

Carter Sanford Component Plant 298 Harvey Faulk Rd Sanford, NC 27332

Phone #:919-775-1450

# Builder: HH Hunt Homes Raleigh

# Morgan HB 3FL TRAY NOOK FL GLH



# THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.

2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.

3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.

4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.

5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.

6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.

7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.

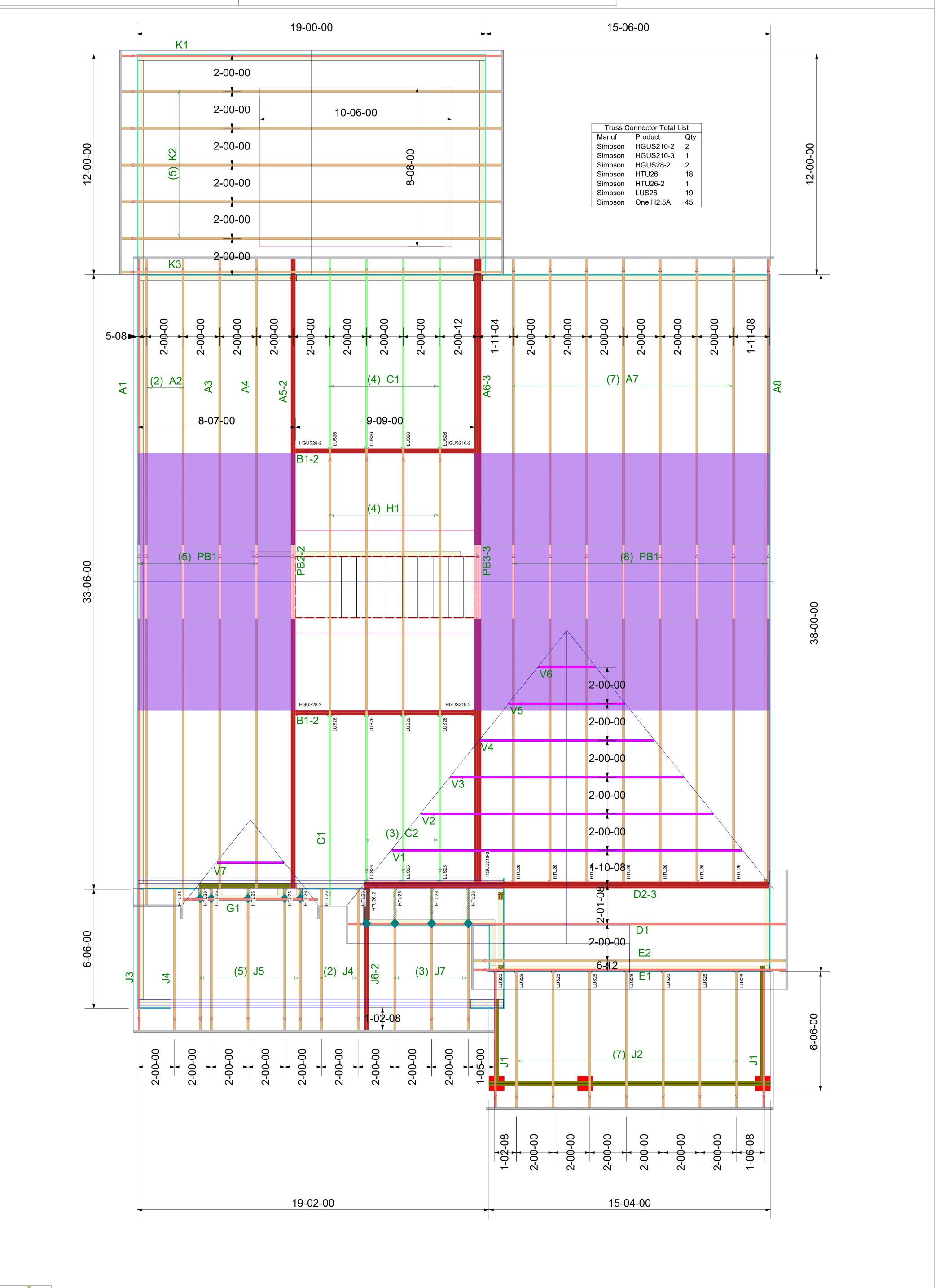
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death. 9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: \_\_\_\_\_

Date: \_\_\_\_\_

\*\* DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

\*\* TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.





TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS

CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

\*\* ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.	T-INCH-SIXTEENTH. ** All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

	HH Hunt Homes Raleigh Durham		<b>THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.</b> These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss	Re\ 00/00/00 00/00/00 00/00/00 00/00/00
NTS Designer: Off Wes Project Num 050180 Sheet Num	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL GLH	CARTER	design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The disign of the tuss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available	000 000 7 00
ber: <b>5</b>	ROOF PLACEMENT PLAN	Lumber	from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179	Vame Vame Vame



Trenco 818 Soundside Rd Edenton, NC 27932

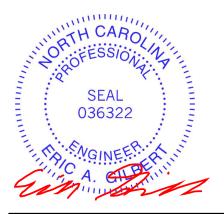
Re: 25050180-01 53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL GLH

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I73807987 thru I73808023

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844

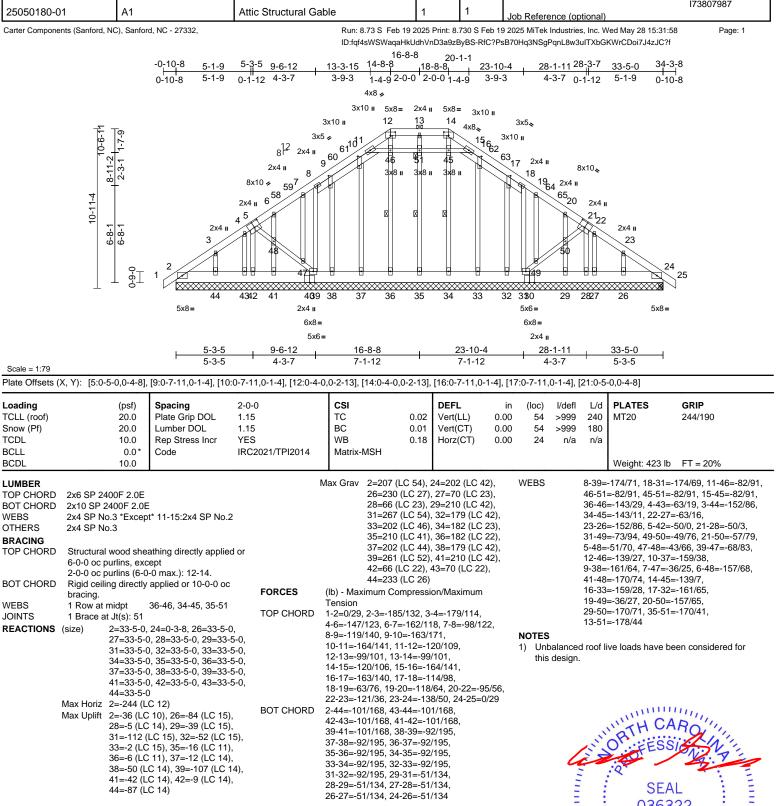


May 29,2025

Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A1	Attic Structural Gable	1	1	Job Reference (optional)





#### continued on page 2

Loading

Snow (Pf)

LUMBER

TCDL

BCLL

BCDL

WEBS

WEBS

JOINTS

OTHERS

Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE WARNING Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A1	Attic Structural Gable	1	1	I73807987 Job Reference (optional)

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-8-8, Interior (1) 2-8-8 to 11-4-6, Exterior(2R) 11-4-6 to 22-0-10, Interior (1) 22-0-10 to 30-8-8, Exterior(2E) 30-8-8 to 34-3-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 3x6 MT20 unless otherwise indicated.
- 9) Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) N/A
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

14) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:31:58 ID:fqf4sWSWaqaHkUdhVnD3a9zByBS-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 2



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SEAL 036322

OR

Contraction of the

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



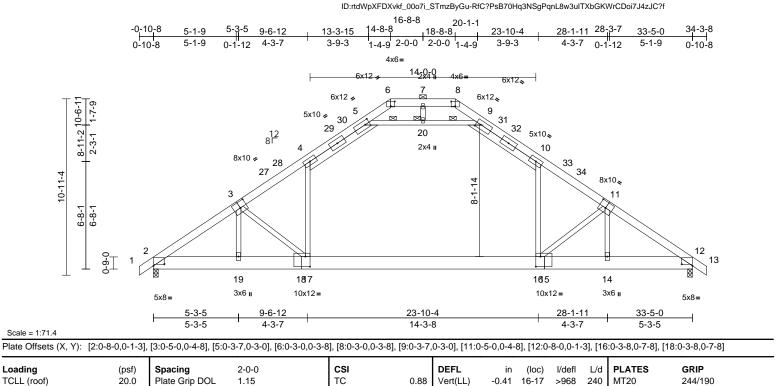
Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A2	Attic	2	1	I73807988 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:00

Page: 1

Carter Components (Sanford, NC), Sanford, NC - 27332,

Looding



Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.88	Vert(LL)	-0.41	16-17	>968	240	MT20	244/190	
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.41	Vert(CT)	-0.69	16-17	>585	180			
TCDL	10.0	Rep Stress Incr	YES		WB	0.64	Horz(CT)	0.03	12	n/a	n/a			
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-MSH		Attic	-0.18	16-17	>941	360			
BCDL	10.0											Weight: 307 lb	FT = 20%	_
	2-2-0 oc purlins, exc 2-0-0 oc purlins (10- Rigid ceiling directly bracing.	t* 5-9:2x4 SP No.2 athing directly applied ept 0-0 max.): 6-8. applied or 10-0-0 oc 5-20, 9-20 12=0-3-8 C 12)	3)	Vasd=103mp II; Exp B; End and C-C Exte 2-5-10 to 11- (1) 22-0-10 to zone; cantiler and right exp MWFRS for r grip DOL=1.6 TCLL: ASCE Plate DOL=1 DOL=1.15); I CS=1.00; Ct=	7-16; Pr=20.0 psf .15); Pf=20.0 psf ( s=1.0; Rough Cat	BCDL=6 envelope 5 2-5-10 11-4-6 to (2E) 30 xposed bers an umber I (roof LL Lum DC B; Fully	.0psf; h=25ft exterior zon Interior (1) 22-0-10, Int 11-6 to 34-3; end vertical d forces & DOL=1.60 plat :: Lum DOL= UL=1.15 Plate Exp.; Ce=0.9	ne erior -8 left ate 1.15 9;						_
FORCES	(lb) - Maximum Com Tension		5)	This truss ha	s been designed f osf or 1.00 times fl									
TOP CHORD	1-2=0/29, 2-4=-3459 5-6=0/1170, 6-7=0/1 8-9=0/1171, 9-10=-2 12-13=0/29		6) /0, 7)	overhangs no Provide adeo This truss ha	on-concurrent with Juate drainage to p s been designed f ad nonconcurrent v	other liv prevent or a 10.0	ve loads. water ponding ) psf bottom	g.						
BOT CHORD	2-19=-14/2877, 17-1 16-17=0/2409 14-16	9=-8/2876, 6=0/2876, 12-14=0/28	8)	* This truss h	as been designed n chord in all areas	for a liv	e load of 20.0				3	WITH CA	RO	
WEBS	,	=0/1370, 5-20=-4063/0 494/110, -16=-689/269,		3-06-00 tall b chord and an Ceiling dead	y 2-00-00 wide wi y other members. load (5.0 psf) on r Vall dead load (5.0	ll fit betv nember	veen the botto	,		4	in	R	BAR	
NOTES	, -			10-16				,		-		CEAL	1 E	
	d roof live loads have	been considered for	11) 12)	chord dead lo Graphical pu or the orienta bottom chord	ecked for L/360 d	ed only t does no long the	o room. 16-1 ot depict the s top and/or	7		111100	A A A A A A A A A A A A A A A A A A A		• -	

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



G mm May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A3	Attic	1	1	I73807989 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries. Inc. Wed May 28 15:32:00

-0.18 15-16

Attic

>941

360

Weight: 304 lb

FT = 20%

Page: 1

Carter Components (Sanford, NC), Sanford, NC - 27332,

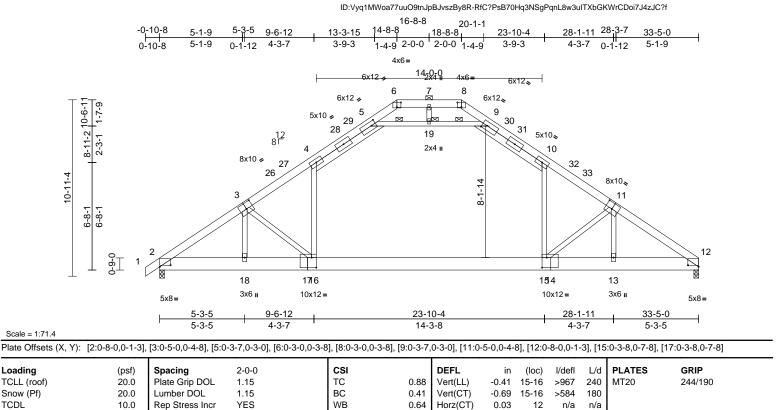
Loading

Snow (Pf)

TCDL

BCLL

BCDL



LUMBER TOP CHORD BOT CHORD WEBS	2x6 SP 2400F 2.0E 2x10 SP 2400F 2.0E 2x4 SP No.3 *Except* 5-9:2x4 SP No.2	2)	Wind: ASCE Vasd=103mp II; Exp B; End and C-C Exte
BRACING TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 6-8.		2-5-10 to 11- (1) 22-0-10 to zone; cantilev and right exp
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	2)	MWFRS for r grip DOL=1.6 TCLL: ASCE
WEBS JOINTS	1 Row at midpt 5-19, 9-19 1 Brace at Jt(s): 19	3)	Plate DOL=1
REACTIONS	(size) 2=0-3-8, 12=0-3-8 Max Horiz 2=240 (LC 13) Max Grav 2=2018 (LC 52), 12=1968 (LC 54)	4)	DOL=1.15); I Cs=1.00; Ct= Unbalanced s design.
FORCES	(lb) - Maximum Compression/Maximum Tension	5)	This truss has load of 12.0 p
TOP CHORD	1-2=0/29, 2-4=-3460/0, 4-5=-2423/83, 5-6=0/1171, 6-7=0/1513, 7-8=0/1513, 8-9=0/1172, 9-10=-2423/83, 10-12=-3464/0	6) 7)	overhangs no Provide adeq This truss has
BOT CHORD	2-18=-22/2871, 16-18=-17/2870, 15-16=0/2403, 13-15=0/2873, 12-13=0/2875	8)	chord live loa * This truss h
WEBS	4-16=0/1370, 10-15=0/1371, 5-19=-4065/0, 9-19=-4065/0, 3-18=-495/110, 11-13=-490/113, 11-15=-696/272, 3-16=-689/267, 7-19=0/203	9)	on the bottom 3-06-00 tall b chord and an Ceiling dead
NOTES	ed sof live leads have been appointed for	0)	5-19, 9-19; V

0.0

10.0

Code

IRC2021/TPI2014

Unbalanced roof live loads have been considered for 1) this design.

oh; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. closed; MWFRS (envelope) exterior zone erior(2E) -0-10-8 to 2-5-10, Interior (1) 4-6, Exterior(2R) 11-4-6 to 22-0-10, Interior o 30-0-14, Exterior(2E) 30-0-14 to 33-5-0 ever left and right exposed ; end vertical left osed:C-C for members and forces & reactions shown; Lumber DOL=1.60 plate 60 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15

7-16; Vult=130mph (3-second gust)

Matrix-MSH

- .15); Pf=20.0 psf (Lum DOL=1.15 Plate Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; =1.10 snow loads have been considered for this
- is been designed for greater of min roof live psf or 1.00 times flat roof load of 20.0 psf on on-concurrent with other live loads.
- quate drainage to prevent water ponding.
- is been designed for a 10.0 psf bottom ad nonconcurrent with any other live loads.
- has been designed for a live load of 20.0psf m chord in all areas where a rectangle by 2-00-00 wide will fit between the bottom ny other members.
- load (5.0 psf) on member(s). 4-5, 9-10, Wall dead load (5.0psf) on member(s).4-16, 10-15
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 15-16
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Attic room checked for L/360 deflection.
- LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932

Job	D	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25	050180-01	A4	Attic	1	1	I73807990 Job Reference (optional)

Scale = 1:73.8

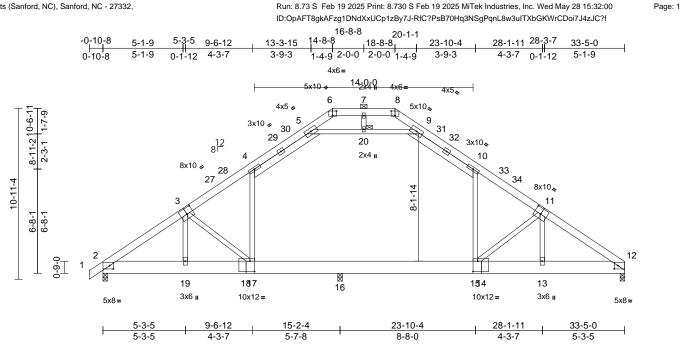


Plate Offsets (X, Y):	Plate Offsets (X, Y): [2:0-2-14,0-2-8], [3:0-5-0,0-4-8], [6:0-3-0,0-3-8], [8:0-3-0,0-3-8], [11:0-5-0,0-4-8], [12:0-2-14,0-2-8], [14:0-3-8,0-8-0], [17:0-3-8,0-8-0]											
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.33	Vert(LL)	-0.19	15-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.30	15-16	>740	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(CT)	0.02	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH		Attic	-0.19	15-16	>999	360		
BCDL	10.0										Weight: 304 lb	FT = 20%

DODL	10.0			_
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD JOINTS REACTIONS	$\begin{array}{llllllllllllllllllllllllllllllllllll$	2) 3) 4)	Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-5-10, Interior (1) 2-5-10 to 11-4-6, Exterior(2R) 11-4-6 to 22-0-10, Interior (1) 22-0-10 to 30-0-14, Exterior(2E) 30-0-14 to 33-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10 Unbalanced snow loads have been considered for this	
	Max Grav 2=1246 (LC 42), 12=1289 (LC 54), 16=1571 (LC 52)	, 5)	design.	
FORCES	(lb) - Maximum Compression/Maximum	5)	This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on	
TOP CHORD	Tension 1-2=0/29, 2-4=-1972/71, 4-5=-1352/160, 5-6=-329/206, 6-7=-172/352, 7-8=-172/352, 8-9=-314/288, 9-10=-1312/166, 10-12=-2279/65	6) 7) 8)	overhangs non-concurrent with other live loads. Provide adequate drainage to prevent water ponding. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf	
BOT CHORD	2-19=-92/1575, 17-19=-92/1576, 16-17=0/1092, 15-16=0/1092, 13-15=0/1858, 12-13=0/1856	0)	3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.	
WEBS	4-17=-205/153, 10-15=0/311, 5-20=-1487/139, 9-20=-1487/139, 3-19=-96/441, 11-13=-51/536,	9)	5	
NOTES 1) Unbalance	11-15=-998/234, 3-17=-701/265, 7-20=0/102 ed roof live loads have been considered for	,	Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 16-17, 15-16	
this desig	٦.	11)	One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at $jt(s)$ 12 and 2. This connection is for uplift only	

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **PCB Building Component Scietur Information**. Building from the Structure Building Component Advance interpretented and the properties of th and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

and does not consider lateral forces.

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A5-2	Attic Girder	1	2	I73807991 Job Reference (optional)

Scale = 1:74.7

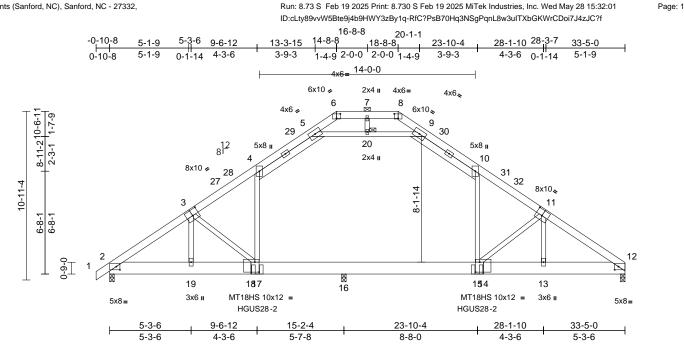


Plate Offsets (	(X, Y): [2:0-2-14,0-2-8	], [3:0-5-0,0-4-8], [6:0	-3-0,0-3-8	3], [8:0-3-0,0-3	-8], [11:0-5-0,0-4-8	], [12:0-	2-14,0-2-8], [	[14:0-3-4	1,0-7-12	], [17:0-3	3-4,0-7	-12]	
Loading TCLL (roof) Snow (Pf) TCDL BCLL	(psf) 20.0 20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC202 <sup>2</sup>	1/TPI2014	CSI TC BC WB Matrix-MSH	0.63 0.58 0.34		in -0.23 -0.39 0.02 -0.23	(loc) 15 15 12 15-16		L/d 240 180 n/a 360	PLATES MT20 MT18HS	<b>GRIP</b> 244/190 244/190
BCDL	10.0											Weight: 609 lb	FT = 20%
FORCES TOP CHORD BOT CHORD WEBS	6-0-0 oc purlins, exc 2-0-0 oc purlins (10- Rigid ceiling directly bracing. 1 Brace at Jt(s): 20 (size) 2=0-3-8, 1 Max Horiz 2=240 (LC Max Uplift 2=-188 (LL 16=-169 ( Max Grav 2=2654 (L 16=3746 ( (lb) - Maximum Com Tension 1-2=0/29, 2-4=-4386 5-6=-112/1252, 6-7= 7-8=-154/1695, 8-9= 9-10=-2348/229, 10 2-19=-343/3637 12 19-17=-72/2462, 15 13-15=-254924 (L2) 13-15=-254924 (L2) 13-15=-254924 (L2) 14-17=-72/2463 (L2) 14-17=-72/2463 (L2) 14-17=-228/404 (L2) 5-20=-4057/469, 9-2	t* 5-9:2x4 SP No.2 athing directly applied to 0-0 max): 6-8. applied or 10-0-0 oc 12=0-3-8, 16=0-3-8 C 11) C 12), 12=-218 (LC 1 LC 12) C 38), 12=2898 (LC (LC 46) pression/Maximum 3/320, 4-5=-2428/265 -154/1695, -154/1695, -148/1388, 12=-507/406 19-4944/3645, 15=-139/1702,	d or 2) 3), 3) 38), 4) , 5) 6) 7)	(0.131"x3") r Top chords of staggered at Bottom chord staggered at Web connec Except mem member 10- All loads are except if note CASE(S) see provided to c unless othen Unbalanced this design. Wind: ASCE Vasd=103mg II; Exp B; En cantilever lef right exposed TCLL: ASCE Plate DOL=1 DOL=1.15); Cs=1.00; Ct Unbalanced design. This truss ha load of 12.0 overhangs n Provide aded All plates are	ds connected as fo :0-9-0 oc. ted as follows: 2x4 ber 4-17 2x4 - 1 ro 15 2x4 - 1 row at 0 considered equal! ed as front (F) or b ction. Ply to ply cor distribute only loads wise indicated. roof live loads hav 7-16; Vult=130mp ph; TCDL=6.0psf; f closed; MWFRS (c ft and right exposed d; Lumber DOL=1. E 7-16; Pr=20.0 psf [.15); Pf=20.0 psf ( Is=1.0; Rough Cat	vs: 2x6 - llows: 2 - 1 row w at 0-5- -5-0 oc. y applie ack (B) nection s noted e been of h (3-sec BCDL=6 envelope d; end v 60 plate (roof LL Lum DC B; Fully been cor or great at roof k other lin prevent v ss other or great	- 2 rows x10 - 2 rows at 0-9-0 oc, -0 oc, Excep d to all plies, face in the LC s have been as (F) or (B), considered for b.0psf; h=25ft a) exterior zor vertical left ar grip DOL=1. .: Lum DOL= DL=1.15 Plate Exp.; Ce=0.9 nsidered for the er of min roof poad of 20.0 p ve loads. water ponding; wise indicate 0 psf bottom	DAD or ; Cat. ne; nd .60 1.15 9; his f live sf on g. ed.	on 3-C 12) Cec 5-2 10- 13) Bo cha 15- 14) On rec UF oni 15) Gr: boi 15) Gr: boi 16-1 stat tru	the botto 6-00 tall brd and a iling deal 20, 9-20; -15 ttom chc brd dead -16 te H2.5A commend LIFT at, ly and de aphical p the orier ttom chc e Simps 6d Trus ss(es) to	om cho I by 2-0 any oth do load Wall o ord live I load (f Simps ded to jt(s) 12 bes not ourlin re tration o rd. on Stro s) or ec 9-7-0 fr b back f	ord in all areas w 20-00 wide will fi lear members. (5.0 psf) on men- dead load (5.0psf) load (40.0 psf) a 5.0 psf) applied constrong-Tie c connect truss to , 2, and 16. This consider lateral apresentation do of the purlin alor prog-Tie HGUS282 quivalent spaced om the left end to face of bottom cl	bearing walls due to connection is for uplift forces. wes not depict the size ong the top and/or 3-2 (36-16d Girder, d at 14-3-0 oc max. to 23-10-0 to connect
	(IIIIII	mmm										Ma	y 29,2025

Continued on page 2 Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTeR% connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) with the SCH trust in the prevent collapse in the providence of the property demoge for property dependence of the property incorporate the property and prevents on the providence of the property and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) with the SCH trust information, evolved the trust set of the storage of the property damage. For another the property dependence on the and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A5-2	Attic Girder	1	2	I73807991 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:01 ID:cLty89vvW5Bte9j4b9HWY3zBy1q-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 2

18) Attic room checked for L/360 deflection.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-4=-60, 4-5=-70, 5-6=-60, 6-8=-60, 8-9=-60, 9-10=-70, 10-12=-60, 17-24=-20, 15-17=-30, 15-21=-20, 5-20=-10, 9-20=-10

Drag: 4-17=-10, 10-15=-10

Concentrated Loads (lb) Vert: 17=-2658 (B), 15=-2658 (B)

an Din



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

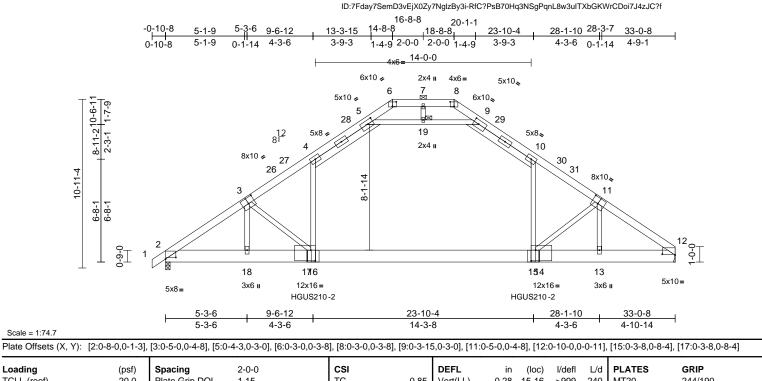


Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A6-3	Attic Girder	1	3	I73807992 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:01

Carter Components (Sanford, NC), Sanford, NC - 27332,

Scale = 1:74.7



Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2021/TPI20		<b>CSI</b> TC BC WB Matrix-MSH	0.85 0.21 0.54	DEFL Vert(LL) Vert(CT) Horz(CT) Attic	in -0.28 -0.51 0.02 -0.09	15-16 12	l/defl >999 >785 n/a >999	L/d 240 180 n/a 360	PLATES MT20 Weight: 905 lb	<b>GRIP</b> 244/190 FT = 20%		
	6-0-0 oc purlins, exc 2-0-0 oc purlins (10- Rigid ceiling directly bracing. 1 Brace at Jt(s): 19	ot* 5-9:2x4 SP No.2 athing directly applied cept -0-0 max.): 6-8. applied or 10-0-0 oc 12= Mechanical C 9) C 12), 12=-261 (LC 1	(0.13 Top o stagg Botto d or Stagg Web Exce mem 2) All lo exce CASI provi unles 3) 3) Ubba	"x3") nai hords con- ered at 0- n chords ered at 0- connecte- to membe- per 10-15 ds are co- to for the format ds are co- to format (S) secti- led to dis s otherwis- anced ro	connected as fol	vs: 2x6 - 1 row w at 0-2 4-0 oc. / applie ack (B) nection ; noted	- 2 rows x10 - 2 rows at 0-9-0 oc, I-0 oc, Except d to all plies, face in the LC s have been as (F) or (B),	DAD	5-1 10- 12) Bot chc 13) Rei 14) Pro bea 12. 15) On rec UP doe 16) Gra or t	9, 9-19; 15 tom choord dead fer to gir vvide me aring pla e H2.5A ommeno LIFT at j es not co aphical p	Wall c rd live load (! der(s) f chanic te capa Simps ded to c t(s) 2. nsider urlin re tation c	load (40.0 psf) ar 5.0 psf) applied oi or truss to truss a l connection (by ble of withstandii on Strong-Tie con connect truss to b This connection is lateral forces.	on member(s).4-16 ad additional bottom nly to room. 15-16 connections. others) of truss to ng 261 lb uplift at join nnectors earing walls due to s for uplift only and s not depict the size	n bint	
FORCES	(lb) - Maximum Com Tension 1-2=0/29, 2-4=-7420 5-6=-204/3506, 6-7= 7-8=-224/4403, 8-9= 9-10=-4578/325, 10	, Vasd , II; Ex cantil	4) Wind: AŠCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60							<ul> <li>17) Use Simpson Strong-Tie HGUS210-2 (46-10d Girder, 16-10d Truss) or equivalent spaced at 14-3-0 oc max. starting at 9-7-0 from the left end to 23-10-0 to connect truss(es) to front face of bottom chord.</li> <li>18) Fill all nail holes where hanger is in contact with lumber.</li> <li>19) Attic room checked for U360 deflection, U360 d</li></ul>					
BOT CHORD	2-18=-477/6436, 16 15-16=-188/5229, 11 12-13=-301/5800	3-15=-302/5794,	Plate DOL:	DOL=1.1 1.15); ls=	7-16; Pr=20.0 psf 15); Pf=20.0 psf (I =1.0; Rough Cat	Lum DC	DL=1.15 Plate	•	,		A.L.	ORTEE89	ROUNT		
WEBS	3-18=-540/126, 3-16 4-16=-252/4242, 10 11-15=-1093/347, 1 5-19=-9561/613, 9-1 7-19=0/420	-15=-248/4137, 1-13=-1016/119,	6) Unba desig 7) This	n. russ has	1.10 now loads have b been designed fo sf or 1.00 times fla	or great	er of min roof	live		4	20	12/	L AL		
NOTES			overh 8) Provi 9) This choro 10) * This on th 3-06-	angs non le adequa russ has live load truss has bottom o 00 tall by	n-concurrent with late drainage to p been designed for nonconcurrent w is been designed chord in all areas 2-00-00 wide will o ther members.	other lin revent or a 10.0 vith any for a liv s where	ve loads. water ponding 0 psf bottom other live loa re load of 20.0 a rectangle	g. ds. Opsf		1100	A A A A A A A A A A A A A A A A A A A	SEA 0363			

Continued on page 2 Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 1/2/2023 BEFORE USE Design valid for use only with MTek connectors. This design is based only upon parameters and property incorporate this design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

May 29,2025

Page: 1



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A6-3	Attic Girder	1	3	I73807992 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:01 ID:7Fday7SemD3vEjX0Zy7NgIzBy3i-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 2

#### LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (lb/ft)

Vert: 1-4=-60, 4-5=-70, 5-6=-60, 6-8=-60, 8-9=-60, 9-10=-70, 10-12=-60, 16-23=-20, 15-16=-30, 15-20=-20, 5-19=-10, 9-19=-10 Drag: 4-16=-10, 10-15=-10 Concentrated Loads (lb)

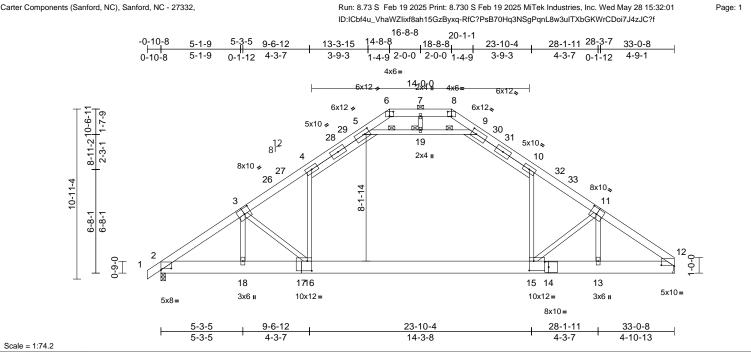
Vert: 16=-2658 (F), 15=-2658 (F)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A7	Attic	7	1	I73807993 Job Reference (optional)

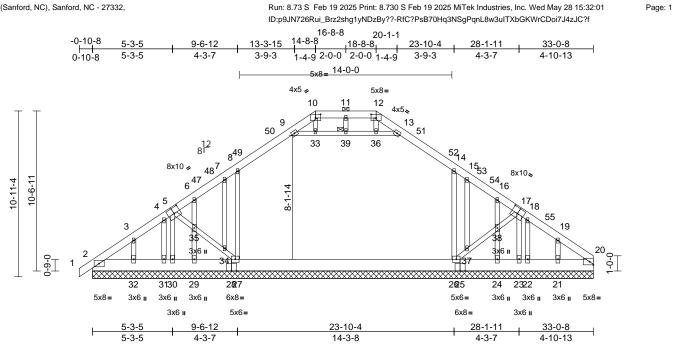


[2:0-8-0,0-1-3], [3:0-5-0,0-4-8], [5:0-3-15,0-3-0], [6:0-3-0,0-3-8], [8:0-3-0,0-3-8], [9:0-3-11,0-3-0], [11:0-5-0,0-4-8], [12:0-10-0,0-0-11], [14:0-3-0,0-4-0], [15:0-3-8,0-7-4], Plate Offsets (X, Y): [16:0-3-8,0-7-8]

		1			1		<b>.</b>					i	
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.87	Vert(LL)	-0.40	15-16	>985	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.41	Vert(CT)	-0.66	15-16	>597	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.63	Horz(CT)	0.02	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021	1/TPI2014	Matrix-MSH		Attic	-0.18	15-16	>946	360		
BCDL	10.0					-						Weight: 302 lb	FT = 20%
LUMBER			2)		7-16; Vult=130m								
TOP CHORD					ph; TCDL=6.0psf;								
BOT CHORD					closed; MWFRS								
WEBS	2x4 SP No.3 *Excep	ot* 5-9:2x4 SP No.2			erior(2E) -0-10-8								
BRACING	$22.0.24 \pm 20.0.44$ Evterior/2E) 20.0.44 to 22.0.9 energy												
TOP CHORD	CHORD Structural wood sheathing directly applied or 2.2.0 consulting except 22-0-2 to 29-8-14, Exterior(2E) 29-8-14 to 33-0-8 zone; cantilever left and right exposed ; end vertical left and												
	2-2-0 oc purlins, exc												
	2-0-0 oc purlins (10-			right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip									
BOT CHORD		applied or 10-0-0 oc		DOL=1.60	20110111, 2011001 2		piato grip						
WEBS	bracing. 1 Row at midpt	5-19, 9-19	3)		7-16; Pr=20.0 ps	sf (roof Ll	L: Lum DOL=	1.15					
JOINTS	1 Brace at Jt(s): 19	5-19, 9-19	,	Plate DOL=1	I.15); Pf=20.0 psf	i (Lum DC	DL=1.15 Plate	9					
REACTIONS	( )	12 Machanical		DOL=1.15);	OL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9;								
REACTIONS	(Size) 2=0-3-8, Max Horiz 2=240 (L0	12= Mechanical		Cs=1.00; Ct:									
	Max Grav 2=1997 (I		54) <sup>4</sup> )		snow loads have	been co	nsidered for th	his					
500050	,	1.	,	design.									
FORCES	(lb) - Maximum Com Tension	npression/iviaximum	5)		as been designed								
TOP CHORD		1/0 1-52376/83			psf or 1.00 times on-concurrent wit			sron					
	5-6=0/1141, 6-7=0/1		6)		quate drainage to			a					
	,	2383/83, 10-12=-3285			as been designed			y.					
BOT CHORD	,		,		ad nonconcurrent			aha				ORTH CA	un,
		5=0/2628, 12-13=0/20	631 8)		nas been designe							WAH CA	Rolly
WEBS	4-16=0/1337, 10-15	=0/1292, 5-19=-3964/			m chord in all area			000			1	R	A LINI
	9-19=-3964/0, 3-18=	=-463/123,		3-06-00 tall I	oy 2-00-00 wide w	vill fit betv	veen the bott	om			1.	O FESS	Dir Vila
	11-13=-652/74, 11-1			chord and a	y other members	3.				4		11 /	City
	3-16=-705/265, 7-19	9=0/199	9)		load (5.0 psf) on					-	-	.0	K
NOTES					Wall dead load (5	6.0psf) on	member(s).4	-16,		-		SEA	1 1 2
	ed roof live loads have	been considered for		10-15						=	:	SLA	• -
this desig	n.		10		d live load (40.0 p					=		0363	22 ; =
					oad (5.0 psf) appl			6		-	3		1 - E
					er(s) for truss to t						1	·	- 1 E -
			12		Irlin representatio			size			20	A. SNOW	EFR. A N
	bottom chore	In dead load (5.0 psr) on member(s). 4-5, 9-10,         In chord live load (5.0 psr) on member(s).4-16,         In chord live load (40.0 psr) and additional bottom         dead load (5.0 psr) applied only to room. 15-16         to girder(s) for truss to truss connections.         cical purlin representation does not depict the size         or chord.         or checked for L/360 deflection.											
					oom checked for L/360 deflection.								
					SE(S) Standard								
					Glanuaru							~	. 00.0005
												May	/ 29,2025

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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	A8	Attic Structural Gable	1	1	I73807994 Job Reference (optional)



# Plate Offsets (X, Y): [5:0-5-0,0-4-8], [10:0-4-0,0-2-13], [12:0-4-0,0-2-13], [17:0-5-0,0-4-8]

			,	1, 1											
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(10	oc) I/de	efl L		LATES	GRIP	
TCLL (roof)	(psi) 20.0	Plate Grip DOL	1.15		TC	0.09	Vert(LL)	n/a	(it	- n			IT20	244/190	
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.03	Vert(CT)	n/a			a 99		1120	244/130	
TCDL	10.0	Rep Stress Incr	YES		WB	0.13	Horz(CT)	0.01			a se ′a n/				
						0.41		0.01		20 11	a n	a			
BCLL BCDL	0.0*	Code	IRC202	1/TPI2014	Matrix-MSH							1	/eight: 318 lb	FT 200/	
BCDL	10.0											VV	veignt: 318 lb	FT = 20%	
LUMBER TOP CHORD			B	OT CHORD	2-32=-80/440, 31-3 30-31=-76/440, 29		,		5)		ced sn	ow loa	ads have been	considered for this	
BOT CHORD	2x6 SP 2400F 2.0E	-			27-29=-76/439, 26				6)	design.	e hae k	h noo	lesigned for an	eater of min roof live	
WEBS	2x10 SP 2400F 2.0E 2x4 SP No.3 *Excep				24-26=-32/187, 23		,							of load of 20.0 psf or	
OTHERS	2x4 SP No.3 Excep 2x4 SP No.3	1 9-13:2x4 SP NO.2			22-23=-32/188, 21								urrent with othe		
	2X4 SP NU.3				20-21=-32/188	22- 02	100,							ent water ponding.	
BRACING		منامنه مانده منابر معمانه	war W	/EBS	8-27=-537/96, 14-2	26=-567	/79.							erwise indicated.	
TOP CHORD	Structural wood she 6-0-0 oc purlins, exc				9-33=-251/114, 33								nuous bottom o		
	2-0-0 oc purlins, exc 2-0-0 oc purlins (6-0				36-39=-251/113, 1								at 2-0-0 oc.	5	
BOT CHORD	Rigid ceiling directly				4-31=-150/26, 3-32				11)	This trus	s has t	een d	designed for a	10.0 psf bottom	
BOTOHORD	bracing.				19-21=-179/79, 5-3			1/2,		chord liv	e load	nonco	oncurrent with a	any other live loads.	
JOINTS	1 Brace at Jt(s): 39				26-37=-33/559, 37									a live load of 20.0psf	
	. ,	20=33-0-8, 21=33-0	-8		17-38=-29/492, 5-3			37,						ere a rectangle	
		3, 23=33-0-8, 24=33-			27-34=0/203, 10-3			170						between the bottom	
		3, 27=33-0-8, 29=33-	,		6-35=-142/47, 29-3									BCDL = 10.0psf.	
	30=33-0-8	3, 31=33-0-8, 32=33-	0-8		15-37=-7/91, 16-3 11-39=-21/21	5=-229/4	15, 24-38=-24	8/49,						ber(s). 8-9, 13-14,	
	Max Horiz 2=239 (LC	C 11)			11-39=-21/21					,	,		,	ad load (5.0psf) on	
	Max Uplift 2=-15 (LC	10), 20=-2 (LC 42),		OTES						member	S).8-2	, 14-2	20		
		LC 52), 24=-555 (LC	<b>Z</b> 1), '		I roof live loads hav	e been	considered to	r							
		C 15), 29=-585 (LC 2		this design.	E 7-16; Vult=130mr	h (2 aa	(10.00 m. 10.00								
		LC 21), 32=-85 (LC			ph; TCDL=6.0psf;			Cat						11	
	Max Grav 2=462 (LC				nclosed; MWFRS (								ATH CA	- 11, in	
		C 11), 23=1060 (LC			terior(2E) -0-10-8 te							11.	ATH CA	ROIL	
		C 42), 26=1018 (LC 5			xterior(2R) 10-0-7			00				10	A SECO	2. 11.1	
		(LC 52), 29=-33 (LC			-8-14, Exterior(2E)			ne:			1	24	FESS	N. Si	
	30=605 (L 32=268 (L	-C 21), 31=88 (LC 22	),		ft and right expose					(				and the second	-
500050	,	,		right expose	ed;C-C for member	s and fo	rces & MWFF	RS			-	:4	ζ.	1 : 5	
FORCES	(lb) - Maximum Com Tension	ipression/iviaximum		for reactions	s shown; Lumber D	OL=1.60	) plate grip				E	1	SFA	L 🕴 🗄	
TOP CHORD	1-2=0/29, 2-3=-556/8	85 3-4561/79		DOL=1.60							Ξ.		0202		
	4-6=-690/70, 6-7=-72		3)		ned for wind loads						1	:	0363	22 : :	
	8-9=-815/175, 9-10=		/96		uds exposed to wir						-	1		1 2	
	11-12=-487/96, 12-1	,	,		rd Industry Gable E						-	1	~	01. 3	
	13-14=-815/175, 14-				ualified building de						1	1	NGING	Enix	
	15-16=-740/85, 16-1		4)		E 7-16; Pr=20.0 ps						1	12	C. GIN	E. F. S.	
	18-19=-269/47, 19-2				1.15); Pf=20.0 psf							11,	AG	ILBUIN	
	, -				Is=1.0; Rough Cat	в; Fully	⊨xp.; Ce=0.9	9;					1111. 0	in the second se	
				Cs=1.00; C1	=1.10								SEA 0363	20 2025	

May 29,2025



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# Continued on page 2

Scale = 1:76

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL				
25050180-01	A8	Attic Structural Gable	1	1	I73807994 Job Reference (optional)				

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 20, 15 lb uplift at joint 2, 11 lb uplift at joint 26, 138 lb uplift at joint 31, 85 lb uplift at joint 32, 512 lb uplift at joint 22, 585 lb uplift at joint 29 and 555 lb uplift at joint 24.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

16) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:01 ID:p9JN726Rui\_Brz2shg1yNDzBy??-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



Page: 2

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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	B1-2	Flat Girder	2	2	I73807995 Job Reference (optional)

2x4 II

4-10-8

4-10-8

4x5 =

Carter Components (Sanford, NC), Sanford, NC - 27332,

Scale = 1:46.1 Loading

TCLL (roof)

Snow (Pf)

LUMBER

TOP CHORD

BOT CHORD

BOT CHORD

FORCES

TOP CHORD

BOT CHORD

WEBS

NOTES

2)

**REACTIONS** (size)

bracing.

Tension

staggered at 0-9-0 oc.

BRACING TOP CHORD

TCDL

BCLL

BCDL

WFBS

Run: 8 73 S. Feb 19 2025 Print: 8 730 S Feb 19 2025 MiTek Industries. Inc. Wed May 28 15:32:02 ID:6Mxb\_R0\_of4nTQx85TUTMfzBy5Z-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

4x5 =

9-9-0

4-10-8

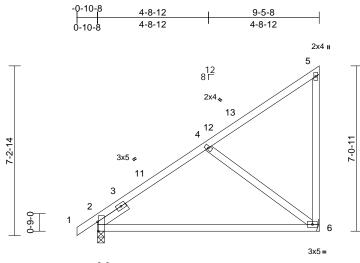
Page: 1

VIIIIIIIIII

\_10 2  $\mathbf{k}$ 8 2 3 6-1-7 6 4 ΠΠ ПП ΠΠ ΠΠ 11 12 5 13 14 3x6 ı 3x6 II 5x8= LUS26 LUS26 LUS26 LUS26 4-10-8 9-9-0 4-10-8 4-10-8 Spacing 2-0-0 CSI DEFL l/defl L/d PLATES GRIP (psf) in (loc) Plate Grip DOL 20.0 1.15 TC 0.57 Vert(LL) -0.02 5 >999 240 MT20 244/190 BC 20.0 Lumber DOL 1 15 0.08 Vert(CT) -0.03 5 >999 180 10.0 Rep Stress Incr NO WB 0.54 Horz(CT) 0.00 4 n/a n/a 0.0 Code IRC2021/TPI2014 Matrix-MSH 10.0 Weight: 205 lb FT = 20% Wind: ASCE 7-16; Vult=130mph (3-second gust) Vert: 1-3=-60, 4-6=-20 3) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. Concentrated Loads (lb) 2x6 SP No.2 2x10 SP 2400F 2.0E II; Exp B; Enclosed; MWFRS (envelope) exterior zone; Vert: 7=-656, 8=-656, 9=-656, 10=-656, 11=-494 (B), 12=-494 (B), 13=-494 (B), 14=-494 (B) cantilever left and right exposed ; end vertical left and 2x4 SP No.3 right exposed; Lumber DOL=1.60 plate grip DOL=1.60 2-0-0 oc purlins (6-0-0 max.): 1-3, except TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 4) end verticals. Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate Rigid ceiling directly applied or 10-0-0 oc DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1 00. Ct=1 10 4= Mechanical, 6= Mechanical 5) Unbalanced snow loads have been considered for this Max Horiz 6=-189 (LC 8) design. Max Uplift 4=-296 (LC 9), 6=-296 (LC 8) Provide adequate drainage to prevent water ponding. 6) Max Grav 4=2678 (LC 1), 6=2678 (LC 1) This truss has been designed for a 10.0 psf bottom 7) (lb) - Maximum Compression/Maximum chord live load nonconcurrent with any other live loads. 8) \* This truss has been designed for a live load of 20.0psf 1-6=-2155/248, 1-2=-1432/133, on the bottom chord in all areas where a rectangle 2-3=-1432/133, 3-4=-2155/248 3-06-00 tall by 2-00-00 wide will fit between the bottom 5-6=-153/157, 4-5=-61/65 chord and any other members. 1-5=-262/2160, 2-5=-2158/160, Refer to girder(s) for truss to truss connections. 3-5=-262/2160 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 6 and 296 lb uplift at joint 4. 1) 2-ply truss to be connected together with 10d MILLIN 11) Graphical purlin representation does not depict the size (0.131"x3") nails as follows: ORT or the orientation of the purlin along the top and/or Top chords connected as follows: 2x4 - 1 row at 0-9-0 bottom chord. oc, 2x6 - 2 rows staggered at 0-9-0 oc. 12) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Bottom chords connected as follows: 2x10 - 2 rows Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-10-8 from the left end to 7-10-8 to Web connected as follows: 2x4 - 1 row at 0-9-0 oc. CONTRACTOR OF THE connect truss(es) to back face of bottom chord. All loads are considered equally applied to all plies, SEAL except if noted as front (F) or back (B) face in the LOAD 13) Fill all nail holes where hanger is in contact with lumber. 14) Hanger(s) or other connection device(s) shall be CASE(S) section. Ply to ply connections have been 036322 provided to distribute only loads noted as (F) or (B), provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the unless otherwise indicated. responsibility of others. LOAD CASE(S) Standard Dead + Snow (balanced): Lumber Increase=1.15, Plate 1) Increase=1.15 GI Uniform Loads (lb/ft) mm May 29,2025 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) 818 Soundside Road and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com) Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	C1	Jack-Closed	5	1	I73807996 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:02 ID:pwvEEn6xScdQzvikCDetprzBy6k-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



3x8 II

ł

9-5-8

Scale = 1:49.2 Plate Offsets (X, Y): [2:0-4-13.Edge]

Plate Offsets (	X, Y): [2:0-4-13,Edge	]										
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MSH	0.52 0.67 0.27	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.20 -0.39 0.03	(loc) 6-9 6-9 2	l/defl >571 >285 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 53 lb	<b>GRIP</b> 244/190 FT = 20%
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Wind: ASC Vasd=103 II; Exp B; I and C-C E to 5-0-13, left and rig exposed;C reactions s DOL=1.60 2) TCLL: ASC Plate DOL DOL=1.15 Cs=1.0; 0	2x4 SP No.2 2x4 SP No.3 Left 2x4 SP No.3 Structural wood she 5-1-10 oc purlins, e Rigid ceiling directly bracing. (size) 2=0-3-8, 6 Max Horiz 2=242 (LC Max Uplift 2=-31 (LC Max Uplift 2=-31 (LC Max Grav 2=476 (LC (Ib) - Maximum Com Tension 1-2=0/29, 2-4=-869/ 5-6=-195/67 2-6=-270/380 4-6=-402/176 CE 7-16; Vult=130mph mph; TCDL=6.0psf; BC Exterior(2R) 5-0-13 to pht exposed ; end vertii shown; Lumber DOL=- CE 7-16; Pr=20.0 psf (L Shown; Carbon St (L) (St (C) (St (C)	athing directly applie xcept end verticals. applied or 10-0-0 oc 5= Mechanical C 13) 14), 6=-70 (LC 11) C 21), 6=514 (LC 21) pression/Maximum 109, 4-5=-176/128, (3-second gust) CDL=6.0psf; h=25ft; tvelope) exterior zon 2-1-8, Interior (1) 2-1 9-3-12 zone; cantile cal left and right orces & MWFRS for 1.60 plate grip roof LL: Lum DOL=1 um DOL=1.15 Plate ; Fully Exp.; C=0.9;	load of 12. overhangs 5) This truss 1 chord live 1 6) * This truss on the bott 3-06-00 tal chord and 7) All bearing 8) Refer to gii 9) Provide me bearing pla 6. 10) One H2.5A recommen UPLIFT at does not ct LOAD CASE(S	has been designed to psf or 1.00 times f non-concurrent with has been designed oad nonconcurrent is has been designed om chord in all area by 2-00-00 wide w any other members is are assumed to be der(s) for truss to tr techanical connection te capable of withst Simpson Strong-Ti ded to connect truss it(s) 2. This connect onsider lateral force b) Standard	lat roof lin o other lin for a 10.1 with any d for a liv s where ill fit betw e User D uss conr n (by oth anding 7 e conne s to bear tion is foi	bad of 20.0 p ve loads. D psf bottom other live loa e load of 20.1 a rectangle veen the bott efined . nections. ers) of truss i 0 lb uplift at j ctors ing walls due	ads. Opsf om to joint e to				SEA 0363	EER A

May 29,2025

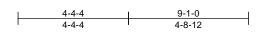
Page: 1



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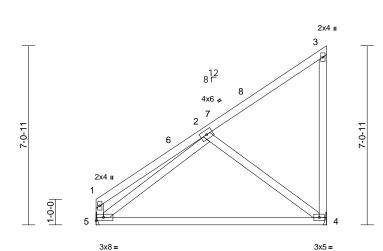
Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	C2	Jack-Closed	3	1	I73807997 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:02 ID:Is4ro5YX\_g9JHRSeJmHJHRzBy6A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





Page: 1



9-1-0



Scale = 1:45.4

		· · · · · · · · · · · · · · · · · · ·			-							
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.58	Vert(LL)	-0.21	4-5	>504	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.71	· · ·	-0.42	4-5	>253	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.23	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 56 lb	FT = 20%
LUMBER			5) * This tr	uss has been designe	d for a liv	ve load of 20.0p	sf					
TOP CHORD	2x4 SP No.2		on the b	ottom chord in all area	as where	a rectangle						
BOT CHORD	2x4 SP No.2			tall by 2-00-00 wide v		ween the bottom	ו					
WEBS	2x4 SP No.3			nd any other members								
BRACING				girder(s) for truss to t								
TOP CHORD				mechanical connection			E					
	6-0-0 oc purlins, ex		and 70	plate capable of withs b uplift at joint 4.	stanuing s	a ib upilit at joiri	5					
BOT CHORD	Rigid ceiling directly bracing.	applied or 10-0-0 c		E(S) Standard								
REACTIONS	0	anical, 5= Mechanic		(-)								
	Max Horiz 5=236 (L	C 11)										
	Max Uplift 4=-76 (LC	C 11), 5=-9 (LC 14)										
	Max Grav 4=493 (L	C 20), 5=403 (LC 20	D)									
FORCES	(lb) - Maximum Con	npression/Maximum	1									
	Tension											
TOP CHORD	,	98/35, 2-3=-173/13	4,									
BOT CHORD	3-4=-199/64 4-5=-124/350											
WEBS	4-5=-124/350 2-5=-210/167, 2-4=-	351/101										
NOTES	2-3-210/107, 2-4-	331/131										
	CE 7-16; Vult=130mph	(3-second quet)										
	Bmph; TCDL=6.0psf; B		" Cat									
	Enclosed; MWFRS (er											11111
	Exterior(2E) 0-6-4 to 3-										IN TH UA	ROUL
5-0-13, Ex	kterior(2R) 5-0-13 to 9-	3-12 zone; cantileve	er							1.	R	in the
	ght exposed ; end verti									53	FES	Ning
	C-C for members and f		r						L			n
DOL=1.60	shown; Lumber DOL=	1.60 plate grip								<u>е</u> у	. ~	1 1 E
	) CE 7-16; Pr=20.0 psf (		1 15						=		SEA	∖L : =
	_=1.15); Pf=20.0 psf (L									:	0363	• -
	5); Is=1.0; Rough Cat E								1		0505	22 : :
Cs=1.00;		, , , ,							-			1 E
3) Unbalance	ed snow loads have be	een considered for t	his						S	2	N. E.	Rich
design.										A A A A A A A A A A A A A A A A A A A	A CA	EFRAN
	has been designed fo									11	C	BEIN
chord live	load nonconcurrent w	ith any other live loa	ads.								11, A. C	il- in

May 29,2025

A. GILLIN

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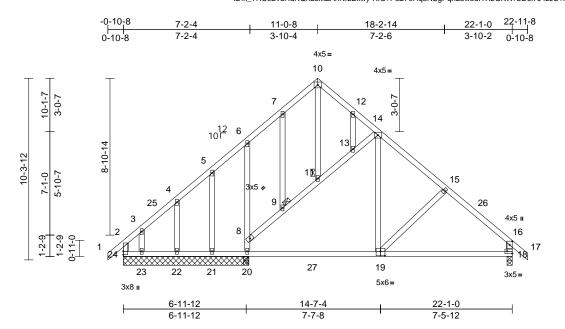


Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	D1	Common Structural Gable	1	1	I73807998 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:02 ID:k\_THUcEvuHdRQnLeMZAhNrzBxwy-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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818 Soundside Road Edenton, NC 27932



Scale = 1:65.4	= 1:65.4	Scale
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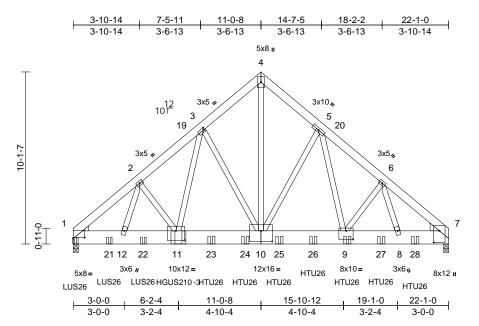
Plate Offsets (X, Y): [16:0-2-8,0-1-12], [18:Edge,0-1-8], [19:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.81	Vert(LL)		19-20	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.53	Vert(CT)	-0.18		>999	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.70	Horz(CT)	0.01	18	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 154 lb	FT = 20%
LUMBER			N	DTES					12) N/A				
TOP CHORD	2x4 SP No.2		1)	Unbalanced	roof live loads have	/e been	considered fo	r					
BOT CHORD	2x4 SP No.2			this design.									
NEBS	2x4 SP No.3		2)		7-16; Vult=130m								
OTHERS	2x4 SP No.3 *Except	ot* 20-6:2x4 SP No.2			ph; TCDL=6.0psf;								
BRACING					closed; MWFRS (								es not depict the size
TOP CHORD	Structural wood she	athing directly applie	ed or		ner(3E) -0-10-8 to							of the purlin along	g the top and/or
	5-8-8 oc purlins, ex				ner(3R) 8-0-8 to 1			1-0-8		tom cho			
BOT CHORD		applied or 10-0-0 or	>		Corner(3E) 19-11-				LOAD	CASE(S	) Sta	ndard	
	bracing.				t and right expose								
JOINTS	1 Brace at Jt(s): 11,				d;C-C for member			RS					
	9				shown; Lumber D	OL=1.6	) plate grip						
REACTIONS	(size) 18=0-3-8	, 20=7-1-8, 21=7-1-8		DOL=1.60									
		, 23=7-1-8, 24=7-1-8			ed for wind loads								
	Max Horiz 24=-257				ids exposed to wi								
	Max Uplift 18=-44 (L		4).		d Industry Gable I								
		(LC 14), 22=-54 (LC	1/1)		alified building de								
		(LC 14), 24=-51 (LC			7-16; Pr=20.0 ps								
	Max Grav 18=794 (				1.15); Pf=20.0 psf								
	,	C 36), 22=266 (LC 2			Is=1.0; Rough Ca	( B; Fully	Exp.; Ce=0.9	9;					
		LC 12), 24=511 (LC 1	27)	Cs=1.00; Ct		h	aldonad fr 4						
ORCES	,	npression/Maximum	5)		snow loads have	Deen CO	isidered for tr	115				WITH CA	1111
	Tension		C)	design.	e boon decigned	for groat	or of min roof	livo				WAH CA	Rolly
FOP CHORD	1-2=0/39, 2-3=-424/	/87. 3-4=-318/59.	6)		s been designed						1	R	- City
		259/43, 6-7=-238/67,			on-concurrent wit			sion		/	S	O'.FESS	100.10
	7-10=-208/119, 10-	, , ,	7)	0	e 2x4 MT20 unles					4		AP 1	the second
	12-14=-223/74, 14-	,	7) 8)							4		10 10	44
		-17=0/39, 2-24=-378			ully sheathed from					-	5 5		. : =
	16-18=-714/151, 8-9				spaced at 2-0-0 c		lagonal web)	•				SEA	L : =
	9-11=-510/130, 11-				spaced at 2-0-0 0 as been designed		) nef hottom			=	:	0363	22 : =
	13-14=-514/129	,	10		ad nonconcurrent			de		-	8	0303	: :
BOT CHORD	23-24=-72/286, 22-2	23=-72/286,	11		has been designe						2. 9	N	1 - S
	,	21=-72/286, 18-20=0	/590		n chord in all area			hai			5	·	air S
NEBS	10-11=-71/76, 7-9=-	-63/55, 8-20=-565/22	0,		by 2-00-00 wide w			m			2.5	SEA 0363	FERMAN
		=-105/93, 4-22=-169/			by 2-00-00 wide w						11	710	E. S.
	3-23=-102/184, 12-	13=-70/52,		choru anu al	iy other members	, which be	DE = 10.0psi	•				A G	ILBUIN
	14-19=-18/406, 15-	19=-151/165										Thun the	in the second se
													29,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	D2-3	Common Girder	1	3	I73807999 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:02 ID:6ry9W?khhNErlqqTdcOmP?zBxv0-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:67.8

	, , t , , ,	]; [ö:ö ö :⊇;ö : ö]; [:	, -	-1, [ ,-		-1,1	1						
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC202	21/TPI2014	<b>CSI</b> TC BC WB Matrix-MSH	0.28 0.26 0.93	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.09 -0.17 0.03	(loc) 10-11 10-11 7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 685 lb	<b>GRIP</b> 244/190 FT = 20%
	6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 1=0-3-8, 7 Max Horiz 1=-216 (L	t* 10-4:2x4 SP No.2 athing directly applied applied or 10-0-0 oc 7=0-3-8 C 36)	5) 6)	<ul> <li>this design.</li> <li>Wind: ASCE Vasd=103m; II; Exp B; En cantilever lef right exposed</li> <li>TCLL: ASCE Plate DOL=1 DOL=1.15); Cs=1.00; Ct= Unbalanced design.</li> </ul>	snow loads have l	oh (3-sed BCDL=6 envelope d ; end v 60 plate f (roof Ll (Lum DC B; Fully peen cor	cond gust) .0psf; h=25ft e) exterior zon vertical left ar grip DOL=1. .: Lum DOL= L=1.15 Plate Exp.; Ce=0. asidered for th	; Cat. ne; nd 60 1.15 9;	C	oncentra Vert: 11 21=-383	ted Lo =-4387 3 (B), 2 1698	7 (B), 9=-1698 (B)	), 13=-392 (B), 1698 (B), 24=-1698
FORCES TOP CHORD BOT CHORD WEBS	Max Grav       1=9999 (LC 21), 7=10837 (LC 22)       7)       This truss has been designed for a 10.0 psf bottom         (lb) - Maximum Compression/Maximum       7)       This truss has been designed for a 10.0 psf bottom         Tension       8)       * This truss has been designed for a live load of 20.0psf         1.2=-12437/0, 2-3=-13226/0, 3-4=-9538/0,       6)       * This truss has been designed for a live load of 20.0psf         4-5=-9537/0, 5-6=-12781/0, 6-7=-13258/0       3-06-00 tall by 2-00-00 wide will fit between the bottom								ROUTIN				
<ul> <li>NOTES</li> <li>1) 3-ply truss to be connected together with 10d (0.131*x3") nais as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 5 rows staggered at 0-4-0 oc. Web connected as follows: 2x4 - 1 row at 0-9-0 oc.</li> <li>2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.</li> <li>11) Use Simpson Strong-Tie HGUS210-3 (46-10d Girder, 16-10d Truss) or equivalent at 6-2-4 from the left end to connect truss(es) to back face of bottom chord.</li> <li>12) Use Simpson Strong-Tie HTU26 (20-10d Girder, 14-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 c max. starting at 8-1-8 from the left end to 20-1-8 to connect truss(es) to back face of bottom chord.</li> <li>13) Fill all nail holes where hanger is in contact with lumber.</li> <li>LOAD CASE(S) Standard</li> <li>14) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15, Uniform Loads (Ib/ft)</li> </ul>													

May 29,2025

Page: 1

ENGINEERING BY REENCO A MITCH Affilian

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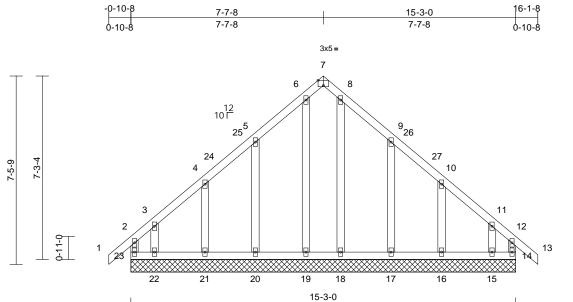
Plate Offsets (X, Y): [1:0-8-0,0-0-11], [3:0-0-12,0-1-8], [7:0-8-3,0-4-0], [9:0-5-0,0-6-0], [10:0-8-0,0-7-8], [11:0-6-0,0-6-12]

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	E1	Common Supported Gable	1	1	I73808000 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:02 ID:ZH7\_MfUVwJA?tJysE?REQ9zBxqB-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

16-1-8 0-10-8

Page: 1



Scale = 1:45.6

Plate Offsets (X, Y): [7:0-2-8,Edge]

		1	-				1						
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.14	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.08	Vert(CT)	n/a	-	n/a	999	-	
TCDL	10.0	Rep Stress Incr	YES		WB	0.13	Horz(CT)	0.00	14	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TP	912014	Matrix-MR								
BCDL	10.0		110202 1/11	12011								Weight: 101 lb	FT = 20%
LUMBER TOP CHORD	2x4 SP No.2	•	WEBS		6-19=-167/8, 8-18= 1-21=-157/98, 3-22	,		19,				en designed for rd in all areas wh	a live load of 20.0psf
BOT CHORD	2x4 SP No.2				9-17=-221/120, 10-								between the bottom
WEBS	2x4 SP No.3				1-15=-122/125		,					er members.	
OTHERS	2x4 SP No.3		NOTE	s					13) Pro	vide me	chanic	al connection (by	others) of truss to
BRACING					roof live loads have	been	considered fo	r	bea	aring plat	e capa	ble of withstandi	ng 117 lb uplift at joint
TOP CHORD	Structural wood she	eathing directly applied	,	s design.				•					ft at joint 20, 68 lb
	6-0-0 oc purlins, ex				7-16; Vult=130mpl	h (3-seo	cond aust)						22, 96 lb uplift at
BOT CHORD		applied or 6-0-0 oc	Ý Va II;	asd=103mp Exp B; End	oh; TCDL=6.0psf; E closed; MWFRS (e	3CDL=6 nvelope	6.0psf; h=25ft; e) exterior zor	ne	15.				66 lb uplift at joint
	(size) 14=15-3- 17=15-3- 20=15-3- 23=15-3- Max Horiz 23=-190 Max Uplift 14=-80 (I 16=-68 (I 20=-95 (I 22=-176 Max Grav 14=161 (I 16=189 (I 16=189 (I 16=189 (I 20=262 (I)))		-0, to -0, to 1-0, lef ex 5), re; 5), 3) Tr 12) on 3), se 2), 0 (1), 4) TC 1), Pli 1) DC	4-7-8, Exte 13-1-8, Ext t and right posed;C-C DL=1.60 uss design ly. For stu e Standarc consult qu CLL: ASCE ate DDL=1 DL=1.15); I	erior(2E) -0-10-8 to prior(2R) 4-7-8 to 1 terior(2E) 13-1-8 to exposed ; end vert for members and own; Lumber DOL= ed for wind loads in ds exposed to wind Industry Gable Er alified building des 7-16; Pr=20.0 psf 15); Pf=20.0 psf 15); Pf=20.0 psf 15); Pf=20.0 psf	0-7-8, I b 16-1-8 ical left forces a 1.60 pl n the pl d (norm nd Deta igner a (roof LI Lum DC	nterior (1) 10- b zone; cantile and right & MWFRS for ate grip ane of the tru: al to the face) ils as applicat s per ANSI/TF .: Lum DOL= <sup>2</sup>	-7-8 ever ss ), ble, PI 1. 1.15	LOAD	CASE(S)	) Star	Hodard	RO
FORCES		npression/Maximum		s=1.00; Ct=	snow loads have b	een cor	nsidered for th	nis		/	53	OFFESS	C. Jan
TOP CHORD		77/85, 5-6=-86/144, 71/116, 8-9=-86/144, =-72/82, 11-12=-158/1 :-124/58 22=-91/133, 20=-91/133, 18=-91/133,	de 6) Th loa 07, 7) All 8) Ga 9) Tr br 10) Ga 11) Th	esign. his truss ha ad of 12.0 p erhangs no l plates are able require uss to be fi aced again able studs his truss ha	s been designed for osf or 1.00 times fit on-concurrent with 2x4 MT20 unless es continuous bott ully sheathed from st lateral movemer spaced at 2-0-0 oc s been designed for d nonconcurrent w	or great at roof le other li otherwi om chor one fac one fac or a 10.1	er of min roof bad of 20.0 ps ve loads. se indicated. d bearing. se or securely liagonal web). 0 psf bottom	live sf on		Junnum.		SEA 0363	EER.K

May 29,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

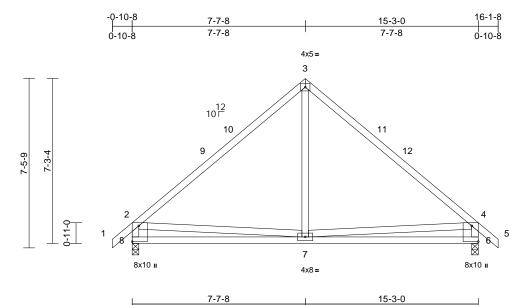


Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	E2	Common	1	1	I73808001 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:kPl8gQcPKiZRh0lzNp8pNTzBxq0-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

7-7-8

Page: 1



### Plate Offsets (X, Y): [6:Edge,0-3-8], [8:Edge,0-3-8]

Scale = 1:50.8

	∧, T). [0.⊏uge,0-3-8],	[8.Euge,0-3-8]											
Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.86	Vert(LL)	-0.06	6-7	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.52	Vert(CT)	-0.13	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.27	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021	I/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 89 lb	FT = 20%
LUMBER			4)	Unbalanced	snow loads have l	been cor	nsidered for t	his					
TOP CHORD	2x4 SP No.1		,	design.									
BOT CHORD	2x4 SP No.2		5)	This truss ha	is been designed f	for great	er of min roof	f live					
WEBS	2x4 SP No.3 *Except	t* 8-2,6-4:2x4 SP N	o.2		psf or 1.00 times f			sf on					
BRACING				0	on-concurrent with								
TOP CHORD	Structural wood shea	athing directly applie	ed or 6)	The Fabricat	ion Tolerance at jo	2 = 2	16%, joint 4 =	= 16%					
	5-4-10 oc purlins, ex	xcept end verticals.		<del>.</del>									
BOT CHORD	Rigid ceiling directly	applied or 9-7-8 oc	()		is been designed f ad nonconcurrent			do					
	bracing.		0)		has been designed								
	(size) 6=0-3-8, 8		0)		n chord in all area			opsi					
	Max Horiz 8=-190 (L	,			by 2-00-00 wide wi		0	om					
	Max Uplift 6=-61 (LC	,, , , ,			y other members.								
	Max Grav 6=729 (LC	C 22), 8=729 (LC 21	) 9)		Simpson Strong-Ti		ctors						
FORCES	(lb) - Maximum Com	pression/Maximum		recommende	ed to connect truss	s to bear	ing walls due	to					
	Tension			UPLIFT at jt	s) 8 and 6. This co	onnectio	n is for uplift	only					
TOP CHORD	1-2=0/39, 2-3=-690/	, , ,		and does no	t consider lateral f	orces.							
	4-5=0/39, 2-8=-665/	,	LC	AD CASE(S)	Standard								
BOT CHORD	7-8=-368/617, 6-7=-2												
WEBS	3-7=0/333, 2-7=-238	3/403, 4-7=-249/409											
NOTES													
,	ed roof live loads have	been considered fo	r									TH CA	1111
this design		( <b>A</b> )										"TH CA	Rolly
	CE 7-16; Vult=130mph		Cat								5	R	. Alala
vasu=103	mph; TCDL=6.0psf; B0	ວບL=0.0psi; n=25π;	, Udl.									U . FFS	D' L'HII

- 2) Wind: ASCE 7-16; Vull=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 4-7-8, Exterior(2E) 13-1-8 to 10-7-8, Interior (1) 10-7-8 to 13-1-8, Exterior(2E) 13-1-8 to 16-1-8 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

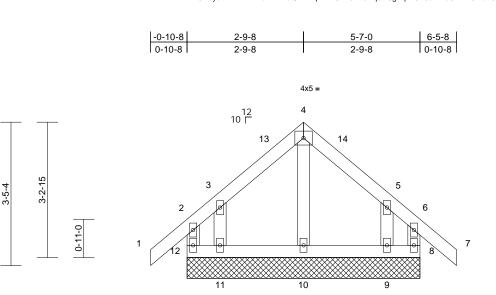
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	G1	Common Supported Gable	1	1	I73808002 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:OXbyuzNwV2VVIhc4ZYmbQmzBxp1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



5-7-0

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Scale = 1:27.6	6												
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL LUMBER TOP CHORD		Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021 3)	only. For stu	CSI TC BC WB Matrix-MR ed for wind loads ids exposed to wi	ind (norm	al to the face	e),	(loc) - - 8	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 32 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD	2x4 SP No.3 2x4 SP No.3 Structural wood she 5-7-0 oc purlins, ex		5)	or consult qu TCLL: ASCE Plate DOL=1 DOL=1.15); I Cs=1.00; Ct= Unbalanced design.	snow loads have	esigner as sf (roof Ll (Lum DC at B; Fully been cor	s per ANSI/TI .: Lum DOL= DL=1.15 Plate Exp.; Ce=0.9	PI 1. 1.15 9; his					
REACTIONS	5 (size) 8=5-7-0, 1 11=5-7-0, Max Horiz 12=-96 (L Max Uplift 8=-55 (LC 11=-84 (L Max Grav 8=151 (LC	C 11), 9=-81 (LC 15), .C 14), 12=-67 (LC 10 C 22), 9=148 (LC 31), .C 22), 11=155 (LC 29	5), 10	load of 12.0 p overhangs no All plates are Gable require Truss to be f braced again ) Gable studs	s been designed osf or 1.00 times on-concurrent wite e 2x4 MT20 unles es continuous bol ully sheathed fror ist lateral movem spaced at 2-0-0 c s been designed	flat roof le h other li s otherwi ttom chor m one fac ent (i.e. c oc.	bad of 20.0 p ve loads. se indicated. rd bearing. ce or securely liagonal web)	sf on					
FORCES	3-4=-57/153, 4-5=-5 6-7=0/65, 6-8=-141/	- =0/65, 2-3=-50/61, 7/153, 5-6=-39/51, 135	12	chord live loa * This truss h on the bottor 3-06-00 tall b	ad nonconcurrent has been designe n chord in all area by 2-00-00 wide w hy other members	with any d for a liv as where vill fit betw	other live loa e load of 20.0 a rectangle	0psf					99977
BOT CHORD WEBS NOTES	8-9=-49/107	11=-49/107, 9-10=-49/ 154/136, 5-9=-144/14	10	bearing plate	hanical connectio capable of withs ft at joint 8, 84 lb 9.	tanding 6	67 lb uplift at j	joint			AN AN	ORTH CA	ROUN
this desig 2) Wind: AS Vasd=10 II; Exp B; and C-C to 3-5-8, and right C for mer	ced roof live loads have gn. GCE 7-16; Vult=130mph 3mph; TCDL=6.0psf; B Enclosed; MWFRS (er Corner(3E) -0-10-8 to 2 Corner(3E) 3-5-8 to 6-5 exposed ; end vertical mbers and forces & MW umber DOL=1.60 plate	(3-second gust) CDL=6.0psf; h=25ft; C nvelope) exterior zone 2-1-8, Corner(3R) 2-1- 5-8 zone; cantilever lef left and right exposed VFRS for reactions	Cat. 8 ft	DAD CASE(S)	Standard					<b>5</b>			22 ERA

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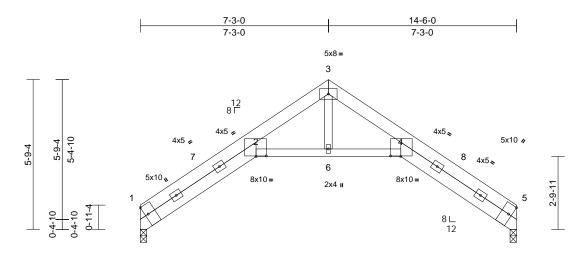
818 Soundside Road Edenton, NC 27932

May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	H1	Roof Special	4	1	I73808003 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:EwOhOnUS\_lpLwts2pdY2DIzByXN-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



0-3-0	4-5-8	7-3-0	ر 10-0-7 I	14-3-0	14-6-0
0-3-0	4-2-8	2-9-8	2-9-7	4-2-9	0-3-0

#### Scale = 1:44.4 Plate Offsets (X, Y): [1:Edge,0-1-7], [2:0-4-12,0-0-4], [4:0-4-12,0-0-4], [5:Edge,0-1-7]

Loading TCLL (roof) Snow (Pf)	(psf) 20.0 20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15		CSI TC BC	0.64 0.71	DEFL Vert(LL) Vert(CT)	in -0.25 -0.47	(loc) 2-6 2-6	l/defl >686 >365	L/d 240 180	PLATES MT20	<b>GRIP</b> 244/190
TCDL	10.0	Rep Stress Incr	YES		WB	0.24	Horz(CT)	0.48	20	>000 n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 80 lb	FT = 20%
LUMBER			4)	Unbalanced design.	snow loads have	been co	nsidered for t	his					
BOT CHORD	2x6 SP 2400F 2.0E 2x6 SP No.2 *Excep 2.0E	t* 2-4:2x4 SP 2400F	= 5)	This truss ha	s been designed t			ads					
WEBS	2x4 SP No.3		6)		as been designed								
BRACING					n chord in all area								
TOP CHORD	Structural wood she	athing directly applie	ed or		y 2-00-00 wide w y other members		ween the bott	tom					
BOT CHORD	6-0-0 oc purlins. Rigid ceiling directly bracing.	applied or 10-0-0 of	c 7)	Bearing at jo using ANSI/1	nt(s) 1, 5 conside PI 1 angle to grai	rs parall n formul	a. Building	llue					
	(size) 1=0-3-0, 5 Max Horiz 1=117 (LC Max Uplift 1=-40 (LC Max Grav 1=656 (LC	C 11) C 14), 5=-40 (LC 15)		One H2.5A S recommende UPLIFT at jt(	uld verify capacity simpson Strong-Ti d to connect truss s) 1 and 5. This co	e conne to bear onnectio	ctors ing walls due						
FORCES	(lb) - Maximum Com	<i>.</i>	,	and does not DAD CASE(S)	consider lateral f	orces.							
	Tension	1100/110 0 1 110		5/12 6/102(0)	olandara								
TOP CHORD	1-2=-310/130, 2-3=- 4-5=-308/84	1129/112, 3-4=-112	9/112,										
BOT CHORD	2-6=0/1087, 4-6=0/1	087											
WEBS	3-6=-16/595												
NOTES												ORTH CA	11111
<ol> <li>Unbalance this design</li> </ol>	ed roof live loads have	been considered for	r									WTH CA	Roille
0	E 7-16; Vult=130mph	(3-second gust)									3	A	D. Inil
	mph; TCDL=6.0psf; B		Cat.							1	SA	FESC	Mini
	Enclosed; MWFRS (er									4			ng?
	xterior(2E) 0-2-0 to 3- erior(2R) 4-3-0 to 10-0									3			1 I E
	terior(2E) 11-4-0 to 14									=	:	SEA	• -
	exposed ; end vertical l									Ξ		0363	22 : 3
	bers and forces & MW									-	3	<b>N</b> 1997	1 2
snown; Lur	mber DOL=1.60 plate	grip DOL=1.60									5	·	air E
3) TCLL: ASC	CE 7-16; Pr=20.0 psf (	roof LL: Lum DOL=1	1.15									S. GIN	EFF
Plate DOL:	=1.15); Pf=20.0 psf (L	um DOL=1.15 Plate									1	CA C	II BEIN
	); Is=1.0; Rough Cat E	3; Fully Exp.; Ce=0.9	);									A. C	
Cs=1.00; C	JI=1.1U												v 29,2025



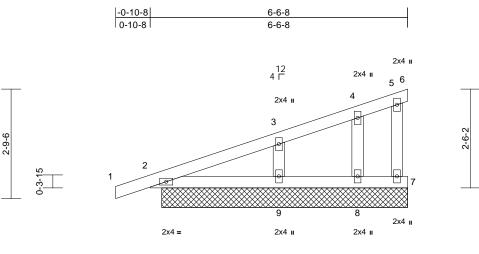
May 29,2025

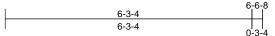
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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J1	Monopitch Supported Gable	2	1	I73808004 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:h821wAf\_rQW5dNRizaCDOqzBxof-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:29.3

Scale = 1.29.3													
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	21/TPI2014	CSI TC BC WB Matrix-MP	0.12 0.08 0.06	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 6	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 28 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 Structural wood she 6-0-0 oc purlins, ex Rigid ceiling directly bracing. (size) 2=6-3-0, ( 9=6-3-0 Max Horiz 2=92 (LC Max Uplift 2=-46 (LC (LC 14), 8 14) Max Grav 2=235 (LC	cept end verticals. <sup>2</sup> applied or 10-0-0 o 6=6-3-0, 7=6-3-0, 8= 11) C 10), 6=-4 (LC 11), 3=-25 (LC 10), 9=-45	c 6; =6-3-0, 7; 7=-7 9 (LC , 7=53 <sup>9</sup> ;	<ul> <li>Plate DOL= DOL=1.15); Cs=1.00; Ct</li> <li>Unbalanced design.</li> <li>This truss ha load of 12.0 overhangs r</li> <li>Gable studs</li> <li>This truss ha chord live lo</li> <li>* This truss on the botto 3-06-00 tall chord and a</li> <li>Provide mec bearing plate 7 lb uplift at</li> </ul>	E 7-16; Pr=20.0 p 1.15); Pf=20.0 ps Is=1.0; Rough C =1.10 snow loads have as been designed psf or 1.00 times ion-concurrent w spaced at 2-0-0 as been designed ad nonconcurrent has been designed ad nonconcurrent has been designed ad nonconcurrent has been designed ad nonconcurrent has been designed in chord in all are by 2-00-00 wide ny other member chanical connecti e capable of with joint 7, 46 lb uplii uplift at joint 8 a	of (Lum DC at B; Fully e been cor d for greats flat roof li th other lin oc. d for a 10. t with any ed for a liv ass where will fit betw 's. on (by oth standing 4 ft at joint 2	DL=1.15 Plate Exp.; Ce=0.1 sidered for t er of min roof bad of 20.0 p ve loads. D psf bottom other live loa e load of 20.2 e load of 20.2 a rectangle veen the bott ers) of truss i l b uplift at jo , 49 lb uplift at	e 9; flive sfon dds. 0psf om to int 6, at				Trogn. 2018	
FORCES	(lb) - Maximum Com Tension 1-2=0/17, 2-3=-115/			0) Non Standa OAD CASE(S)	rd bearing condit Standard	ion. Revie	ew required.						
BOT CHORD WEBS NOTES 1) Wind: ASC Vasd=103 II; Exp B; and C-CC to 6-6-8 zc vertical lef forces & M DOL=1.6C 2) Truss des only. For see Stand	4-5=-35/47, 5-6=-3/4	4, 5-7=-42/29 (7/49, 7-8=-27/49 (128/127 (128/127 (128/127 (128/127) (128/12	ne -1-8 ss ), ble,							10 . 111111.	X	SEA 0363	EEP. A

Truss designed for wind loads in the plane of the truss 2) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

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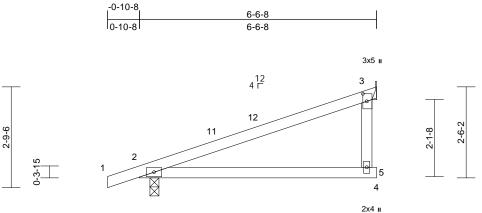


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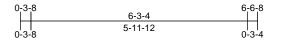
May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J2	Monopitch	7	1	I73808005 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:O3fp1bnFUVnhqwCdZhNZowzBxoV-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



3x5 =



Scale = 1:31.8

Plate Offsets (X, Y): [3:0-2-8,0-1-4]

	.]						
Loading         (psi           TCLL (roof)         20.           Snow (Pf)         20.           TCDL         10.           BCLL         0.           BCDL         10.	Plate Grip DOL     1.1       Lumber DOL     1.1       Rep Stress Incr     YE       Code     IR	5 5	CSI           TC         0.78           BC         0.55           WB         0.00           Matrix-MP	DEFL         in           Vert(LL)         -0.09           Vert(CT)         -0.17           Horz(CT)         0.01	5-10 >	/defl L/d 809 240 453 180 n/a n/a	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD         6-0-0 cc purlins, Rigid ceiling dire bracing.           REACTIONS         (size)         2=0-3 Max Horiz         2=90 Max Uplift           Max Grav         2=417	(LC 10), 3=-39 (LC 14) (LC 21), 3=324 (LC 21) Compression/Maximum 11/72, 3-5=0/121 =0/0 nph (3-second gust) f; BCDL=6.0psf; h=25ft; Cat. (envelope) exterior zone to 2-1-8, Interior (1) 2-1-8 to 6-3-4 zone; cantilever left cal left and right exposed;C- MWFRS for reactions ate grip DOL=1.60 psf (roof LL: Lum DOL=1.15 if (Lum DOL=1.15 Plate at B; Fully Exp.; Ce=0.9; e been considered for this d for greater of min roof live a flat roof load of 20.0 psf on	<ul> <li>chord live loa</li> <li>* This truss f on the bottor 3-06-00 tall b chord and ar</li> <li>7) Refer to gird</li> <li>8) Provide mec bearing plate 3.</li> <li>9) One H2.5A S recommende UPLIFT at jt( does not cor</li> <li>10) Gap between</li> </ul>	as been designed for a 10. as been designed for a liv mas been designed for a liv m chord in all areas where by 2-00-00 wide will fit betw ny other members. ler(s) for truss to truss com- chanical connection (by oth e capable of withstanding 3 Simpson Strong-Tie conne ed to connect truss to bear (s) 2. This connection is for nsider lateral forces. n inside of top chord bearin vertical web shall not exceet Standard	other live loads. e load of 20.0psf a rectangle veen the bottom nections. ers) of truss to 39 lb uplift at joint ctors ing walls due to r uplift only and ng and first			 EER A U

May 29,2025

Page: 1

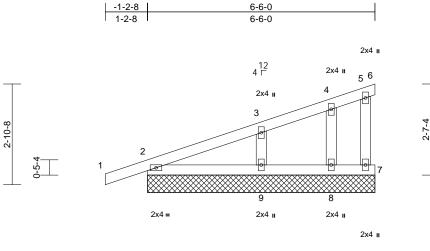


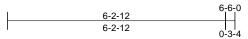
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J3		1	1	I73808006 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:doiCwguuMGvPPIOLb41ggqzBxoM-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:32.9

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14		n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	n/a	-	n/a	999	-	
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014		0.01	1.0.2(0.1)	0.00	Ũ		, a		
BCDL	10.0										Weight: 29 lb	FT = 20%
LUMBER				SCE 7-16; Pr=20.0								
TOP CHORD				DL=1.15); Pf=20.0 p								
BOT CHORD				15); Is=1.0; Rough (	Cat B; Fully	Exp.; Ce=0.9	9;					
WEBS	2x4 SP No.3			); Ct=1.10								
OTHERS	2x4 SP No.3		,	iced snow loads hav	ve been co	nsidered for ti	าเร					
BRACING			design. 5) This tru	ss has been designe	od for aroot	or of min roof	livo					
TOP CHORD				2.0 psf or 1.00 time								
	6-0-0 oc purlins, ex		overbor	gs non-concurrent v			51 011					
BOT CHORD	<ol> <li>Rigid ceiling directly bracing.</li> </ol>	applied or 10-0-0 o		equires continuous l								
DEACTIONO	5		7) Gable s	uds spaced at 2-0-0		0						
REACTIONS	(size) 2=6-6-0, 9=6-6-0	6=6-6-0, 7=6-6-0, 8=	=6-6-0, 8) This tru	ss has been designe	ed for a 10.	0 psf bottom						
	Max Horiz 2=93 (LC	13)		e load nonconcurre								
	Max Uplift 2=-54 (LC			uss has been desig			)psf					
		3=-22 (LC 10), 9=-5	1/IC On the L	ottom chord in all a								
	14)	(,	3-06-00	tall by 2-00-00 wide		veen the botto	om					
	Max Grav 2=227 (L	C 21), 6=12 (LC 21)		nd any other member mechanical connect		ore) of truce t	•					
		3=125 (LC 21), 9=33		plate capable of wit								
	21)			plift at joint 6, 7 lb u								
FORCES	(lb) - Maximum Con Tension	npression/Maximum	joint 9, 2	2 lb uplift at joint 8								
TOP CHORD	0 1-2=0/22, 2-3=-136/		LOAD CAS	E(S) Standard							OR. EES	and the second
	4-5=-35/47, 5-6=-3/											11111
BOT CHORD											IN THUA	ROUL
WEBS	3-9=-249/220, 4-8=-	114/114								N	A	D. Inter
NOTES		(a							/	シン	FESC	PN
	SCE 7-16; Vult=130mph		0-4						4	0		Neg-
	3mph; TCDL=6.0psf; B ; Enclosed; MWFRS (ei									2 13	. *	
	Corner(3E) -1-2-8 to 1-								=		SEA	
	zone; cantilever left and	, , ,							=	:	0363	• -
	eft and right exposed;C								1		0363	22 ; :
	MWFRS for reactions s									8		1 S S
DOL=1.6	0 plate grip DOL=1.60									5	·	airs
	signed for wind loads ir									25	S GIN	EFICAN
	r studs exposed to wind									11	10	BEN
	dard Industry Gable En										0363	ALLUN
or consul	It qualified building desi	gner as per ANSI/TI	PI I.								in the second	IIII.

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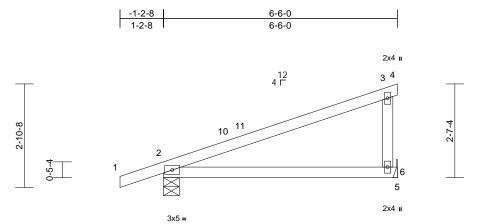
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May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J4		3	1	I73808007 Job Reference (optional)

#### Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:\_mV5zNy1BoYhV3HJNddrNtzBxoH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





#### Scale = 1:32

00010 = 1102												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.11	6-9	>694	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.59	Vert(CT)	-0.19	6-9	>386	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI201	Matrix-MP								
BCDL	10.0										Weight: 25 lb	FT = 20%
UMBER				ss has been designe								
	2x4 SP No.2			e load nonconcurrer								
	2x4 SP No.2			uss has been design			)pst					
	2x4 SP No.3			ottom chord in all are tall by 2-00-00 wide		•						
BRACING			obord o	nd any other member		veen the bollo						
	Structural wood she 2-7-8 oc purlins, ex	athing directly appli	ea or	girder(s) for truss to		nections.						
		applied or 10-0-0 o	Ó Dura dela	mechanical connecti			0					
	bracing.		bearing	plate capable of with	nstanding 4	6 lb uplift at jo	oint					
REACTIONS (s	0	6= Mechanical	6.									
· · ·	lax Horiz 2=96 (LC			.5A Simpson Strong-								
		C 10), 6=-46 (LC 14)		ended to connect tru at it(s) 2. This conne		0						
		C 21), 6=363 (LC 21	UFLIFI	t consider lateral for		upint only an	iu					
	(lb) - Maximum Con Tension	npression/Maximum		E(S) Standard								
TOP CHORD	1-2=0/23, 2-3=-136/ 3-6=-265/152	75, 3-4=-8/0,										
BOT CHORD	2-6=-35/128, 5-6=0/	0										
OTES												
1) Wind: ASCE	7-16; Vult=130mph	(3-second gust)										
		CDL=6.0psf; h=25ft										
		nvelope) exterior zor										1111
		-9-8, Interior (1) 1-9									WHILL CA	ADOUL
		0 zone; cantilever le left and right expose								1	alli	and the
	ers and forces & MV		50,0-							S	0'.EE8	Con Vin
	ber DOL=1.60 plate								6	27		This Mill
									1		21	104.01
		roof LL: Lum DOL=							-		SEA	u : =
		um DOL=1.15 Plate							=		JL/	• -
		3; Fully Exp.; Ce=0.9	9;						=		0363	322 <u>:</u> E
Cs=1.00; Ct=		een considered for th	hie							8		1 5
design.	SHOW IDAUS HAVE DE		110							2	·	a !
	as been designed fo	r greater of min roof	live							3.5	NGIN	FERIAS
		t roof load of 20.0 p								14	710	THE ALL N
overhangs n	on-concurrent with	other live loads.									SEA 0363	ALPIN
											111111	mm
											Ma	v 20 2025

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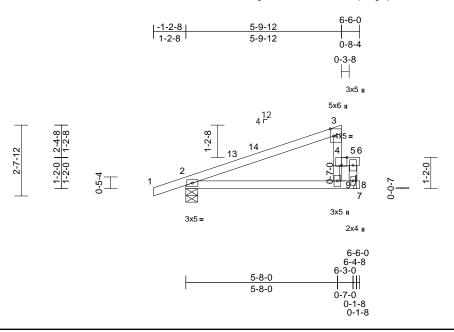
818 Soundside Road Edenton, NC 27932

May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J5		5	1	I73808008 Job Reference (optional)

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:ao\_XIE98tD0euHLKgwv3tmzBxmk-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:43.1

Plate Offsets (X, Y): [3:0-3-3,Edge], [4:0-2-8,Edge]

Plate Offsets (A	K, Y): [3:0-3-3,Edge],	[4:0-2-8,Edge]										-	
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2021	/TPI2014	<b>CSI</b> TC BC WB Matrix-MR	0.90 0.34 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.03 -0.05 0.01	(loc) 9-12 9-12 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 25 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD NOTES 1) Unbalanced this design. 2) Wind: ASC Vasd=103r II; Exp B; E and C-C Ex 2-8-0, Exte and right ex C for memb	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she 5-11-12 oc purlins, ( 2-0-0 oc purlins (6-0 Rigid ceiling directly bracing. (size) 2=0-5-4, 8 Max Horiz 2=109 (LC Max Grav 2=459 (LC (Ib) - Maximum Com Tension 1-2=0/32, 2-3=-386/ 3-4=-519/33, 4-5=- 5-8=-555/267 2-9=-153/368, 8-9=-	applied or 10-0-0 oc 3= Mechanical C 14) C 10), 8=-98 (LC 14) C 38), 8=617 (LC 38) apression/Maximum 151, 4-9=-58/133, 186/119, 5-6=0/0, 86/204, 7-8=0/0 been considered for (3-second gust) CDL=6.0psf; h=25ft; C avelope) exterior zone -9-8, Interior (1) 1-9-8 0 zone; cantilever left left and right exposed /FRS for reactions	5) f or 6) and 7) 8) 9) 10) 11) 12) 13) Cat. to 14) ;C-	design. This truss ha load of 12.0 U overhangs nu Provide adec This truss ha chord live loa * This truss ha chord live loa * This truss ha chord live loa * This truss ha chord and ar Refer to gird, Provide mect bearing plate 8. One H2.5A S recommende UPLIFT at jt( does not con Graphical pu or the orienta bottom chorc Hanger(s) or provided suff Ib down and Ib up at 6-6- such connec In the LOAD of the truss a	other connection d icient to support co 201 lb up at 5-8-0, 0 on top chord. Th tion device(s) is the CASE(S) section, I re noted as front (F Standard ww (balanced): Lum	r great at roof I revent to r a 10.1 ith any for a liv where fit betv ss conne to bear on is foi does no ong the levice(s e desig e respoi oads a r) or ba	er of min roof pad of 20.0 pr (ve loads. water ponding psf bottom other live loa e load of 20.0.1 a rectangle veen the bottom ections. ers) of truss t 8 lb uplift at j ctors ing walls due top and/or b depict the s top and/or ) shall be ated load(s) 3 8 lb down and n/selection of msibility of oth pplied to the f ck (B).	f live sf on g. dds. Opsf om to joint to size size d68 d 32 f f hers. face		4		SEA 0363	
Plate DOL=	CE 7-16; Pr=20.0 psf ( =1.15); Pf=20.0 psf (L ;; Is=1.0; Rough Cat B Ct=1.10	um DOL=1.15 Plate	15	Uniform Loa Vert: 1-3 Concentrate		-60, 7-	10=-20					A. C	EER. KINN

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (lb/ft)
  - Vert: 1-3=-60, 4-5=-60, 5-6=-60, 7-10=-20 Concentrated Loads (lb)

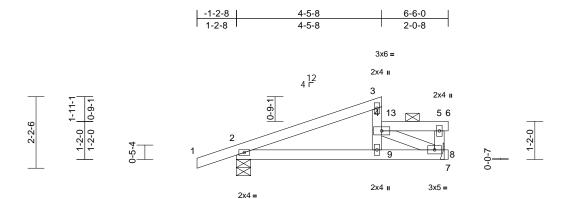


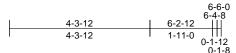
munn May 29,2025

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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J6-2	Half Hip Girder	1	2	I73808009 Job Reference (optional)

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:03 ID:kYbZHRnFmuMsQdR2u\_IyAWzBxjL-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





Scale = 1:35.4

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		тс	0.60	Vert(LL)	-0.02	9-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.24	Vert(CT)	-0.04	9-12	>999	180		
TCDL	10.0	Rep Stress Incr	NO		WB	0.14	Horz(CT)	0.00	8	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-MP								
BCDL	10.0											Weight: 54 lb	FT = 20%
LUMBER			4)		7-16; Vult=130m				1) D	ead + Sr	now (ba	alanced): Lumbe	r Increase=1.15, Pla
TOP CHORD					ph; TCDL=6.0psf					crease=			
BOT CHORD					closed; MWFRS				U	niform Lo			
WEBS	2x4 SP No.3 *Excep	ot* 3-9:2x4 SP No.2			ft and right expos							4-13=-60, 5-13=	-680 (F=-620),
BRACING				right expose	d; Lumber DOL=	1.60 plate	grip DOL=1.	60	0			620), 7-10=-20	
TOP CHORD					7-16; Pr=20.0 p	of /roof		4 4 5	C	oncentra		· · /	
		cept end verticals, ar	id <sup>5)</sup>		1.15); Pf=20.0 p					Vert: 3=	-300 (	F)	
	2-0-0 oc purlins: 4-9				Is=1.0; Rough Ca								
BOT CHORD	0 0 ,	applied or 10-0-0 oc		Cs=1.00; Ct		at D, T uny	Lxp., 00=0.	σ,					
	bracing.		6)		snow loads have	been co	nsidered for t	his					
REACTIONS		B= Mechanical	0)	design.	onen leade hare								
	Max Horiz 2=75 (LC		7)	This truss ha	as been designed	I for great	er of min root	live					
	Max Uplift 2=-111 (L		, N	load of 12.0	psf or 1.00 times	flat roof l	oad of 20.0 p	sf on					
	Max Grav 2=614 (L0	<i>,</i>	)		on-concurrent wi								
FORCES	(lb) - Maximum Com	pression/Maximum	8)		quate drainage to			g.					
	Tension	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	9)		as been designed								
TOP CHORD	,				ad nonconcurrent								
	3-4=-202/64, 4-5=-1 5-8=-901/147	1/8, 5-6=0/0,	10		has been designe			Opsf					
BOT CHORD		183/1300 7-8-0/0			n chord in all are								
WEBS	4-8=-1429/210	103/1300, 7-0=0/0			by 2-00-00 wide way other members		veen the bott	om					
	4-0=-1423/210		11		er(s) for truss to		octions						
NOTES		the model of the second			hanical connection			'n				, mmm	11111
	s to be connected toge ") nails as follows:	ther with 10d	12		e capable of with							TH CA	Rall
	s connected as follows.	$2 \times 1 = 1 \text{ row at } 0 = 1 = 0$	h	8.		stantaning 1		. joint			1	A	CAL MAN
oc.		5. 2.4 - 1 10w at 0-4-0	, 13	) One H2.5A \$	Simpson Strong-T	Tie conne	ctors				1.	O' FESC	Idin A
	nords connected as foll	ows: 2x4 - 1 row at			ed to connect trus			to		6	25	in a	Visi
0-9-0 oc.				UPLIFT at jt	(s) 2. This conne	ction is fo	r uplift only a	nd		~			
	nected as follows: 2x4 -	1 row at 0-9-0 oc.		does not cor	sider lateral force	es.				-		054	1 1 3
2) All loads a	are considered equally	applied to all plies,	14		Irlin representatio			size				SEA	L
except if r	noted as front (F) or ba	ck (B) face in the LO	٩D		ation of the purlin	along the	e top and/or					0363	22 : =
CASE(S)	section. Ply to ply conr	nections have been		bottom chore						-			: :
	to distribute only loads	noted as (F) or (B),	15		other connection						2	N	1 2
	nerwise indicated.				ficient to support			34			2.	N. En	-cRik S
	ed roof live loads have	been considered for			48 lb up at 4-3-1						31	S. GIN	EFRAN
this desig	n.			responsibility	tion of such conr	lection de	vice(s) is the			1111111111111	1	C .	BEIN
												11, A. C	all

LOAD CASE(S) Standard

May 29,2025

Page: 1

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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	J7	Half Hip	3	1	I73808010 Job Reference (optional)

4-5-8

-1-2-8 1-2-8

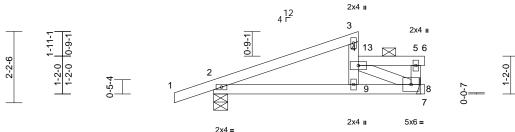
Carter Components (Sanford, NC), Sanford, NC - 27332,

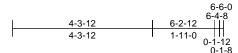
Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:O1XWL1Z4wM?sANIUZ4yfAkzBxh1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

6-6-0



4-5-8 2-0-8 3x6 =





Scale = 1:35.5

Scale = 1:35.5														
Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.99	Vert(LL)	0.05	9-12	>999	240	MT20	244/190	
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.36	Vert(CT)	-0.06	9-12	>999	180			
TCDL	10.0	Rep Stress Incr	NO		WB	0.25	Horz(CT)	0.01	8	n/a	n/a			
BCLL	0.0*	Code	IRC20	21/TPI2014	Matrix-MP									
BCDL	10.0		-									Weight: 27 lb	FT = 20%	
LUMBER			5	) This truss ha	as been designed	d for areat	er of min roof	flive						
TOP CHORD	2x4 SP No.2				psf or 1.00 times									
BOT CHORD				overhangs n	on-concurrent w	ith other liv	/e loads.							
WEBS	2x4 SP No.3 *Excep	ot* 3-9:2x4 SP No.2	6	6) Provide adequate drainage to prevent water ponding.										
BRACING			7		as been designe									
TOP CHORD	Structural wood she	athing directly applie	d or		ad nonconcurrer									
		xcept end verticals, a			has been design			0psf						
	2-0-0 oc purlins: 4-9	, 4-6.			m chord in all are									
BOT CHORD	Rigid ceiling directly	applied or 7-2-4 oc			by 2-00-00 wide		een the bott	om						
	bracing.				ny other member ler(s) for truss to		antiona							
REACTIONS	(size) 2=0-5-4, 8	8= Mechanical			chanical connecti			to						
	Max Horiz 2=75 (LC	14)	1		e capable of with									
	Max Uplift 2=-103 (L	.C 10), 8=-106 (LC 1	1)	8.	e capable of with	istanung i		t joint						
	Max Grav 2=560 (L0	C 38), 8=736 (LC 37)	1		Simpson Strong-	Tie conne	ctors							
FORCES	(lb) - Maximum Com	pression/Maximum			ed to connect tru			to						
	Tension				(s) 2. This conne									
TOP CHORD	1-2=0/32, 2-3=-757/	376, 4-9=0/101,			sider lateral for									
	3-4=-415/258, 4-5=-	11/16, 5-6=0/0,	1	2) Graphical pu	urlin representati	on does no	ot depict the	size						
	5-8=-278/156			or the orient	ation of the purlin	n along the	top and/or							
BOT CHORD		663/1272, 7-8=0/0		bottom chor										
WEBS	4-8=-1389/747		1		r other connectio									
NOTES					ficient to support								11	
1) Unbalance	ed roof live loads have	been considered for			268 lb up at 4-3							111110	10/11	
this desig					tion of such con	nection de	vice(s) is the				1	N'TH UT	NOIL	
	CE 7-16; Vult=130mph			responsibilit							5	A SECO	12. 11.11	
	3mph; TCDL=6.0psf; B		out.		CASE(S) sectio			face		/	52	S FEU	PN: Si	
	Enclosed; MWFRS (er				are noted as fron	IL (F) OF DA	СК (D).			4			n	
	Exterior(2E) -1-2-8 to 6			OAD CASE(S)				Dista			<u>е</u> у	. ~	N 1 E	
	exposed ; end vertical		d;C- 1		ow (balanced): L	umber Inc	rease=1.15, 1	Plate		Ξ		SEA	AL E	
	nbers and forces & MW			Increase=1						=			• -	
snown; Lt	umber DOL=1.60 plate	grip DOL=1.60		Uniform Lo	ads (ID/IT) =-60, 4-13=-60,	5 12- 100	(E_ 120)			1		0363	22 : 3	
3) TCLL-AS	CE 7-16; Pr=20.0 psf (		15		=-60, 4-13=-60, ) (F=-120), 7-10=		(r = 120),			-	2		1 2	
	L=1.15); Pf=20.0 psf (L		.15		ed Loads (lb)	20					-	·	A 1. 3	
	E = 1.13, $1 = 20.0$ psi (E										- 1	N. ENOW	FEN. X N	

- 3) TOLL: ASCE 7-16; PT=20.0 psi (tool LL: Lum DOL=1.15 Plate DOL=1.15); IS=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

ENGINEERING BY

A. GILD

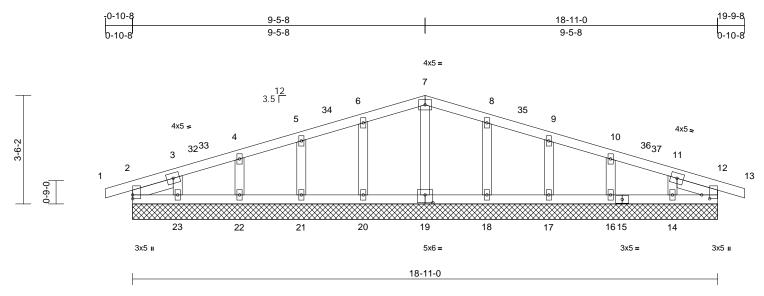
May 29,2025

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Vert: 3=-400 (F)

Job		Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-0	01	K1	Common Supported Gable	1	1	I73808011 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:?40x7umBcnTpYbpWsMEthczBxfU-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:37.3

#### Plate Offsets (X, Y): [2:0-1-8,0-0-1], [12:0-1-8,0-3-1], [19:0-3-0,0-3-0]

Plate Offsets (	х, т): [2:0-1-8,0	)-0-1],	[12:0-1-8,0-3-1], [19	:0-3-0,0-3	3-0]										
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	20 20 10	osf) 0.0 0.0 0.0 0.0* 0.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 YES IRC202	21/TPI2014	CSI TC BC WB Matrix-MSH	0.07 0.03 0.04	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - 12	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 87 lb	<b>GRIP</b> 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS SLIDER BRACING TOP CHORD	No.3 1-4-15		-4-15, Right 2x4 SP athing directly applie	v N	OT CHORD /EBS OTES	20-21=-6/39, 18-2 16-17=-6/39, 14-1 7-19=-102/19, 6-2 4-22=-169/56, 3-2 9-17=-166/53, 10-	0=-6/39, 6=-6/39, 0=-182/8 3=-101/6 16=-169	17-18=-6/39, 12-14=-6/39 31, 5-21=-166, 34, 8-18=-182, /56, 11-14=-1	/53, /81, 01/64	<ul> <li>11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.</li> </ul>					
BOT CHORD	6-0-0 oc purlin	IS.	applied or 6-0-0 oc		<ol> <li>Unbalanced roof live loads have been considered for this design.</li> <li>Wind: ASCE 7-16; Vult=130mph (3-second gust)</li> <li>Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.</li> </ol>										
	(size) 2=11 14= 17= 19= 21= 23= Max Horiz 2=4 Max Uplift 2=-2 14= 17= 20= 20= 20= 20= 20= 14= 17= 14= 17= 19= 21=	18-11- 18-11- 18-11- 18-11- 18-11- 6 (LC 20 (LC -35 (LI -30 (LI -30 (LI -31 (LI 12 (LC 131 (L 204 (L 204 (L 204 (L		1), 3 5), 4), 4) 4) 4 22), 22), 22), 1), 5	<ul> <li>II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 6-5-8, Exterior(2R) 6-5-8 to 12-5-8, Interior (1) 12-5-8 to 16-9-8, Exterior(2E) 16-9-8 to 19-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.</li> <li>4) TCLL: ASCE 7-16; Pr=20.0 psf (toof LL: Lum DOL=1.15) Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10</li> </ul>							Ŵ	ORTH CA	ROUNT	
FORCES TOP CHORD	(lb) - Maximum Tension 1-2=0/14, 2-3= 4-5=-29/46, 5-6 7-8=-38/88, 8-9	n Com =-19/28 6=-30/ 9=-30/	pression/Maximum	6 7 8 9	load of 12. overhangs All plates a Gable requ Gable stud ) This truss l	has been designed to 0 psf or 1.00 times f non-concurrent with re 2x4 MT20 unless iires continuous bott s spaced at 2-0-0 o has been designed to oad nonconcurrent	lat roof I n other li s otherw tom cho c. for a 10.	bad of 20.0 ps ve loads. ise indicated. rd bearing. 0 psf bottom	sf on		111WA	A A A A A A A A A A A A A A A A A A A		22	

May 29,2025

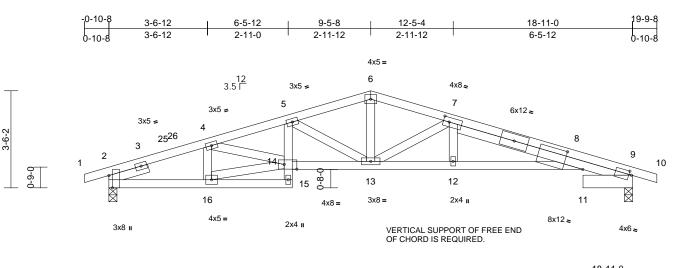
Page: 1

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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	K2	Roof Special	5	1	I73808012 Job Reference (optional)

Run: 8.73 E May 9 2024 Print: 8.730 E May 9 2024 MiTek Industries, Inc. Thu May 29 08:48:40 ID:QSHLTMQ3uNH81uyu1iCXt?zBxee-VQml3l2Te1vaNstypeKrgSlbX0iYIQmATGAizozBiTM

Page: 1



1	3-6-12	6-7-8	9-5-8	12-5-4	17-1-7	17-1-8	I.
	3-6-12	3-0-12	2-10-0	2-11-12	4-8-3	0-0-1 1-9-8	٦

Scale = 1:41.6

Plate Offsets (X, Y): [2:0-5-6,Edge], [7:0-2-8,0-2-0], [8:0-8-8,0-5-8], [9:0-1-2,0-7	1-8], [14:0-5-8,0-2-0]
--	------------------------

Loading         (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	Plate Grip DOL       1.         Lumber DOL       1.         Rep Stress Incr       YI	15	CSI TC BC WB 014 Matrix-MSH	0.84 0.78 0.55	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.18 -0.34 0.12	(loc) 8-12 8-12 9	l/defl >999 >670 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 101 lb	<b>GRIP</b> 244/190 FT = 20%
14-8:2x4 SP No.1           VEBS         2x4 SP No.3           SLIDER         Left 2x4 SP No.3           - c-6-10	ept* 15-5:2x4 SP No.3, .11-9:2x6 SP No.2 - 1-6-0, Right 2x4 SP No.1 heathing directly applied or tly applied or 10-0-0 oc 0-3-8, 9=805/0-3-8 C 14) (LC 10), 9=-129 (LC 11) LC 21), 9=876 (LC 22) ompression/Maximum 95/327, 4-5=-2263/488, 7=-1497/378, 9=-1066/196, 9-10=0/17 15-16=-39/210, 14-15=0/81 14=-399/2203, 8-12=-370/2090, 9-11=0/0 13=-113/682, 7-12=0/104, 16=-414/119, I-16=-226/1318	Vasda II; Exp and C to 6-5 to 16- left ar expos reacti DOL= 3) TCLL Plate DOL= 3) TCLL Plate Cs=1. 4) Unbal desigg 5) This t load c overh 6) This t chord 7) * This t on the 3-06-1 chord 8) One F truss This c latera	: ASCE 7-16; Pr=20.0 ps DOL=1.15); Pf=20.0 psf =1.15); Is=1.0; Rough Cat .00; Ct=1.10 lanced snow loads have	BCDL=6 envelope 2 2-1-8, 12-4-1, I o 19-9-4 trical left forces a =1.60 pl f (roof LL Lum DC B; Fully been con or great at roof I o ther li o ther li o r a 10. with any I for a liv s where II fit betw commen PLIFT at	.0psf; h=25ft s) exterior zor Interior (1) 2- interior (1) 2-	ne 1-8 -4-1 ever r 1.15 e 9; his f live sf on ds. 0psf om ect				SEA 0363	ROUTING TO THE ROUTING

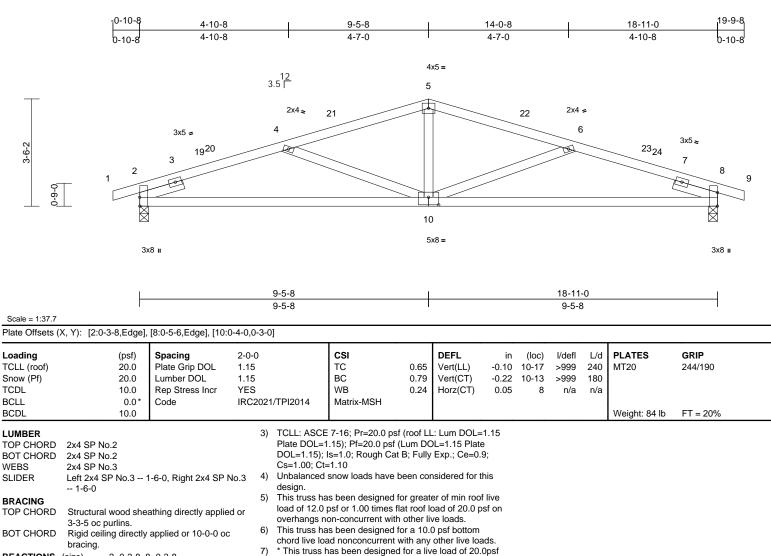




G minin May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	КЗ	Common	1	1	I73808013 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:IIjaeTpIOv0e4gCfnRCf5YzBxYz-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



REACTIONS (size) 2=0-3-8.8=0-3-8 Max Horiz 2=48 (LC 14) Max Uplift 2=-120 (LC 10), 8=-120 (LC 11) Max Grav 2=873 (LC 21), 8=873 (LC 22) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/15, 2-4=-1696/398, 4-5=-1252/298, 5-6=-1252/298, 6-8=-1696/398, 8-9=0/15 BOT CHORD 2-8=-311/1582 WEBS 5-10=0/432, 6-10=-468/168, 4-10=-468/168

NOTES

 Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 6-5-8, Exterior(2R) 6-5-8 to 12-5-8, Interior (1) 12-5-8 to 16-9-8, Exterior(2E) 16-9-8 to 19-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

on the bottom chord in all areas where a rectangle

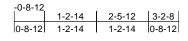
LOAD CASE(S) Standard



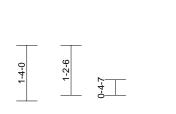
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

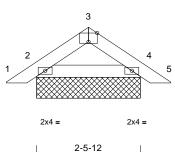
Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	PB1	Piggyback	13	1	I73808014 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:r80jyzoxu\_h0poeCCSsbAQzBxAS-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



12 8 Г





3x5 =

Scale = 1:27.6

5)

design.

Unbalanced snow loads have been considered for this

## Plate Offsets (X, Y): [3:0-2-8.Edge]

Plate Offsets (.	X, Y): [3:0-2-8,Edge]												
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI	12014	<b>CSI</b> TC BC WB Matrix-MP	0.03 0.03 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 10	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 244/190 FT = 20%
	Structural wood she 4-0-0 oc purlins. Rigid ceiling directly bracing.	applied or 10-0-0 or 4=2-5-12 2 12) 2 14), 4=-14 (LC 15)	loa ove 7) Ga 9) Thi chc 10) * Ti on 3-0 chc	d of 12.0   erhangs no ble require ble studs is truss ha ord live loa his truss h the bottor 06-00 tall b ord and ar	s been designed for osf or 1.00 times fl on-concurrent with es continuous both spaced at 2-0-0 oc s been designed for d nonconcurrent v as been designed n chord in all areas y 2-00-00 wide wil y other members.	at roof le other li om chor c. or a 10. vith any for a liv s where	bad of 20.0 ps ve loads. d bearing. ) psf bottom other live loa e load of 20.0 a rectangle	sf on Ids. Opsf					
FORCES TOP CHORD BOT CHORD NOTES 1) Unbalance this design	(lb) - Maximum Com Tension 1-2=0/23, 2-3=-86/3 2-4=-1/65 ed roof live loads have	, 3-4=-87/35, 4-5=0	Det	tail for Co nsult quali	d Industry Piggyba nection to base tr ïed building desigr Standard	uss as a							
<ol> <li>Wind: ASC Vasd=1033 II; Exp B; E and C-C E exposed; members a Lumber D0</li> <li>Truss desi only. For s see Standa or consult</li> <li>TCLL: ASC Plate D0L D0L=1.15 Cs=1.00; 0</li> </ol>	CE 7-16; Vult=130mph mph; TCDL=6.0psf; Bi Enclosed; MWFRS (er end vertical left and rig and forces & MWFRS DL=1.60 plate grip DC gned for wind loads in studs exposed to wind ard Industry Gable En qualified building desig CE 7-16; Pr=20.0 psf (L =1.15); Pf=20.0 psf (L ); Is=1.0; Rough Cat E	CDL=6.0psf; h=25ft; ivelope) exterior zon ilever left and right ght exposed;C-C for for reactions shown V=1.60 the plane of the trus l (normal to the face) d Details as applicat gner as per ANSI/TF roof LL: Lum DOL=1 um DOL=1.15 Plate 3; Fully Exp.; Ce=0.9	e ss , , , , , , , , , , , , , , , , , ,							4		SEA 0363	L

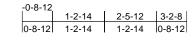


Page: 1

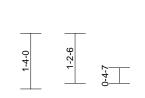
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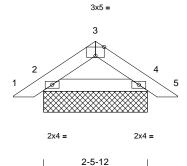
Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	PB2-2	Piggyback	1	2	I73808015 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:LJNoPF4sdlNLNh0LDxEho3zBx7W-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



12 8 Г





Scale = 1:27.6

# Plate Offsets (X, Y): [3:0-2-8,Edge]

Loading         (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0*           BCDL         10.0	Spacing2-0Plate Grip DOL1.1Lumber DOL1.1Rep Stress IncrYECodeIRC	5 5	CSI TC BC WB Matrix-MP	0.02 0.02 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 10	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 22 lb	<b>GRIP</b> 244/190 FT = 20%
How the second secon	<ul> <li>2 14), 4=-14 (LC 15)</li> <li>C 21), 4=163 (LC 22)</li> <li>apression/Maximum</li> <li>7, 3-4=-86/36, 4-5=0/23</li> <li>ther as follows:</li> <li>(0.131"x3") nails as</li> <li>0d (0.131"x3") nails as</li> <li>applied to all plies,</li> <li>ck (B) face in the LOAD</li> <li>nections have been</li> <li>noted as (F) or (B),</li> <li>been considered for</li> <li>ascond gust)</li> <li>CDL=6.0psf; h=25ft; Cat.</li> <li>velope) exterior zone</li> <li>ilever left and right</li> <li>ght exposed; C-C for</li> <li>for reactions shown;</li> </ul>	<ul> <li>only. For st see Standar or consult q</li> <li>TCLL: ASCI Plate DOL= DOL=1.15); Cs=1.00; Ct</li> <li>Unbalanced design.</li> <li>This truss h load of 12.0 overhangs r</li> <li>Gable requi</li> <li>Gable studs</li> <li>This truss on the botto 3-06-00 tall chord and a</li> <li>N/A</li> <li>See Standa Detail for Co</li> </ul>	I snow loads have b as been designed for psf or 1.00 times fil- ion-concurrent with res continuous botto spaced at 4-0-0 oc as been designed mad nonconcurrent v has been designed m chord in all areas by 2-00-00 wide wil ny other members.	d (norm nd Deta signer a: (roof LL Lum DC B; Fully been cor or great at roof le other lif om chor : or a 10. vith any for a liv s where Il fit betw	al to the face ils as applical is per ANSI/TF JL=1.15 Plate Exp.; Ce=0.9 asidered for the er of min roof bad of 20.0 ps re loads. d bearing. D psf bottom other live loa e load of 20.0 a rectangle ween the bottom s Connection	), ble, PI 1. 1.15 9; his 9; live sf on dds. Dpsf om				SEA 0363	EER A LUN

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

May 29,2025

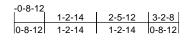
Page: 1



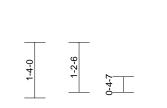
Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	PB3-3	Piggyback	1	3	I73808016 Job Reference (optional)

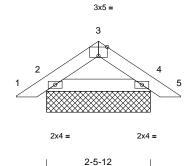
Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:mR7nAvX3v615JNLSuDOMoszBx6x-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



12 8 Г





Scale = 1:27.6

# Plate Offsets (X, Y): [3:0-2-8,Edge]

								i	
Loading         (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0*           BCDL         10.0	Spacing2-0-Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC	5 5	CSI           TC         0.01           BC         0.01           WB         0.00           Matrix-MP	DEFL Vert(LL) Vert(CT) Horz(CT)	in (loc n/a n/a 0.00 10	n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 32 lb	<b>GRIP</b> 244/190 FT = 20%
4-0-0 oc purlins. BOT CHORD Rigid ceiling directly bracing. REACTIONS (size) 2=2-5-12, Max Horiz 2=-27 (LC Max Uplift 2=-18 (LC Max Grav 2=157 (LC FORCES (lb) - Maximum Com Tension	4=2-5-12 :12) :14), 4=-14 (LC 15) :21), 4=163 (LC 22) pression/Maximum 8, 3-4=-86/36, 4-5=0/23 ther as follows: (0.131"x3") nails as :0d (0.131"x3") nails as applied to all plies, ck (B) face in the LOAD tections have been noted as (F) or (B), been considered for (3-second gust) CDL=6.0psf; h=25ft; Cat. ivelope) exterior zone lever left and right pht exposed;C-C for for reactions shown;	<ul> <li>only. For stise Standar or consult qi</li> <li>TCLL: ASCE Plate DOL=</li> <li>DOL=1.15); Cs=1.00; Ct</li> <li>Unbalanced design.</li> <li>This truss ha load of 12.0 overhangs ri</li> <li>Gable requii</li> <li>Gable studs</li> <li>This truss is chord live lo</li> <li>* This truss is on the botto 3-06-00 tall chord and a</li> <li>N/A</li> <li>See Standar Detail for Co</li> </ul>	I snow loads have been co as been designed for great psf or 1.00 times flat roof I oon-concurrent with other I res continuous bottom cho spaced at 4-0-0 oc. as been designed for a 10. ad nonconcurrent with any has been designed for a li m chord in all areas where by 2-00-00 wide will fit bet ny other members.	hal to the face), iils as applicable s per ANSI/TPI 1 L: Lum DOL=1.1: DL=1.15 Plate r Exp.; Ce=0.9; nsidered for this ter of min roof livio oad of 20.0 psf of ve loads. rd bearing. 0 psf bottom other live loads. re load of 20.0ps a rectangle ween the bottom ss Connection	5 n			in nu	EER ALU

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NGINEERING

818 Soundside Road Edenton, NC 27932

May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	V1	Valley	1	1	I73808017 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:04 ID:MqTvfCzFAzGr3OnH09eZ0rzByxs-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

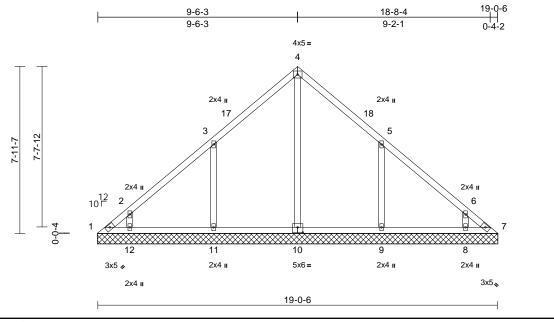


Plate Offsets (X, Y):	[10:0-3-0,0-3-0]

Scale = 1:54.9

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.31	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf) CDL	20.0	Lumber DOL Rep Stress Incr	1.15 YES		BC WB	0.17 0.20	Vert(TL) Horiz(TL)	n/a 0.00	- 7	n/a n/a	999 n/a		
SCLL	10.0 0.0*	Code		1/TPI2014	Matrix-MSH	0.20		0.00	/	n/a	n/a		
BCDL	10.0	Code	IRCZUZ	1/1712014	Maurix-IVISH							Weight: 89 lb	FT = 20%
UMBER OP CHORD			2)	Vasd=103m	7-16; Vult=130mp ph; TCDL=6.0psf;	BCDL=6	6.0psf; h=25ft;						
OT CHORD THERS	2x4 SP No.2 2x4 SP No.3			and C-C Ext	closed; MWFRS ( erior(2E) 0-0-0 to 3	3-0-0, İn	terior (1) 3-0-	0 to					
RACING					or(2R) 6-6-8 to 12								
OP CHORD	<ul> <li>Structural wood she</li> <li>6-0-0 oc purlins.</li> </ul>	eathing directly applied	d or	and right exp	rior(2E) 16-1-0 to bosed ; end vertica	al left an	d right expose						
SOT CHORD	<ul> <li>Rigid ceiling directly bracing.</li> </ul>	y applied or 10-0-0 oc			ers and forces & M ber DOL=1.60 plat								
REACTIONS	9=19-1-0 12=19-1- Max Horiz 1=182 (L 8=-86 (L 11=-72 (L 8=-86 (L 11=-175 Max Grav 1=111 (L 8=317 (L 10=372 ( 12=324 (	C 11) C 12), 7=-28 (LC 13), C 15), 9=-175 (LC 15) (LC 14), 12=-92 (LC 1 C 14), 7=81 (LC 15), C 30), 9=480 (LC 6), LC 27), 11=480 (LC 5 LC 24)	-0, 3) , 4) , 4) , 5)	only. For sti see Standar or consult qi TCLL: ASCE Plate DOL= DOL=1.15); Cs=1.00; Ct: Unbalanced design.	ed for wind loads uds exposed to wird d Industry Gable E Jalified building de E 7-16; Pr=20.0 psf 1.15); Pf=20.0 psf Is=1.0; Rough Cat =1.10 snow loads have I	nd (norm ind Deta signer a f (roof Ll (Lum DC B; Fully been co	al to the face ils as applical s per ANSI/TF L: Lum DOL= DL=1.15 Plate Exp.; Ce=0.§ nsidered for th	), ble, PI 1. 1.15 9;					
ORCES	Tension	npression/Maximum	7) 8)		es continuous bott spaced at 4-0-0 o		rd bearing.						111111
OP CHORD	,	-185/116, 3-4=-209/16 -143/62, 6-7=-184/97	61, 9)	This truss ha	as been designed f	for a 10.		ds			15	RTHU	NOL
OT CHORD	1-12=-77/139, 11-1 8-9=-60/139, 7-8=-0	2=-60/139, 9-11=-60/´ 60/139	139, 10	) * This truss I	has been designed m chord in all area	d for a liv	ve load of 20.0			4	i	AT.	
VEBS	4-10=-180/5, 3-11= 5-9=-379/222, 6-8=	-379/222, 2-12=-262/ -262/171	173,	3-06-00 tall I	by 2-00-00 wide winny other members.	ill fit betv	veen the botto						
NOTES			11		chanical connection					-		SEA	• –
) Unbalanc this desig	ced roof live loads have n.	e been considered for		1, 28 lb uplif at joint 12, 1 8.	e capable of withst t at joint 7, 175 lb u 75 lb uplift at joint	uplift at j 9 and 80	oint 11, 92 lb 3 lb uplift at jo	uplift int				0363	22 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
			12		e or shim required truss chord at join			g			111	BIC	EERIN
			L	OAD CASE(S)	Standard							A. C	il Lunin
													v 29 2025



May 29,2025

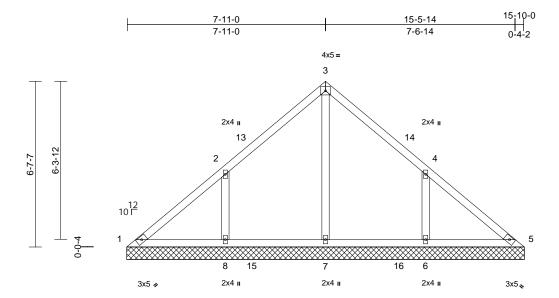
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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	V2	Valley	1	1	I73808018 Job Reference (optional)

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Thu May 29 13:06:53 ID:q01HsY\_txGOihYMTas9oZ3zByxr-YSGe?6ed6k?lonosJ9FMu3GrVxq7lDrhrXJ0q8zBoM2

Page: 1



15-10-0

Scale = 1:46
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Scale = 1:46											-		
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MSH	0.32 0.16 0.21		in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 70 lb	<b>GRIP</b> 244/190 FT = 20%	
	2x4 SP No.3 Structural wood she 6-0-0 oc purlins. Rigid ceiling directly bracing. All bearings 15-10-10 Max Horiz 1=-151 (L Max Uplift All uplift 1 except 6= 14) Max Grav All reactin (s) 1, 5 ex	_C 10) 100 (lb) or less at joint =-170 (LC 15), 8=-173	d or d or d or d or d or d or d or d or	SCE 7-16; Pr=20.0 DL=1.15); Pf=20.0 p 15); Is=1.0; Rough ( ); Ct=1.10 aced snow loads have equires continuous b tuds spaced at 4-0-0 ss has been designe re load nonconcurre uss has been designe ottom chord in all at tall by 2-00-00 wide ad any other membe mechanical connec plate capable of with plate spate (Jt=Ib) 8=172, 6	sf (Lum DC Cat B; Fully ve been co bottom cho o c. ed for a 10. nt with any ned for a 10. nt with any ned for a lin reas where will fit bett ers, with BC tion (by oth hstanding	DL=1.15 Plate v Exp.; Ce=0.5 nsidered for th rd bearing. 0 psf bottom other live loa ve load of 20.0 a rectangle ween the botto CDL = 10.0psf hers) of truss t	ds. Dpsf om o						
FORCES	(lb) - Max. Comp./M (lb) or less except w	lax. Ten All forces 2 /hen shown.		plate or shim requin with truss chord at ju			g						
WEBS	3-7=-273/0, 2-8=-38	34/208, 4-6=-384/207											
NOTES													
,	ed roof live loads have	e been considered for										1111	
this design		(*									N''ILCI	D	
	CE 7-16; Vult=130mph mph; TCDL=6.0psf; B		Cat							1	OR. FESS	NO III	
	Enclosed; MWFRS (er									13	O'. FESS	SIGN SV S	
	Exterior(2E) 0-0-0 to 3-		to						4	ès		A.	
	terior(2R) 4-11-5 to 10 12-10-10, Exterior(2E		10						3		·Q.		
	tilever left and right ex								=		SEA SEA	AL : E	
and right e	exposed;C-C for memb	bers and forces &							=		0363	22 : E	
	or reactions shown; Lu	umber DOL=1.60 plat	e								: 0000	: :	
grip DOL= 3) Truss des	1.60 signed for wind loads in	n the plane of the true	20						-	5	SEA 0363	EER A	
	studs exposed to wind									20	S. SNOW	EER. A.S	
	ard Industry Gable En									1	P/ GIN	A CAN A	
	qualified building desi									1	ICA C	BEIN	

Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

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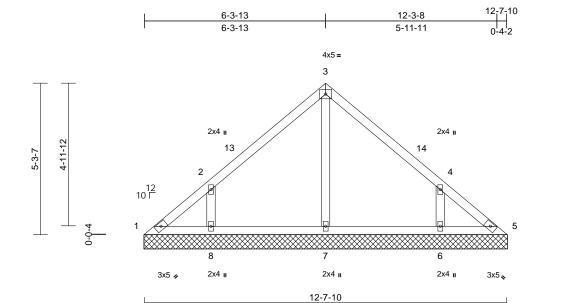
818 Soundside Road Edenton, NC 27932

G mmm May 29,2025

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	V3	Valley	1	1	I73808019 Job Reference (optional)

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Thu May 29 13:07:24 ID:q01HsY\_txGOihYMTas9oZ3zByxr-5ol7xP1JLt93FspCkbL5j08UHE6T\_AtYRApGTjzBoLX

Page: 1



Sca	le	_ `	1.4	0 2	>

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC20	21/TPI2014	<b>CSI</b> TC BC WB Matrix-MSH	0.32 0.12 0.09	<b>DEFL</b> Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 53 lb	<b>GRIP</b> 244/190 FT = 20%
	2x4 SP No.2 2x4 SP No.3 Structural wood shea 6-0-0 oc purlins. Rigid ceiling directly bracing. All bearings 12-8-3. Max Horiz 1=120 (LC Max Uplift All uplift 1 1, 5 excep (LC 14) Max Grav All reactio (s) 1, 5 ex	applied or 10-0-0 or C 13) 00 (lb) or less at joir ot 6=-139 (LC 15), 8 ns 250 (lb) or less a cept 6=436 (LC 21) C 20), 8=436 (LC 20 ax. Ten All forces hen shown.	ed or 6 c 7 s t(s) =-142 t joint 1	<ul> <li>Plate DOL=1 DOL=1.15); I Cs=1.00; Ct=</li> <li>Unbalanced design.</li> <li>Gable requirity</li> <li>Gable studs</li> <li>Gable studs</li> <li>This truss ha chord live loa</li> <li>* This truss ha on the bottor 3-06-00 tall b chord and ar</li> <li>Provide mecibearing plate (s) 1 except i</li> <li>Beveled plate</li> </ul>	snow loads have es continuous bo spaced at 4-0-0 s been designed d nonconcurren as been designe n chord in all are y 2-00-00 wide y other member nanical connection capable of with: jt=lb) 8=142, 6=	f (Lum DC at B; Fully been cor oc. if for a 10.0 t with any ed for a liv as where will fit betw s. on (by oth standing 1 139. ed to provi	DL=1.15 Plate Exp.; Ce=0.9 Insidered for the d bearing. D psf bottom other live load e load of 20.0 a rectangle ween the botthers) of truss the 00 lb uplift at	e ); ids. Dpsf om i joint					
this design 2) Wind: ASC	ed roof live loads have n. CE 7-16; Vult=130mph	(3-second gust)										TH CA	Rout

- 2) Wind: ASCE 7-16; Vult=130mpn (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 3-4-2, Exterior(2R) 3-4-2 to 9-4-2, Interior (1) 9-4-2 to 9-8-3, Exterior(2E) 9-8-3 to 12-8-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

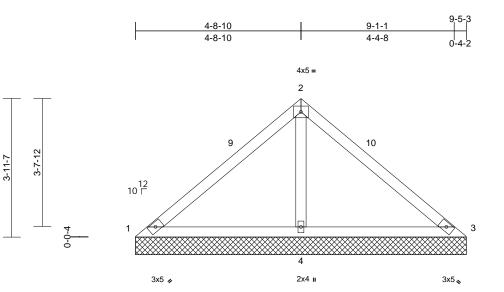


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	Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
2	25050180-01	V4	Valley	1	1	I73808020 Job Reference (optional)

Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Thu May 29 13:07:39 ID:q01HsY\_txGOihYMTas9oZ3zByxr-9h9o4XDkpU2xZ9T56F6cqBF1rH9W?xJlt?xZVMzBoLI



9-5-3

Scale = 1:32.9

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		тс	0.42	Vert(LL)	n/a	(100)	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.41	Vert(TL)	n/a	-	n/a	999		210,000
TCDL	10.0	Rep Stress Incr	YES		WB	0.17	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC202	21/TPI2014	Matrix-MSH		. ,						
BCDL	10.0											Weight: 36 lb	FT = 20%
LUMBER TOP CHORD			4	Plate DOL=1	7-16; Pr=20.0 ps 1.15); Pf=20.0 psf	(Lum DC	DL=1.15 Plate						
BOT CHORD					Is=1.0; Rough Ca	t B; Fully	Exp.; Ce=0.9	9;					
OTHERS	2x4 SP No.3		5	Cs=1.00; Ct	=1.10 snow loads have	hoon cor	acidorod for th	aic					
BRACING				design.	Show loads have	been coi		115					
TOP CHORD	9-5-3 oc purlins.	eathing directly applied	d or 6	•	es continuous bot	tom chor	d bearing.						
BOT CHORD		/ applied or 6-0-0 oc	7 8	) This truss ha	spaced at 4-0-0 o as been designed	for a 10.							
REACTIONS	(size) 1=9-5-3, Max Horiz 1=88 (LC Max Uplift 1=-44 (LC	,	9	) * This truss I on the bottor	ad nonconcurrent has been designed m chord in all area	d for a liv as where	e load of 20.0 a rectangle	Opsf					
	4=-104 (L	,, ,, ,,	=748 1	chord and ar 0) Provide med	by 2-00-00 wide w by other members hanical connectio e capable of withs	n (by oth	ers) of truss t	0					
FORCES	(lb) - Max. Comp./M (lb) or less except w	lax. Ten All forces 2 /hen shown.	50		t at joint 3 and 104			oint					
TOP CHORD	1-9=-110/254, 2-9=- 3-10=-110/254	-93/358, 2-10=-93/358	3,										
WEBS	2-4=-617/264												
NOTES													
<ol> <li>Unbalanc this desig</li> </ol>	ed roof live loads have n.	been considered for											11111

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
   Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-5 to 3-0-5, Exterior(2R) 3-0-5 to 6-5-8, Exterior(2E) 6-5-8 to 9-5-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

SEAL 036322 MGINEER May 29,2025

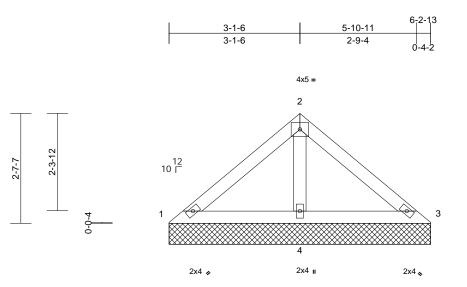
Page: 1

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ENGINEERING BY A MI Tek Affiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	V5	Valley	1	1	I73808021 Job Reference (optional)

Run; 8,73 E Nov 16 2023 Print; 8,730 E Nov 16 2023 MiTek Industries, Inc. Thu May 29 13:07:51 ID:q01HsY\_txGOihYMTas9oZ3zByxr-o?uKbeMF\_AYE??NOpmKQJjl8j7JgpO\_WetsBxfzBoL6





PEFL /ert(LL) /ert(TL)	in (lo n/a	c) l/defl - n/a	L/d	PLATES	GRIP
/ert(LL)		· .	L/d		CDID
( )	n/a	n/o		1 6/1160	GNIF
		- n/a	999	MT20	244/190
en(IL)	n/a	- n/a	999		
loriz(TL)	0.00	4 n/a	n/a		
				Weight: 23 lb	FT = 20%
	bottom			· · ·	Weight: 23 lb

OTHERS	2x4 SP N	0.3
BRACING		
TOP CHORD	Structura	I wood sheathing directly applied or
	6-2-13 oc	purlins.
BOT CHORD	Rigid ceil	ing directly applied or 6-0-0 oc
	bracing.	
REACTIONS	(size)	1=6-2-13, 3=6-2-13, 4=6-2-13

- Max Horiz 1=-57 (LC 10) Max Uplift 3=-3 (LC 10), 4=-54 (LC 14) Max Grav 1=101 (LC 20), 3=101 (LC 21), 4=424 (LC 21) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250
- (lb) or less except when shown. WEBS 2-4=-327/156
- NOTES
- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00: Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.

on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 3 and 54 lb uplift at joint 4.



Page: 1

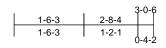
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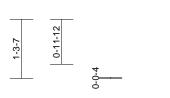
Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	V6	Valley	1	1	I73808022 Job Reference (optional)

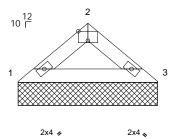
Run: 8.73 E Nov 16 2023 Print: 8.730 E Nov 16 2023 MiTek Industries, Inc. Thu May 29 13:08:03 ID:HhbuFygw6XUiavNJHLHM9kzBxv6-SJcs6kWn9s3XRrliWHYEoFFCHyQRdqwHPkmqLzzBoKw

Page: 1



3x5 =





3-0-6

Scale = 1:25

# Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	20.0 Pla 20.0 Lur	ate Grip DOL mber DOL p Stress Incr de	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-MP	0.07 0.07 0.00	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 9 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD 2x4 SP No BOT CHORD 2x4 SP No BRACING TOP CHORD Structural 3-0-6 oc p BOT CHORD Rigid ceilin bracing. REACTIONS (size) Max Horiz Max Uplift Max Grav FORCES (lb) - Max. (lb) or less NOTES 1) Unbalanced roof live lo this design. 2) Wind: ASCE 7-16; Vul	2.2 wood sheathin urlins. ng directly appl 1=3-0-6, 3=3-0 1=26 (LC 13) 1=-10 (LC 14), 1=140 (LC 20) Comp./Max. T s except when s bads have beer t=130mph (3-si	, 3=-10 (LC 15) ), 3=140 (LC 21) 'en All forces 2 shown. n considered for econd gust)	9) d or 10) 250	chord live lo. * This truss l on the bottoo 3-06-00 tall l chord and an Provide med bearing plate	as been designed f ad nonconcurrent has been designed in chord in all area by 2-00-00 wide w by other members hanical connection e capable of withst uplift at joint 3.	with any I for a liv s where ill fit betv n (by oth	other live load e load of 20.0 a rectangle veen the botto ers) of truss to	)psf om o				Weight: 9 lb	FT = 20%
<ul> <li>Vasd=103mph; TCDL= II; Exp B; Enclosed; M and C-C Exterior(2E) z exposed ; end vertical members and forces &amp; Lumber DOL=1.60 pla</li> <li>Truss designed for wir only. For studs exposs see Standard Industry or consult qualified bui</li> <li>TCLL: ASCE 7-16; Pr= Plate DOL=1.15); Is=1.0; Rc Cs=1.00; Ct=1.10</li> <li>Unbalanced snow load design.</li> <li>Gable requires continu</li> <li>Gable studs spaced at</li> </ul>	=6.0psf; BCDL= WFRS (enveloy cone; cantilevel left and right e: MWFRS for re te grip DOL=1. nd loads in the ed to wind (nor Gable End Dei Iding designer =20.0 psf (roof I 20.0 psf (Lum E bugh Cat B; Ful ds have been co ious bottom cho	=6.0psf; h=25ft; pe) exterior zonor r left and right xposed;C-C for eactions shown; .60 plane of the true; mal to the face), tails as applicab as per ANSI/TP LL: Lum DOL=1 DOL=1.15 Plate Ily Exp.; Ce=0.9; considered for thi	e ss le, l 1. .15							N		SEA 0363	EER REPUT



May 29,2025

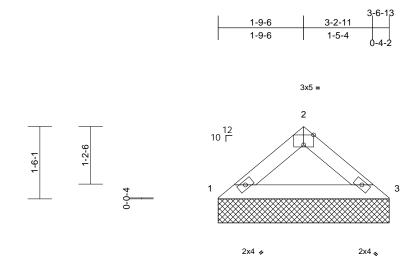
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Job	Truss	Truss Type	Qty	Ply	53 Magnolia Acres-Roof-Morgan HB 3FL TRAY NOOK FL
25050180-01	V7	Valley	1	1	I73808023 Job Reference (optional)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed May 28 15:32:05 ID:ZaiHxqmJbVeA8rjOBP9rpRzBy8T-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

3-6-13

Page: 1



Scale = 1:24

# Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	<b>CSI</b> TC BC WB Matrix-MP	0.10 0.08 0.00	<b>DEFL</b> Vert(LL) Vert(TL) Horiz(TL)	in n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD BOT CHORD BOT CHORD REACTIONS (A M FORCES TOP CHORD BOT CHORD BOT CHORD BOT CHORD NOTES 1) Unbalanced this design. 2) Wind: ASCE Vasd=103m II; Exp B; Er	2x4 SP No.2 2x4 SP No.2 Structural wood she 3-6-13 oc purlins. Rigid ceiling directly bracing.	applied or 10-0-0 or 3=3-6-13 13) 2 14), 3=-12 (LC 15) 2 20), 3=167 (LC 21) pression/Maximum 17/82 been considered for (3-second gust) CDL=6.0psf; h=25ft; ivelope) exterior zon	8) This truss chord live 9) * This trus on the bot 3-06-00 ta chord and 10) Provide m bearing pl 1 and 12 l LOAD CASE(	ds spaced at 4-0-0 of has been designed load nonconcurrent s has been designe tom chord in all are: Il by 2-00-00 wide v any other members echanical connection ate capable of withs b uplift at joint 3. S) Standard	for a 10. with any d for a liv as where vill fit betw s. on (by oth	other live loa e load of 20.1 a rectangle veen the bott ers) of truss	Opsf com to				Weight: 11 lb	FT = 20%
<ul> <li>exposed ; ei members ar Lumber DO</li> <li>3) Truss desig only. For st see Standar or consult q</li> <li>4) TCLL: ASCI Plate DOL= DOL=1.15); Cs=1.00; Ct</li> <li>5) Unbalanced design.</li> </ul>	nd vertical left and ri nd forces & MWFRS L=1.60 plate grip DC ned for wind loads in uds exposed to winc rd Industry Gable En ualified building desi E 7-16; Pr=20.0 psf ( 1.15); Pf=20.0 psf ( Is=1.0; Rough Cat E	ght exposed;C-Č for for reactions shown JL=1.60 (normal to the face) d Details as applicat gner as per ANSI/TF roof LL: Lum DOL=1 um DOL=1.15 Plate B; Fully Exp.; Ce=0.9 even considered for th	ss , ole, 11. 1.15 ;						4.000000		SEA 0363	EER ALU



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

