.3 or 2x4 SPF Stud	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEDGE			
1 - 44 - 04 - 4 - 6			

Left: 2x4 SP or SPF No.3 or Stud

REACTIONS. (size) 3=Mechanical, 1=0-3-0 Max Horz 1=60(LC 8) Max Uplift 3=-41(LC 8), 1=-16(LC 8) Max Grav 3=234(LC 18), 1=234(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8-11)

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) Unbalanced snow loads have been considered for this design.

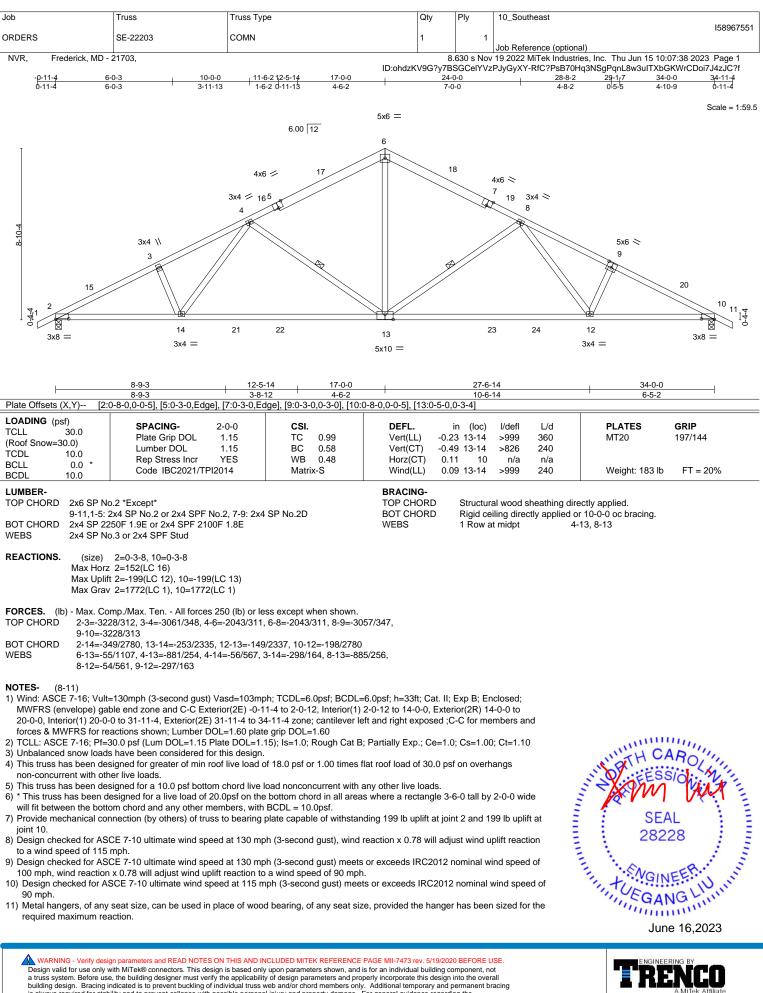
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 3 and 16 lb uplift at joint 1.
- 8) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 9) Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 10) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.
- 11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



June 16,2023

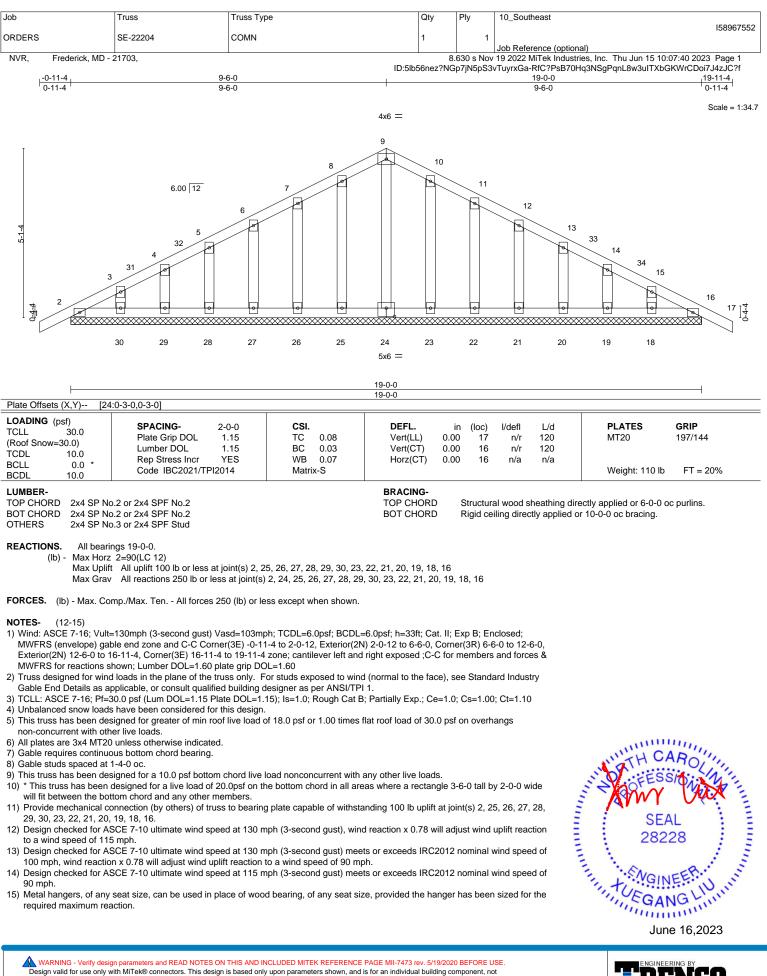


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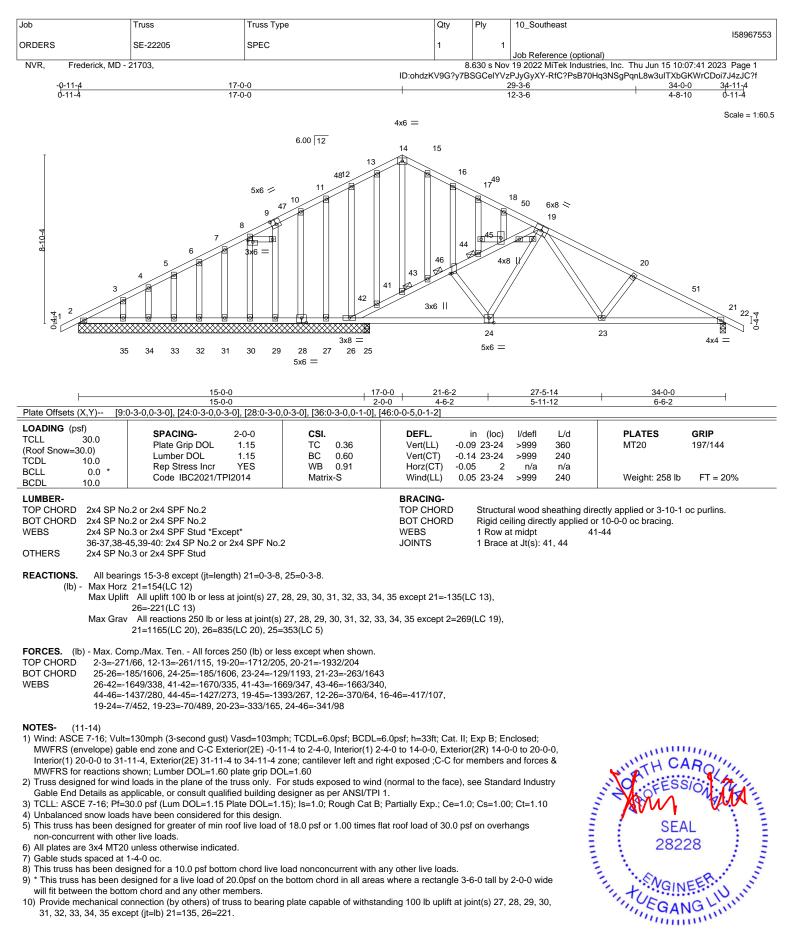
besign valid to less only with with twe commendations. This besign is based only upon parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	10_Southeast
					158967553
ORDERS	SE-22205	SPEC	1	1	
					Job Reference (optional)
NVR, Frederi	ick, MD - 21703,		8	.630 s Nov	19 2022 MiTek Industries, Inc. Thu Jun 15 10:07:41 2023 Page 2

ID:ohdzKV9G?y7BSGCelYVzPJyGyXY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

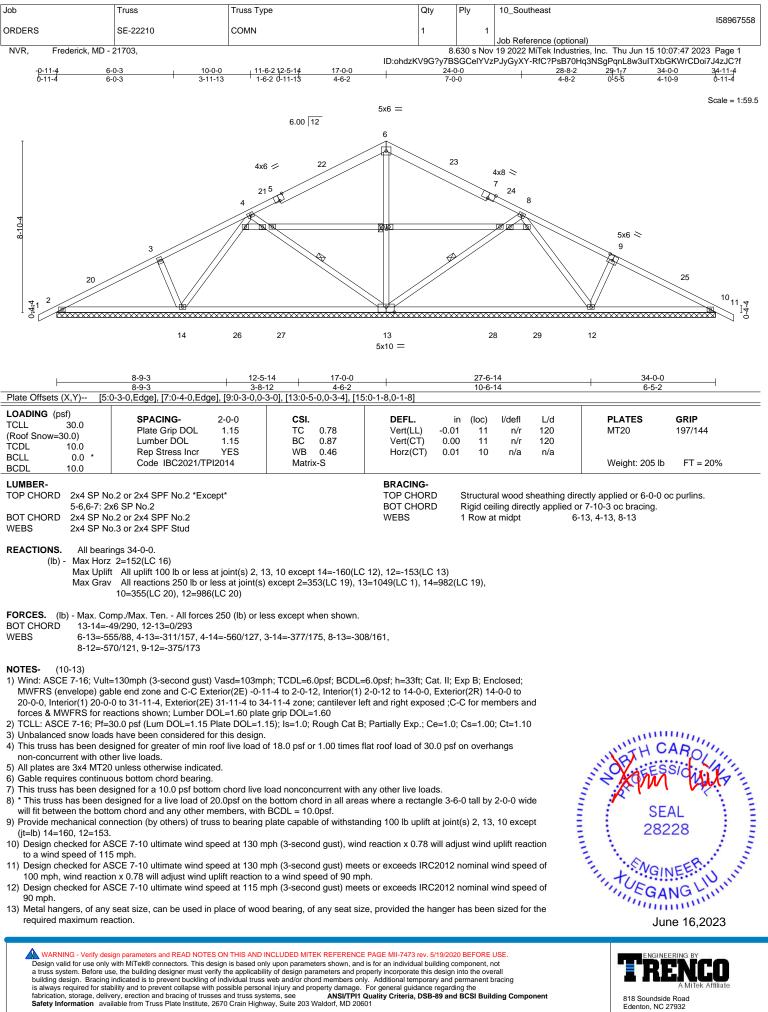
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 Design checked for ASCE 7-10 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

13) Design checked for ASCE 7-10 ultimate wind speed at 115 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 90 mph.

14) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

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