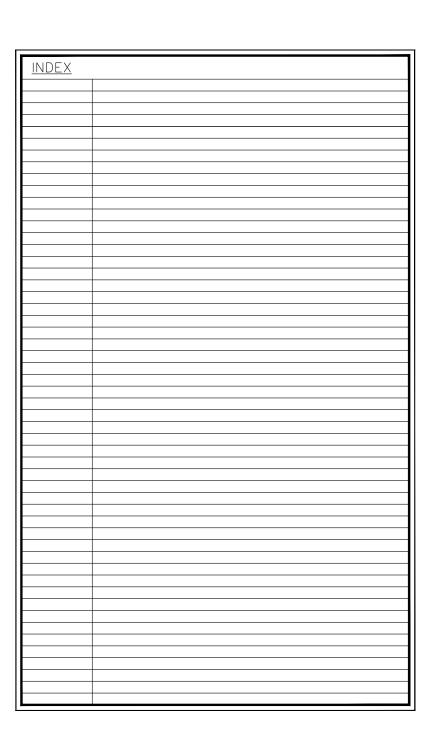
STONEHAVEN-RALE

RALEIGH- LOT 00.0077 BLAKE POND SF

(MODEL# 3656) ELEVATION 1 - GL



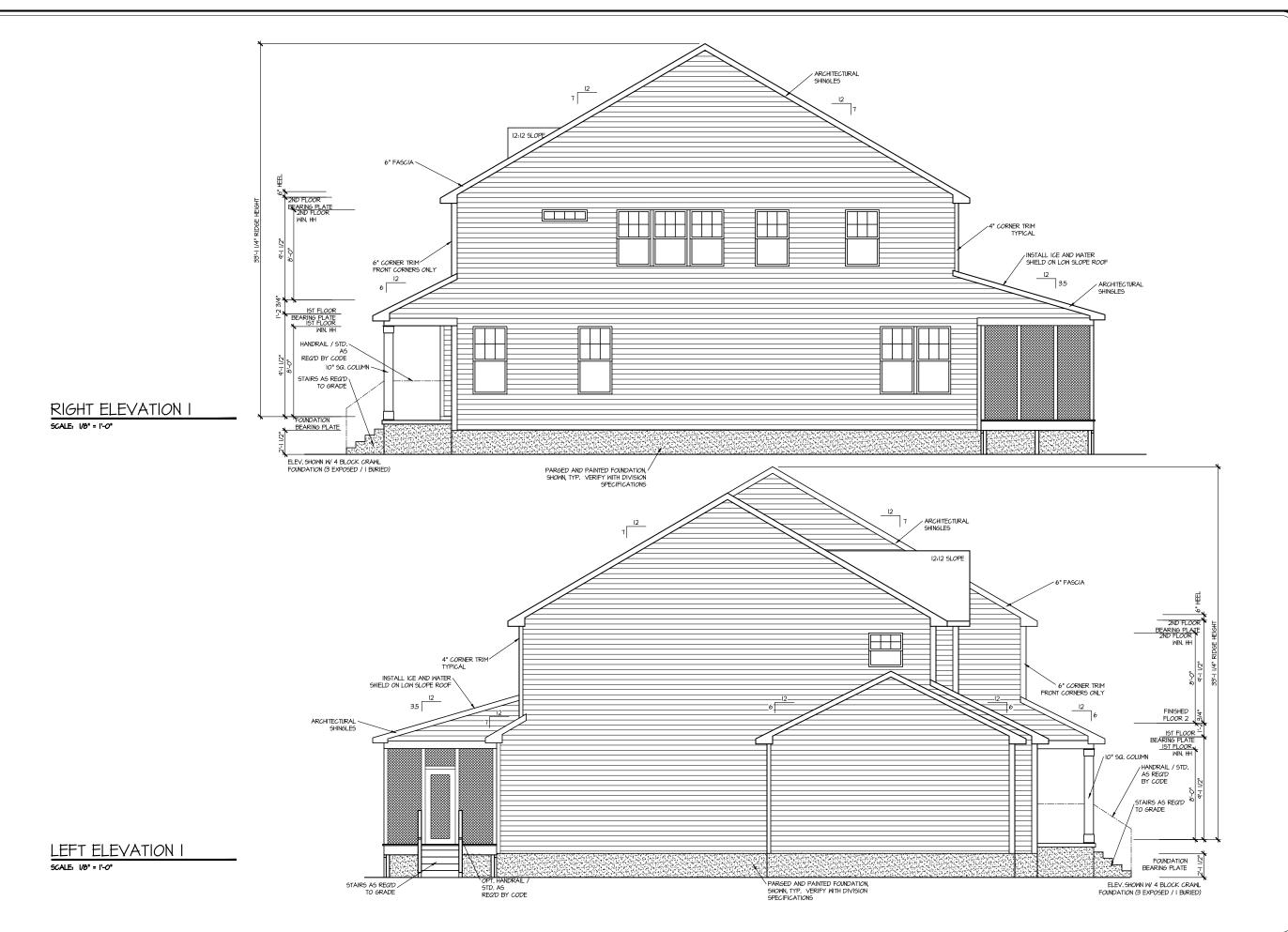


AREA CALCULATIONS ELEVATION 1 FIRST FLOOR	HEATED 1724 SF	COVERED / UNHEATED	UNCOVERED
	1/24 SF		
GARAGE		487 SF	
FRONT PORCH - ELEVATION 1		169 SF	
SECOND FLOOR	1939 SF		
OPTIONS			
3RD CAR GARAGE		264 SF	
SCREENED PORCH		150 SF	
BEDROOM 5	26 SF		
TOTAL	3689 SF	1070 SF	
	1	-	

140 Bronze Leaf Drive

LOT	<u>SPECIFIC</u>	
1	LOT 00.0077	BLAKE POND SF
		STONEHAVEN R2 ELEVATION 1
2	ADDRESS	140 BRONZE LEAF DRIVE LILLINGTON, NC 27546
		_





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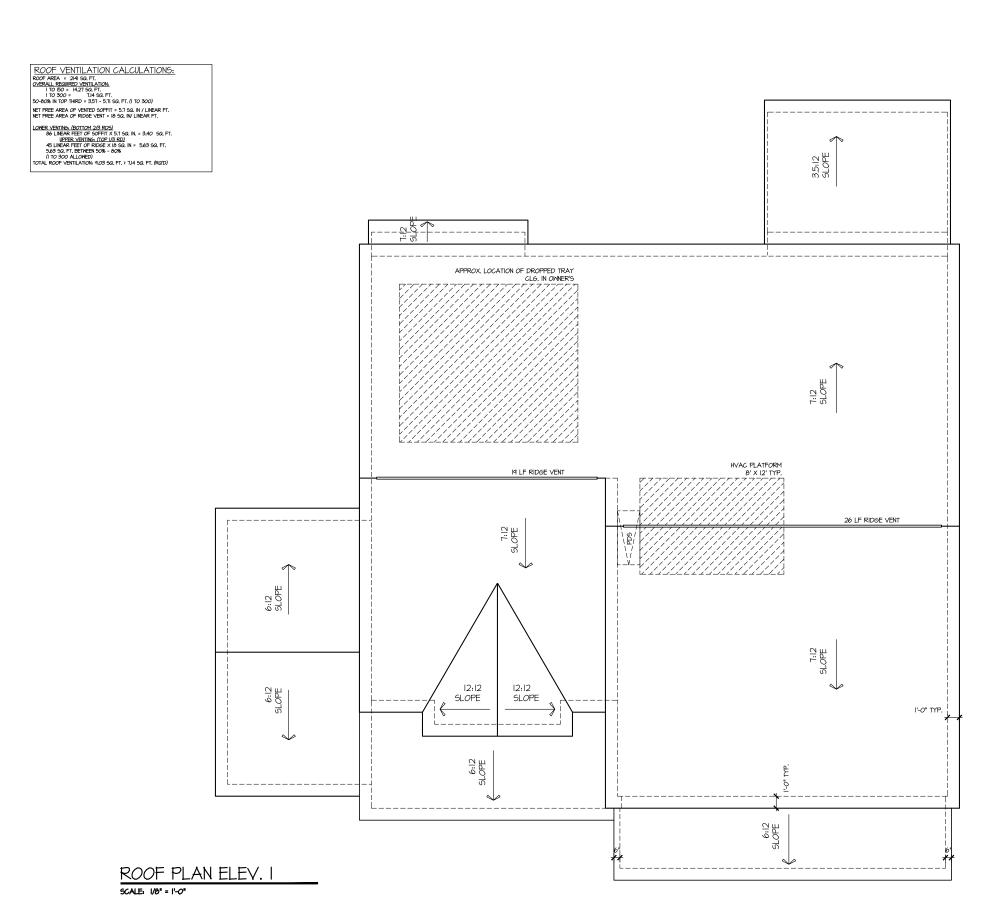
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STONEHAVEN
DRAWING TITLE <u>...</u> ⊥ ⊒ ⊒ ∃ ₩

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SHEET No.



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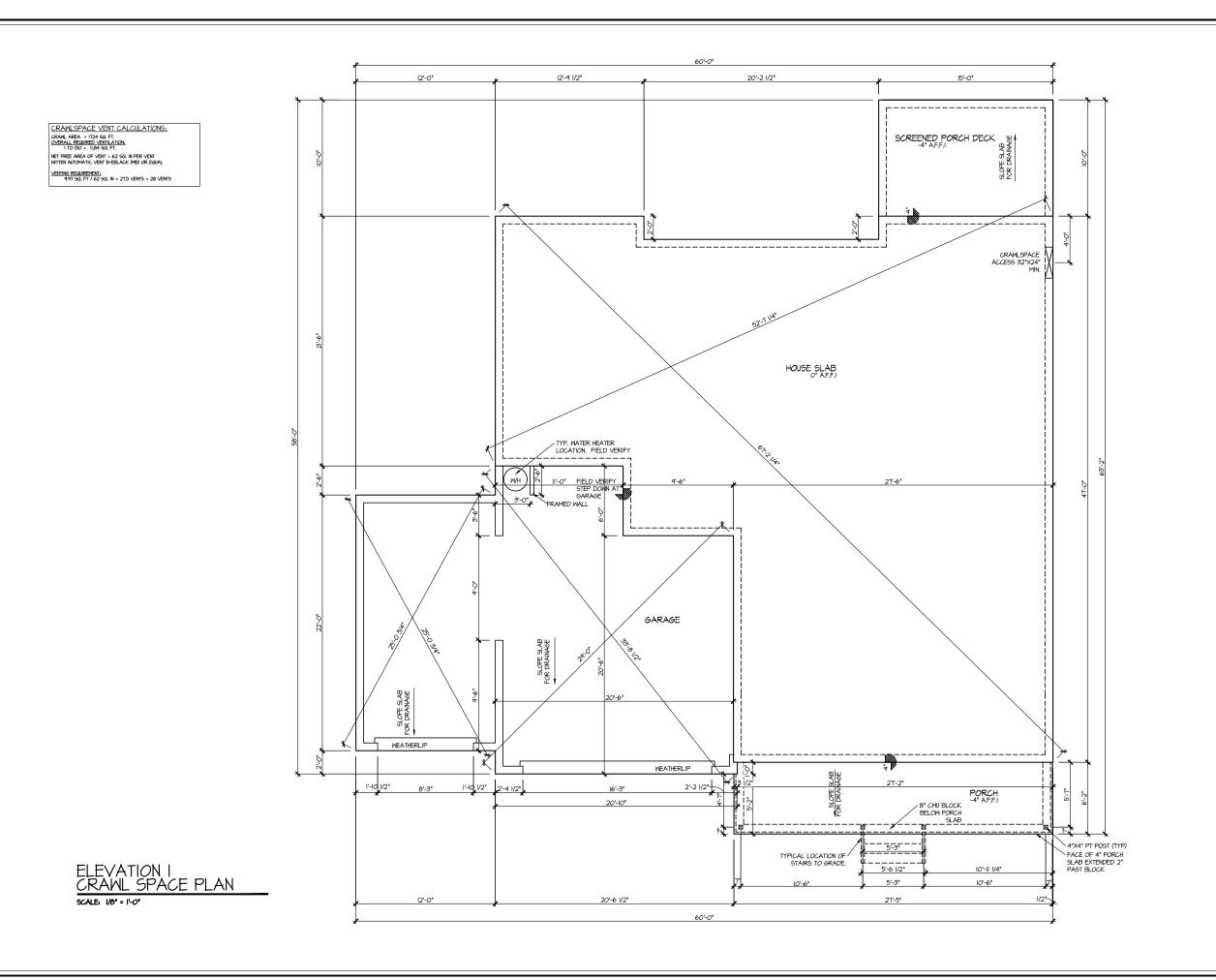
PLAN NO. 3656



HOUSE NAME:
STONEHAVEN
DRAWING TITLE
ROOF PLAN

SHEET No.

AI.3



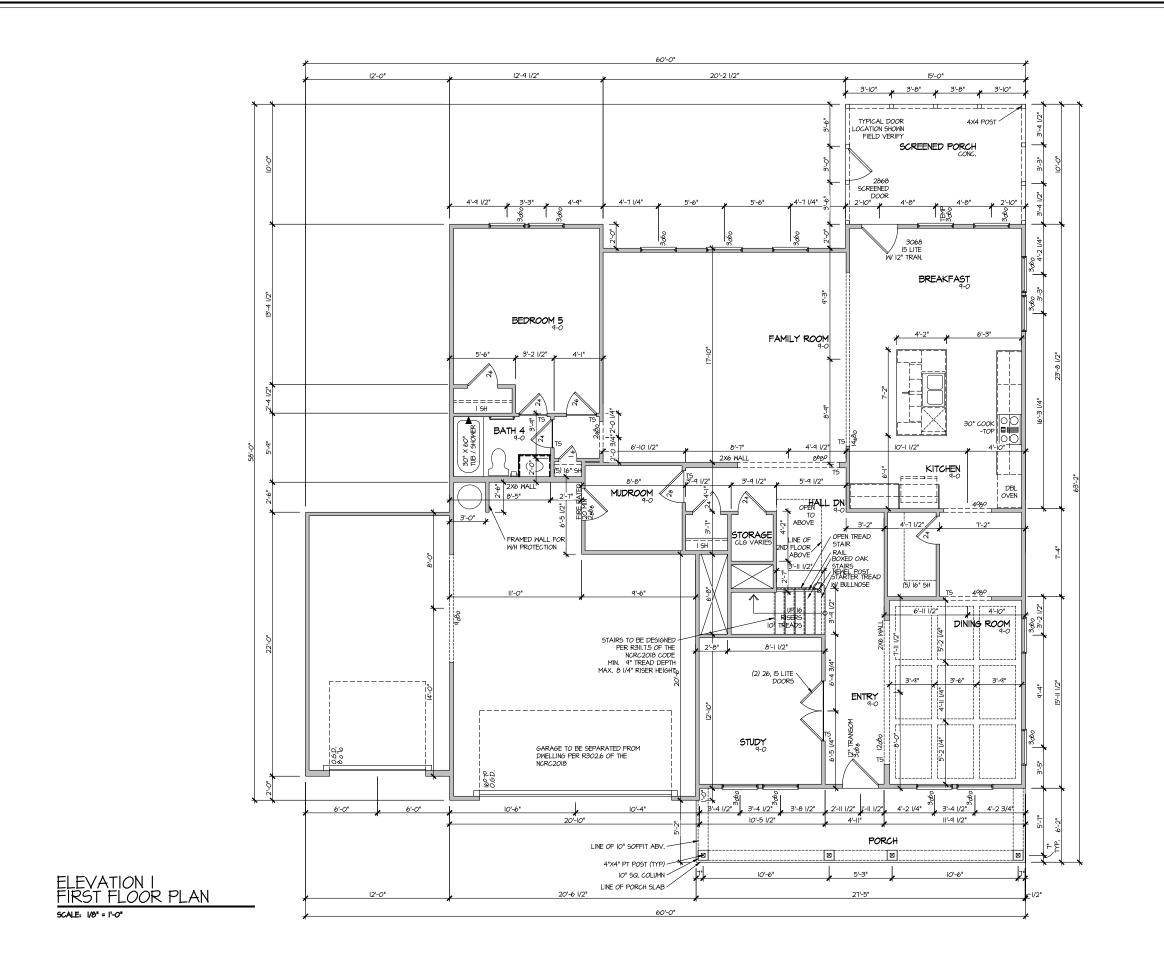
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SPACE PLAN

HOUSE NAME:
STONEHAVEN
DRAWING TITLE
CRAML SPACE PL

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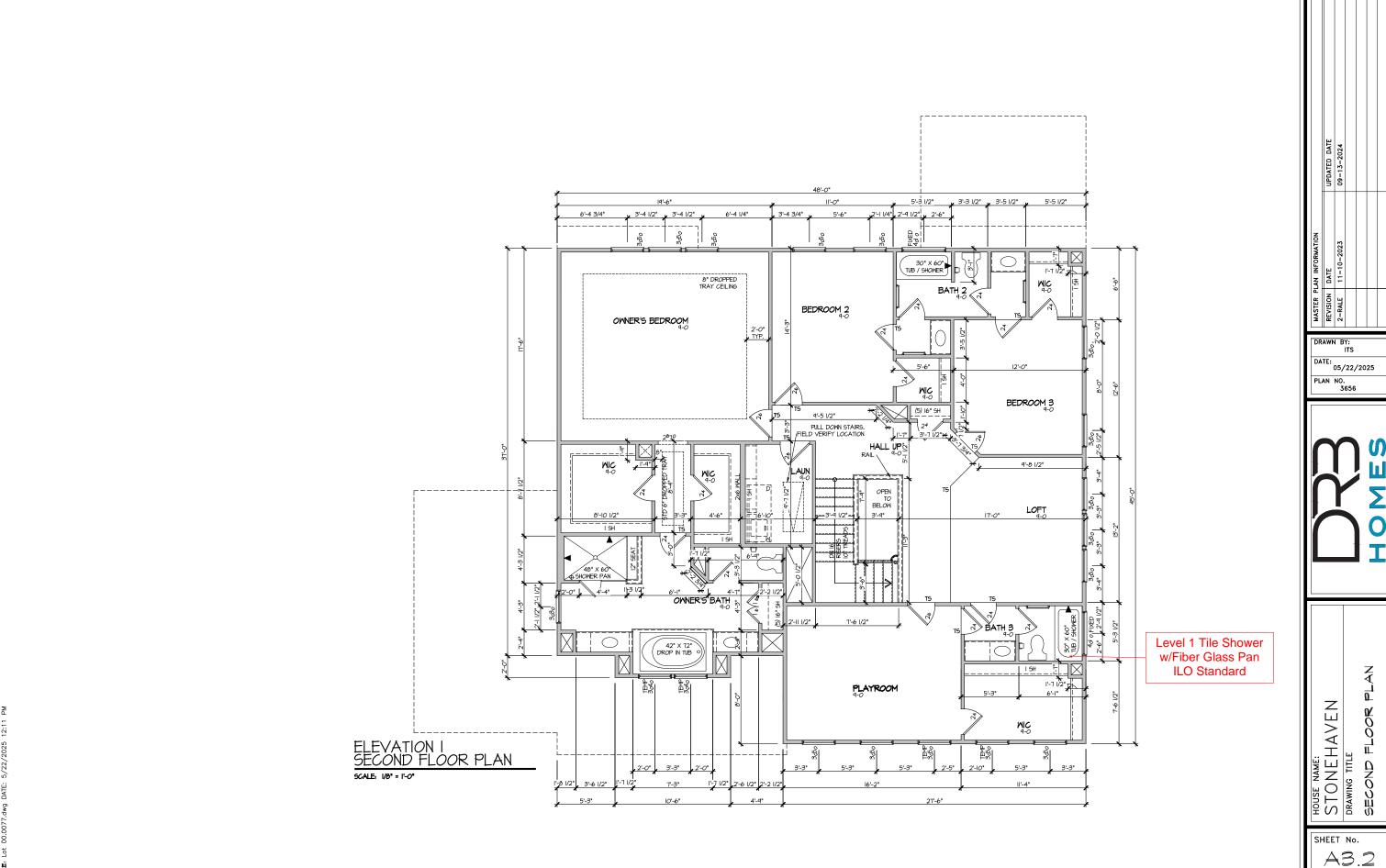
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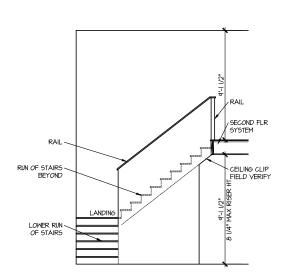


HOUSE NAME:
STONEHAVEN
DRAWING TITLE
FIRST FLOOR PLA

SHEET No.

A3.





STAIR SECTION SCALE: 1/8" = 1'-0"

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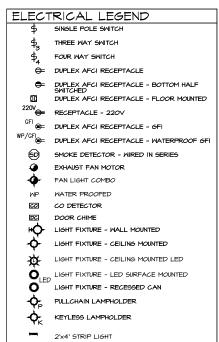
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PLAN NO. 3656

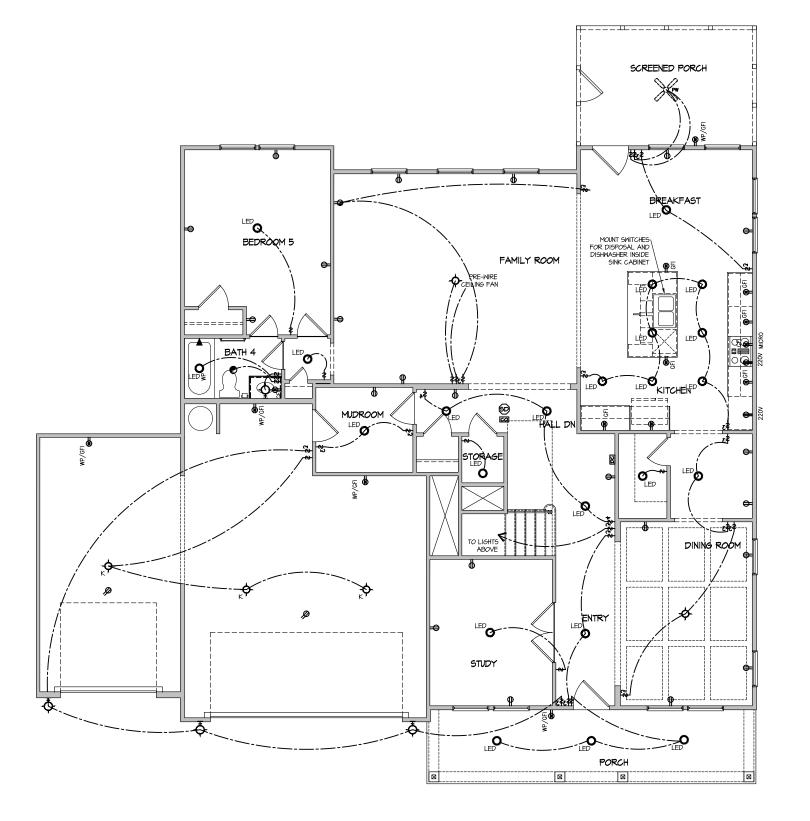


HOUSE NAME:
STONEHAVEN
DRAWING TITLE
BUILDING SECTION

SHEET No. **A4**.



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



ELECTRICAL PLAN FIRST FLOOR - ELEV. I

SHEET No.

HOUSE NAME: STONEHAVE DRAWING TITLE

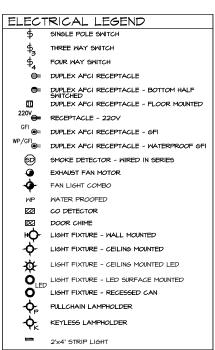
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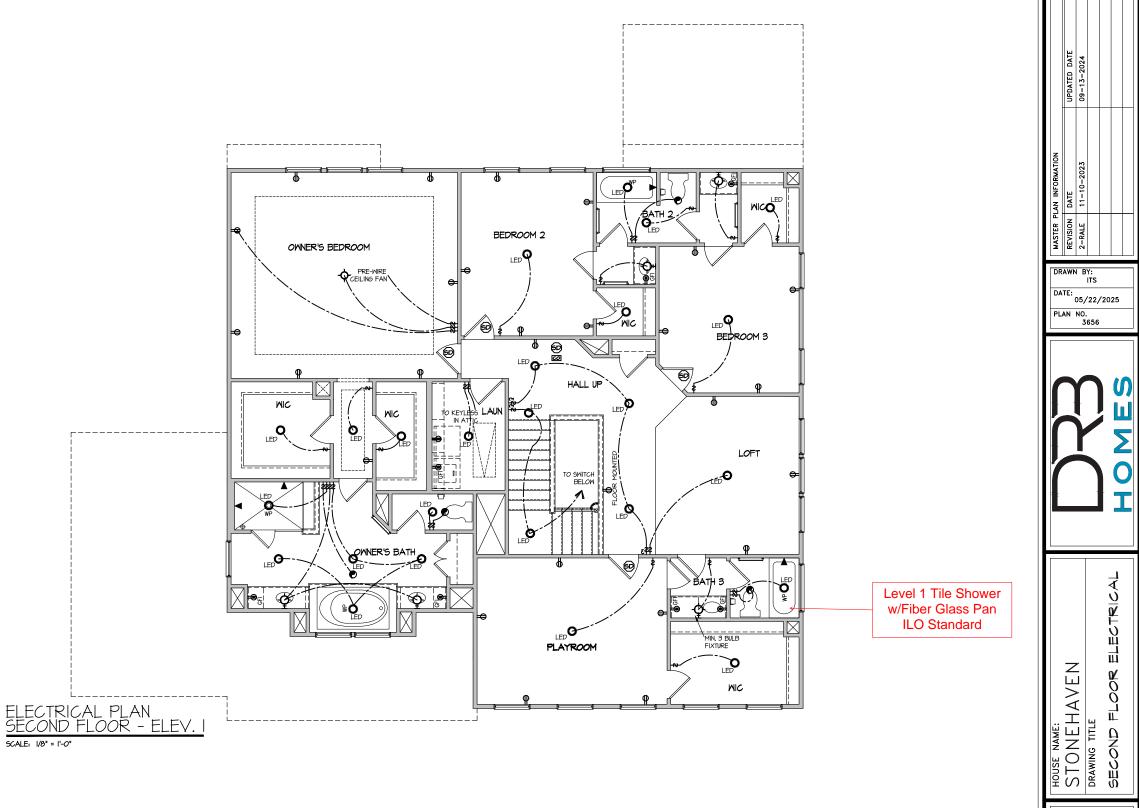
DRAWN BY:

DATE: 05/22/2025 PLAN NO. 3656

SCALE: 1/8" = 1'-0"



NOTE. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



-ILE: Lot 00.0077.dwg DATE: 5/22/2025 12:11 PM

SHEET No.

DESCRIPTION OF BLDG. ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS 🛭 4" O.C.	(3) NAILS 💇 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS @ 8" O.C.	TOENAILS @ 6" o.c.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS ⊘ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" o.c.	NAILS @ 16" O.C.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP ② CORNERS \$ INTERSECTING WALLS	(2) NAILS	(2) NAILS

* 2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE * ARE SHOWN)

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS IOTED OTHERWISE ON PLAN.

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO
- FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

ROOF DEAD = 7 PSF T.C., 10 PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES)

(ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION, ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP, U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
 I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (I)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8' ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- 'LSL' Fb=2325 psi; Fv=3i0 psi; E=1.55xi0^6 psi
- 'LVL' Fb=2600 psi; Fv=265 psi; E=2.0x10^6 psi
- 'PSL' FB=2400 PSI; FV=240 PSI; E=2.0XIO^6 PSI M+K SHALL BE FILLY INDEMNIFIED FOR ANY AND ALL ISSUES
- RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THE OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
- FOR 2 \$ 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x31/5" SIMPSON SDS SCREWS (OR 3½" TRUSSLOK SCREWS) & 16" O/C. USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM FDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF k''"x6" SIMPSON SDS SCREMS (OR 6 $3\!\!\!/\!\!\!/$ " TRUSSLOK SCREMS) • 16" O/C. USE A MINIMUM OF 4 ROMS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE. A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MEK FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I 1/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 1 x 0.131" NAILS @ 6"04. @ PANEL EDGES & @ 12"04. FIELD.
- 2 🖣 × 0.120" NAILS 4" O.C. PANEL EDGES € 8" O.C. FIELD. - 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN, SCREMS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO:
 - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS w/ (3) 3"x0.131" NAILS • 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" × 0.131" NAILS 6"o.c. PANEL EDGES € 12" O.C. FIELD. - w/ 2 3 × 0.120" NAILS • 4"o.c. • PANEL EDGES € • 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"0.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.

VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size
3'-0"	20 FT. MAX	L3"x3"x/4"
6'-0"	3 FT, MAX	L3"x3"x/4"
	I2 FT. MAX	L4"x3"x1/4"
	20 FT. MAX	L5"x3½"x¾"
Ð'-O"	3 FT, MAX	L4"x4"x4" *
	I2 FT. MAX	L5"x3½"x%;"
	I6 FT. MAX	L6"x31/2"x7%"
4-6"	I2 FT. MAX	L6"x31/2"x3/6"
16'-0"	2 FT. MAX	L7"x4"x½" **
	3 FT. MAX	L8"x4"x½" **
	1.6	

SHALL SUPPORT 2 1%" - 3 ½" VENEER W/ 40 psf MAXIMUM WEIGHT < 16' SHALL HAVE 4" MIN. BEARING

- = 16' SHALL HAVE 8" MIN, BEARING (16' SHALL NOT BE FASTENED BACK TO HEADER.
- 16' SHALL BE EASTENED BACK TO WOOD HEADER IN WALL 448" W/S" DIA. x 3 S" LONG LAG SCRENS IN 2" LONG VERTICALLY SLOTTED HOLES.
- MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
 WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 ½" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT
- FINEHING.

 SEE STRICTURAL PLANS FOR ANY LINTEL CONDITION NOT BICOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL FASTENED BACK TO BEAM, FASTENERS SHALL MAINTAIN A 25' (MINIMAN) CLEAR DISTANCE FROM BOTTOM OF BEAM.
- FOR QUEEN VENEER USE L4x3x * FOR 3%" VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF

SD2. | REFERS TO SD2. | A FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RO

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301211) EXP. B. RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NOSEC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

EXT. WALL SHEATHING SPECIFICATION

- 1/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ・ALT. STAPLE CONNECTION SPEC: 1 名" 16 GA STAPLES (1/6" CROWN) • 3" O.C. AT EDGES \$ • 6" O.C IN FIELD.

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 3/8" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 3/4" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C IN FIELD, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN. IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- <u>PRE-MANUFACTURED PANELIZED WALLS:</u> FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWAL OR 3" O.C. OSB SHEARWALL.

► INDICATES HOLDOWN BELOW

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING
- I/2" DIA. ANCHOR BOLTS 6'-0" O.C, 7" MIN. EMBEDMENT
- (CONC), 15" MIN. EMBEDMENT (CMU)
 SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS @ 2'-8" O.C. (CMU) (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACI 318, CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: FOUNDATION WALLS 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE
- leq 000,00 BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS) - TALLER WALLS MUST BE ENGINEERED.
- NOMINAL WIDTH (91/5" FOR 10" THICK WALL).
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC. ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL WITH 2" CLEAR, REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.
- * FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 W (2)2x6 JACK STUDS, U.N.O.
- LARGER OPENINGS SHALL BE PER PLAN.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT
- ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP
- . JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR
- 15'-0" O.C. (MAXIMUM) JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS
- POSSIBLE (IJ RATIO) WITH A MAXIMUM OF IJ 5 RATIO • CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (F/m=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 € 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORGEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS. FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE * DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
- BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY

HOLD-DOWN SCHEDULE

5YMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM U.N.O.) -OR- MSTC66B3 ALTERNATE
► HD-3	SIMPSON STHDI4/STHDI4R I

***** UTILIZE THE SSTB24 ANCHOR BOLT ● ALL MONOSLAB & INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN
THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 15" (FOR 7/8" DIA.) MIN. EMBEDMENT INTO CONCRETE, INSTALL PER MANUF, INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D.

DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE

6/3/25 "H CAR NOFESSIO, Ö ENGINE SEPH T. R

N+KCC NEW ENNIN



I&K project numbe 126-2306

oject mgr: JTR rawn by: ssue date: 06-03-2

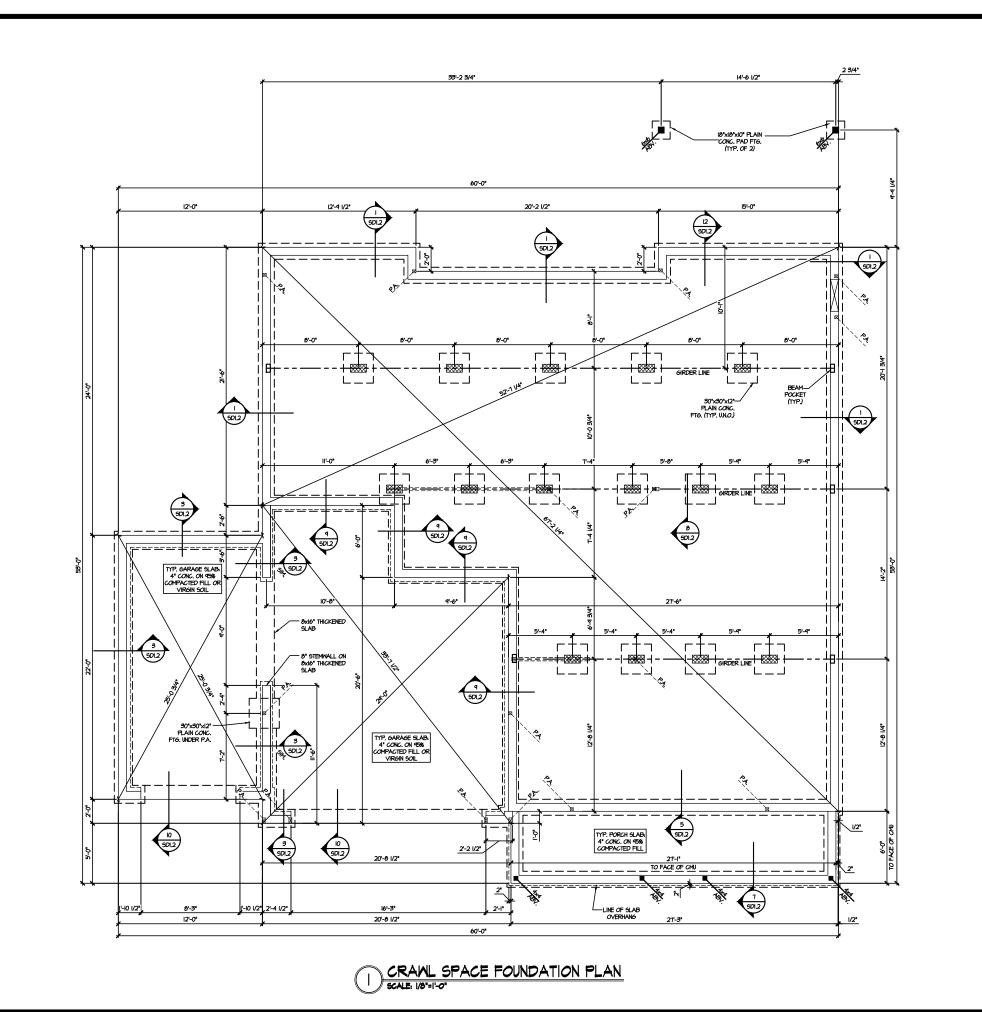
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COMMUNIT AVEN 1 $\sum_{i=1}^{n}$ POND CC Stonehave I. nc Щі

M F

BL/ LOT RAL



al: 6/3/25

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENSINEERI



M&K project number: 126-23061

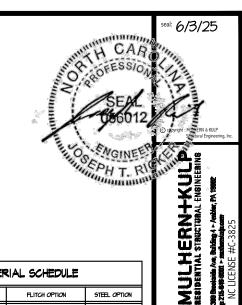
JTR drawn by: issue date: 06-03-25

BLAKE POND COMMUNITY Lot 77 - Stonehaven 1 raleigh, nc **OUNDATION PLANS**

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

- IIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- ---- BEAM / HEADER • = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE,
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

S1.0



ENGINEERED BEAM MATERIAL SCHEDULE LVL OPTION STEEL OPTION PSL OPTION LSL OPTION FLITCH OPTION (2)2xi0 + (1) ¼"x4¼" STEEL FLITCH PLATES - F 001 (2)13/4"x9 /4" - F 36'x9 &' - F M8xIO - F 002 (2)134"x18" - FB 3½"xi8" - FB WI2xI9 - F N/A (3)2xi2 + (2) ½"xil¼" STEEL FLITCH PLATES - F 002A 5¼"x16" - FB (2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATES - F 003 (2)13/4"x14" - F 3%"xl4" - F (2)|%"x|4" - 1 WI2xI4 - F (2)2x12 + (1) %"x114" STEEL FLITCH PLATES - F 004 (2)|%"x|4" - F (3)13/4"x14" - F WI2xI4 - F 3%"x|4" - F (4)2x12 + (3) %"x114" STEE FLITCH PLATES - D 005 (4)134"x18" - D ΝA 7"x18" - D WI2x30 - D 006 (2)1¾"x11¾" - H (3)134"x1136" - H N/A 3½"xII%" - H (2)2xl2 + (1) %"xll¼" STEEL FLITCH PLATES - H 001 (2)13/4"x14" - H 3½"x14" - H (3)13/4"x14" - H (3)2xi0 + (2) ¼"xi¼" STEEI FLITCH PLATES - F 008 (3)13/4"x14" - F 5¼"x|4" - F WI2xI4 - F (3)2xi2 + (2) ½"xil¼" STEEI FLITCH PLATES - H 009 (3)134"×18" - H 5¼"x16" - H NΑ N/A (2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - H 010 (2)134"x944" - H 3½"x9¼" - H (2)134"×94" - H M8xIO - H (3)2x10 + (2) ¼"x11¼" STEEL FLITCH PLATES - F OII (3)134"×944" - F (3)134"×94" -M8xIO - F (4)2xi2 + (3) ¼"xil¼" Steel Flitch Plates - D 012 (4)134"x1136" - D T"xII%" - D N/A MI2xI4 - D (3)2x12 + (2) ½"x1¼" STEEL FLITCH PLATES - F (3)134"x16" - FT 5¼"xl6" - FT ΝA W12x19 - F (2)2xl2 + (l) %"xl以" STEEL FLITCH PLATES - F 014 (2)134"x14" - F (3)13/4"x14" - F WI2xI4 - F 3½"xl4" - F

- BEAM NOTATION: "F" INDICATES FLUSH BEAM "FT" INDICATES FLUSH TOP BEAM
- "FB" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM
 "H" INDICATES DROPPED OPENING HEADER
- REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
 REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
 FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN
- PLATES IN SUCCESSION W/ (2) 3"X0.120" NAILS @ 8" Q.C.

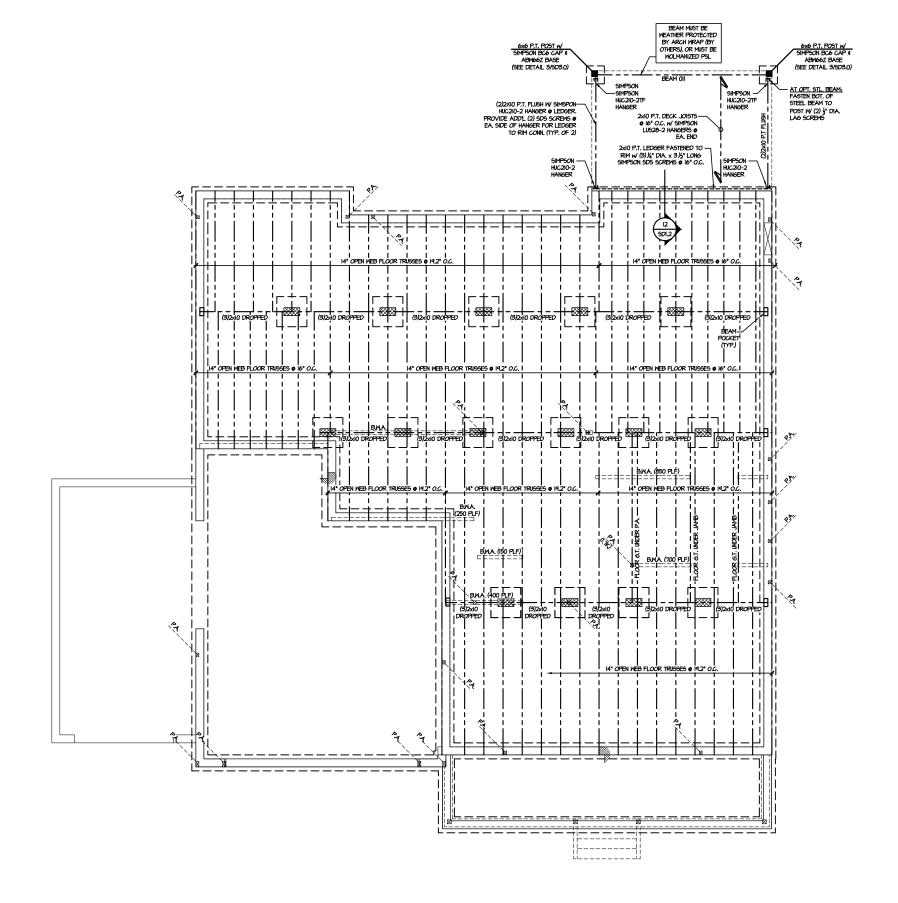
FOR FLUSH BOTTOM BEAMS PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W (2) 3'X0.120" NAILS @ 8" O.C.

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- ■ INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



IST FLOOR FRAMING PLAN

l&K project number:

FVISIONS

126-23061

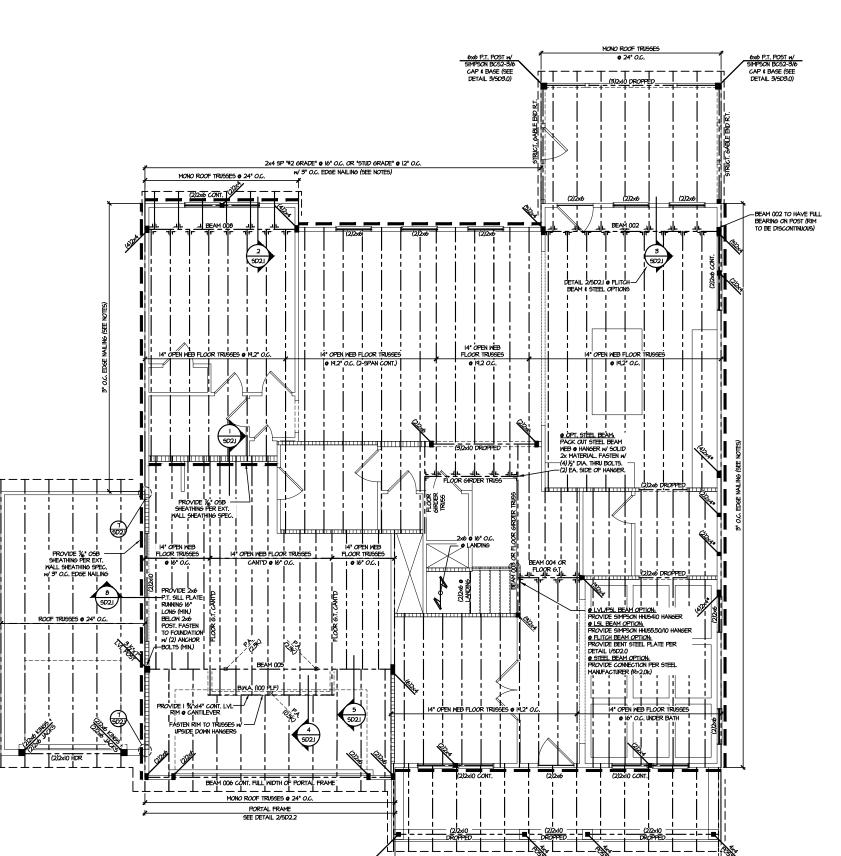
ssue date: 06-03-2

JTR

initial:

ANS

BLAKE POND COMMUNITY Lot 77 - Stonehaven 1 raleigh, nc OR



ENGINEERED BEAM MATERIAL SCHEDULE STEEL OPTION LVL OPTION PSL OPTION LSL OPTION FLITCH OPTION (2)2x10 + (1) ¼"x9¼" STEEL FLITCH PLATES - F 001 (2)|%"x9 ½" - I 36"x4 6" - F W∂xIO - F (2)2xi2 + (i) i"xiik;" steel Flitch plates - f 002 (2)1¾"x18" - FB W12x19 - F 3½"x18" - FB N/A 002A (3)13/4"x16" - FB N/A WI2x19 - F 5¼"x16" - FB (2)2xl2 + (I) 从"xli以" STEEL FLITCH PLATES - F 003 (2)13/4"x14" - F 3%"xl4" - F (2)13/4"x14" - F WI2xI4 - F 2)2x12 + (I) %"xIK" STEEI FLITCH PLATES - F 004 (2)13/4"x14" - F 3%"xl4" - F (3)13/4"x14" - F WI2xI4 - F 4)2xl2 + (3) %"xll4" STEEL FLITCH PLATES - D (4)134"x18" - D NΑ (2)1¾"×11%" - H (3)134"x1136" - H NΑ NΑ 3½"xII%" - H (2)2x12 + (1) %"x114" STEE1 FLITCH PLATES - H 001 (2)13/4"x14" - H (3)13/4"x14" - H NΑ (3)2x10 + (2) ¼"xII¼" STEEL FLITCH PLATES - F 008 (3)13/4"×14" - F WI2xI4 - F 5½"x|4" - F (3)2x12 + (2) ½"x1½" STEEL FLITCH PLATES - H (3)13/4"×18" - H N/A NΑ 5¼"xl8" - H (2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - H 010 (2)134"×94" - H (3)2xi0 + (2) ¼"xil¼" STEEL FLITCH PLATES - F OII (3)134"x94" - F 54"x94" - F (3)13/4"×91/4" - F WBxIO - F 4)2x12 + (3) ¼"xII¼" STEEL FLITCH PLATES - D 012 (4)|%"\|I%" - D 7"x11%" - D N/A WI2xI4 - D (3)2xi2 + (2) ½"xil½" STEEL FLITCH PLATES - F (3)13/4"x16" - FT 5¼"xl6" - FT N/A WI2xI9 - F 014 (2)13/4"×14" - F (3)134"x14" - F WI2xI4 - F

BEAM NOTATION: - "F" INDICATES FLUSH BEAM - "FT" INDICATES FLUSH TOP BEAM

- "FB" INDICATES FLUSH BOTTOM BEAM

- "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER

REFER TO DETAIL DISD20 FOR TYPICAL FLITCH BEAM CONNECTIONS

REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN

PLATES IN SUCCESSION W/ (2) 3"X0 120" NAILS @ 8" O.C.

FOR FLUSH BOTTOM BEAMS PROVIDE 2X STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W (2) 3"X0,120" NAILS & 8" O.C.

SD2, I REFERS TO SD2, IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- ■ INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING

JL METAL HANGER

BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

TYPICAL STRUCTURAL NOTES & SCHEDULES

REFER TO SO.O FOR

4x4 P.T. POST w/ SIMPSON BC52-2/4 CAP & ABW44Z BASE (SEE DETAIL 3/SD3.0) 2ND FLOOR FRAMING PLAN

MONO ROOF TRUSSES @ 24" O.C.

2x4 SP "#2 GRADE" @ 16" O.C. OR "STUD GRADE" @ 12" O.C.

SEPH T. R MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERI

6/3/25

TH CAR

1&K project number: 126-23061

JTR rawn by: ssue date: 06-03-2

FVISIONS initial:

ANS

BLAKE POND COMMUNITY Lot 77 - Stonehaven 1 raleigh, nc OR

S3.0

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number: 126-23061

JTR drawn by: issue date: 06-03-25

ROOF FRAMING PLANS BLAKE POND COMMUNITY LOT 77 - STONEHAVEN 1 RALEIGH, NC

LEGEND

 INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE

• — BEAM / HEADER

• == INDICATES SHEAR WALL & EXTENT

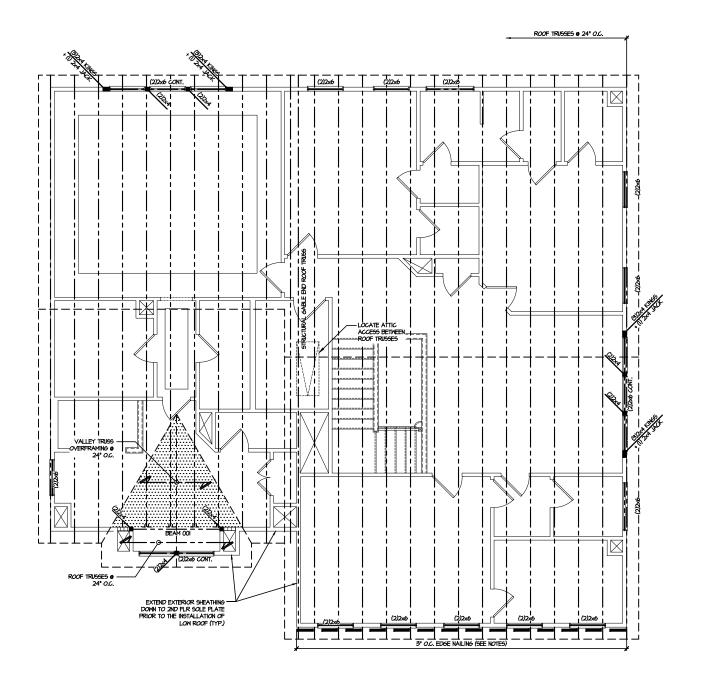
EXTENT OF OVERFRAMING

JL METAL HANGER

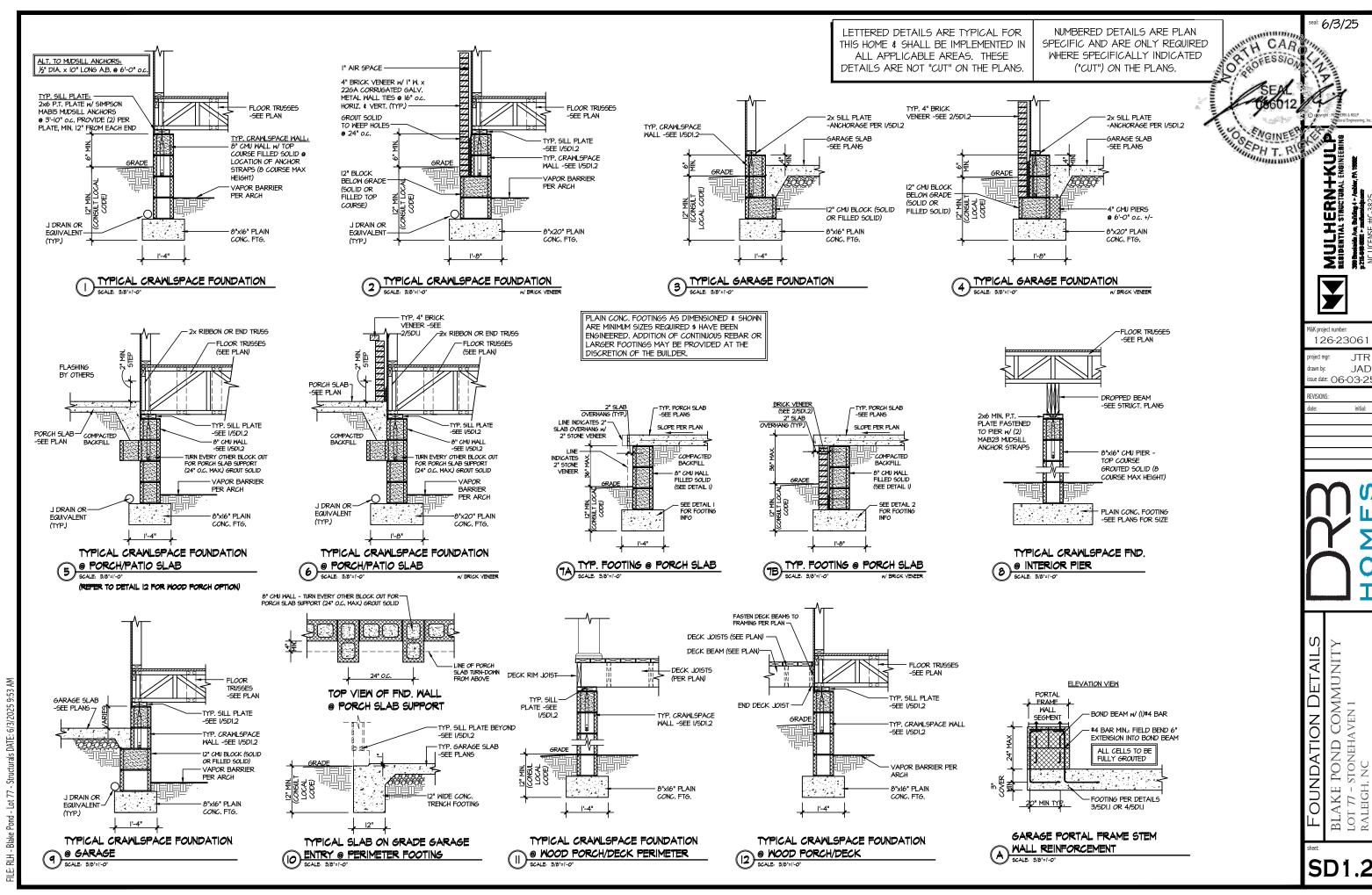
* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

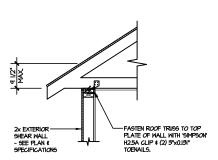
REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



ROOF FRAMING PLAN SCALE: 1/8"=1'-0"



initial:



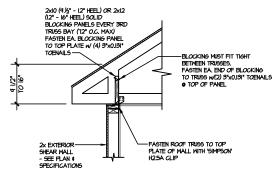
TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

SCALE: 3/8'=1'-0' HEEL HEIGHT LESS THAN HEEL HEIGHT LESS THAN 9½" NO BLOCKING REQD

TYPICAL SHEAR TRANSFER DETAIL

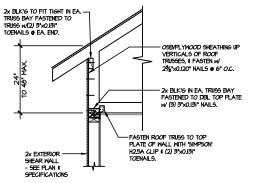
BETWEEN FLOORS @ EXTERIOR WALL
SCALE 500'-11-0' PARALLE FRANCE



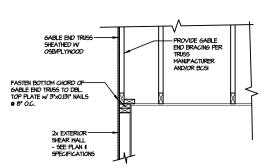
TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

SCALE: 9/8"=1"-0" HEEL HEIGHT BETWEEN 9. HEEL HEIGHT BETWEEN 9½" - 16" BLOCKING REQ'D



TYPICAL SHEAR TRANSFER DETAIL @ RAISED HEEL TRUSS



TYPICAL GABLE END DETAIL SOLLE SIDE STATE

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERINS

M&K project number 126-23061

6/3/25

TH CAR

SEPH T. R

JTR drawn by: JAD issue date: 06-03-2

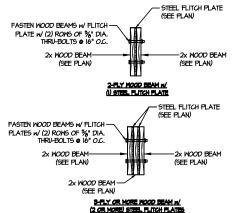
REVISIONS

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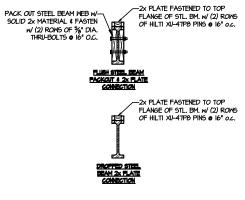
BLAKE POND COMMUNIT Lot 77 - Stonehaven 1 Raleigh, nc DETAILS Ŋ

- 2x EXT, WALL -2x EXT. WALL LADDER TRUSS W/ VERTICALS @ 24" o.c. (MAX) - FASTEN EXTERIOR SHEATHING FASTEN SOLE PLATE TO LADDER TRUSS W/ 3"x0.131" NAILS @ 6" O.C. OR 3"x0.120" NAILS @ 4" O.C FASTEN SOLE PLATE TO SHEATHING SPLICE SOLE PLATE OR BOTTOM OF SOLE RIBBON W/ 3"x0.131" NAILS @ 6" O.C. OR 3"x0.120" NAILS @ 4" O.C TO TOP & BOTTOM CHORDS WITH 23/2"x0.113" NAILS @ 6" o.c. SPLICE EXTERIOR WALL SHEATHING OVER PLATE AS FLOOR TRUSSES -FLOOR TRUSSES (SEE PLAN) (SEE PLAN) 2x EXT. WALL (SEE PLAN) (SEE PLAN)

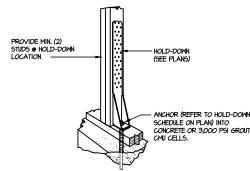
> TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL
> SCALE 305-1-07 FEREDICLAR FRAS



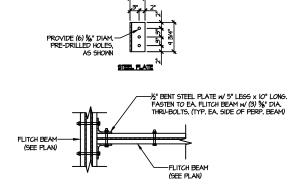




TYPICAL STEEL BEAM CONNECTION DETAIL



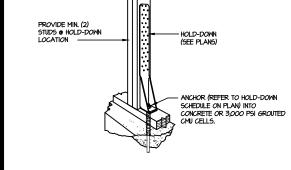
TYPICAL HOLD DOWN INSTALLATION SCALE: NT.5.



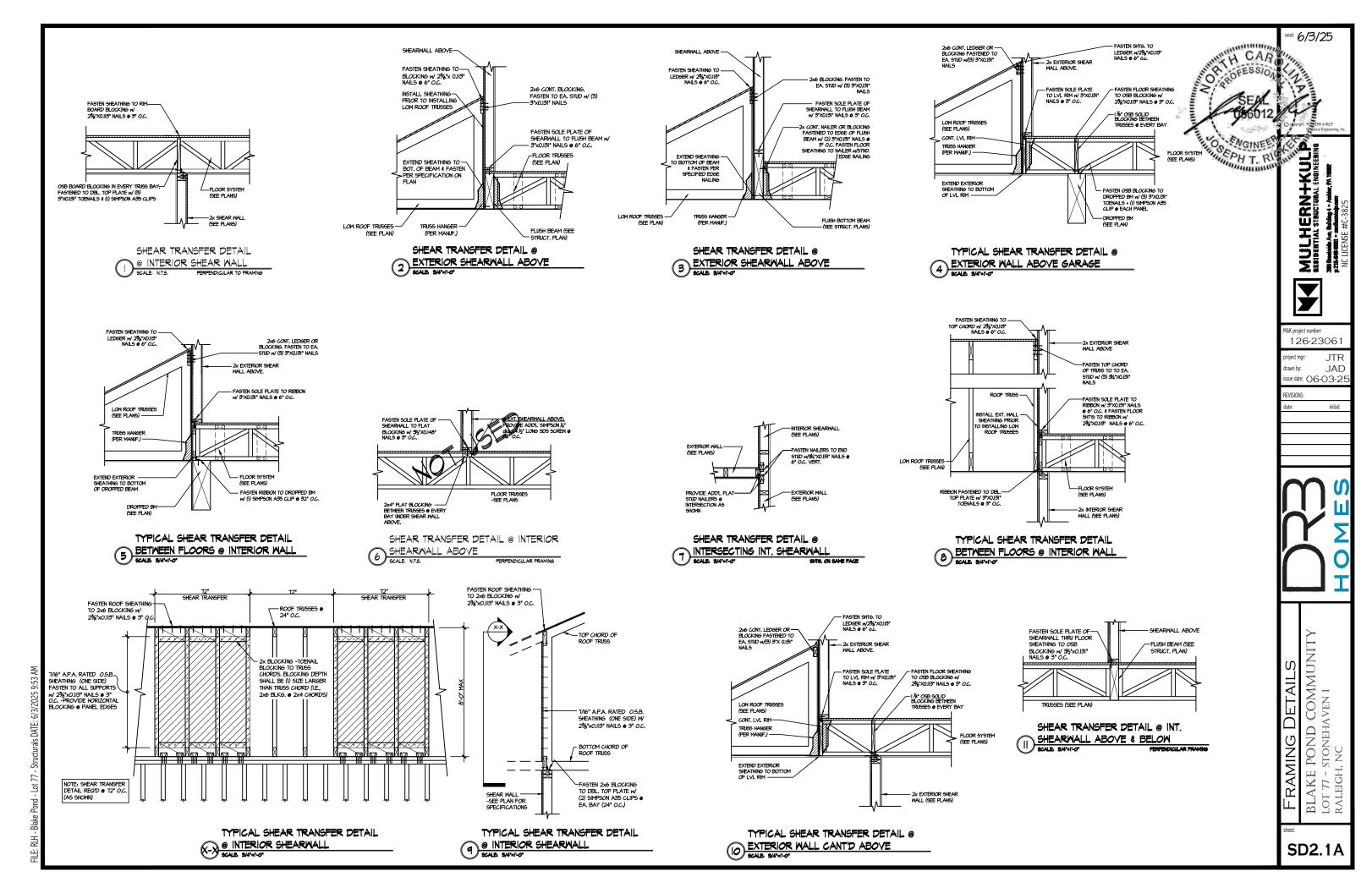
TYPICAL FLITCH BEAM TO FLITCH BEAM CONNECTION DETAIL

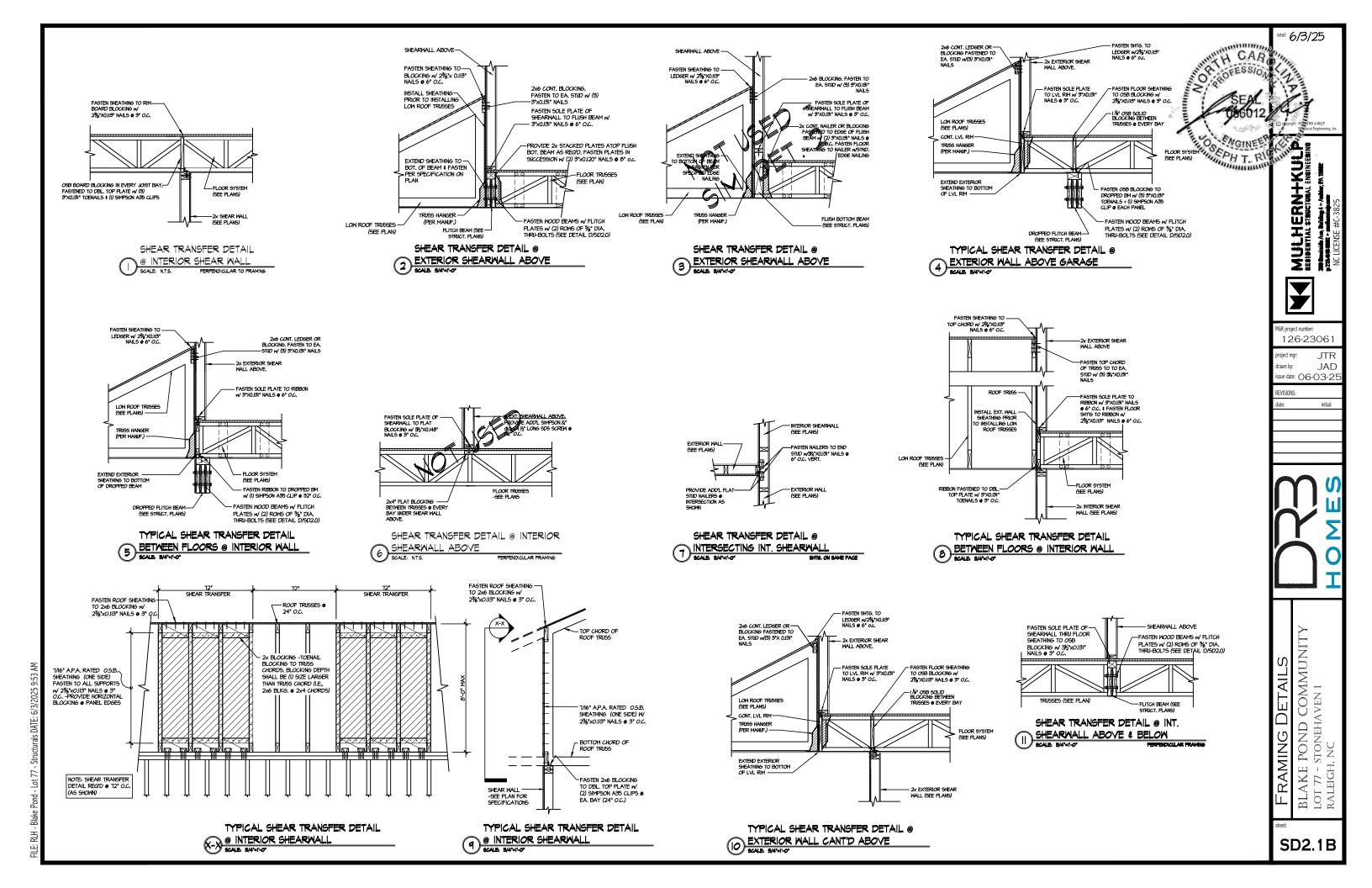
NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

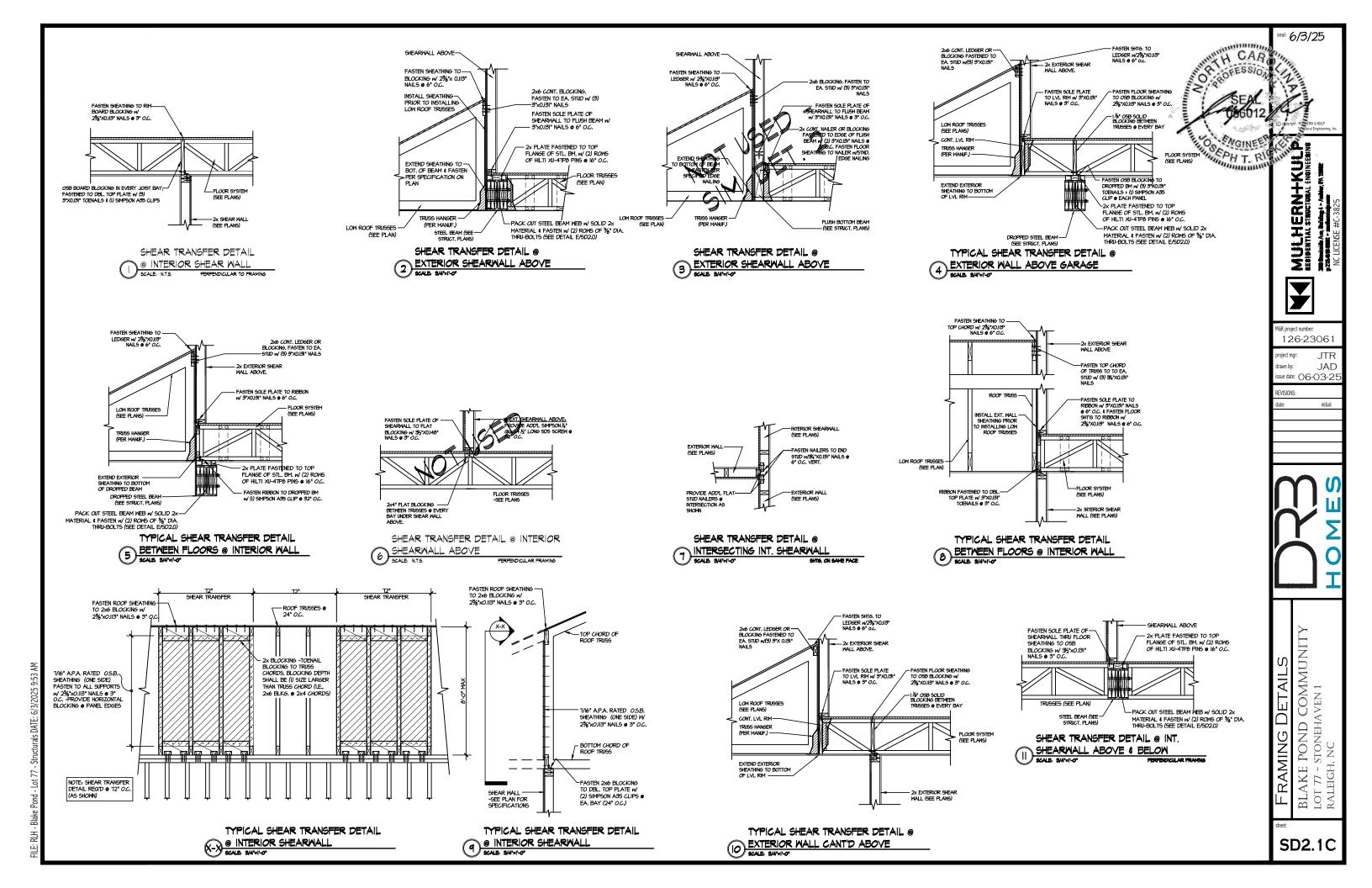
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

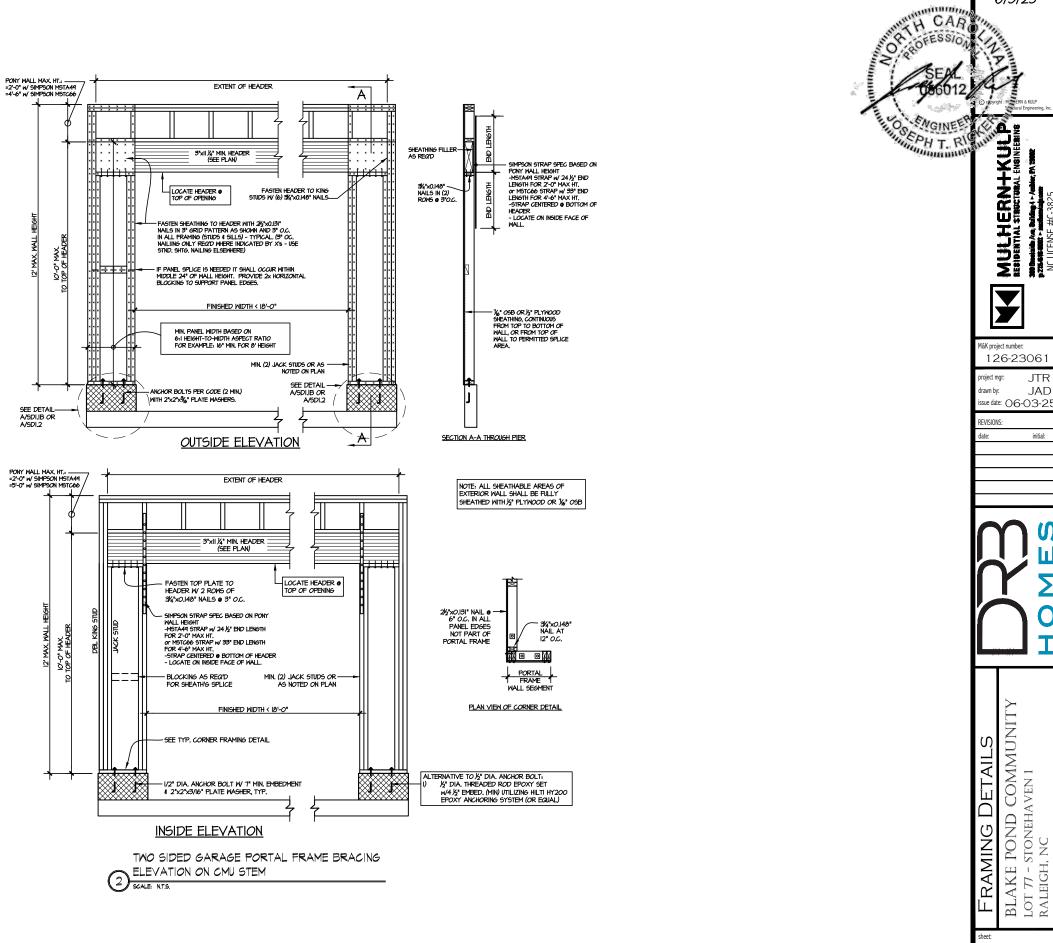


SPLICE EXTERIOR WALL SHEATHING OVER PLATE AS SHOWN









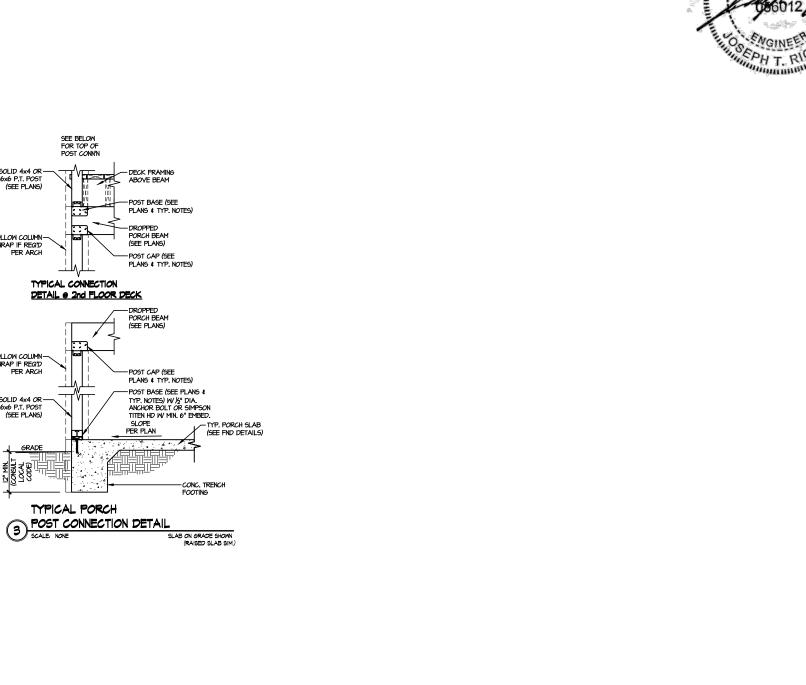
SEE DETAIL— A/SDI.IB OR A/SDI.2

al: 6/3/25

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initial:



SOLID 4x4 OR -6x6 P.T. POST (SEE PLANS)

HOLLOW COLUMN-WRAP IF REQ'D PER ARCH

HOLLOW COLUMN — WRAP IF REQ'D PER ARCH

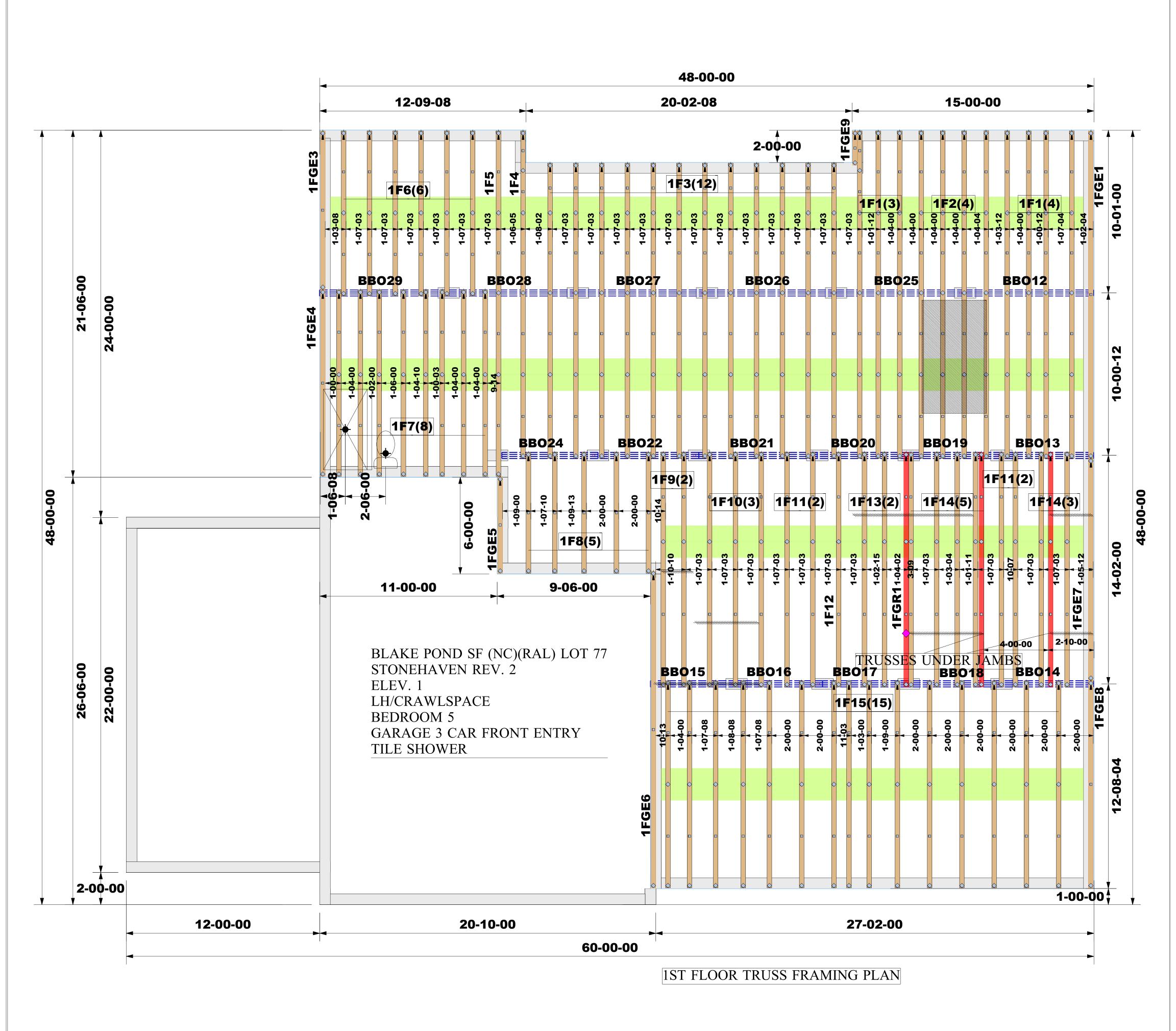
SOLID 4x4 OR -6x6 P.T. POST (SEE PLANS)

al: 6/3/25 MULHERN+KUL RESIDENTIAL STRUCTURAL ENSINEERI

M&K project number: 126-23061

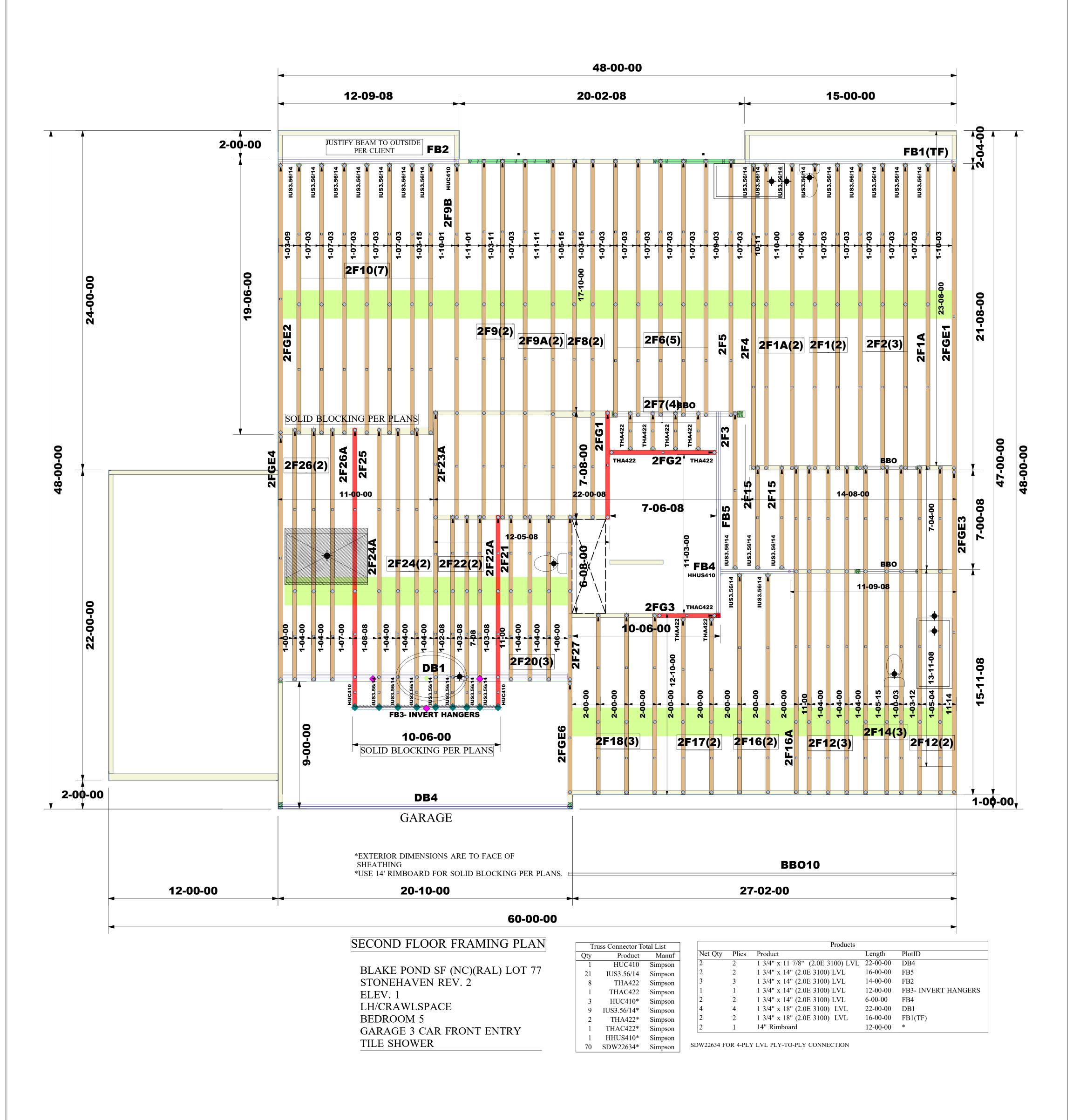
JTR drawn by: JAD issue date: 06-03-2

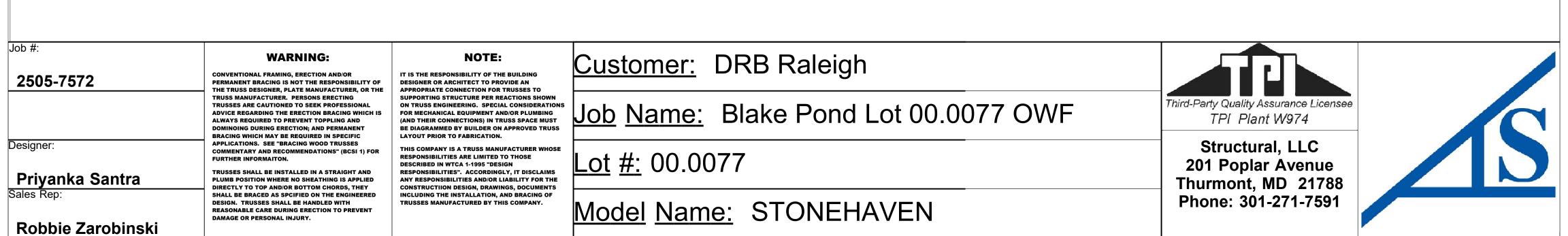
BLAKE POND COMMUNITY Lot 77 - Stonehaven 1 Raleigh, nc

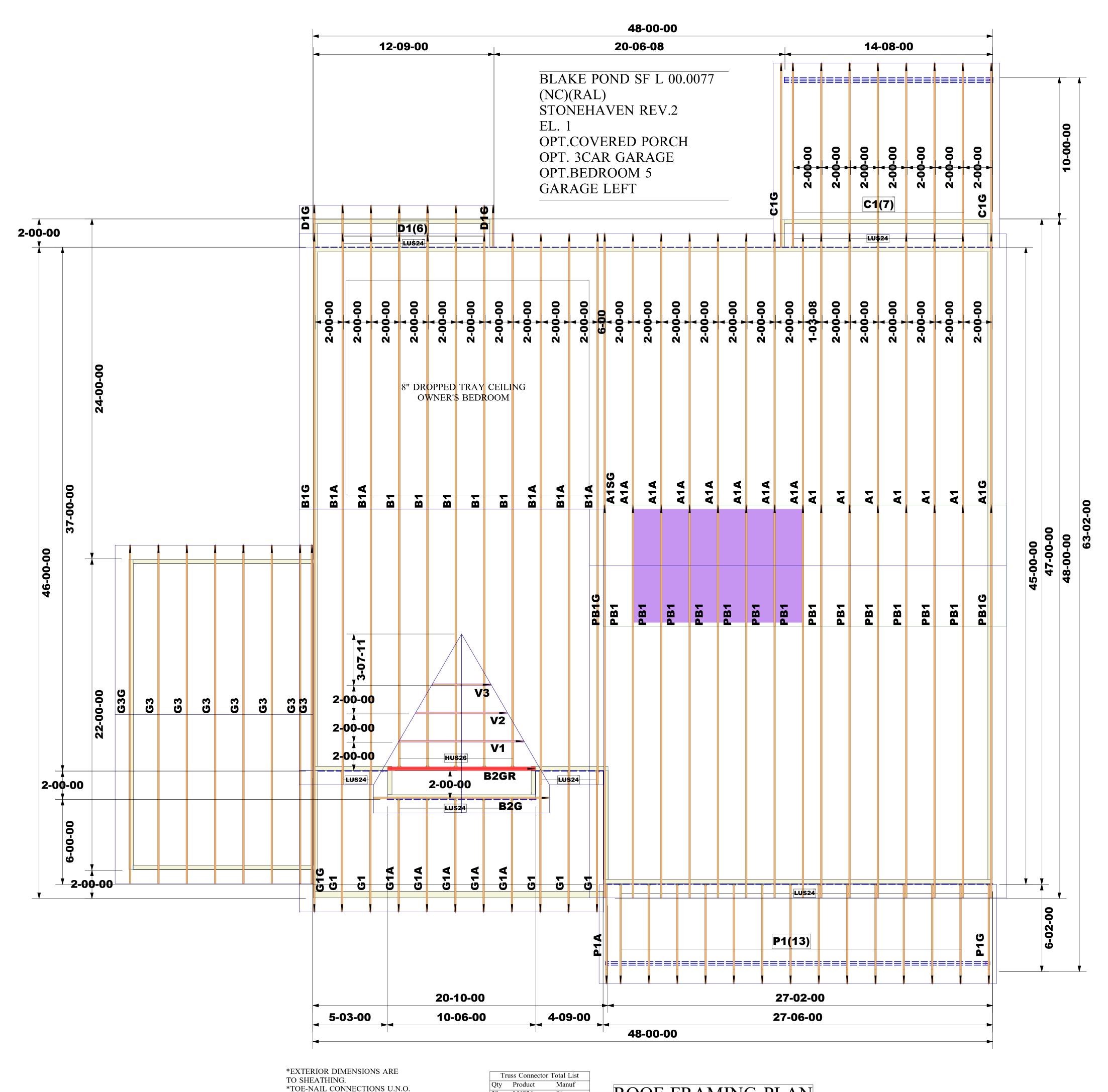


Job #: **WARNING: NOTE:** Customer: DRB Raleigh IT IS THE RESPONSIBILITY OF THE BUILDING CONVENTIONAL FRAMING, ERECTION AND/OR 2505-7572 PERMANENT BRACING IS NOT THE RESPONSIBILITY OF DESIGNER OR ARCHITECT TO PROVIDE AN THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN TRUSS MANUFACTURER. PERSONS ERECTING Job Name: Blake Pond Lot 00.0077 OWF TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS ADVICE REGARDING THE ERECTION BRACING WHICH IS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING ALWAYS REQUIRED TO PREVENT TOPPLING AND (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST **DOMINOING DURING ERECTION; AND PERMANENT** BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS **BRACING WHICH MAY BE REQUIRED IN SPECIFIC** LAYOUT PRIOR TO FABRICATION. APPLICATIONS. SEE "BRACING WOOD TRUSSES Designer: THIS COMPANY IS A TRUSS MANUFACTURER WHOSE COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR <u>ot #:</u> 00.0077 RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND Priyanka Santra Sales Rep: PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED INCLUDING THE INSTALLATION, AND BRACING OF DESIGN. TRUSSES SHALL BE HANDLED WITH TRUSSES MANUFACTURED BY THIS COMPANY. Model Name: STONEHAVEN REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY. Robbie Zarobinski









*TRUSSES @ 2' O/C U.N.O. *SEE PROFILE DWGS. FOR TRUSS ORIENTATION BEFORE INSTALLATION. *INSTALL SIMPSON H2.5A HURRICANE ANCHORS AT EACH BEARING POINT

LUS24 Simpson HUS26 Simpson 115 One H2.5A Simpson

ROOF FRAMING PLAN

Job #: 2505-7573	WARNING: CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF	NOTE: IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN	Customer: DRB Raleigh		
	THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	Job Name: Blake Pond Lot 00.0077 Roof	Third-Party Quality Assurance Licensee TPI Plant W974	1
Designer: Priyanka	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON. TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS	<u>Lot #:</u> 00.0077	Structural, LLC 201 Poplar Avenue Thurmont, MD 21788	
Sales Rep: Robbie Zarobinski	SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED	INCLUDING THE INSTALLATION, AND BRACING OF	Model Name: STONEHAVEN	Phone: 301-271-7591	