

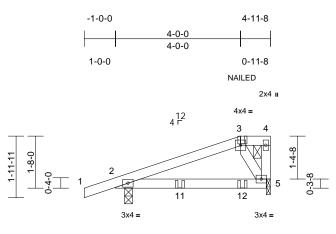
RE: M	00210-В -	Arlington Re	v.1-Elev	4-Roc	of			Trenco 818 Soundside Rd	
	ck:		ıh Proje			Raleigh Mode n: DRB Raleig		Edenton, NC 27932	
Addres	is:			0					
City: Genera	l Truss Er	ngineering C	riteria &		ate: NC n Loads ((Individual Tr	uss Design		
Drawin	gs Show S	Special Load	ing Cond		5):	-	-		
Wind Co Wind Sp	Code: IR(ode: ASCE beed: 120 n ad: 40.0 ps	nph	14		Ι	Design Program Design Method Floor Load: N/	: MWFRS (E	20 25.2 nvelope)/C-C hybrid	Wind ASCE 7-16
	oof Height					Exposure Categ	gory: B		
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	e Date		
123456789011234 11234 11234 11234 1678901223 226789 222222	173898299 173898300 173898300 173898302 173898303 173898304 173898306 173898306 173898307 173898307 173898307 173898310 173898311 173898312 173898313 173898314 173898314 173898315 173898315 173898316 173898316 173898319 173898319 173898320 173898323	M1 B1G B1 A1G A1 A1A	6/3/25 6/3/25	35 36 37 38 39 40 41 43 44 45	173898333 173898334 173898336 173898335 173898336 173898340 173898340 173898342 173898342	6 A2ST 7 B1T 3 B1A 9 S1GT 0 S1G 1 S1	6/3/25 6/3/25 6/3/25 6/3/25 6/3/25 6/3/25 6/3/25 6/3/25 6/3/25 6/3/25		
34	173898332		6/3/25	1					
Truss E provide	ngineering d by Structu		lirect supe	ervision		the parameters	NIN OFF	CAROL	
					olina is De	cember 31, 202	5. 2	T.	
IMPOR that the of designs shown (e given to TRENCO preparat applicab the build	TANT NO engineer name comply with Al e.g., loads, sup MiTek or TRE D's customers ion of these de ility of the desi ing designer s	TE: The seal on the seal on the seal is licensed in the NSI/TPI 1. These sports, dimensions NCO. Any projec file reference purpasigns. MiTek or T gn parameters or thould verify applic gns into the overal	these truss of e jurisdiction designs are , shapes and t specific info ose only, an RENCO has the designs ability of des	compone h(s) identi based u d design ormation d was no s not inde for any p sign para	nt designs is ified and that pon paramet codes), whic included is fo taken into a ependently vo articular build meters and p	a certification the ers h were or MiTek's or account in the erified the ding. Before use, properly		GILBERT	2
moorpon				Sign per	1 of 1		Cilbert Eric		June 3,2025

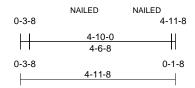
Gilbert, Eric

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P1G	Half Hip Girder	2	1	I7 Job Reference (optional)	3898299

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:41 ID:1g9wQ0DaF_KqUhCtBYUW_MzBxin-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:36.9

Scale = 1.30.9													
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2021/	/TPI2014	CSI TC BC WB Matrix-MP	0.40 0.48 0.05	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.03 -0.07 0.00 0.03	(loc) 5-10 5-10 2 5-10	l/defl >999 >872 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 20 lb	GRIP 244/190 FT = 20%
	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she 4-11-8 oc purlins, e 2-0-0 oc purlins: 3-4 Rigid ceiling directly bracing. (size) 2=0-3-0, § Max Horiz 2=39 (LC Max Uplift 2=-71 (LC Max Grav 2=455 (LC	xcept end verticals, ; applied or 10-0-0 oc 5=0-1-8 11) 2 8), 5=-49 (LC 9)	8) 9) ed or and 5, 10) 11) 12)	about its cen This truss ha chord live los * This truss h on the bottor 3-06-00 tall t chord and ar Bearing at jo using ANSI/7 designer sho Provide mcc bearing plate One H2.5A S	as been designed for ad nonconcurrent v has been designed in chord in all areas by 2-00-00 wide wil hy other members. int(s) 5 considers p IPI 1 angle to grain juld verify capacity hanical connection	or a 10. vith any for a liv s where I fit betw parallel of bear (by oth e conne	D psf bottom other live loa: e load of 20.0 a rectangle veen the botto o grain value a. Building ng surface. ers) of truss to ctors	ds.)psf om o					
FORCES	(lb) - Maximum Com Tension			UPLIFT at jt(and does no	(s) 2 and 5. This co t consider lateral fo	nnectio rces.	n is for uplift o	only					
BOT CHORD WEBS	1-2=0/26, 2-3=-148/ 2-5=-139/116 3-5=-228/33	120, 3-4=-13/9, 4-5=	,		Irlin representation ation of the purlin a d.			size					
NOTES			14)	"NAILED" ind	dicates 3-10d (0.14	8"x3") d	or 3-12d						
 Unbalance this design Wind: ASC Vasd=95m II; Exp B; I and right e 	ed roof live loads have CE 7-16; Vult=120mph .ph; TCDL=6.0psf; BC Enclosed; MWFRS (er exposed ; end vertical l and right exposed; Lur	(3-second gust) DL=6.0psf; h=25ft; C avelope); cantilever le left and right expose	15) Cat. LO eft 1) d;	(0.148"x3.25 In the LOAD of the truss a AD CASE(S)	") toe-nails per ND CASE(S) section, are noted as front (Standard bw (balanced): Lun .15	S guidli loads a F) or ba	nes. oplied to the f ck (B).			2	- AL	ORTH CA	ROUN

Vert: 1-3=-51, 3-4=-61, 5-6=-20

Vert: 3=-72 (B), 11=-143 (B), 12=-37 (B)

Concentrated Loads (lb)

- porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially
- Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.



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mmm June 3,2025

SEAL

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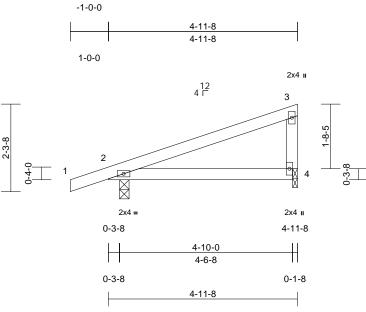
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road Edenton, NC 27932

VIIIIIIII

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P1	Monopitch	18	1	Job Reference (optional)	173898300

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:41 ID:s52cFuV0q2EZIETI1tPvPDzBxiQ-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:30.2

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.31	Vert(LL)	-0.02	4-9	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.21	Vert(CT)	-0.04	4-9	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.03	4-9	>999	240		
BCDL	10.0										Weight: 19 lb	FT = 20%
LUMBER	2x4 SP No.2			nas been designe oad nonconcurrei								

TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
WEBS	2x4 SP N	0.3
BRACING		
TOP CHORD		l wood sheathing directly applied, nd verticals.
BOT CHORD		ing directly applied.
REACTIONS	(size)	2=0-3-0, 4=0-1-8
	Max Horiz	2=46 (LC 15)
	Max Uplift	2=-54 (LC 12), 4=-27 (LC 12)
	Max Grav	2=329 (LC 23), 4=199 (LC 23)

- FORCES (Ib) Maximum Compression/Maximum Tension
- TOP CHORD
 1-2=0/26, 2-3=-75/126, 3-4=-142/101

 BOT CHORD
 2-4=-140/109

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 4-9-12 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.

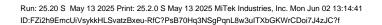
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

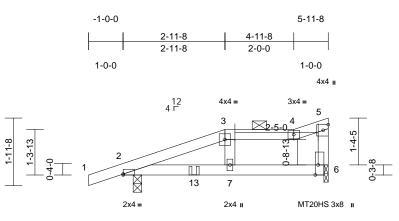
LOAD CASE(S) Standard

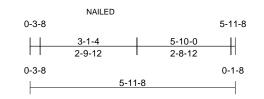


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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P1A	Roof Special Girder	2	1	Job Reference (optional)	173898301







Scale = 1:33.5

Plate Offsets (X, Y): [2:Edge,0-0-5], [6:0-3-8,Edge]

	(, 1). [Z.Euge,0 0 0],		-										
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2021/T	TPI2014	CSI TC BC WB Matrix-MP	0.72 0.46 0.02	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.03 -0.05 0.00 0.02	(loc) 7-12 7-12 6 7-12	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240		GRIP 244/190 187/143 FT = 20%
BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalancee this design. 2) Wind: ASC Vasd=95m II; Exp B; E and right ex porch left a grip DOL=1 3) TCLL: ASC Plate DOL=1 1.15 Plate I Exp.; Ce=1	5-11-8 oc purlins, e: 2-0-0 oc purlins; 3-4 Rigid ceiling directly bracing. (size) 2=0-3-0, 6 Max Horiz 2=37 (LC Max Uplift 2=-75 (LC Max Grav 2=453 (LC (Ib) - Maximum Com Tension 1-2=0/26, 2-3=-283/ 4-5=-209/40, 5-6=-1: 2-7=-139/251, 6-7=-: 3-7=-6/57 d roof live loads have E 7-16; Vult=120mph ph; TCDL=6.0psf; BC inclosed; MWFRS (en chosed; curtical 1 und right exposed; Lun 1.60 E 7-16; Pr=20.0 psf (=1.15); Pg=20.0 psf (=1.15); Pg=20.0 psf (=1.15); log=20.0 psf (=1.15); log=1.0; ct=1.10,	applied or 10-0-0 oc S=0-1-8 11) : 8), 6=-40 (LC 8) : 38), 6=252 (LC 2) pression/Maximum 125, 3-4=-250/43, 40/22 37/250 been considered for (3-second gust) DL=6.0psf; h=25ft; Ca welope); cantilever lef eft and right exposed; nber DOL=1.60 plate roof LL: Lum DOL=1.1 Y=20.4 psf (Lum DOL	or 8) F (d) 9) - (d) 9)	load of 12.0 p overhangs no Provide adeco Plates check about its cen This truss ha chord live loa "This truss ha chord live loa "This truss ha on the botton 3-06-00 tall b bearing plate One H2.5A S recommende UPLIFT at jt(and does not One H2.5A S recommende UPLIFT at jt(and does not (0.148"x3.25 In the LOAD of the truss a D CASE(S) Dead + Snc Increase=1. Uniform Loa Vert: 1-3=	s been designed i d nonconcurrent as been designed n chord in all area y 2-00-00 wide w y other members int(s) 6 considers PI 1 angle to grai uld verify capacity nanical connection at joint(s) 6. isimpson Strong-Ti d to connect truss s) 2 and 6. This c consider lateral f rlin representation d to connect truss s) 2 and 6. This c consider lateral f rlin representation ticon of the purlin a licates 3-10d (0.1- ') toe-nails per NU CASE(S) section, re noted as front Standard w (balanced): Lun 15 ads (lb/ft) =-51, 3-4=-61, 4-5 ed Loads (lb)	lat roof lo n other lin prevent v sess other inus 5 do for a 10.0 with any d for a 110.0 with any d for a 110.0 with any d for a 110.0 with any for a 10.0 with any parallel 1 n formula y of bear n formula y of bear n (by oth ie conne- s to bear onces. n does n d does n along the 48"x3") o DS guidli , loads a (F) or ba	bad of 15.4 p ve loads. water pondin wise indicate agree rotation D psf bottom other live load e load of 20. a rectangle veen the bott o grain value a. Building ng surface. ers) of truss ctors ing walls due n is for uplift bt depict the se top and/or or 3-12d nes. oplied to the ck (B). rease=1.15,	esf on g. ed. n ads. Opsf om to to e to only size face				SEA 0363	EER. K

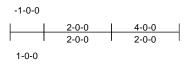
June 3,2025

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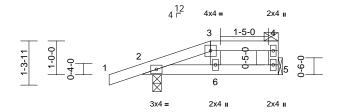
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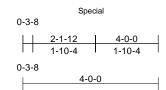
Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P2G	Half Hip Girder	2	1	I738983 Job Reference (optional))2

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:41 ID:Z7zPNcAeVZV1AUdM9UyLVDzBxk7-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Special





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Scale = 1:34.1												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2021/TPI2014	CSI TC BC WB Matrix-MP	0.14 0.30 0.03	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.03 0.00 0.02	(loc) 6 6 2 6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 15 lb	GRIP 244/190 FT = 20%
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E and right e porch left a grip DOL= 3) TCLL: ASC Plate DOL= 1.15 Plate Exp; Ce=1 4) Unbalance design. 5) This truss I load of 12. overhangs	4-0-0 oc purlins, exit 2-0-0 oc purlins; 3-4 Rigid ceiling directly bracing. (size) 2=0-3-0, 5 Max Horiz 2=22 (LC Max Uplift 2=-48 (LC Max Grav 2=262 (LC (Ib) - Maximum Com Tension 1-2=0/26, 2-3=-66/9, 2-6=-14/50, 5-6=-7/5 3-6=-134/22 d roof live loads have E 7-16; Vult=120mph ph; TCDL=6.0psf; BC Enclosed; MWFRS (er xposed; end vertical I and right exposed; Lur 1.60 E 7-16; Pr=20.0 psf; (=1.15); Pg=20.0 psf; (=1.15); Pg=20.0 psf; (=1.15); Ibg=1.0; fl. .0; Cs=1.00; Ct=1.10, d snow loads have be has been designed for 0 psf or 2.00 times flat non-concurrent with c	applied or 10-0-0 oc 5= Mechanical 11) 8), 5=-24 (LC 9) 34), 5=163 (LC 33) pression/Maximum 3-4=-7/5, 4-5=-75/9 been considered for (3-second gust) DL=6.0psf, h=25ft; C welope); cantilever le eft and right exposed her DOL=1.60 plate roof LL: Lum DOL=1. 2f=20.4 psf (Lum DOL Rough Cat B; Partially Lu=50-0-0 ten considered for this rigreater of min roof lit roof load of 15.4 psf other live loads.	about its ce 8) This truss f chord live live 9) * This truss on the botto 3-06-00 tall chord and a 10) Refer to gir 11) Provide me bearing pla 5. 12) One H2.5A recommend UPLIFT at does not co 13) Graphical p or the orient bottom cho 14) Hanger(s) of provided su down and 2 down and 2	as been designed bad nonconcurrent has been designed by 2-00-00 wide w any other members der(s) for truss to t chanical connectio te capable of withs Simpson Strong-T led to connect trus: t(s) 2. This connec unsider lateral force urlin representation tation of the purlin rd. or other connection tation of support (2 lb up at 2-0-0 or (4 lb up at 2-0-0 or (4 lb up at 2-0-0 or (4 lb up at 2-0-0 or (5 CASE(S) section are noted as front) Standard now (balanced): Lu 1.15 bads (lb/ft) 3=-51, 3-4=-61, 5-7 ted Loads (lb)	for a 10.1 with any d for a liv is where ill fit betw - rruss con n (by oth tanding 2 ie conne s to bear tion is for s. n does no along the device(s concentra n top cho n bottom ection de , loads a (F) or ba	0 psf bottom other live load re load of 20.1 a rectangle veen the bott nections. ers) of truss t 24 lb uplift at j ctors ing walls due r uplift only ar ot depict the s e top and/or s) shall be ated load(s) 1 rd, and 40 lb chord. The vice(s) is the pplied to the s	ads. Opsf om to joint to nd size 8 lb face			22	SEA 0363	L 22 EERER LIN
6) Provide ad	equate drainage to pr	event water ponding.										1111

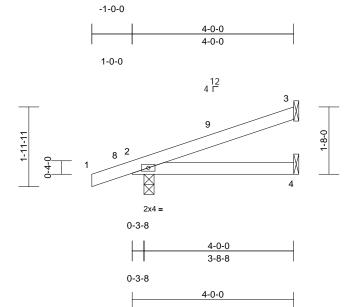
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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P2A	Jack-Open	2	1	I738 Job Reference (optional)	398303

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:41 Page: 1 ID:ZxYh42odUEwonJ19eiMJ3_zBxjK-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:28.5

	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.01	4-7	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	-0.03	4-7	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.02	4-7	>999	240		
BCDL	10.0										Weight: 14 lb	FT = 20%

BRACING		
TOP CHORD	Structura	I wood sheathing directly applied.
BOT CHORD	Rigid ceil	ing directly applied.
REACTIONS	(size)	2=0-3-0, 3= Mechanical, 4=
		Mechanical
	Max Horiz	2=39 (LC 12)
	Max Uplift	2=-43 (LC 12), 3=-19 (LC 12), 4=-8
		(LC 12)
	Max Grav	2=269 (LC 23), 3=121 (LC 23),
		4=70 (LC 7)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	
TOP CHORD	1-2=0/26,	2-3=-69/34

BOT CHORD 2-4=-69/52

NOTES

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this 3) design.
- This truss has been designed for greater of min roof live 4) load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 5 degree rotation about its center.

- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections. 8)
- Provide mechanical connection (by others) of truss to 9) bearing plate capable of withstanding 19 lb uplift at joint 3 and 8 lb uplift at joint 4.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



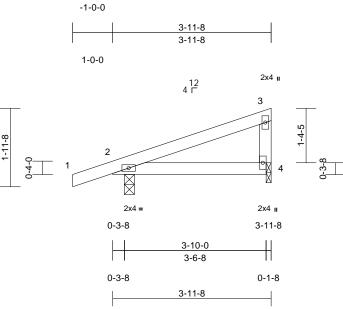
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof
M00210-B	P2	Monopitch	6	1	I73898304 Job Reference (optional)

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Page: 1



alo - 1.29 9 So

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.01	4-7	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.02	4-7	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP		Wind(LL)	0.02	4-7	>999	240		
BCDL	10.0										Weight: 15 lb	FT = 20%

LOWIDEN		
TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
WEBS	2x4 SP N	0.3
BRACING		
TOP CHORD	Structura	I wood sheathing directly applied or
	3-11-8 oc	purlins, except end verticals.
BOT CHORD	Rigid ceil	ing directly applied or 10-0-0 oc
	bracing.	
REACTIONS	(size)	2=0-3-0, 4=0-1-8
REACTIONS	` '	2=0-3-0, 4=0-1-8 2=37 (LC 15)
REACTIONS	Max Horiz	
REACTIONS	Max Horiz Max Uplift	2=37 (LC 15)
REACTIONS	Max Horiz Max Uplift Max Grav	2=37 (LC 15) 2=-45 (LC 12), 4=-23 (LC 12)
	Max Horiz Max Uplift Max Grav	2=37 (LC 15) 2=-45 (LC 12), 4=-23 (LC 12) 2=263 (LC 23), 4=169 (LC 23)
	Max Horiz Max Uplift Max Grav (lb) - Max Tension	2=37 (LC 15) 2=-45 (LC 12), 4=-23 (LC 12) 2=263 (LC 23), 4=169 (LC 23)
FORCES	Max Horiz Max Uplift Max Grav (lb) - Max Tension 1-2=0/26,	2=37 (LC 15) 2=-45 (LC 12), 4=-23 (LC 12) 2=263 (LC 23), 4=169 (LC 23) imum Compression/Maximum 2-3=-65/36, 3-4=-115/81

NOTES

Unbalanced roof live loads have been considered for 1) this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this desian.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 7) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 20.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof
M00210-B	P2B	Jack-Open	2	1	I73898305 Job Reference (optional)

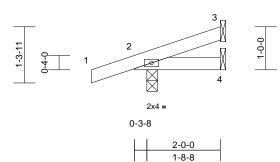
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Structural, LLC, Thurmont, MD - 21788.

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:41 ID:g4CTzRwTR6r09FWJQa3Q8ozBxkS-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

2-0-0 2-0-0 1-0-0 12 4 Г





2-0-0

Scale = 1:26.8

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-MP	0.10 0.07 0.00	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in 0.00 0.00 0.00 0.00	(loc) 4-9 5 3 9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 8 lb	GRIP 244/190 FT = 20%
	2-0-0 oc purlins. Rigid ceiling direct bracing. (size) 2=0-3-0 Mechan Max Horiz 2=25 (L Max Uplift 2=-42 ((LC 13)		: 8) 9) !=-3 10	about its cen This truss ha chord live loa * This truss h on the bottor 3-06-00 tall b chord and ar Refer to gird Provide mec bearing plate and 5 lb uplil 0) One H2.5A S recommende UPLIFT at jtt	as been designed ad nonconcurrent has been designed in chord in all are by 2-00-00 wide w hy other members er(s) for truss to thanical connection e capable of withs	I for a 10.0 t with any ed for a liv as where will fit betw s. truss con on (by oth standing 3 Tie conne- ss to bear ction is for	D psf bottom other live loa e load of 20.0 a rectangle veen the botto nections. ers) of truss t I b uplift at jo ctors ing walls due	ds. Dpsf om int 4 to					

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/26, 2-3=-75/128

TOP CHORD BOT CHORD 2-4=-141/108

NOTES

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 2) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this 3) design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

does not consider lateral forces. LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P2C	Jack-Open	4	1	Job Reference (optional)	173898306

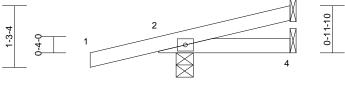
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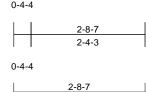




12 2.83 Г







Scale = 1:23.7

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	0.00	4-9	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	0.00	4-9	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP		Wind(LL)	0.00	5	>999	240		
BCDL	10.0										Weight: 10 lb	FT = 20%

TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
BRACING		
TOP CHORD	Structura	wood sheathing directly applied or
	2-8-7 oc p	ourlins.
BOT CHORD	Rigid ceili bracing.	ing directly applied or 10-0-0 oc
REACTIONS	(size)	2=0-4-4, 3= Mechanical, 4=
		Mechanical
	Max Horiz	2=24 (LC 12)
	Max Uplift	2=-60 (LC 12), 3=-5 (LC 16), 4=-5 (LC 22)
	Max Grav	2=277 (LC 23), 3=41 (LC 23), 4=33 (LC 7)
FORCES	· · /	imum Compression/Maximum
	Tension	
TOP CHORD	1-2=0/26,	2-3=-194/249
BOT CHORD	2-4=-267/	236

NOTES

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this 3) design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle
- 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections. 8)
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 3
- and 5 lb uplift at joint 4. 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to
- UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces. LOAD CASE(S) Standard

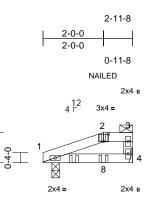


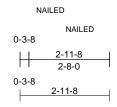
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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P3G	Half Hip Girder	2	1	I738 Job Reference (optional)	898307

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Page: 1





Scale = 1:38.1

Plate Offsets (X, Y): [2:0-2-0,0-2-13]

	()							(1)				
Loading	(psf)	Spacing	2-0-0	CSI	0.00	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	0.00	4-7	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	0.20	Vert(CT)	-0.01	4-7	>999	240		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI20	4 Matrix-MR		Wind(LL)	0.00	4-7	>999	240		
BCDL	10.0				-						Weight: 10 lb	FT = 20%
UMBER			7) This ti	uss has been designed	d for a 10.	0 psf bottom						
TOP CHORD	2x4 SP No.2		chord	live load nonconcurren	t with any	other live loa	ads.					
BOT CHORD	2x4 SP No.2		8) * This	truss has been designe	ed for a liv	e load of 20.	0psf					
NEBS	2x4 SP No.3		on the	bottom chord in all are	eas where	a rectangle						
BRACING				0 tall by 2-00-00 wide		veen the bott	om					
TOP CHORD	Structural wood she	athing directly appli		and any other member								
	2-11-8 oc purlins, e		and 9) Refer	to girder(s) for truss to								
	2-0-0 oc purlins: 2-3		10) Provid	e mechanical connecti								
BOT CHORD	Rigid ceiling directly	applied or 10-0-0 c		g plate capable of with	standing 2	24 lb uplift at	joint					
	bracing.		4.									
REACTIONS	(size) 1=0-3-0, 4	4= Mechanical		2.5A Simpson Strong-								
	Max Horiz 1=18 (LC	11)		mended to connect tru								
	Max Uplift 1=-28 (LC			T at jt(s) 1. This conne ot consider lateral forc		r upliπ only a	na					
	Max Grav 1=241 (L0					ot donict the	0170					
FORCES	(lb) - Maximum Corr	,. , ,	12) Olapi	ical purlin representation orientation of the purlir			SIZE					
ONOLO	Tension	ipression/maximum		onentation of the putility chord.	i along the	e top anu/or						
FOP CHORD	1-2=-162/21, 2-3=-1	39/25 3-4=-83/13		ED" indicates 3-10d (0.	1/0"v2")	or 2 12d						
BOT CHORD	1-4=-20/139	00,20,0 . 00,10		"x3.25") toe-nails per N								
NOTES	20/100			LOAD CASE(S) sectio			face					
	d roof live loads have	been considered fo	· · · ·	truss are noted as fron			lace					
this design		been considered it	1	SE(S) Standard	. (.) 0. 20							115
	E 7-16; Vult=120mph	(2 second quist)		+ Snow (balanced): L	umber Inc	rease-1 15	Plato				11111	A - 111
	iph; TCDL=6.0psf; BC			ase=1.15		16436-1.13,	i late			1.	IN THUR	ROUL
	Enclosed; MWFRS (er			rm Loads (lb/ft)						1	A	Declare
	xposed ; end vertical			rt: 1-2=-51, 2-3=-61, 4	-5=-20					27		PAINT
	and right exposed; Lui			entrated Loads (lb)	0-20				1		19 10	14.1
grip DOL=				rt: 7=-148 (F), 8=1 (B)							.4	19.12
	CE 7-16; Pr=20.0 psf (roof LL: Lum DOL=		n. 1 = 110 (1), 0 = 1 (D)							SEA	AL : =
	=1.15); Pg=20.0 psf; I								=	:		• -
	DOL = 1.15); Is=1.0;								=		0363	322 : =
	1.0; Cs=1.00; Ct=1.10		-							- B		
I) Unbalance	d snow loads have be	en considered for t	his							-	A	- 1 - E
design.										11	N. SNO.	-ERIX S
 Provide ad 	lequate drainage to pr	event water pondin	g.							1		Et. A.N
) Plates che	cked for a plus or min	us 5 degree rotatior	n							1	C A	ALBE I'I''
about its or	ontor										1. A (

- grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- Provide adequate drainage to prevent water ponding. 5) 6) Plates checked for a plus or minus 5 degree rotation
- about its center.

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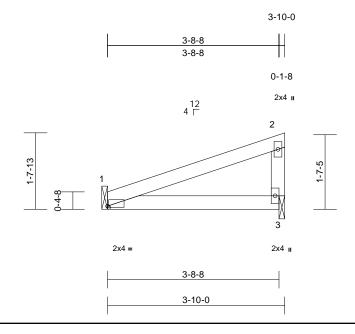
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100000 June 3,2025

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P3B	Jack-Open	2	1	I7389830 Job Reference (optional)	08

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:42 ID:TDell_WmVMGtHukOJSu3hQzBxfp-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:24.9

Plate Offsets (X, Y): [1:0-0-5,0-0-5]

Plate Olisets	(X, Y): [1:0-0-5,0-0-5]												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TP		CSI TC BC WB Matrix-MP	0.21 0.19 0.02	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.02 0.00 0.02	(loc) 3-6 3-6 1 3-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 13 lb	GRIP 244/190 FT = 20%
BCDL 10.0 Weight: 13 lb FT = 20% LUMBER TOP CHORD 2x4 SP No.2 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. BCALING TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins. * Refer to girde(rs) for truss to truss connections. BOT CHORD SDT CHORD BCT CHORD BCT CHORD Max Horiz 1=27 (LC 12) Max Koriay 1=278 (LC 12) Max Grav 1=168 (LC 23), 3=168 (LC 23). * Refer to girde tar joint(s) 3. FORCES BOT CHORD DT CHORD DT CHORD DT 2-B8/36 BOT CHORD DT 2-B8/36 BOT CHORD DT 2-B8/36 BOT CHORD DT 2-B8/36 BOT CHORD DT 2-B8/36 BOT CHORD 1.3-e68/81 WEBS 2.3-a-112/77 NOTES 10 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1. 10 One H2.25A Simpson Strong-Tie connectors recommended to connect trus to bearing walls due to UPLIFT at jt(s) 3. This connection is for uplift only and does not consider lateral forces. NoTES 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25f; Cat. II; Exp is Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 3-8-4 zone; cantilever left and right exposed ; end vertical left													
and right members Lumber D 2) TCLL: AS Plate DOI 1.15 Plate Exp.; Ce= 3) Unbalanc design. 4) Plates chu about its o	exposed; porch left and and forces & MWFRS DOL=1.60 plate grip DO CCE 7-16; Pr=20.0 psf; F e DOL = 1.15); Pg=20.0 psf; F e DOL = 1.15); Is=1.0; I =1.0; Cs=1.00; Ct=1.10 eed snow loads have be ecked for a plus or min	d right exposed;C-C for reactions shown DL=1.60 roof LL: Lum DOL= 7=15.4 psf (Lum DC Rough Cat B; Partia een considered for th us 5 degree rotation	for ; 1.15 DL = Ily nis									SEA 0363	EER REALITY

- Unbalanced snow loads have been considered for this 3) design.
- 4) Plates checked for a plus or minus 5 degree rotation about its center.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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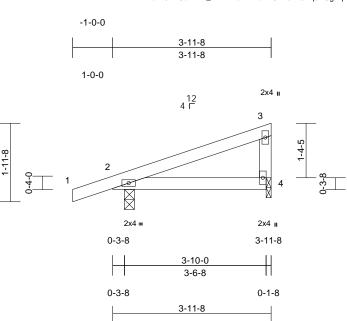
June 3,2025

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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	P3	Monopitch	4	1	Job Reference (optional)	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:41 ID:hnTkeZz5PLGuYNW_NH7H1ezBxho-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.21	Vert(LL)	-0.01	4-7	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.02	4-7	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP		Wind(LL)	0.02	4-7	>999	240		
BCDL	10.0										Weight: 15 lb	FT = 20%

BOT CHORD	2x4 SP N	0.2
WEBS	2x4 SP N	0.3
BRACING		
TOP CHORD	Structural	wood sheathing directly applied or
	3-11-8 oc	purlins, except end verticals.
BOT CHORD	Rigid ceili	ing directly applied or 10-0-0 oc
	bracing.	
REACTIONS	(size)	2=0-3-0, 4=0-1-8
	Max Horiz	2=37 (LC 15)
	Max Uplift	2=-45 (LC 12), 4=-23 (LC 12)
	Max Grav	2=263 (LC 23), 4=169 (LC 23)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	
	1 2-0/26	2 2 65/26 2 1 115/01

TOP CHORD 1-2=0/26, 2-3=-65/36, 3-4=-115/81 BOT CHORD 2-4=-52/59 NOTES

Unbalanced roof live loads have been considered for 1) this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 3-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this desian.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 7) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 20.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



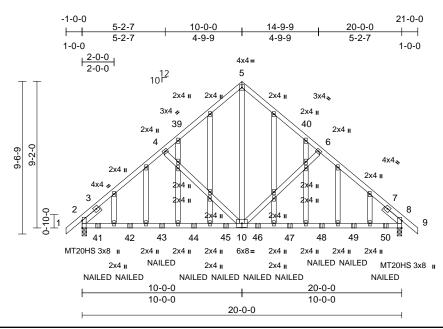
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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	G1G	Common Girder	2	1	Job Reference (optional)	173898310

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:40 Page: 1 ID:8FjzAQrxXSfH1U7EWzh1UJzBxPu-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:72.1

Plate Offsets (X, Y): [2:0-5-7,Edge], [8:0-5-7,Edge], [8:0-0-0,0-0-0]

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.51	Vert(LL)		10-33	>999	360		244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.63	Vert(CT)		10-33	>999	240	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO		WB	0.22	Horz(CT)	0.03	8	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-MS		Wind(LL)	0.03	10-33	>999	240		
BCDL	10.0					-						Weight: 162 lb	FT = 20%
UMBER			4)		7-16; Pr=20.0 p								
TOP CHORD	2x4 SP No.2				.15); Pg=20.0 ps								
BOT CHORD	2x4 SP No.2				OL = 1.15); ls=1.		Cat B; Partia	illy					
VEBS	2x4 SP No.3		_); Cs=1.00; Ct=1.								
OTHERS	2x4 SP No.3		5)		snow loads have	been co	nsidered for t	his					
SLIDER	Left 2x4 SP No.3 1	1-6-0, Right 2x4 SP	No.3	design.	a haan daalamad		on of min roof	. live					
	1-6-0		6)		is been designed psf or 2.00 times								
BRACING	.				on-concurrent wit			31 011					
TOP CHORD	Structural wood she	athing directly appli	ed or 7		MT20 plates un			h					
BOT CHORD	6-0-0 oc purlins. Rigid ceiling directly	applied at 10.0.0 a			ed for a plus or n								
SOT CHURD	bracing.	applied of 10-0-0 0	C -	about its cer			9						
	0		9)	Gable studs	spaced at 2-0-0 of	DC.							
REACTIONS	(size) 2=0-3-8, 8 Max Horiz 2=-147 (L		10)) This truss ha	is been designed	for a 10.	0 psf bottom						
	Max Uplift 2=-42 (LC				ad nonconcurrent								
	Max Grav 2=816 (LC		1		nas been designe			0psf					
FORCES	(lb) - Maximum Com				n chord in all are								
ORCES	Tension	pression/maximum			by 2-00-00 wide v		veen the bott	om					
FOP CHORD		136 4-5=-681/108	1.		Simpson Strong-T		ctore						
	5-6=-681/108, 6-8=-		14		ed to connect trus			to					
BOT CHORD		000,101,0000,02			s) 2 and 8. This of							SALLE.	
WEBS	5-10=-82/464, 6-10=	-248/108, 4-10=-24	8/108		t consider lateral		into tot apint	0,				1111 00	1111
NOTES	,	,	1;		dicates 3-10d (0.1		or 3-12d					"TH UA	ROUL
	ed roof live loads have	been considered fo	r	(0.148"x3.25	") toe-nails per N	DS guidli	nes.				1	A SECO	ich i
this design			14		CASE(S) section			face			10	in	Nº si
2) Wind: ASC	CE 7-16; Vult=120mph	(3-second gust)		of the truss a	are noted as front	(F) or ba	ck (B).					:07	1.1
Vasd=95n	nph; TCDL=6.0psf; BC	DL=6.0psf; h=25ft;	Cat. L	DAD CASE(S)	Standard					-	<u>е</u> в	. *	1 1 1 E
	Enclosed; MWFRS (en				ow (balanced): Lu	umber Inc	rease=1.15,	Plate		=		SEA	L : E
	exposed ; end vertical I		ed;	Increase=1							:	0202	• -
	OL=1.60 plate grip DO			Uniform Lo						1		0363	~~ : :
	igned for wind loads in				=-51, 5-9=-51, 31	-35=-20							
	studs exposed to wind				ed Loads (lb)						3	·	air S
	lard Industry Gable En				25 (F), 42=25 (F)						15	A SAGINI	EFICAN
or consult	quanneu bunung desi	grier as per ANOI/11	11.		F), 46=25 (F), 47=	=∠ə (F), 4	¤=∠5 (F), 49=	=20			11	710	aFin
				(F), 50=2	5 (F)							SEA 0363	ILDIN
												A. G	innin,
												lur	0 3 2025

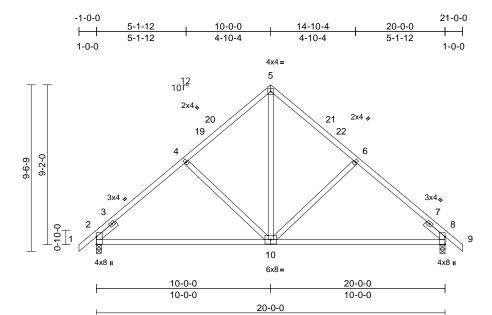
June 3,2025

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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	G1	Common	2	1	Job Reference (optional)	173898311

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:40 Page: 1 ID:1f72Pg7RZ0?FInEcjDaungzBxOE-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:66.1

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-AS	0.34 0.81 0.22	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.28 0.02	(loc) 10-13 10-13 2 10-13	l/defl >999 >866 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 109 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Unbalance, this design. 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E Exterior(2E Exterior(2E Exterior(2E Exterior(2E Exterior(2E) 21-0-0 zon vertical left forces & M DOL=1.60 3) TCLL: ASC Plate DOL= 1.15 Plate I	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x4 SP No.3 1-6-0 Structural wood she Rigid ceiling directly (size) 2=0-3-8, 8 Max Horiz 2=-147 (L Max Grav 2=860 (LG (lb) - Maximum Corr Tension 1-2=0/52, 2-4=-907/ 5-6=-720/105, 6-8=- 2-8=-135/661 5-10=-33/555, 6-10= d roof live loads have	8=0-3-8 C 14) C 2), 8=860 (LC 2) ppression/Maximum 80, 4-5=-720/105, 907/80, 8-9=0/52 =-249/117, 4-10=-249 been considered for (3-second gust) DL=6.0psf; h=25ft; C rivelope) and C-C rior (1) 2-0-0 to 10-0- terior (1) 13-0-0 to ight exposed ; end hown; Lumber roof LL: Lum DOL=1. 2f=15.4 psf (Lum DOI Rough Cat B; Partiall	 load of 12.0 overhangs n 6) Plates check 7) This truss ha chord live lo 8) * This truss de son the botto 3-06-00 tall 3-06-00 tall chord and a 9) This truss de structural we chord and 1, the bottom c LOAD CASE(S) 	as been designed ad nonconcurrent nas been designe m chord in all are by 2-00-00 wide w ny other members ssign requires tha bod sheathing be 2" gypsum sheet hord.	flat roof lo th other liv ninus 5 de for a 10.0 t with any ed for a liv as where will fit betw s. tt a minim applied di	bad of 15.4 p: ve loads. egree rotation 0 psf bottom other live loa e load of 20.1 a rectangle veen the botto um of 7/16" irectly to the f	sf on n nds. 0psf om top			2 m	SEA ORTEES SEA 0363	ROLL
	d snow loads have be		is								A. G	EL N

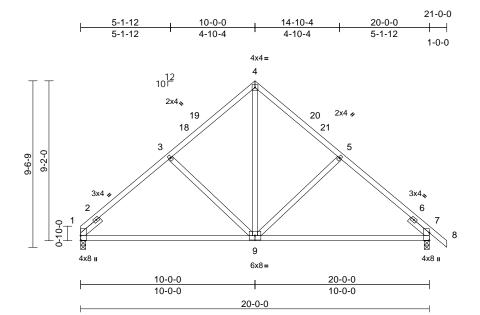
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
 - WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



GI 11111111 June 3,2025

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	G1A	Common	6	1	Job Reference (optional)	173898312

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:40 Page: 1 ID:s40kEXPs85v_0JU1ZZVHCWzBxNt-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:66.1

Loading TCLL (roof) Snow (Pf/Pg)	(psf) 20.0 15.4/20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15		CSI TC BC	0.34 0.81	DEFL Vert(LL) Vert(CT)	in -0.14 -0.28	(loc) 9-16 9-12	l/defl >999 >858	L/d 360 240	PLATES MT20	GRIP 244/190
TCDL	10.0	Rep Stress Incr	YES		WB	0.22	Horz(CT)	0.02	1	n/a	n/a		
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-AS		Wind(LL)	0.02	9-12	>999	240		
BCDL	10.0											Weight: 108 lb	FT = 20%
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E Exterior(2E Exterior(2F 21-0-0 zon vertical left forces & M DOL=1.60 3) TCLL: ASC Plate DOL 1.15 Plate Exp;; Ce=1	2x4 SP No.2 2x4 SP No.3 Left 2x4 SP No.3 1-6-0 Structural wood she Rigid ceiling directly (size) 1=0-3-8, ' Max Horiz 1=-142 (L Max Grav 1=-799 (Ld (lb) - Maximum Com Tension 1-3=-796/81, 3-4=-7 5-7=-909/80, 7-8=0/ 1-7=-131/665 4-9=-35/556, 5-9=-2	7=0-3-8 C 14) C 2), 7=862 (LC 2) ppression/Maximum 23/106, 4-5=-722/105 52 49/117, 3-9=-253/118 been considered for (3-second gust) DL=6.0psf; h=25ft; C velope) and C-C ior (1) 3-0-0 to 10-0-0 terior (1) 3-0-0 to 1	6) lo.3 7) d. 8) 9) 5, LO 3 at.), 15 y	load of 12.0 p overhangs no Plates check about its cen This truss ha chord live loa * This truss h on the botton 3-06-00 tall b chord and ar This truss de structural wo	s been designed d nonconcurrent as been designed h chord in all area y 2-00-00 wide w y other members sign requires tha od sheathing be "gypsum sheet hord.	flat roof lo h other lin hinus 5 do for a 10.0 with any d for a liv as where vill fit betv s. t a minim applied d	bad of 15.4 p ve loads. agree rotation 0 psf bottom other live loa e load of 20. a rectangle veen the bott um of 7/16"	sfon n ads. Opsf om top		4	V	SEA 0363	L

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

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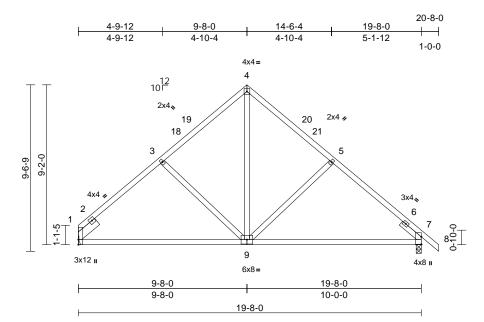
818 Soundside Road Edenton, NC 27932

G١ 111111111 June 3,2025

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	G1B	Common	6	1	Job Reference (optional)	173898313

 Run: 25.20 S
 May 13 2025 Print: 25.2.0 S
 May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:40
 Page: 1

 ID:2SiK66lQYhR?MddQCR?rLQzBxNR-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f
 Figure 1
 Figure 2



Scale = 1:66.1

21-0-0 zone; cantilever left and right exposed ; end

vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15

Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially

Unbalanced snow loads have been considered for this

DOL=1.60 plate grip DOL=1.60

Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3)

4)

design.

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-AS	0.38 0.77 0.22	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.15 -0.30 -0.02 0.02	(loc) 9-16 9-16 1 9-16	l/defl >999 >796 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 108 lb	GRIP 244/190 FT = 20%
	1-6-0 Structural wood she Rigid ceiling directly (size) 1= Mecha Max Horiz 1=-142 (L Max Grav 1=785 (LC (lb) - Maximum Com Tension 1-3=-870/80, 3-4=-6 5-7=-792/79, 7-8=0// 1-7=-128/628	nical, 7=0-3-8 C 12) C 2), 7=848 (LC 2) pression/Maximum 95/106, 4-5=-701/105	 load of 12.0 overhangs n Plates check about its cer This truss ha chord live lo * This truss lo on the botton 3-06-00 tall chord and ai Refer to gird 10) This truss de structural we chord and 1 	as been designed to ad nonconcurrent has been designed in chord in all area by 2-00-00 wide w hy other members er(s) for truss to tr besign requires that bod sheathing be a (2" gypsum sheetre hord.	lat roof k n other liv inus 5 de for a 10. with any d for a liv s where ill fit betv uss conr a minim upplied d	ad of 15.4 p: re loads. gree rotation) psf bottom other live loa e load of 20.1 a rectangle reen the botto rections. um of 7/16" rectly to the f	sf on n ds. Dpsf om top					
NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(28)	ed roof live loads have	been considered for (3-second gust) DL=6.0psf; h=25ft; Ca ivelope) and C-C ior (1) 3-4-0 to 10-0-0,								A. M. M.	OR FESE	ROLIN



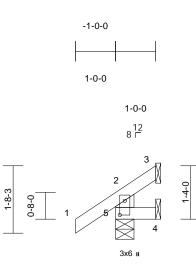
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	M2	Jack-Open	20	1	Job Reference (optional)	73898314

Run: 25.20 E May 15 2025 Print: 25.2.0 E May 15 2025 MiTek Industries, Inc. Tue Jun 03 08:34:00 ID:iFH?Q4y5GW?QJWCGNCtS9QzBxSK-F7qX1Y0k5qp6ZnatLH3v3cKlu66D5o7UM2BDeizACts

Page: 1

1-0-0



1-0-0

Scale = 1:28.8

Plate Offsets (X, Y):	[5:0-4-4,0-1-8]
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Plate Olisets (<u>, , , , , , , , , , , , , , , , , , , </u>													
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TP		CSI TC BC WB Matrix-MR	0.14 0.06 0.00	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in 0.00 0.00 0.00 0.00	(loc) 4-5 4-5 3 5	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 6 lb	GRIP 244/190 FT = 20%	
FORCES NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(21 vertical lef forces & M DOL=1.60 3) TCLL: ASC Plate DOL 1.15 Plate Exp.; Ce=: 4) Unbalance design. 5) This truss load of 12.	2x4 SP No.2 2x6 SP DSS Structural wood sheat 1-0-0 oc purlins, exu Rigid ceiling directly bracing. (Ib/size) 3=-9/ Mec Mechanic Max Horiz 5=23 (LC Max Uplift 3=-35 (LC Max Grav 3=4 (LC 1 (LC 22)) (Ib) - Max. Comp./Ma (Ib) or less except will ed roof live loads have	cept end verticals. applied or 10-0-0 oc chanical, 4=-15/ al, 5=140/0-5-8 13) 22), 4=-31 (LC 22) 2), 4=5 (LC 7), 5=19 ax. Ten All forces 2 hen shown. been considered for (3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C and right exposed ; C for members and hown; Lumber roof LL: Lum DOL=1 2f=15.4 psf (Lum DO Rough Cat B; Partial en considered for th r greater of min roof 1	ab 7) Th ch 8) * T d or 3-(9) Re 10) Pr be 4 a 11) Or 7 UF 250 LOAD Cat. end .15 L = ly is	out its cente is truss has ord live load This truss ha the bottom D6-00 tall by ord and any offer to girder ovide mecha aring plate c and 35 lb up he H2.5A Sin commended PLIFT at jt(s)	been designed for nonconcurrent v s been designed chord in all areas 2-00-00 wide wil other members. (s) for truss to tr anical connection apable of withsta lift at joint 3. mpson Strong-Tie to connect truss b. This connecti ider lateral forces	or a 10. with any for a liv s where Il fit betw uss con u (by oth anding 3 e conne to bear ion is for	 a) psf bottom other live load e load of 20.0 a rectangle veen the botto nections. arcs) of truss to 1 lb uplift at jo ctors ng walls due 	ds. Ipsf om Doint				SEA 0363	EER.	
												Ju	ne 3,2025	

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

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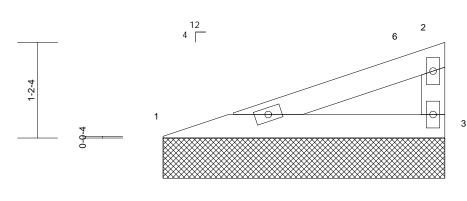


Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	VG1	Valley	2	1	Job Reference (optional)	173898315

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:43 ID:i73?TGwGiVfFVXY3OJEbntzBxLw-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

2x4 🛛

Page: 1



2x4 🚅

3-6-0

2x4 🛛

Scale =	1.11 2
Scale =	: 1:14.3

Scale = 1:14.3												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MP	0.13 0.27 0.00	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 10 lb	GRIP 244/190 FT = 20%
 FORCES TOP CHORD BOT CHORD NOTES 1) Unbalanced this design. 2) Wind: ASC Vasd=95mm II; Exp 8; E Exterior(2E zone; cantil and right exp MWFRS fo grip DOL=1 3) Truss desig only. For s see Standa or consult c 4) TCLL: ASC Plate DOL= 1.15 Plate I Exp.; Ce=1 	Max Horiz 1=24 (LC Max Grav 1=151 (LC (Ib) - Maximum Com Tension 1-2=-283/88, 2-3=-9 1-3=-150/261 d roof live loads have F 7-16; Vult=120mph ph; TCDL=6.0psf; BC nclosed; MWFRS (er) 0-0-12 to 3-0-12, Int lever left and right exg cposed; C-C for membr r reactions shown; Lu	cept end verticals. applied or 10-0-0 oc 3=3-6-0 13) C 22), 3=151 (LC 22) pression/Maximum 1/63 been considered for (3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C terior (1) 3-0-12 to 3- oosed ; end vertical I bers and forces & mber DOL=1.60 plat the plane of the trus (normal to the face) d Details as applicat gner as per ANSI/TP roof LL: Lum DOL=1 2f=15.4 psf (Lum DO	about its 7) Gable re 8) Gable st 9) This trus chord liv 10) * This tru on the bo 3-06-00 f chord an LOAD CASE 0 Cat. 5-0 eft te ss , le, 11. 15 L = ly	necked for a plus or n center. quires continuous bo uds spaced at 4-0-0 s has been designed le load nonconcurrent stan been designe tom chord in all are: all by 2-00-00 wide v d any other members (S) Standard	ttom choi oc. for a 10. with any d for a liv as where vill fit bety	d bearing. 0 psf bottom other live loa e load of 20.0 a rectangle	ids. Opsf				SEA 0363	EER RUU



GIL June 3,2025

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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	C1G	Common	1	1	Job Reference (optional)	173898316

Structural LLC Thurmont MD - 21788

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries. Inc. Mon Jun 02 13:14:39 ID:6gfN4ansdUNfu9K58nz2_7zBwin-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

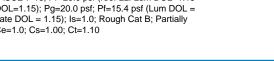
Page: 1

GRIP

244/190

FT = 20%

4-0-0 -1-0-0 13-0-0 6-0-0 12-0-0 6-0-0 6-0-0 1-0-0 1-0-0 4x6 = 12 4 Г 3 2x4 ı 2x4 II 18 19 2-7-11 2-5-8 17 20 4 5 ம் \boxtimes 6 3x4 = 2x4 II 2x4 II 2x4 II 3x4 = 6-0-0 12-0-0 6-0-0 6-0-0 12-0-0 L/d 2-0-0 CSI DEFL in l/defl PLATES (psf) Spacing (loc) 20.0 Plate Grip DOL 1.15 тс 0.45 Vert(LL) -0.05 6-13 >999 360 MT20 BC 0.39 Vert(CT) -0.08 240 6-13 >999 WB 0.10 Horz(CT) 0.01 4 n/a n/a Matrix-AS Wind(LL) 0.04 6-13 >999 240 Weight: 47 lb snow loads have been considered for this s been designed for greater of min roof live sf or 2.00 times flat roof load of 15.4 psf on on-concurrent with other live loads. ed for a plus or minus 5 degree rotation ter. spaced at 2-0-0 oc. s been designed for a 10.0 psf bottom d nonconcurrent with any other live loads. as been designed for a live load of 20.0psf h chord in all areas where a rectangle y 2-00-00 wide will fit between the bottom y other members. impson Strong-Tie connectors d to connect truss to bearing walls due to s) 2 and 4. This connection is for uplift only consider lateral forces. sign requires that a minimum of 7/16" tural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. LOAD CASE(S) Standard Lumber DOL=1.60 plate grip DOL=1.60





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818 Soundside Road Edenton, NC 27932



Scale = 1:40.3

Loading

TCLL (roof)

Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	
TCDL	10.0	Rep Stress Incr	YES	
BCLL	0.0*	Code	IRC2021	/TPI2014
BCDL	10.0			
LUMBER			5)	Unbalanced si
TOP CHORD	2x4 SP No.2		,	design.
BOT CHORD	2x4 SP No.2		6)	This truss has
WEBS	2x4 SP No.3			load of 12.0 ps
OTHERS	2x4 SP No.3			overhangs nor
BRACING			7)	Plates checke
TOP CHORD	Structural wood she	athing directly applied	l. a.	about its cente
BOT CHORD	Rigid ceiling directly	applied.	,	Gable studs sp
REACTIONS	(size) 2=0-4-8, 4	1=0-4-8	9)	This truss has chord live load
	Max Horiz 2=19 (LC	16)	10)	* This truss ha
	Max Uplift 2=-87 (LC	; 12), 4=-87 (LC 13)	10)	on the bottom
	Max Grav 2=541 (LC	C 23), 4=541 (LC 24)		3-06-00 tall by
FORCES	(lb) - Maximum Com	pression/Maximum		chord and any
	Tension		11)	One H2.5A Sir
TOP CHORD		430, 3-4=-844/430,		recommended
	4-5=0/26			UPLIFT at jt(s)
BOT CHORD	, -	339/753		and does not o
WEBS	3-6=-79/254		12)	This truss des
NOTES				structural woo

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) 2) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 13-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown:
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	C1	Common	2	1	Job Reference (optional)	173898317

Loading

TCDL

BCLL

BCDL

LUMBER

WFBS

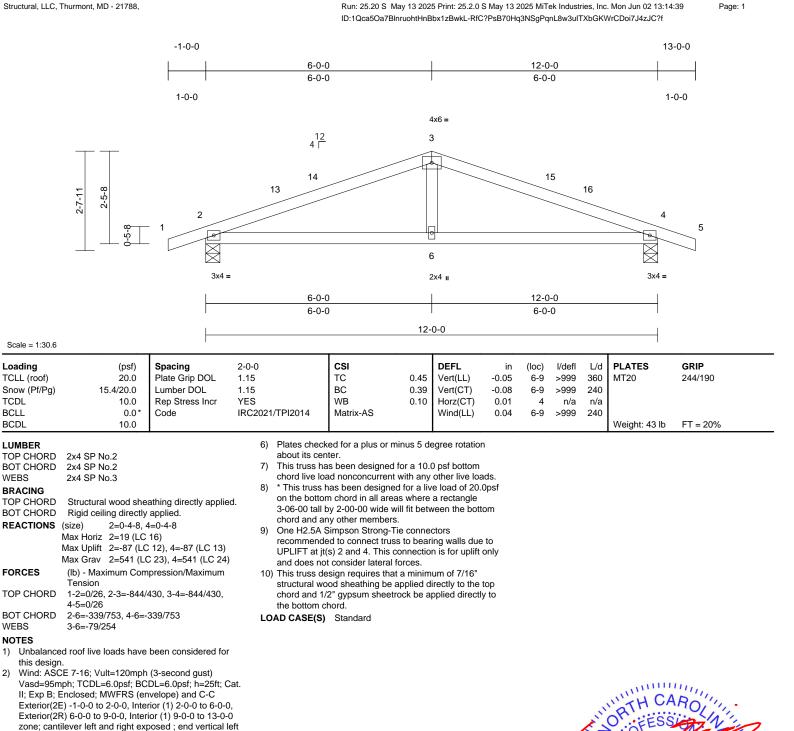
FORCES

WEBS

NOTES

1)

2)



Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown;

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

C 11111111111 SEAL 036322 G minin June 3,2025

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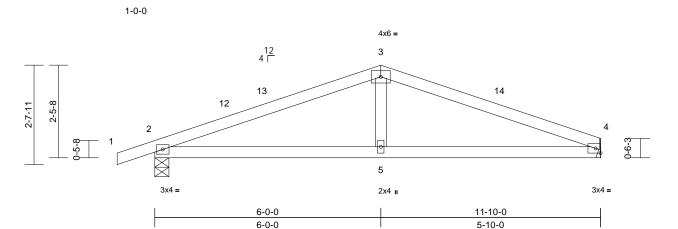


Edenton, NC 27932

Jo	b	Truss	Truss Type	Qty	Ply		
М	00210-B	C1A	Common	2	1	Job Reference (optional)	173898318

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Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries. Inc. Mon Jun 02 13:14:39 Page: 1 ID:_MoBfi0jipJkCKRoOLq1OdzBwjn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 6-0-0 11-10-0 6-0-0 5-10-0



Scale - 1:30.6

Scale = 1.50.0													
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		тс	0.45	Vert(LL)	-0.05	5-11	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.39	Vert(CT)	-0.08	5-11	>999	240		
TCDL	10.0	Rep Stress Incr	YES		WB	0.09	Horz(CT)	0.01	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-AS		Wind(LL)	0.04	5-11	>999	240		
BCDL	10.0											Weight: 41 lb	FT = 20%
LUMBER			6)		ked for a plus or	minus 5 de	egree rotatio	n					
TOP CHORD	2x4 SP No.2			about its ce									
BOT CHORD	2x4 SP No.2		7)		as been designe								
WEBS	2x4 SP No.3				ad nonconcurrer								
BRACING	o		,	8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle									

11-10-0

TOP CHORD	Structural	l wood sheathing directly applied.					
BOT CHORD	Rigid ceili	Rigid ceiling directly applied.					
REACTIONS	(size)	2=0-4-8, 4= Mechanical					
	Max Horiz	2=23 (LC 12)					
	Max Uplift	2=-87 (LC 12), 4=-65 (LC 13)					
	Max Grav	2=537 (LC 23), 4=489 (LC 24)					
FORCES	(lb) - Max	imum Compression/Maximum					
	Tension						

TOP CHORD 1-2=0/26, 2-3=-828/426, 3-4=-829/437 BOT CHORD 2-5=-364/737. 4-5=-364/737 3-5=-79/248

WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 11-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this 4) design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. Refer to girder(s) for truss to truss connections. 9)
- 10) Provide mechanical connection (by others) of truss to
- bearing plate capable of withstanding 65 lb uplift at joint 4 11) One H2.5A Simpson Strong-Tie connectors
- recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

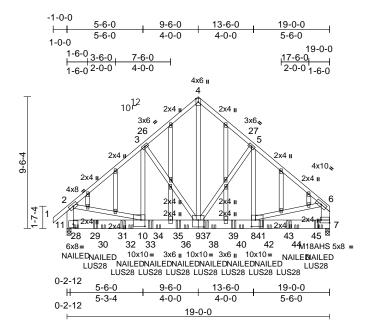
LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
М00210-В	A1GR	Common Girder	2	2	Job Reference (optional)	173898319

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:37 ID:5MFCicanfRV5NISA?OhCxXzByTN-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:83.4

Plate Offsets (X, Y): [7:Edge,0-2-8], [8:0-3-8,0-5-12], [9:0-5-0,0-6-0], [10:0-3-8,0-5-8]

												1	
Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.56	Vert(LL)	-0.07	9-10	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.29		-0.13	8-9	>999	240	M18AHS	186/179
TCDL	10.0	Rep Stress Incr	NO		WB	0.84		0.01	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-MS		Wind(LL)	0.01	9-10	>999	240		
BCDL	10.0											Weight: 398 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x8 SP DSS 2x4 SP No.3 *Excep 11-2:2x6 SP No.2, 7 2x4 SP No.3 Structural wood she 4-6-3 oc purlins, ex Rigid ceiling directly bracing. (size) 7=0-3-8, (Max Horiz 11=165 (I Max Grav 7=7750 (I (Ib) - Maximum Com Tension 1-2=0/64, 2-3=-660/ 4-5=-5135/0, 5-6=-6 6-7=-5374/0	-6:2x8 SP DSS athing directly applied cept end verticals. applied or 10-0-0 oc (req. 0-3-15), 11=0-3-1 _C 50) _C 27), 11=6811 (LC apression/Maximum 4/0, 3-4=-5119/0, 660/0, 2-11=-5409/0,	4) or 5) 3 6) 26) 7)	this design. Wind: ASCE Vasd=95mpf II; Exp B; En and right exp Lumber DOL Truss design only. For stu see Standard or consult qu TCLL: ASCE Plate DOL=1 1.15 Plate DD Exp.; Ce=1.0 Unbalanced design. This truss ha load of 12.0	roof live loads hav 7-16; Vult=120m; ;; TCDL=6.0psf; E closed; MWFRS (osed; end vertice =1.60 plate grip D ed for wind loads ds exposed to wind l ndustry Gable E alified building de 7-16; Pr=20.0 psf DL = 1.15); Is=1.0 ; Cs=1.00; Ct=1.1 snow loads have I s been designed 1 osf or 2.00 times f on-concurrent with	bh (3-sec CCDL=6.0 envelope al left and OCL=1.60 in the pla od (norm End Deta signer as f (roof LL ; Pf=15.4 ; Rough 0 been cor for greate lat roof lo	cond gust) Opsf; h=25ft; i); cantilever d right expose) ane of the tru al to the face ils as applica s per ANSI/TI :: Lum DOL= 4 psf (Lum DC Cat B; Partia nsidered for tl er of min roof pad of 15.4 p	Cat. left ed; lss), ble, PI 1. 1.15 DL = lly his	Tru: 2-0- bac 18: Use SD9 at 2 18-0 cho 19) Fill 20) "NA (0.1 LOAD (1) De Inc Ur	ss) or ec 12 from k face o 2012 Tru- 0-0 oc 0-12 to c 0-12 to c 0-12 to c dll nail h ILLED" ir 48"x3.2 CASE(S ead + Sr crease= iform Lo Vert: 1-2 oncentra Vert: 29	quivale the le f botto on Stro uss, Si max. s connect oles w ndicate 5") toe) Sta now (b 1.15 oads (l 2=-51, ted Lo =-104	png-Tie LUS28 (6 nng-Tie LUS28 (7 nnt spaced at 2-0 ft end to 8-0-12 f m chord. nng-Tie LUS28 (7 ngle Ply Girder) istarting at 10-0-1: truss(es) to back where hanger is in as 3-10d (0.148"s) -nails per NDS (2 ndard alanced): Lumbe b/ft) 2-4=-51, 4-6=-5 vads (lb) 8 (B), 31=-1048 0, 147 (D) 2	S-10d Girder, 4-10d -0 oc max. starting at to connect truss(es) to S-SD9112 Girder, 4- or equivalent spaced 2 from the left end to ck face of bottom n contact with lumber. (3") or 3-12d juidlines. er Increase=1.15, Plate 1, 7-11=-20 (B), 33=-1048 (B),
BOT CHORD WEBS	10-11=0/995, 8-10= 4-9=0/6258, 5-9=-20 3-9=-1953/0, 3-10=0		10)	All plates are Plates check about its cen	MT20 plates unle ed for a plus or m ter.	ess other inus 5 de	wise indicate egree rotatior	١		35=-104 (B), 43=	-1174	(B), 45=-1175 (E	9=-1174 (B), 41=-1174 3)
NOTES	6-8=0/4079		11)		ully sheathed from st lateral moveme						S	ORTH CA	Ling
 2-ply truss (0.131"x3") Top chords oc, 2x6 - 2 staggered Bottom cho staggered Web conne All loads ai except if no CASE(5) s provided to 	to be connected toge of nails as follows: s connected as follows rows staggered at 0-6 at 0-9-0 oc. ords connected as foll at 0-4-0 oc. ected as follows: 2x4 - re considered equally oted as front (F) or ba ection. Ply to ply conr o distribute only loads erwise indicated.	s: 2x4 - 1 row at 0-9-0 9-0 oc, 2x8 - 2 rows ows: 2x8 - 2 rows - 1 row at 0-9-0 oc. applied to all plies, ck (B) face in the LOA rections have been	13) 14) 15)	 Gable studs This truss ha chord live loa * This truss ha on the bottor 3-06-00 tall b chord and ar WARNING: F than input be Bearing at jo using ANSI/T 	spaced at 2-0-0 o s been designed ad nonconcurrent has been designed n chord in all area y 2-00-00 wide w y other members. Required bearing	c. for a 10.0 with any d for a liv s where ill fit betw size at jo s parallel n formula	D psf bottom other live loa e load of 20.0 a rectangle veen the botto int(s) 7 great I to grain valu a. Building	ads. Opsf om er		6		SEA 0363	EER. R. L

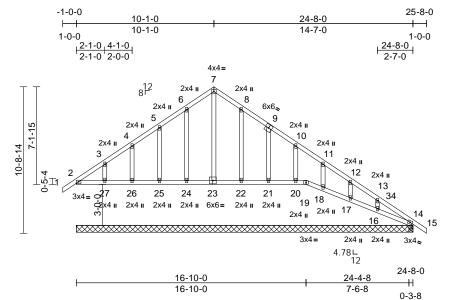
June 3,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	AVG	Roof Special	2	1	Job Reference (optional)	173898320

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:38 Page: 1 ID:UFkVJih3tlw1ljgxF6jK5pzByIv-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





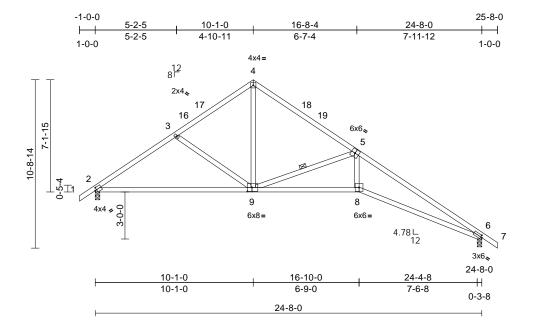
- ·			
Scale	= '	1:84.4	

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/7	FPI2014	CSI TC BC WB Matrix-AS	0.09 0.05 0.15	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in 0.00 0.00 0.00 0.00	(loc) 16-33 16-33 14 16-33	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 138 lb	GRIP 244/190 FT = 20%	
	2x4 SP No.2 2x4 SP No.3 Structural wood shee Rigid ceiling directly (size) 2=24-8-0, 17=24-8-0 20=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 23=24-8-0 17=-9 (L0 19=-9 (L0 21=-17 (L 24=-13 (L 26=-14 (L 16=204 (L 16=204 (L 20=150 (L 22=226 (L 22=226 (L 24=219 (L)	14=0-3-8, 16=24-8-(), 18=24-8-0, 19=24-), 21=24-8-0, 22=24-), 24=24-8-0, 25=24-), 27=24-8-0 C 12) 12), 16=-31 (LC 17) 17), 18=-18 (LC 17) C 17), 22=-10 (LC 17 C 17), 22=-10 (LC 17 C 16), 25=-17 (LC 16 C 16), 27=-19 (LC 16	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	ES Jnbalanced his design. Wind: ASCE Vasd=95mph (3E) -1-0-0 to (3E) -1-0	7-23=-166/62, 6-24 4-26=-121/61, 3-27 3-21=-135/65, 10-2 12-17=-114/57, 13- roof live loads have 7-16; Vult=120mp n; TCDL=6.0psf; BC closed; MWFRS (e 0 2-1-0, Exterior(2) 0 13-1-0, Exterior(2) 0 13-1-0, Exterior(2) 0 13-1-0, Exterior(2) 0 0 13-1-0, Exterior(2) 0 0 13-1-0, Exterior(2) 0 0 13-1-0, Exterior(2) 0 0 13-1-0, Exterior(2) 0 13-1-0, Exterior(2) 13-1-0, Ext	r=-123/6 0=-116, 16=-14 e been of h (3-sec CDL=6. nvelopes v) 2-1-0 2N) 13- kposed ubers an umber I n the pli- d (norm nd Deta signer as (roof LL Pf=15.4 Rough 0	1, 8-22=-185, 58, 11-18=-1: 4/76 considered for cond gust) opsf; h=25ft; (and C-C Cc to 10-1-0, Co 1-0 to 25-8-0 ; end vertical d forces & DOL=1.60 pla ane of the trus al to the face) ils as applicat s per ANSI/TF .: Lum DOL=1 li psf (Lum DC Cat B; Partial	/55, 24/63, 24/63, r Cat. orner rmer left te ss s, ble, PI 1. 1.15 DL = ly	usir des 12) Pro bea 19, upli 13) N/A 14) This stru cho	ng ANSI igner sh vide me ring plat 18 lb up ft at join s truss d tottural w rd and 1 bottom CASE(S	TPI 1 a ould ve chanica ie capa lift at jo t 16. esign r ood sh /2" gyp chord.) Star	angle to grain for rify capacity of b al connection (by ble of withstandi ble of withstandi ble of withstandi ble of withstandi ble of the stand requires that a mi eathing be applie ssum sheetrock b	earing surface. others) of truss to ng 9 lb uplift at joint at joint 17 and 31 lb nimum of 7/16" d directly to the top e applied directly to	b
FORCES TOP CHORD BOT CHORD	7-8=-127/194, 8-10=	104, 3-4=-110/97, 103/154, 6-7=-128/18 -102/150, 10-11=-58 =-70/32, 13-14=-112 '=-66/146, '5=-66/146, 2=-67/148, 0=-65/146, 8=-75/162,	6) - 96, 7) 1 /57, 7) 1 /72, 8) 0 9) - 10) 7	oad of 12.0 poverhangs no Plates check about its cen Gable studs This truss ha chord live loa This truss h on the bottor 3-06-00 tall b	is been designed for psf or 2.00 times fits on-concurrent with ed for a plus or mini- ter. spaced at 2-0-0 oc is been designed for ad nonconcurrent w has been designed in chord in all areas by 2-00-00 wide will by other members.	at roof lo other liv nus 5 do c. or a 10.0 vith any for a liv s where	bad of 15.4 ps ve loads. egree rotation 0 psf bottom other live load e load of 20.0 a rectangle	sf on ds. Ipsf			25	SEA 0363	i i i	

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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A1V	Roof Special	4	1	Job Reference (optional)	173898321

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:37 Page: 1 ID:4rGnEAIUY9g9D9LVYYbW_szByGq-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:73.5

Plate Offsets (X, Y): [2:0-1-4,0-1-14], [6:0-1-1,0-1-8], [9:0-3-8,0-3-0]

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		тс	0.60	Vert(LL)	-0.21	9-12	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.88	Vert(CT)	-0.46	9-12	>648	240		
TCDL	10.0	Rep Stress Incr	YES		WB	0.44	Horz(CT)	0.19	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-AS		Wind(LL)	0.07	8-15	>999	240		
BCDL	10.0											Weight: 117 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD WEBS REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES	2x4 SP No.2 2x4 SP No.3 Structural wood she Rigid ceiling directly 1 Row at midpt	5-9 6=0-3-8 C 12) -C 2), 6=1047 (LC 2) pression/Maximum 2/70, 3-4=-1165/72, 0/45 0/2518	8) 9) 10)	load of 12.0 overhangs n Plates check about its cen This truss ha chord live loa * This truss h on the botton 3-06-00 tall I chord and ar Bearing at jou using ANSI/ designer sho This truss de structural wo	as been designed for ad nonconcurrent v has been designed in chord in all areas by 2-00-00 wide will yo other members. int(s) 6 considers p FPI 1 angle to grain build verify capacity esign requires that sod sheathing be an 2" gypsum sheetro hord.	at roof le other lin nus 5 de or a 10. vith any for a liv s where l fit betw parallel s of bear a minim oplied d	bad of 15.4 p ve loads. ggree rotation 0 psf bottom other live loa e load of 20.4 a rectangle veen the bott o grain value a. Building ng surface. um of 7/16" rectly to the	sfon n ads. Opsf om e					
	ed roof live loads have	been considered for											
this design	n.											MILLIN.	111.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 1-0-0 to 2-0-0, Interior (1) 2-0-0 to 10-1-0, Exterior(2R) 10-1-0 to 13-1-0, Interior (1) 13-1-0 to 25-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.



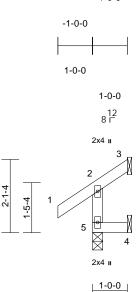
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	M1	Jack-Open	20	1	I738983 Job Reference (optional)	22

Run: 25.20 E May 15 2025 Print: 25.2.0 E May 15 2025 MiTek Industries, Inc. Tue Jun 03 08:34:38 ID:LSzdR6ouCHgLO6cVzTHty2zBymS-cdhHCuUCvpjakn3N7zG28qRQgqP9kRnEKd8nWyzACtF

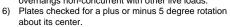
Page: 1

1-0-0



~ 4.00.0

Scale = 1:33.2						-							
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2027	1/TPI2014	CSI TC BC WB Matrix-MR	0.17 0.08 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 7 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x4 S WEBS 2x4 S BRACING 5truc 1-0-0 BOT CHORD Rigid bracin REACTIONS (lb/size Max Hi Max Uj	oc purlins, ex ceiling directly ng.) 3=-11/ Me Mechanic oriz 5=33 (LC plift 3=-43 (LC	athing directly applie cept end verticals. applied or 10-0-0 or echanical, 4=1/ ial, 5=128/0-3-8 13) 2 22), 4=-15 (LC 13) 4), 4=22 (LC 14), 5:	8) ed or 9) c 10 LC	chord live loa * This truss h on the bottor 3-06-00 tall h chord and ar Refer to gird)) Provide mec bearing plate	as been designed as been designe n chord in all area by 2-00-00 wide w ny other members er(s) for truss to hanical connectio e capable of withs uplift at joint 3. Standard	with any d for a liv as where vill fit betv s. truss con on (by oth	other live loa e load of 20.0 a rectangle veen the botto nections. ers) of truss t	0psf om to					
	()	ax. Ten All forces hen shown.	250										
 II; Exp B; Enclose Exterior(2E) zone vertical left and rig forces & MWFRS DOL=1.60 plate g TCLL: ASCE 7-16 Plate DOL=1.15); 1.15 Plate DOL = Exp.; Ce=1.0; Cs Unbalanced snow design. This truss has ber load of 12.0 psf of 	; Vult=120mph DL=6.0psf; BC d; MWFRS (er ; cantilever left ght exposed;C- for reactions s ;rip DOL=1.60 6; Pr=20.0 psf (Pg=20.0 psf ; 1.15); Is=1.0; l=1.00; Ct=1.10 v loads have be en designed fo r 2.00 times fla	(3-second gust) DL=6.0psf; h=25ft; (vvelope) and C-C and right exposed ; C for members and hown; Lumber roof LL: Lum DOL= ² Pf=15.4 psf (Lum DC Rough Cat B; Partia	Cat. end 1.15 DL = Illy live							N. HILLING		SEA 0363	L 22 EEER A





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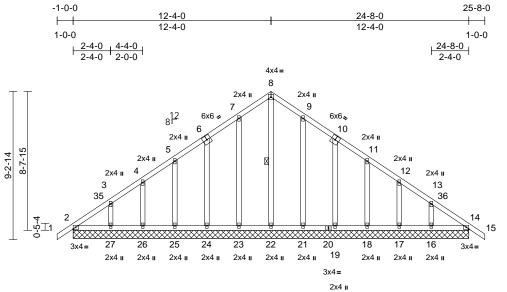
818 Soundside Road Edenton, NC 27932

June 3,2025

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	B1G	Common Supported Gable	2	1	Job Reference (optional)	173898323

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:39 Page: 1 ID:44rgDf_opbLp_mN5WGW5F?zByje-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

> 818 Soundside Road Edenton, NC 27932



24-8-0

Scale = 1:71.8

Plate Offsets (X, Y): [10:0-0-0,0-0-0]

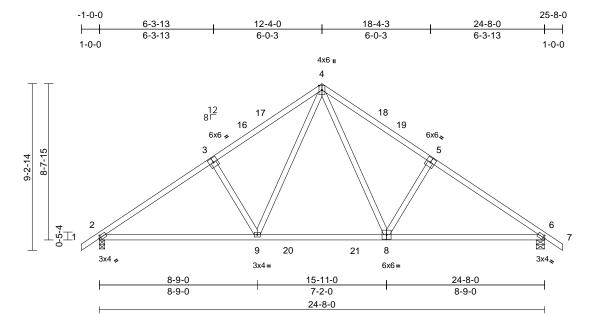
	(,, ,). [.0.0 0 0,0 0 0												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2	CSI TC BC WB 014 Matrix-AS	0.09 0.04 0.17	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 14	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 158 lb	GRIP 244/190	
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she Rigid ceiling directly 1 Row at midpt (size) 2=24-8-0, 17=24-8-0 24=24	applied. 8-22 14=24-8-0, 16=24-8 0, 18=24-8-0, 19=24- 0, 22=24-8-0, 23=24- 0, 25=24-8-0, 26=24- 0 C 14) 2 12), 16=-21 (LC 17) C 17), 21=-11 (LC 17) C 17), 21=-11 (LC 17) C 16), 24=-16 (LC 10 C 16), 24=-16 (LC 10 C 16), 26=-13 (LC 10 C 31), 17=156 (LC 2) C 31), 14=166 (LC 2) C 31), 14=166 (LC 2) C 31), 19=175 (LC 2) C 31), 26=156 (LC 2) C 30), 26=156 (LC 2) C 30) pression/Maximum 108, 3-4=-108/86, 1/123, 7-8=-111/166 -85/123, 11-12=-62/	WEBS -0, 8-0, 8-0, 1) Unbis this s () 2) Wind 7), 11; E: 3), (3E) 5), 20 (3R)	25-26=-54/125, 23-24=-55/126, 21-22=-55/126, 18-19=-54/125, 16-17=-54/125, 8-22=-135/45, 7 5-25=-118/59, 4 9-21=-184/55, 1 12-17=-119/61, 21-	24-25=-54 22-23=-55 19-21=-55 17-18=-54 14-16=-54 7-23=-184/5 1-26=-119/6 0-19=-135 13-16=-13 have been of mph (3-sec ; BCDL=6. S (envelope r(2N) 2-0-0 ior(2N) 15- ht exposed ds in the pl wind (norm e End Deta designer a: psf (roof LL psf; Pf=15.4 1.0; Rough 1.10 // e been cor ed for great ts flat roof le vith other li	(125, (126, (126, (125,	/67, 18/59, r Cat. orner orner left ate ss), bble, PI 1. 1.15 DL = Ily his live sf on	 9) Gal 10) Thi chc 11) * Ti on * 3-0 chc 12) Pro beaz 2, 1 at ju 11 join 18 13) Thi strucho the strucho the strucho the strucho the strucho the strucho 	ble stude s truss h ord live k his truss the bottk 6-00 tall ord and a wide me aring pla to uplif a b uplif to	s space nas bee pad non has be m cho by 2-0 any oth chanic te capa te capa ti at joint 13 lb u at joint lb uplif ta joint 13 lb u ta joint lb uplif ta joint	ntinuous bottom ed at 2-0-0 oc. an designed for a nconcurrent with een designed for rd in all areas wi 00-00 wide will fit er members. al connection (by able of withstand int 23, 16 lb uplif plift at joint 26, 2 21, 17 lb uplift a ft at joint 17, 21 l 2. requires that a m reeathing be appli psum sheetrock ndard SEA 0363	chord bearing. a 10.0 psf bottom any other live loa a live load of 20 here a rectangle between the bot y others) of truss ing 18 lb uplift at joint t at joint 24, 14 lb 3 lb uplift at joint 16 inimum of 7/16" ed directly to the be applied direct	ads. 0.0psf ttom t joint b uplift 27, uplift at 5 and e top

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	B1	Common	12	1	Job Reference (optional)	173898324

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:39 ID:kC9URBkKzyHs2ShCi?8sIHzByif-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:63.8

Plate Offsets (X, Y): [2:0-1-1,0-1-8], [6:0-1-1,0-1-8]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	I/TPI2014	CSI TC BC WB Matrix-AS	0.45 0.75 0.26	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.12 -0.27 0.04 0.04	(loc) 9-12 9-12 6 9-12	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 125 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Unbalance	Rigid ceiling directly (size) 2=0-3-8, 0 Max Horiz 2=-142 (L Max Grav 2=1167 (L (lb) - Maximum Com Tension 1-2=0/45, 2-4=-1528 6-7=0/45 2-9=-33/1325, 6-9=- 2-9=-33/1325, 6-9=-	6=0-5-8 .C 14) .C 30), 6=1167 (LC 3 hpression/Maximum 3/120, 4-6=-1528/120 22/1230 58/126, 4-9=-18/681,	8) (1) 9) ,	load of 12.0 overhangs n Plates check about its cer This truss ha chord live lo * This truss on the botto 3-06-00 tall chord and a This truss de structural wo	as been designed for ad nonconcurrent w has been designed m chord in all areas by 2-00-00 wide wil ny other members, ssign requires that a bod sheathing be a /2" gypsum sheetro hord.	at roof li other li nus 5 do or a 10. for a liv for a liv s where I fit betw with BC a minim oplied d	bad of 15.4 p ve loads. egree rotation 0 psf bottom other live loa ve load of 20. a rectangle veen the bott CDL = 10.0ps um of 7/16" irectly to the	osf on n ads. Opsf tom tf.					
this desigr	ո. CF 7-16՝ Vult=120mph	(3-second gust)										anno	11

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 12-4-0, Exterior(2R) 12-4-0 to 15-4-0, Interior (1) 15-4-0 to 25-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.



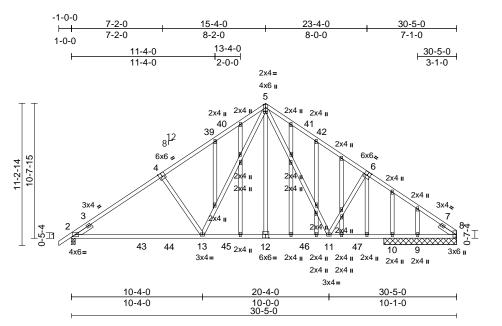
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A1G	Common Structural Gable	2	1	Job Reference (optional)	173898325

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:37 ID:PC6Pd3sLRvuS8WdvvuVwTzzByLG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:91

Plate Offsets (2	X, Y): [4:0-3-0,Edge],	[5:0-2-0,0-0-4], [6:0-	3-0,0-3-4]	, [8:0-2-8,0-0-3	3], [12:0-3-0,0-3-4	1]							
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-AS	0.92 0.83 0.35	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.57 0.06	(loc) 11-13 11-13 8 13-33	l/defl >843 >527 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 240 lb	GRIP 244/190 FT = 20%
	10=5-9-0 Max Horiz 2=170 (LC Max Uplift 9=-10 (LC Max Grav 2=1447 (L	1-6-0, Right 2x4 SP f athing directly applie applied. 3=5-9-0, 9=5-9-0, C 13) C 230, 8=1253 (LC 3 7), 10=153 (LC 30) apression/Maximum 7/149, 5-8=-1829/170 (3=0/1048, 0=-19/1445,	d. 5) 6) 7) 31), 8) 9) 10	only. For sti see Standar or consult qu TCLL: ASCE Plate DOL=' 1.15 Plate DD Exp.; Ce=1.0 Unbalanced design. This truss ha load of 12.0 overhangs n Plates check about its cer Gable studs This truss ha chord live lo. 0) * This truss la on the bottoo 3-06-00 tall l	need for wind loads ads exposed to w d Industry Gable alified building de 7-16; Pr=20.0 ps 0.15); Pg=20.0 ps as been designed ad nonconcurrent nas been designed nord in all are by 2-00-00 wide w ny other members	ind (norm End Deta esigner as is (root LL f; Pf=15.4 0; Rough 10 been cor for great flat roof k th other lin ninus 5 de cc. for a 10.4 with any ed for a liv as where vill fit betw	al to the face Is as applica s per ANSI/T : Lum DOL= : psf (Lum DO Cat B; Partia usidered for t er of min rool bad of 15.4 p re loads. agree rotation 0 psf bottom other live loa e load of 20.4 a rectangle veen the bott	e), ble, PI 1. 1.15 DL = elly his f live sf on n n dds. 0psf om			A	NHTH CA	ROUTIN
 Unbalance this design Wind: ASC Vasd=95m II; Exp B; E Exterior(2E Sterior(2F 30-5-0 zon vertical left 	ed roof live loads have CE 7-16; Vult=120mph hph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) -1-0-0 to 2-0-6, Inte R) 15-4-0 to 18-4-6, In e; cantilever left and r t and right exposed;C- IWFRS for reactions s	(3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C rior (1) 2-0-6 to 15-4- terior (1) 18-4-6 to ight exposed ; end G for members and	at.	structural wo		applied d	rectly to the			A CHILLEN		SEA 0363	•

II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-6, Interior (1) 2-0-6 to 15-4-0, Exterior(2R) 15-4-0 to 18-4-6, Interior (1) 18-4-6 to 30-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



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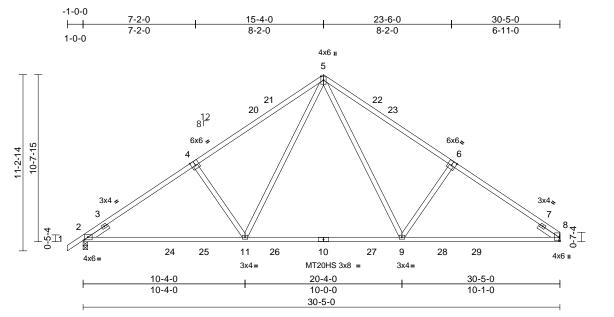
June 3,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A1	Common	8	1	Job Reference (optional)	173898326

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:35 ID:IKFcJ8ATNbelipdCgy9rBmzByZ3-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:73.5

Plate Offsets (X, Y): [2:0-2-12,0-2-1], [4:0-3-0,Edge], [6:0-3-0,0-3-4], [8:Edge,0-0-11]

	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					_							
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/	TPI2014	CSI TC BC WB Matrix-AS	0.97 0.53 0.34	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.27 -0.41 0.04 0.04	(loc) 9-11 9-11 8 11-14	l/defl >999 >875 n/a >999	L/d 360 240 n/a 240		GRIP 244/190 187/143 FT = 20%
	2x4 SP No.2 2x4 SP SS 2x4 SP No.3 Left 2x4 SP No.3 1 1-6-0 Structural wood she: Rigid ceiling directly (size) 2=0-3-8, 8 Max Horiz 2=170 (LC ((b) - Maximum Com Tension 1-2=0/52, 2-5=-1961 2-11=-5/1700, 9-11= 5-11=-9/876, 4-11=- 6-9=-406/155	athing directly applied applied. 3= Mechanical C 30), 8=1406 (LC pression/Maximum /141, 5-8=-1943/14 0/1084, 8-9=0/1553	6) No.3 7) ed. 9) 31) 10) 11) 3	load of 12.0 overhangs n All plates are Plates check about its cer This truss ha chord live loa * This truss h on the bottor 3-06-00 tall h chord and ar Refer to gird This truss de structural wo	is been designed ad nonconcurrent has been designe in chord in all area y 2-00-00 wide w hy other members er(s) for truss to t issign requires that od sheathing be a 2" gypsum sheetr hord.	flat roof li h other li ess othen inus 5 de for a 10. with any d for a liv as where vill fit betv s, with BC russ conr t a minim applied d	bad of 15.4 p <i>i</i> loads. wise indicate agree rotation 0 psf bottom other live load e load of 20. a rectangle veen the bott DL = 10.0ps nections. um of 7/16" irectly to the	ed. n ads. Opsf f. top					
this design 2) Wind: ASC Vasd=95m II; Exp B; I Exterior(2I 30-4-4 zor vertical lef forces & M DOL=1.60 3) TCLL: ASC Plate DOL 1.15 Plate Exp.; Ce=	ed roof live loads have CE 7-16; Vult=120mph nph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) -1-0-0 to 2-0-8, Inter R) 15-4-0 to 18-4-8, Inti- ne; cantilever left and r tand right exposed;C- MWFRS for reactions s 0 plate grip DOL=1.60 CE 7-16; Pr=20.0 psf (.=1.15); Pg=20.0 psf; (.DOL = 1.15); ls=1.0; f 1.0; Cs=1.00; Ct=1.10 ed snow loads have be	(3-second gust) DL=6.0psf; h=25ft; (vivelope) and C-C rior (1) 2-0-8 to 15-4 terior (1) 18-4-8 to ight exposed ; end C for members and hown; Lumber roof LL: Lum DOL=' Pf=15.4 psf (Lum DO Rough Cat B; Partia	Cat. I-0, 1.15 DL = Ily							(N. 111111)		SEA 0363	• -

4) Unbalanced snow loads have been considered for this design.

minim June 3,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

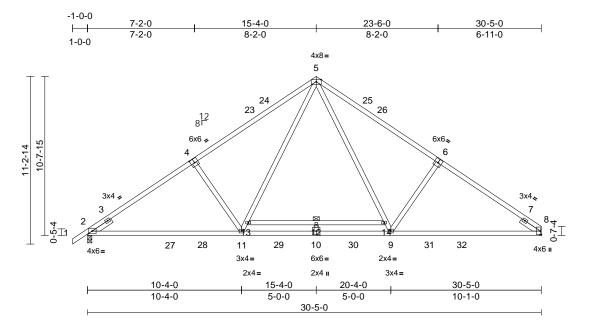


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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A1A	Common	10	1	Job Reference (optional)	173898327

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:36 ID:byT0M3JMVbS0xokuUSx2IVzBye2-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:77.2

Plate Offsets (X, Y): [2:0-2-12,0-2-1], [4:0-3-0,0-3-4], [8:Edge,0-0-11]

], [,], [
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL LUMBER	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	,	Unbalanced	CSI TC BC WB Matrix-AS	0.55 0.56 0.53 been cor	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.43 0.05 0.04	(loc) 11-17 10 8 11-17	l/defl >999 >850 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 171 lb	GRIP 244/190 FT = 20%
		t* 13-14:2x4 SP No. 1-6-0, Right 2x4 SP athing directly applie applied. 13-14 3= Mechanical C 13)	5) 2 No.3 6) d. 7) 8) 9)	load of 12.0 p overhangs n 250.0lb AC u from left end Plates check about its cen This truss ha chord live loa * This truss h on the bottor	s been designed osf or 2.00 times f on-concurrent with nit load placed or supported at two ed for a plus or m ter. s been designed id nonconcurrent ias been designed n chord in all area y 2-00-00 wide w	ilat roof k n other liv p points, s inus 5 de for a 10.0 with any d for a liv is where	bad of 15.4 p ve loads. om chord, 15 5-0-0 apart. egree rotation 0 psf bottom other live loa e load of 20. a rectangle	sf on 5-4-0 n uds. Opsf					
FORCES TOP CHORD BOT CHORD WEBS	(b) - Maximum Com Tension 1-2=0/52, 2-5=-2095 2-11=0/1812, 9-11=1 11-13=0/899, 5-13=1 9-14=0/860, 4-11=-4 10-12=0/81, 12-13=:	npression/Maximum 9/0, 5-8=-2080/0 0/1230, 8-9=0/1663 0/961, 5-14=0/922, 114/166, 6-9=-392/10	10) 11) 59,	chord and ar Refer to gird This truss de structural wo	y other members er(s) for truss to tr sign requires that od sheathing be a 2" gypsum sheetn nord.	, with BC uss conr a minim applied d	DL = 10.0ps nections. um of 7/16" irectly to the	f. top					
 this design Wind: ASC Vasd=95m II; Exp B; E Exterior(2E Exterior(2F 30-4-4 zon vertical left forces & M DOL=1.60 TCLL: ASC Plate DOL= 	ed roof live loads have E 7-16; Vult=120mph hph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) -1-0-0 to 2-0-8, Inte R) 15-4-0 to 18-4-8, In e; cantilever left and r t and right exposed;C- IWFRS for reactions s plate grip DOL=1.60 CE 7-16; Pr=20.0 psf; F DOL = 1.15); Is=1.0; I	(3-second gust) DL=6.0psf; h=25ft; (ivelope) and C-C rior (1) 2-0-8 to 15-4 terior (1) 18-4-8 to ight exposed ; end C for members and hown; Lumber roof LL: Lum DOL=1 Pf=15.4 psf (Lum DC	.15 L =							A contraction of the second se		SEA 0363	• –

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

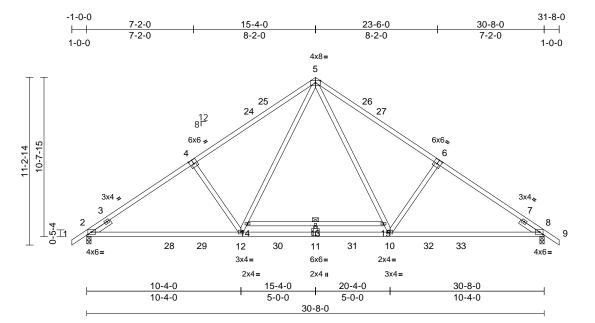


G minin June 3,2025

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A1B	Common	4	1	Job Reference (optional)	173898328

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:36 ID:QLFPVNNdWWawYnXBLAQXZ8zByWD-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:77.2

Plate Offsets (X, Y): [2:0-2-12,0-2-1], [4:0-3-0,0-3-4], [6:0-3-0,0-3-4], [8:0-2-12,0-2-1]

], [1.0 0 0,0 0 1], [0.0	,,.	-,,								
Loading TCLL (roof) Snow (Pf/Pg) TCDL	(psf) 20.0 15.4/20.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI TC BC WB	0.55 0.56 0.51	DEFL Vert(LL) Vert(CT) Horz(CT)	-0.42 0.05	(loc) 12-18 11 8	l/defl >999 >873 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0* 10.0	Code	IRC2021/TPI201	4 Matrix-AS		Wind(LL)	0.04	12-18	>999	240	Weight: 173 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP SS *Except* 2x4 SP SS 2x4 SP No.3 *Except Left 2x4 SP No.3 1-6-0 Structural wood she Rigid ceiling directly 1 Row at midpt (size) 2=0-3-8, 8 Max Horiz 2=174 (LC Max Grav 2=1555 (L (lb) - Maximum Com Tension 1-2=0/52, 2-5=-2111 2-12=0/1829, 10-12: 12-14=0/898, 5-14=1	t* 14-15:2x4 SP No.2 1-6-0, Right 2x4 SP No.2 athing directly applied applied. 14-15 3=0-3-8 C 15) C 30), 8=1555 (LC 3 pression/Maximum /0, 5-8=-2111/0, 8-9= =0/1248, 8-10=0/170 0/958, 5-15=0/958, 415/166, 6-10=-415/	design 5) This tri load of overha 6) 250.0ll from le d. 7) Plates about i 8) This tri chord l 9) * This 0 n the 3-06-0 chord a 10) This tri 5-052 structu 2 chord a the bol	Inced snow loads hav uss has been designe 12.0 psf or 2.00 time: ngs non-concurrent w o AC unit load placed ft end, supported at to checked for a plus or iss center. Uss has been designe ive load nonconcurrer russ has been designe bottom chord in all ar o tall by 2-00-00 wide and any other membe uss design requires the ral wood sheathing be and 1/2" gypsum sheet tom chord. SE(S) Standard	ed for great s flat roof lk vith other liv on the bott wo points, s minus 5 de ed for a 10.0 nt with any need for a liv eas where will fit betv ers, with BC	er of min roo pad of 15.4 p ve loads. om chord, 1: 5-0-0 apart. egree rotatio 0 psf bottom other live loa e load of 20. a rectangle veen the bot iDL = 10.0ps irectly to the	f live ssf on 5-4-0 n ads. 0psf tom tf.					
 this design Wind: ASC Vasd=95m II; Exp B; E Exterior(2E 15-4-0, Ex 18-4-13 to exposed; members a Lumber DC TCLL: ASC Plate DOL 1.15 Plate 	ed roof live loads have DE 7-16; Vult=120mph nph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) -10-0 to 2-0-13, Int tterior(2R) 15-4-0 to 18 31-8-0 zone; cantilev end vertical left and rig and forces & MWFRS OL=1.60 plate grip DC CE 7-16; Pr=20.0 psf (=.1.15); Pg=20.0 psf; F DOL = 1.15); Is=1.0; I 1.0; Cs=1.00; Ct=1.10	(3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C erior (1) 2-0-13 to 3-4-13, Interior (1) er left and right ght exposed; C-C for for reactions shown; iL=1.60 Droof LL: Lum DOL=1. Pf=15.4 psf (Lum DOI	15 - =						A. martines		SEA 0363	EER AU

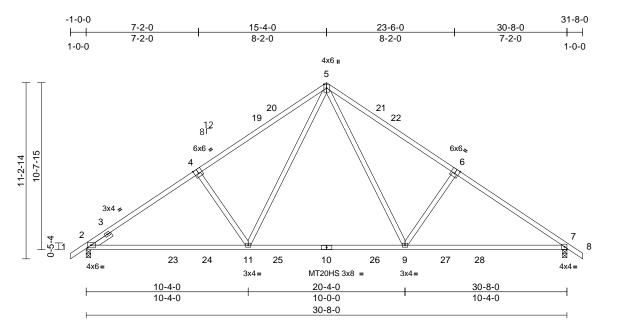
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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A1C	Common	4	1	Job Reference (optional)	173898329

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:36 ID:g9PWfiJwOAG?Hcw_LrJaANzByV0-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:73.5

Plate Offsets (X, Y): [2:0-2-12,0-2-1], [4:0-3-0,Edge], [6:0-3-0,Edge]

],[,									
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	I/TPI2014	CSI TC BC WB Matrix-AS	0.90 0.56 0.35	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.26 -0.39 0.04 0.05	(loc) 9-11 9-14 7 9-14	l/defl >999 >938 n/a >999	L/d 360 240 n/a 240	PLATES MT20HS MT20 Weight: 157 lb	GRIP 187/143 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP SS 2x4 SP No.3 Left 2x4 SP No.3 Structural wood she Rigid ceiling directly (size) 2=0-3-8, 7 Max Horiz 2=175 (LC Max Grav 2=1483 (L (lb) - Maximum Com Tension 1-2=0/52, 2-5=-1983 7-8=0/45 2-11=0/1725, 9-11=1 5-11=-10/870, 5-9=- 6-9=-446/158	eathing directly applie v applied. 7=0-3-8 C 15) LC 30), 7=1475 (LC npression/Maximum 3/141, 5-7=-2019/14 0/1111, 7-9=-27/162	9) 31) 10 1, 24 LC	load of 12.0 overhangs n All plates are Plates check about its cer This truss ha chord live lo * This truss d on the botto 3-06-00 tall chord and au) This truss de structural wo	as been designed ad nonconcurrent has been designed m chord in all area by 2-00-00 wide w ny other members esign requires that bod sheathing be a /2" gypsum sheetr hord.	ilat roof lin n other lin ess other inus 5 de for a 10.1 with any d for a liv s where ill fit betw , with BC a minim applied d	bad of 15.4 p ve loads. wise indicate egree rotation D psf bottom other live loa e load of 20. a rectangle veen the bott DL = 10.0ps um of 7/16" irectly to the	osf on ed. n ads. Opsf tom tf.					
this design 2) Wind: ASC	ed roof live loads have n. CE 7-16; Vult=120mph aph: TCDI =6 0pcf: PC	n (3-second gust)										TH CA	Route

- Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-13, Interior (1) 2-0-13 to 15-4-0, Exterior(2R) 15-4-0 to 18-4-13, Interior (1) 18-4-13 to 31-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.



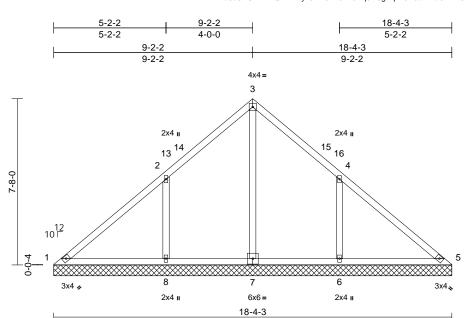
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A MiTek Affi 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	V5	Valley	2	1	Job Reference (optional)	173898331

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:43 ID:YkLBIMa6aSBUXiLl4rQLP2zByk9-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-AS	0.30 0.37 0.38	Vert(TL)	in n/a n/a 0.01	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 83 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.3 2x4 SP No.3 Structural wood shea Rigid ceiling directly (size) 1=18-4-3, 7=18-4-3, Max Horiz 1=120 (LC Max Uplift 1=-8 (LC 16) Max Grav 1=104 (LC	5=18-4-3, 6=18-4-3, 8=18-4-3 12), 6=-65 (LC 17), 8= 30), 5=104 (LC 36), 30), 7=542 (LC 29),	6) 7) 8) -67	Plate DOL=' 1.15 Plate D Exp.; Ce=1.0 Unbalanced design. Plates check about its cer Gable studs This truss ha chord live loo 0) * This truss l on the bottoo 3-06-00 tall l	F7-16; Pr=20.0 I.15); Pg=20.0 p OL = 1.15); Is= 0; Cs=1.00; Ct= snow loads hav ted for a plus or ter. es continuous b spaced at 4-0-0 is been designe ad nonconcurre has been design ad nonconcurre has been design op 2-00-00 wide yo other membe	s; Pf=15.4 1.0; Rough 1.10 re been cor minus 5 de bottom chor) oc. d for a 10.0 nt with any eed for a liv eas where will fit betw	 \$\$ perform a performance of the perfor	DL = Illy his n ds. Dpsf om					
FORCES TOP CHORD BOT CHORD WEBS NOTES	(lb) - Maximum Com Tension 1-2=-112/305, 2-3=-3 4-5=-104/272 1-8=-153/103, 6-8=- 3-7=-369/0, 2-8=-337	34/222, 3-4=-34/208, 153/97, 5-6=-153/97		 Provide med bearing plate 67 lb uplift a This truss de structural wo 	hanical connect capable of with t joint 8 and 65 sign requires th od sheathing b 2" gypsum shee	tion (by oth hstanding 8 b uplift at jo hat a minim e applied d	ers) of truss f 8 lb uplift at jo pint 6. um of 7/16" irectly to the f	to int 1, top					University

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 9-2-6, Exterior(2R) 9-2-6 to 12-2-6, Interior (1) 12-2-6 to 18-4-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

LOAD CASE(S) Standard

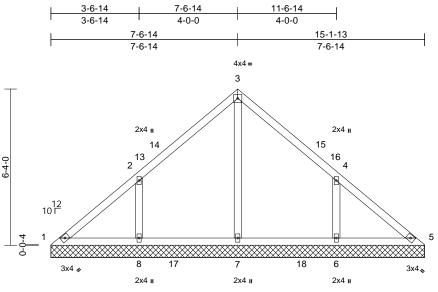


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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	V4	Valley	2	1	Job Reference (optional)	173898332

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:42 Page: 1 ID:ja_w1JVL?cRLpnubkaJx9nzBykF-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





Scale = 1:46.7

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-AS	0.23 0.16 0.17	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 66 lb	GRIP 244/190 FT = 20%
	2x4 SP No.3 Structural wood sh Rigid ceiling direct (size) 1=15-1- 7=15-1- Max Horiz 1=98 (L Max Uplift 1=-7 (LC (LC 16) Max Grav 1=117 (13, 5=15-1-13, 6=15-1 13, 8=15-1-13 C 13) C 12), 6=-52 (LC 17), 8 LC 30), 5=100 (LC 2), LC 30), 7=427 (LC 29	5) ed. 6) 1-13, 7) 8) 8=-53 9) 10	Plate DOL=1 1.15 Plate D Exp.; Ce=1.0 Unbalanced design. Plates check about its cen Gable studs This truss ha chord live loa 0) * This truss f on the bottor 3-06-00 tall t	es continuous bo spaced at 4-0-0 is been designed ad nonconcurren has been designe n chord in all are by 2-00-00 wide v	if; Pf=15.4 0; Rough 10 been cor ninus 5 de ttom chor oc. I for a 10.4 t with any d for a liv as where vill fit betv	\$ psf (Lum DG Cat B; Partia nsidered for t egree rotation d bearing. 0 psf bottom other live loa re load of 20. a rectangle veen the bott	DL = ally his n ads. 0psf om					
FORCES	Tension 1-2=-139/143, 2-3	mpression/Maximum =-118/109, 3-4=-118/9) Provide mec bearing plate	ny other member hanical connection capable of with joint 8 and 52 lb	on (by oth standing 7	ers) of truss Ib uplift at jo	to					
BOT CHORD WEBS	5-6=-63/88	-63/73, 6-7=-63/73, 295/142, 4-6=-295/142		structural wo	sign requires that od sheathing be 2" gypsum sheet hord.	applied d	irectly to the						
NOTES 1) Unbalance	d roof live loads hav	e been considered for		DAD CASE(S)								TH CA	RO

- this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
 U: Exp 8: Enclosed: MWFERS (anyeline) and C. C.
- II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 7-7-3, Exterior(2R) 7-7-3 to 10-7-3, Interior (1) 10-7-3 to 15-2-2 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

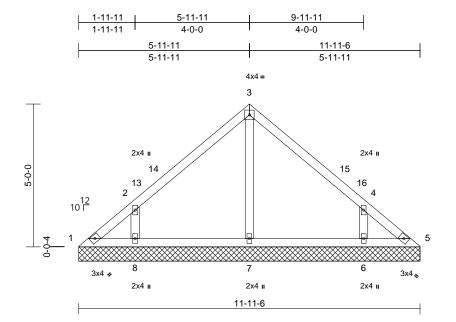


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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	V3	Valley	2	1	Job Reference (optional)	173898333

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:42 Page: 1 ID:UrxW8EOi6rlcEOitiBfqltzBykO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Figure 1 Figure 2



Scale = 1:40.3

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-AS	0.23 0.21 0.07	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 49 lb	GRIP 244/190 FT = 20%
	7=11-11-6 Max Horiz 1=77 (LC Max Uplift 1=-18 (LC 8=-44 (LC Max Grav 1=80 (LC	applied. 6, 5=11-11-6, 6=11-1 6, 8=11-11-6 15) 2 12), 6=-42 (LC 17), 2 16)	6) 1-6, 7) 8) 9) =348	Plate DOL=1 1.15 Plate D Exp.; Ce=1.0 Unbalanced design. Plates check about its cen Gable studs This truss ha chord live loa 0) * This truss h on the bottor 3-06-00 tall b	7-16; $Pr=20.0$.15); $Pg=20.0 p$ OL = 1.15); $Is='0; Cs=1.00; Ct='snow loads havded for a plus orter.es continuous bspaced at 4-0-0is been designead nonconcurrenhas been designab noconcurrenhas been designop 2-00-00 wideby 0 ther membe$	sf; Pf=15.4 I.0; Rough I.10 e been cor minus 5 de ottom chor oc. d for a 10.0 nt with any eed for a liv eas where will fit betw	 psf (Lum DC Cat B; Partia partia psidered for the egree rotation d bearing. D psf bottom other live load e load of 20.0 a rectangle 	DL = Ily nis ds. Dpsf					
FORCES TOP CHORD	(lb) - Maximum Com Tension 1-2=-98/69, 2-3=-16		11	 Provide mec bearing plate 	hanical connect capable of with at joint 8 and 4	ion (by oth Istanding 1	8 lb uplift at j						
BOT CHORD WEBS	4-5=-78/42 1-8=-16/63, 7-8=-16 5-6=-16/58 3-7=-167/0, 2-8=-30	, , ,		2) This truss de structural wo	esign requires th od sheathing be 2" gypsum shee	at a minim applied d	um of 7/16" irectly to the t						
NOTES	ed roof live loads have		L	DAD CASE(S)								TH CA	Ro

- this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 11-11-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- grip DOL=1.60
 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP11.



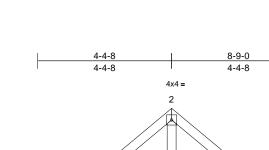
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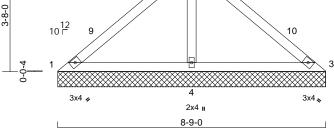
Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	V2	Valley	2	1	Job Reference (optional)	173898334

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries. Inc. Mon Jun 02 13:14:42 ID:UaQ5aQB17d91hnvbC6sr6IzBykf-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



4-4-8



Scale = 1:37.6

BOT CHORD

TOP CHORD

BOT CHORD

REACTIONS

FORCES

WEBS

NOTES

1)

2)

3)

4)

TOP CHORD

BOT CHORD

this design.

grip DOL=1.60

OTHERS

BRACING

2x4 SP No.3

2x4 SP No.3

(size)

Max Grav

Tension

Exp.; Ce=1.0; Cs=1.00; Ct=1.10

2-4=-485/210

Structural wood sheathing directly applied.

Max Uplift 1=-33 (LC 23), 3=-33 (LC 22)

(lb) - Maximum Compression/Maximum

1=8-9-0, 3=8-9-0, 4=8-9-0

1=66 (LC 22), 3=66 (LC 23), 4=648

Rigid ceiling directly applied.

(LC 2)

1-2=-112/282, 2-3=-107/282

1-4=-198/162, 3-4=-198/162

Unbalanced roof live loads have been considered for

Truss designed for wind loads in the plane of the truss

only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1.

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially

Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 4-4-13, Exterior(2R) 4-4-13 to 7-4-13, Interior (1) 7-4-13 to 8-9-5 zone; cantilever left and right exposed ; end vertical left and right exposed:C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

Max Horiz 1=-56 (LC 12)

Loading	(psf)	Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.49	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 33 lb	FT = 20%

- 6) Plates checked for a plus or minus 5 degree rotation
- about its center.
- Gable requires continuous bottom chord bearing. 7)
- Gable studs spaced at 4-0-0 oc. 8)
- This truss has been designed for a 10.0 psf bottom 9) chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle
- 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1 and 33 lb uplift at joint 3.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



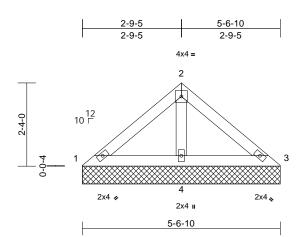
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof
M00210-B	V1	Valley	2	1	I73898335 Job Reference (optional)

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:42 ID:qjeiwieAyUMZAjNmWS13FbzByIN-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

2-9-5



Scale = 1:32.3

	· · · · ·	1				· · · ·					i	
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg) 15.4/20.0	Lumber DOL	1.15	BC	0.20	Vert(TL)	n/a	-	n/a	999		
TCDL	, 10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	3	n/a	n/a		
			IRC2021/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 20 lb	FT = 20%
LUMBER TOP CHORE BOT CHORE OTHERS BRACING TOP CHORE BOT CHORE BOT CHORE BOT CHORE BOT CHORE WEBS NOTES 1) Unbalan this desi 2) Wind: AS Vasd=95 II; Exp B Exterior(vertical li forces & DOL=1.6 3) Truss de only. Fo see Star	 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3 Structural wood she Rigid ceiling directly (size) 1=5-6-10, Max Horiz 1=-34 (LC Max Grav 1=80 (LC (LC 2)) (lb) - Maximum Corr Tension 1-2=-73/119, 2-3=-7 1-4=-92/90, 3-4=-92 2-4=-227/111 ced roof live loads have gn. SCE 7-16; Vult=120mph STOL=6.0psf; BC (ET C) (2) (2) (2) (2) (2) (2) (2) (2) (2) (2	applied. 3=5-6-10, 4=5-6-10 (22), 3=80 (LC 23), 4 apression/Maximum 3/119 /90 been considered for (3-second gust) DL=6.0psf; h=25ft; (velope) and C-C and right exposed; C for members and hown; Lumber the plane of the trust (normal to the face) d Details as applicat	8) Gable stu 9) This truss chord live 10) * This truss ed. 3-06-00 ta chord and 11) This truss structural chord and the bottor LOAD CASE		oc. for a 10. with any d for a liv as where vill fit betw s. t a minim applied d	D psf bottom other live loa re load of 20.0 a rectangle veen the botto um of 7/16" irectly to the t	Opsf om top				Weight: 20 lb	ROLIN
4) TCLL: A Plate DC 1.15 Plat	only. For study exposed to Wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pi=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10) Unbalanced snow loads have been considered for this						• -					
5) Unbalan design.	ced snow loads have be	een considered for th								111	AUGIN	EEEER
 Plates cl about its 	hecked for a plus or min center.	us 5 degree rotation									11, A. C	allouin

6) Plates checked for a plus or minus 5 degree rotation about its center.

June 3,2025

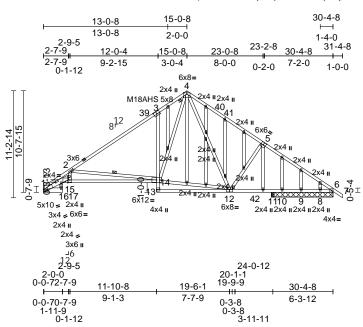
GI A. GILLIN

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Science Use Component Categories (http://www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	A2ST	Roof Special Structural Gable	1	1	Job Reference (optional)	173898336

Run: 25.20 E May 15 2025 Print: 25.2.0 E May 15 2025 MiTek Industries, Inc. Tue Jun 03 08:40:16 ID:6M6Kclz6V3WpE7kz_S3LRTzByDN-jf638SaDJfldHjvKxyKJr6R0u4cAx5i_2C0gMMzACnz

Page: 1



Scale =	1:120.9
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Plate Offsets (X, Y): [1:0-1-9,Edge], [1:0-2-6,0-0-15], [1:1-5-9,0-0-12], [2:0-2-13,0-1-8], [3:0-4-0,0-3-0], [5:0-3-0,0-3-4], [15:0-3-1,0-1-8], [17:0-0-6,0-2-8]	
Loading (psf) Spacing 2-0-0 CSI DEFL in (loc) l/defl L/d PLATE TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.84 Vert(LL) -0.24 14.15 >999 360 MT20 Snow (Pf/Pg) 15.4/20.0 Lumber DOL 1.15 BC 0.72 Vert(CT) -0.49 14.15 >587 240 M18AH TCDL 10.0 Rep Stress Incr YES WB 0.88 Horz(CT) 0.16 36 n/a n/a BCLL 0.0* Code IRC2021/TPI2014 Matrix-AS Wind(LL) 0.08 14-15 >999 240	244/190
	be applied directly to the top eetrock be applied directly to

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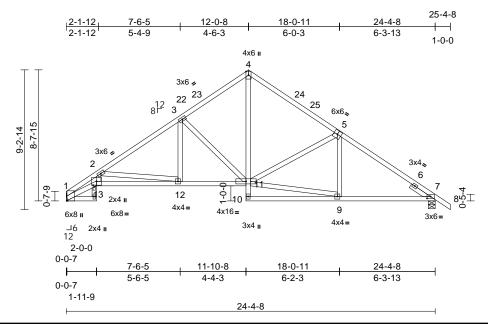


818 Soundside Road Edenton, NC 27932

June 3,2025

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	B1T	Roof Special	5	1	Job Reference (optional)	173898337

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:39 Page: 1 ID:kC9URBkKzyHs2ShCi?8sIHzByif-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:76.1

Plate Offsets (X, Y): [1:0-0-4,Edge], [7:0-2-13,0-1-8], [13:0-1-6,0-1-0]

- 1010 0110010 ()	, , , , , , , , , , , , , , , , , , ,	[°]									
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.54	Vert(LL)	-0.12	12-13	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.84	Vert(CT)	-0.26	12-13	>999	240		
TCDL	10.0	Rep Stress Incr	YES		WB	0.59	Horz(CT)	0.15	7	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-AS		Wind(LL)	0.07	12-13	>999	240		
BCDL	10.0					_						Weight: 149 lb	FT = 20%
LUMBER			3)		7-16; Pr=20.0 p	sf (roof I I	· I um DOI =	1 15					
TOP CHORD	2x4 SP No.2		0)		1.15); Pg=20.0 ps								
BOT CHORD	2x4 SP No.2 *Excep	t* 1-13:2x4 SP SS.			OL = 1.15); Is=1.								
	4-10,1-14:2x4 SP No			Exp.; Ce=1.0); Cs=1.00; Ct=1.	.10							
WEBS	2x4 SP No.3		4)		snow loads have	been cor	sidered for t	his					
SLIDER	Right 2x4 SP No.3 -	- 1-6-0		design.									
BRACING			5)		as been designed								
FOP CHORD	Structural wood she		ed.		psf or 2.00 times			st on					
BOT CHORD	Rigid ceiling directly	applied.	6)	0	on-concurrent wit ted for a plus or n			•					
REACTIONS	(size) 1= Mecha	inical, 7=0-5-8	0)	about its cer			gree rotation	1					
	Max Horiz 1=-138 (L	,	7)		as been designed	for a 10.0) psf bottom						
	Max Grav 1=964 (L0	C 2), 7=1041 (LC 2)	,		ad nonconcurrent			ads.					
FORCES	(lb) - Maximum Com	pression/Maximum	8)	* This truss I	nas been designe	ed for a liv	e load of 20.	0psf					
	Tension				m chord in all are								
TOP CHORD	1-2=-3022/13, 2-3=-		/104,		oy 2-00-00 wide v		veen the bott	om					
BOT CHORD	4-7=-1344/95, 7-8=0 1-13=-53/2580, 12-1		(1017 O)		ny other members								
SOT CHORD	10-11=0/116, 4-11=		/		er(s) for truss to t esign requires that								
	7-9=0/1057	13/100, 3 10=0/103	, IC	,	od sheathing be			ton					
WEBS	2-13=0/959, 3-11=-5	586/60, 9-11=0/977,			2" gypsum sheet								
	5-11=-335/85, 5-9=-	, ,	,	the bottom c				<i>y</i> 10					
	2-12=-1050/61		LC	DAD CASE(S)								UNI CA	
NOTES			_		otandard							TH UA	ROM
1) Unbalance	d roof live loads have	been considered for	r								~	ORIEESE	DAN'
this design										/	SE		Phi sin
	E 7-16; Vult=120mph									<u> </u>		0	W.L.
	ph; TCDL=6.0psf; BC		Cat.							-			
	Enclosed; MWFRS (er		0							=	:	SEA	L : =
	E) 0-3-8 to 3-3-8, Inter R) 12-4-0 to 15-4-0. In		0,							Ξ		0363	22 : =

Vasd=95mph; 1CDL=6.0pst; BCDL=6.0pst; h=25ft; Cat II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-3-8 to 3-3-8, Interior (1) 3-3-8 to 12-4-0, Exterior(2R) 12-4-0 to 15-4-0, Interior (1) 15-4-0 to 25-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



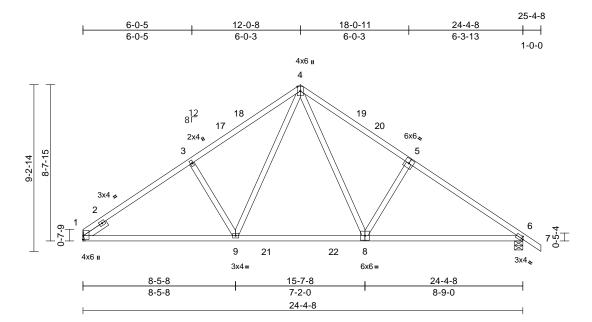
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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	B1A	Common	1	1	Job Reference (optional)	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:39 ID:kC9URBkKzyHs2ShCi?8sIHzByif-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:63.8

Plate Offsets (X, Y): [1:Edge,0-0-3], [6:0-1-1,0-1-8]

		1					· · · · ·						
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.52	Vert(LL)	-0.12	8-16	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.76	Vert(CT)	-0.26	8-16	>999	240		
TCDL	10.0	Rep Stress Incr	YES		WB	0.26	Horz(CT)	0.03	6	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-AS		Wind(LL)	0.03	8-16	>999	240		
BCDL	10.0											Weight: 124 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.3 Left 2x4 SP No.3	athing directly applie applied. anical, 6=0-5-8 .C 12) .C 30), 6=1153 (LC apression/Maximum 1327/124, =0/45 22/1207	8) 31) 9) 10	load of 12.0 overhangs r Plates checi about its cer This truss ha chord live lo * This truss on the botto 3-06-00 tall chord and a Refer to girc)) This truss di structural wo	as been designed ad nonconcurrent has been designed m chord in all area by 2-00-00 wide w ny other members der(s) for truss to tr esign requires that bood sheathing be a /2" gypsum sheetr chord.	flat roof k h other lin inus 5 de for a 10.0 with any d for a liv as where rill fit betv , with BC russ conr : a minim applied d	oad of 15.4 p ve loads. egree rotation 0 psf bottom other live loa re load of 20. a rectangle veen the bott CDL = 10.0ps um of 7/16" irectly to the	osf on n ads. .0psf tom top					
NOTES													
 Unbalance this design 	ed roof live loads have	been considered fo	r									OR EESS	11.
	CE 7-16; Vult=120mph	(3-second gust)										11'''L CA	D'III
	nph; TCDL=6.0psf; BC		Cat.									THUA	NOY.
	Enclosed; MWFRS (er										S	ON. FESS	6 An
II; Exp B;	Enclosed; MWFRS (er	nvelope) and C-C									Sec.	O'. EESS	6: 10%

II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-3-8 to 3-3-8, Interior (1) 3-3-8 to 12-4-0, Exterior(2R) 12-4-0 to 15-4-0, Interior (1) 15-4-0 to 25-8-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

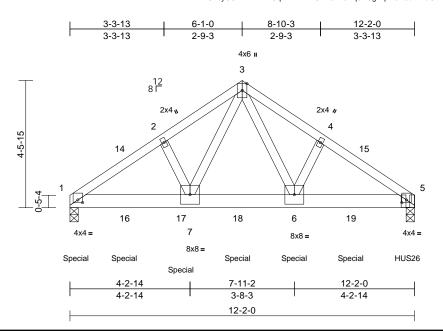


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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	S1GT	Common Girder	1	2	Job Reference (optional)	173898339

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:42 ID:wFt1BIG?syCS21FBZFfDSqzBw47-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:40.7

Plate Offsets (X, Y): [1:0-2-0,0-1-2], [5:0-2-0,0-1-2]

Loading TCLL (roof) Snow (Pf/Pg)	(psf) 20.0 15.4/20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15		CSI TC BC	0.25 0.78	DEFL Vert(LL) Vert(CT)	in -0.04 -0.07	(loc) 6-7 6-7	l/defl >999 >999	L/d 360 240	PLATES MT20	GRIP 244/190
TCDL	10.0	Rep Stress Incr	NO		WB	0.78	Horz(CT)	0.07	5	>999 n/a	240 n/a		
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-MS	0.50	Wind(LL)	0.02	7-10	>999	240		
BCDL	10.0	0000						0.00			2.0	Weight: 138 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD WEBS NOTES 1) 2-ply truss (0.131*x3" Top chord oc. Bottom ch staggered Web conn 2) All loads a except if n CASE(S) 3 provided tu unless oth	2x4 SP No.2 2x6 SP No.2 2x4 SP No.3 Structural wood she: 5-11-7 oc purlins. Rigid ceiling directly bracing. (size) 1=0-3-8, 5 Max Horiz 1=-65 (LC Max Grav 1=4071 (L (lb) - Maximum Com Tension 1-2=-4147/0, 2-3=-4 4-5=-4112/0 1-7=0/3446, 6-7=0/2 3-6=0/2322, 4-6=-22 2-7=-231/43 to be connected toged) nails as follows: s connected as follows: ords connected as follows: ords connected as follows: at 0-9-0 oc. ected as follows: 2x4 - tre considered equally tode as front (F) or bas section. Ply to ply com o distribute only loads iervise indicated.	applied or 10-0-0 oc 5=0-3-8 :8) .C 2), 5=3753 (LC 2) pression/Maximum 096/0, 3-4=-4058/0, 391, 5-6=0/3410 20/47, 3-7=0/2395, ther with 10d s: 2x4 - 1 row at 0-9-0 cows: 2x6 - 2 rows 1 row at 0-9-0 oc. applied to all plies, ck (B) face in the LO. nections have been noted as (F) or (B),	d or 5) 6) 7) 8) 9) 10) 12) AD 10) 12)	Vasd=95mpH II; Exp B; End and right exp Lumber DOL TCLL: ASCE Plate DOL=1 1.15 Plate DOL Exp.; Ce=1.0 Unbalanced design. Plates check about its cen This truss ha chord live loa * This truss ha on the bottom 3-06-00 tall b chord and ar Use Simpsor Truss) or equ connect truss Fill all nail ho Hanger(s) or provided suff Ib down at 0 3-11-4, 944 I 7-11-4, and 2 The design/S responsibility AD CASE(S) Dead + Snc Increase=1. Uniform Loa Vert: 1-33 Concentrate Vert: 6=-i	s been designed f d nonconcurrent t has been designed n chord in all areas y 2-00-00 wide wi y other members. h Strong-Tie HUS2 uivalent at 11-11-4 s(es) to front face e les where hanger other connection cicient to support c -0-0, 944 lb down at 9-1 election of such co of others. Standard wy (balanced): Lur 15	CDL=6.(enveloped) I left and OCL=1.60 f (roof LL); Pf=15.4; Rough 0 been cor inus 5 de for a 10.0 with any 1 for a liv s where II fit betw 26 (14-11 f from the of bottom is in cor device(s oncentra at 1-11- , and 94- 11-4 on h onnectio mber Inc 1=-20 (F), 11=-	Dipsf; h=25ft;); cantilever I right expose) : Lum DOL= psf (Lum DC Cat B; Partial Isidered for the egree rotation) psf bottom other live load e load of 20.1 a rectangle veen the botthe Dd Girder, 4-7 e left end to n chord. tact with lum) shall be ted load(s) 1 4, 944 lb down at pottom chord n device(s) is rease=1.15, 1 8664 (F), 16=-	left ed; 1.15 DL = illy his n dds. Opsf om 10d ber. 292 wn at s the Plate		M. antitute.		SEA 0363	ROW INTERNET

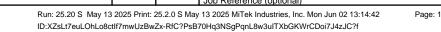
June 3,2025

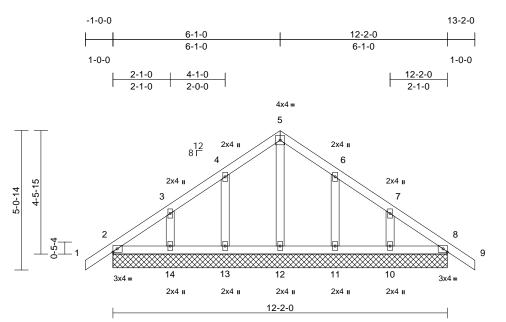
Page: 1

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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	S1G	Common Supported Gable	1	1	Job Reference (optional)	173898340





00010 = 1.41.0													
Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15		CSI TC	0.09	DEFL Vert(LL)	in n/a	(loc) -	l/defl n/a	L/d 999	PLATES MT20	GRIP 244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.03	Vert(CT)	n/a	-	n/a	999	-	
TCDL	10.0	Rep Stress Incr	YES		WB	0.04	Horz(CT)	0.00	19	n/a	n/a		
BCLL BCDL	0.0* 10.0	Code	IRC202	1/TPI2014	Matrix-AS							Weight: 61 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	Rigid ceiling directly (size) 2=12-2-0, 11=12-2-0 14=12-2-0 Max Horiz 2=76 (LC Max Uplift 10=-17 (L 13=-15 (L 10=174 (L 10=174 (L 10=174 (L	, 8=12-2-0, 10=12-2- 0, 12=12-2-0, 13=12 0 15) .C 17), 11=-15 (LC 1 .C 16), 14=-18 (LC 1 C 2), 8=158 (LC 2), LC 24), 11=222 (LC 2) LC 2), 13=222 (LC 2) LC 23)	0, -2-0, 3) 7), 6) 4) 24),	Vasd=95mpl II; Exp B; En (3E) -1-0-0 tr (3R) 6-1-0 to cantilever lef right exposed for reactions DOL=1.60 Truss design only. For stu see Standard or consult qu TCLL: ASCE Plate DOL=1 1.15 Plate D Exp.; Ce=1.0	7-16; Vult=120mp h; TCDL=6.0psf; B closed; MWFRS (o 0 2-1-0, Exterior(2) 0 9-1-0, Exterior(2) t and right expose d;C-C for members shown; Lumber D hed for wind loads uds exposed to wird d Industry Gable E tailified building de: 7-16; Pr=20.0 psf; OL = 1.15); Is=1.0 0; Cs=1.00; Ct=1.1 snow loads have I	CDL=6. envelope N) 2-1-0 J) 9-1-0 d; end v s and fo OL=1.6 in the pl nd (norm nd Deta signer a f (roof LI ; Pf=15.4 ; Rough 0	Opsf; h=25ft; (a) and C-C Co to 6-1-0, Cor to 13-2-0 zon vertical left an rcces & MWFF D plate grip ane of the tru all to the face ils as applical s per ANSI/Tf 2: Lum DOL= 4 psf (Lum DC) C at B; Partia	Cat. orner ner e; id XS ss), ble, PI 1. 1.15 DL = Illy	stru cho	rd and 1 bottom	ood sh /2" gy chord.	psum sheetrock	inimum of 7/16" ed directly to the top be applied directly to
TOP CHORD	Tension 1-2=0/45, 2-3=-74/5		6)	This truss ha	as been designed f								
BOT CHORD	4-5=-75/119, 5-6=-7 7-8=-65/51, 8-9=0/4 2-14=-47/100, 13-14	75/119, 6-7=-65/54, 5 4=-26/100,	7)	overhangs n	psf or 2.00 times fl on-concurrent with ked for a plus or mi iter.	other li	ve loads.					OR FESS	11111
	12-13=-26/100, 11-1 10-11=-26/100, 8-10	,	8)		es continuous bott		d bearing.					TH CA	Roit
WEBS		82/94, 3-14=-133/10	9))1, 1(spaced at 2-0-0 or as been designed f		0 psf bottom				Nº.	ONEESS	in Nin

- WEBS 5-12=-89/0, 4-13=-182/94, 3-14=-133/101, 6-11=-182/94, 7-10=-133/101
- NOTES

1) Unbalanced roof live loads have been considered for this design.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.11) * This truss has been designed for a live load of 20.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 13, 18 lb uplift at joint 14, 15 lb uplift at joint 11 and 17 lb uplift at joint 10.

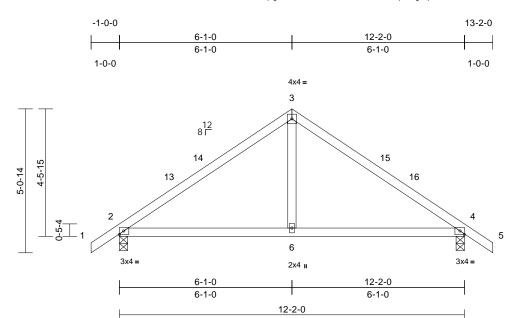


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A MiTek Affili 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof	
M00210-B	S1	Common	2	1	Job Reference (optional)	173898341

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:42 Page: 1 ID:fbC6kBCNGqcgxMivY1ToKOzBwZC-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:40.6

TCLL (roof)	20.0						in	(loc)	l/defl	- L U U	PLATES	GRIP
	20.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.05	6-12	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.38	Vert(CT)	-0.08	6-12	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS		Wind(LL)	0.02	6-9	>999	240		
BCDL	10.0										Weight: 50 lb	FT = 20%

BOT CHORD	2x4 SP N	0.2				
WEBS	2x4 SP N	0.3				
BRACING						
TOP CHORD	Structura	wood sheathing directly applied.				
BOT CHORD	Rigid ceil	ing directly applied.				
REACTIONS	(size)	2=0-3-8, 4=0-3-8				
	Max Horiz 2=76 (LC 15)					
	Max Grav	2=547 (LC 2), 4=547 (LC 2)				
FORCES	(lb) - Max	imum Compression/Maximum				

- Tension TOP CHORD 1-2=0/45, 2-3=-583/88, 3-4=-583/88, 4-5=0/45BOT CHORD 2-6=-17/411, 4-6=-23/411
- WEBS 3-6=0/276

NOTES

- Unbalanced roof live loads have been considered for 1) this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 6-1-0, Exterior(2R) 6-1-0 to 9-1-0, Interior (1) 9-1-0 to 13-2-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this desian.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- about its center.
- 7) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 20.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- This truss design requires that a minimum of 7/16" 9) structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

C The state of the s 11111111111 SEAL 036322 G mm June 3,2025

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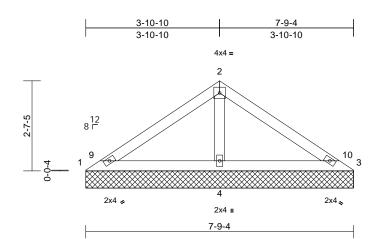


Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof		
M00210-B	V7	Valley	1	1	Job Reference (optional)	173898342	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:43 ID:MyflhHk1YWxvaswmCKDvyyzBvwU-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

3-10-10



Scale = 1:33.4

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.38	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 27 lb	FT = 20%

TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.3
OTHERS	2x4 SP N	0.3
BRACING		
TOP CHORD	Structural	wood sheathing directly applied.
BOT CHORD	Rigid ceili	ing directly applied.
REACTIONS	(size)	1=7-9-4, 3=7-9-4, 4=7-9-4
	Max Horiz	1=-39 (LC 12)
	Max Uplift	1=-10 (LC 23), 3=-10 (LC 22)
	Max Grav	1=79 (LC 22), 3=79 (LC 23), 4=539
		(LC 2)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	

TOP CHORD 1-2=-76/235, 2-3=-76/235 BOT CHORD 1-4=-171/113, 3-4=-171/113 WEBS 2-4=-390/141

NOTES

Unbalanced roof live loads have been considered for 1) this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-6 to 3-0-6, Interior (1) 3-0-6 to 3-11-0, Exterior(2R) 3-11-0 to 6-10-7, Interior (1) 6-10-7 to 7-9-10 zone; cantilever left and right exposed ; end vertical left and right exposed:C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable. or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- design.
- 6) Plates checked for a plus or minus 5 degree rotation
- about its center.
- Gable requires continuous bottom chord bearing. 7)
- Gable studs spaced at 4-0-0 oc. 8)
- This truss has been designed for a 10.0 psf bottom 9) chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom
- chord and any other members. 11) Provide mechanical connection (by others) of truss to
- bearing plate capable of withstanding 10 lb uplift at joint 1 and 10 lb uplift at joint 3.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Arlington Rev.1-Elev 4-Roof
M00210-B	V6	Valley	1	1	I73898343 Job Reference (optional)

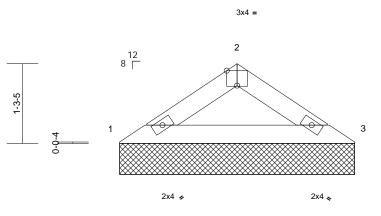
Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Mon Jun 02 13:14:43 ID:?_sPdZhuk_JdT41pQnekFuzBvwZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

818 Soundside Road Edenton, NC 27932



3-9-4



Scale = 1:18.5

Plate Offsets (X, Y): [2:0-2-0, Edge]

	K, f). [2.0-2-0,Euge]											
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MP	0.11 0.15 0.00	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 20%
FORCES TOP CHORD BOT CHORD NOTES 1) Unbalance- this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E vertical left forces & M DOL=1.60 3) Truss desig only. For s see Standa or consult of 1,15 Plate Exp.; Ce=1 5) Unbalance- design.	Max Horiz 1=18 (LC Max Grav 1=152 (LC (Ib) - Maximum Com Tension 1-2=-223/70, 2-3=-2: 1-3=-47/179 d roof live loads have E 7-16; Vult=120mph ph; TCDL=6.0psf; BC inclosed; MWFRS (en c) zone; cantilever left and right exposed;C- WFRS for reactions si plate grip DOL=1.60 gned for wind loads in ituds exposed to wind ard Industry Gable Enq qualified building desig E 7-16; Pr=20.0 psf; F DOL = 1.15); Is=1.0; F .0; Cs=1.00; Ct=1.10 d snow loads have be cked for a plus or minu	applied or 10-0-0 or 3=3-9-4 13) 2 22), 3=152 (LC 23 pression/Maximum 23/70 been considered for (3-second gust) DL=6.0psf; h=25ft; (vielope) and C-C and right exposed ; C for members and hown; Lumber the plane of the trust (normal to the face) d Details as applicat gner as per ANSI/TF roof LL: Lum DOL=- 2f=15.4 psf (Lum DOL 2f=15.4 psf (Lum D	8) Gable stu 9) This truss chord live 10) * This truss chord and LOAD CASE() r Cat. end ss ble, 21.1. 1.15 DL = lly his	uires continuous bo ds spaced at 4-0-0 has been designed load nonconcurren s has been designed tom chord in all are ill by 2-00-00 wide v any other member S) Standard	oc. d for a 10.4 t with any ed for a liv eas where will fit betw	0 psf bottom other live loa re load of 20.0 a rectangle	Opsf				SEA 0363	• -
					U 7470 min 4							2017/2012/15

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