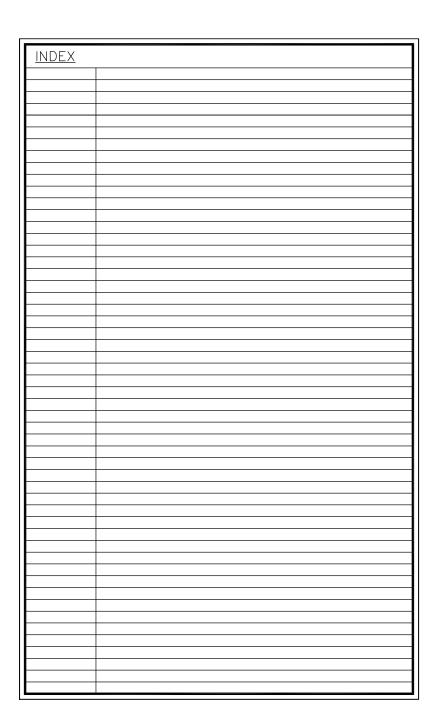
ARLINGTON-RALE

RALEIGH- LOT 00.0124 BLAKE POND SF

(MODEL# 1630) ELEVATION 4 - GL

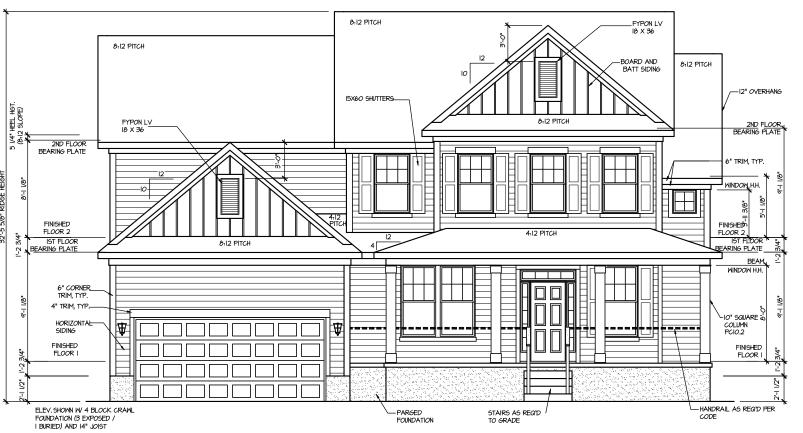




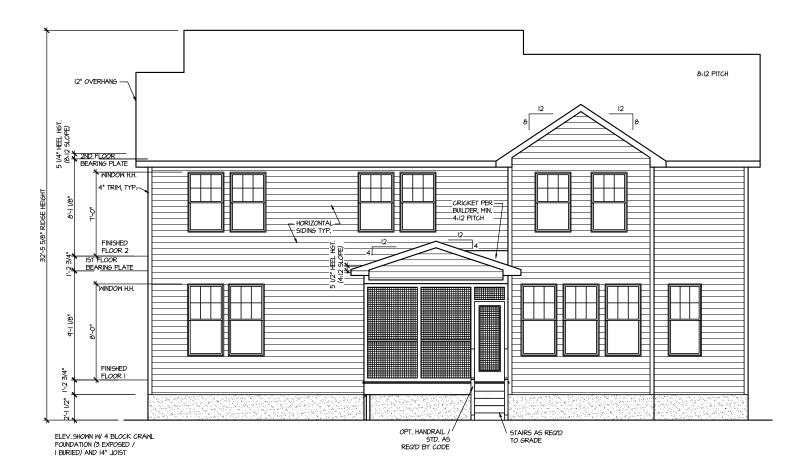
AREA CALCULATIONS ELEVATION 4 FIRST FLOOR GARAGE FRONT PORCH — ELEVATION 4 SECOND FLOOR OPTIONS EXT. BREAKFAST SITTING ROOM SCREENED PORCH		HEATED 1240 SF 1274 SF 60 SF 60 SF	COVERED / UNHEATED 496 SF 201 SF	UNCOVERED
	TOTAL	2634 SF	817 SF	

129 Celtic Lane

LOT	SPECIFIC	
1	LOT 00.0124	BLAKE POND SF
		ARLINGTON REV. RALE 1 ELEVATION 4
2	ADDRESS	129 CELTIC LANE LILLINGTON, NC 27546
	-	
	 	
	-	
	-	
	-	



FRONT ELEVATION 4 SCALE: 1/8" = 1'-0"

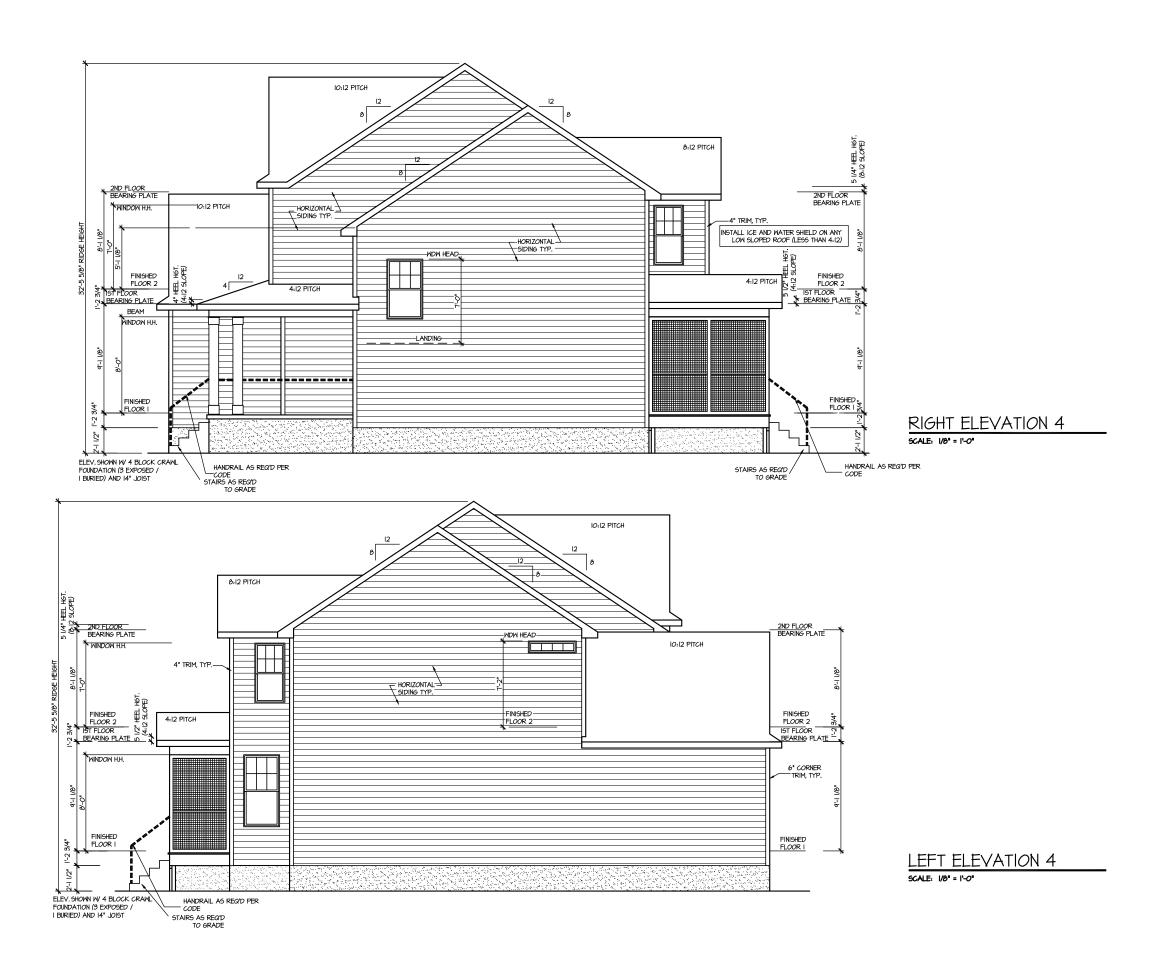


REAR ELEVATION 4

DRAWN BY: DATE: 05/23/2025 PLAN NO. 1630



0 8 0 EVATI ᇳ 07 07 07 HOUSE NAME:
ARLINGTON
DRAWING TITLE
FRONT & REAR



| MASTER PLAN INFORMATION | REVISION | DATE | 10-18-2019 | 04-29-2024 |

DRAWN BY: ITS DATE: 05/23/2025 PLAN NO. 1630



HOUSE NAME:
ARLINGTON
DRAWING TITLE
RIGHT & LEFT ELEVATIONS

SHEET No.

A|.

ATTIC VENT CALCULATION FOR PLAN '4'

ATHE VENT CALCULATION FOR PLAN

ROOF VENTILATION CALCULATIONS:

ROOF MERA I = TID 50. FT.

OMERAL REGISTED VENTILATION.

1 10 300 = 5.72 50. FT.

1 10 300 = 5.72 50. FT.

50% IN TOP THEFD = 2.06 50. FT. (1 TO 300)

MET FREE AREA OF WINTED SOFTT = 5.1 SO. IN / LINEAR FT.

NET FREE AREA OF RIDGE VENTI = 10 50. IN / LINEAR FT. LOHER VENTING. (BOTTOM 2/3 RDS.)
404 LINEAR FEET OF SOFFIT X 5.1 50. IN. = 9.39 50. FT.
407 LINEAR FEET OF RIDGE X 16 50. IN. = 3 50. FT.
3 50. FT. AT 50%
1 TO 300 ALLOWED)
1 TOTAL ROOP VENTILATION 6.33 50. FT. > 5.12 50. FT. (RQTD)

ROOF VENTILATION CALCULATIONS:
ROOF AREA 2 = 191 50, FT.
0/1EASL REGUERS VENTILATION,
110 50 = 10 19 50, FT.
110 500 = 0.60 50, FT.
110 500 = 0.60 50, FT.
110 100 = 0.60 50, FT. NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 10 SQ. IN / LINEAR FT.

LOWER VENTING: (BOTTOM 2/3 RDS)
45 LINEAR FEET OF SOFFIT X 5.7 SQ. IN. = 1.78 SQ. FT.
TOTAL ROOF VENTILATION: 1.78 SQ. FT. > 0.48 SQ. FT. (RQ'D)

NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 18 SQ. IN/ LINEAR FT.

LOVER VINTING. (BOTTOM 2/3 RDS.)

20 LINEAR FEET OF SOFFIT X 5.1 50. IN. = 0.74 50. FT.

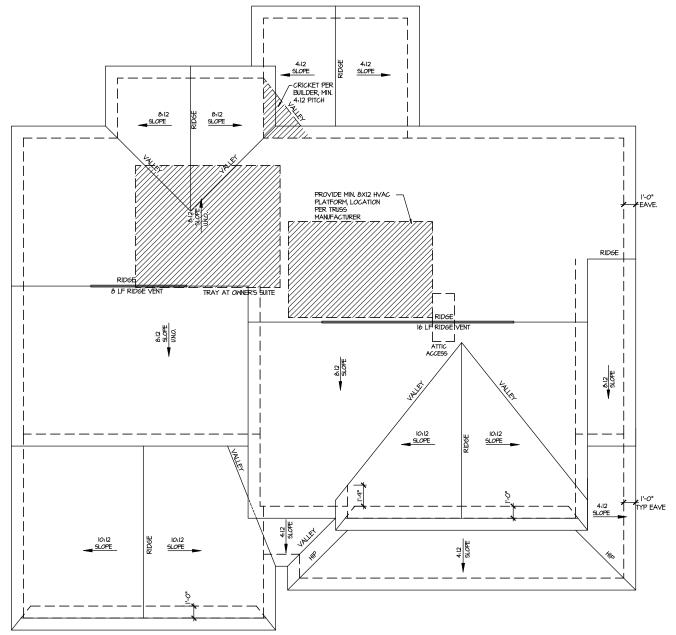
UPDER VERBLES, (RDF 1/3 RD)

9 LINEAR FEET OF RIDGE X ID 50. IN. = 1 50. FT.

150. FT. AT 50%

10 TO 300 ALLOWED.

10 TO ALLOWED.



ROOF PLAN ELEV. 4 SCALE: 1/8" = 1'-0"

HOUSE NAME:
ARLINGTON
DRAWING TITLE

DRAWN BY:

DATE: 05/23/2025 PLAN NO. 1630

SHEET No.

A1.3

Ø 0 ∏

CRAML SPACE VENT CALCULATIONS: ELEV 4
CRAML AREA = 1236 SQ. FT.
OVERALL REQUIRED VENTILATION:
I SQ. IN. PER I SQ. FT. = 1236 SQ. IN.

NET FREE AREA OF VENT = 12 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

<u>VENTING REQUIREMENT:</u> 1238 SQ. IN. / 72 SQ. IN. = 17.2 VENTS = 18 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED.

VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1

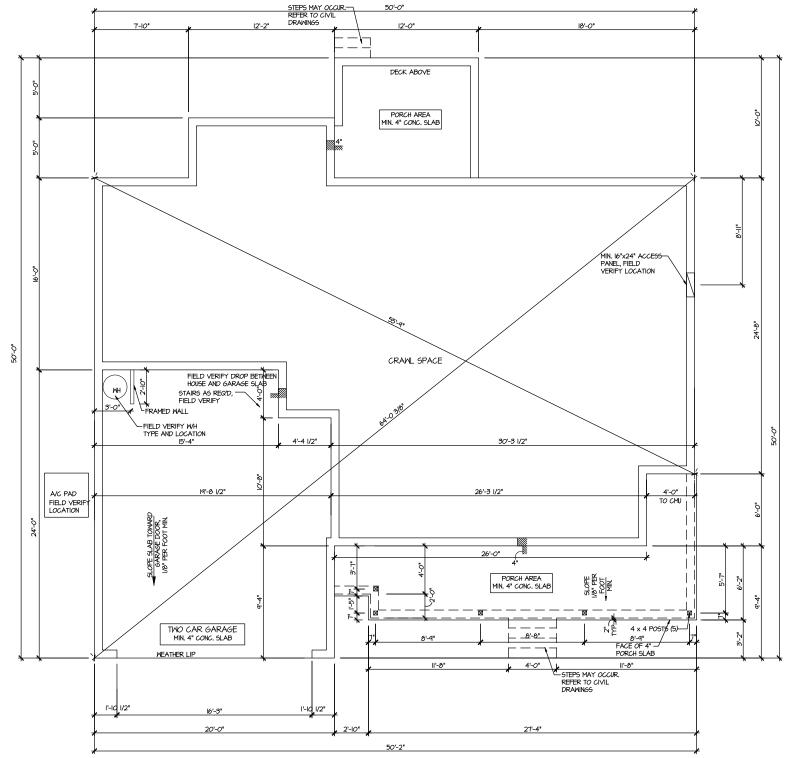
CRAML SPACE VENT CALCULATIONS: OPT. EXT. KIT/BPKFST CRAML AREA = 61 SQ. FT. OVERALL REQUIRED VENTILATION. I SQ. IN. PER I SQ. FT. = 61 SQ. IN.

NET FREE AREA OF VENT = 72 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

VENTING REQUIREMENT: 61 SQ. IN. / 72 SQ. IN. = .8 VENTS = 1 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED.

VENTS SHALL BE INSTALLED PER R322.2.2 - R322.2.2.1



ELEVATION 4 CRAWL SPACE PLAN SCALE: 1/8" = 1'-0"

DRAWN BY:

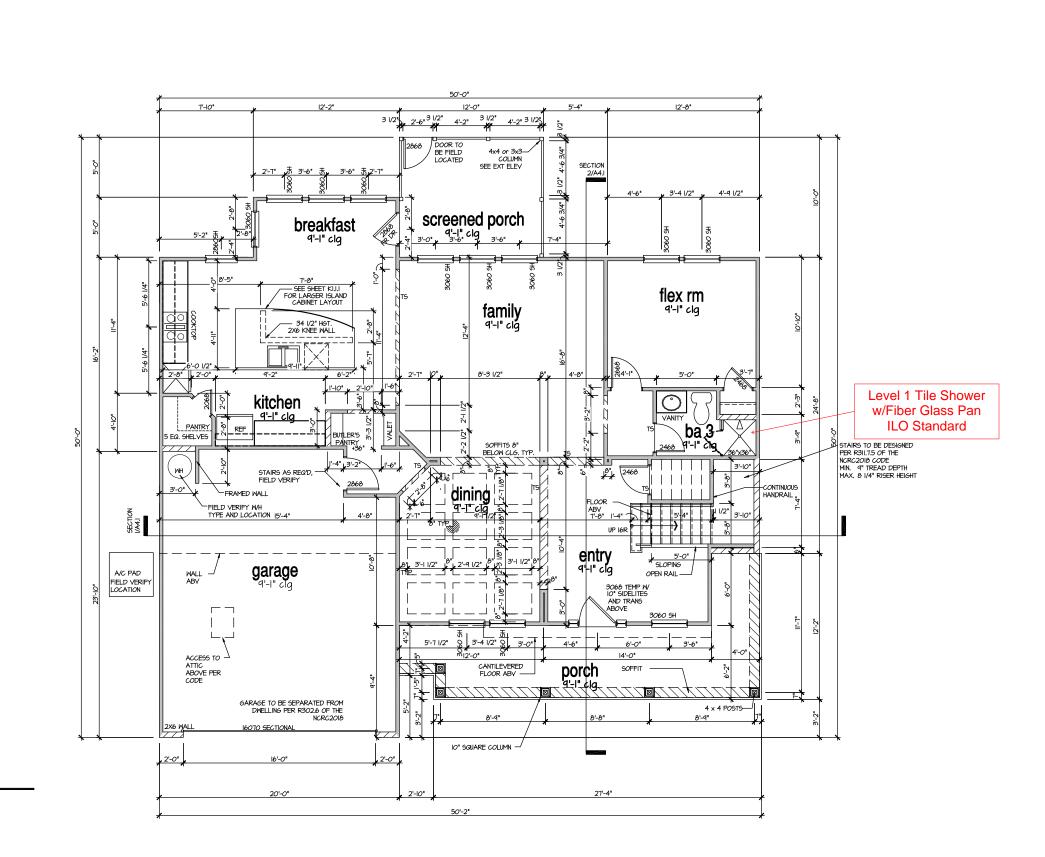
DATE: 05/23/2025 PLAN NO. 1630



HOUSE NAME:
ARLINGTON
DRAWING TITLE SPACE

SHEET No.

A2.



ELEVATION 4
FIRST FLOOR PLAN
SCALE: 1/0"

| MASTER PLAN INFORMATION | REVISION | DATE | 10-18-2019 | 04-29-2024 |

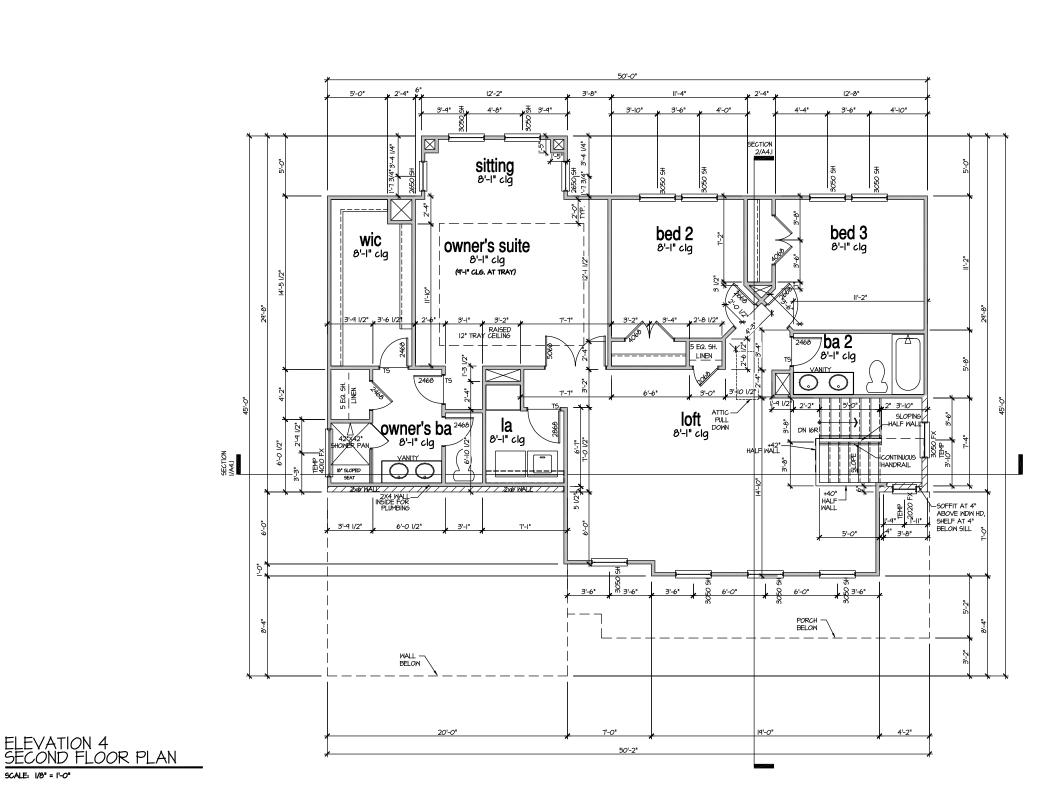
DRAWN BY: ITS DATE: 05/23/2025 PLAN NO. 1630



HOUSE NAME:
ARLINGTON
DRAWING TITLE
FIRST FLOOR PLAN

SHEET No.

A3.



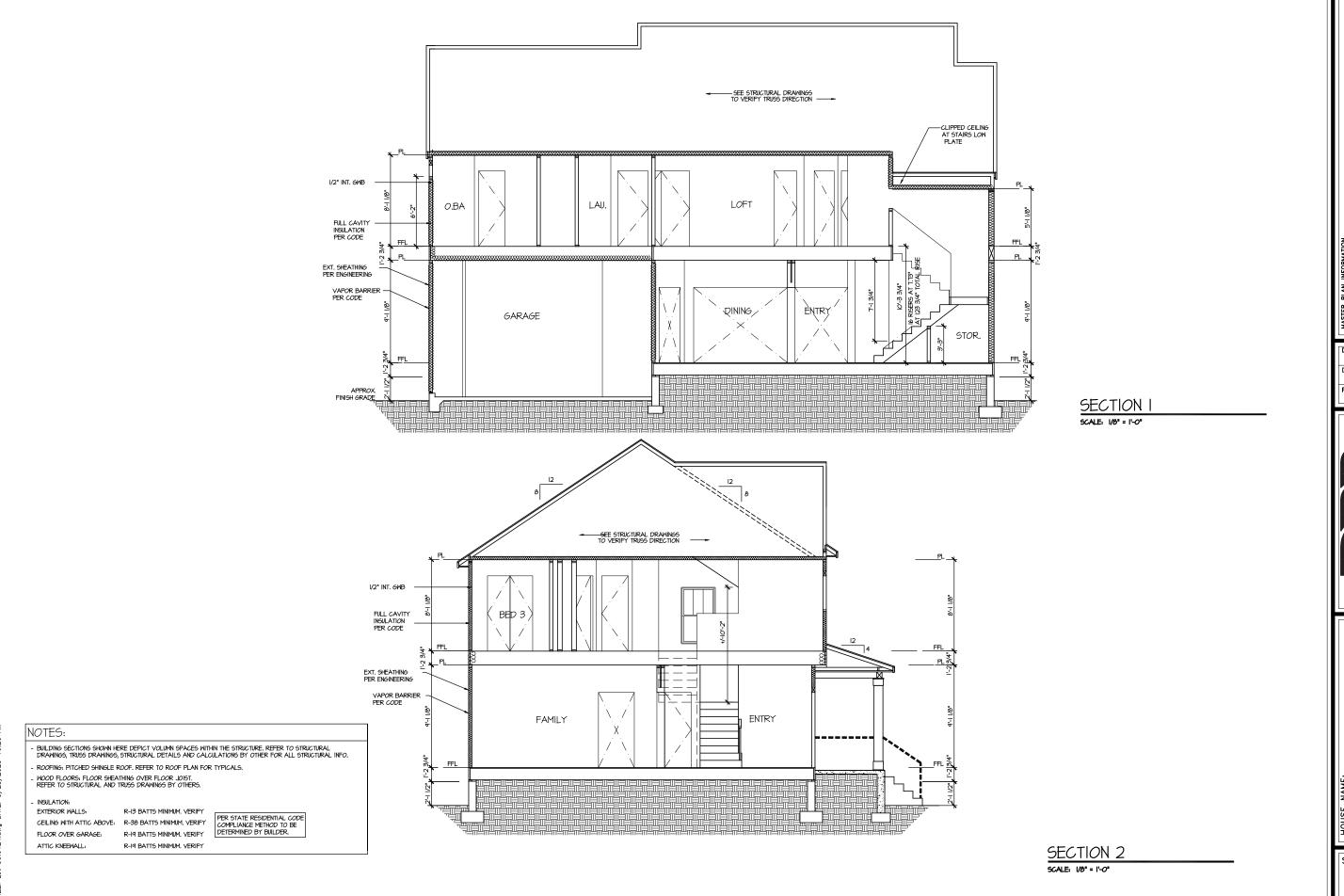
DRAWN BY: DATE: 05/23/2025 PLAN NO. 1630

HOUSE NAME:
ARLINGTON
DRAWING TITLE
SECOND FLOOF

SHEET No.

A3.2

SCALE: 1/8" = 1'-0"



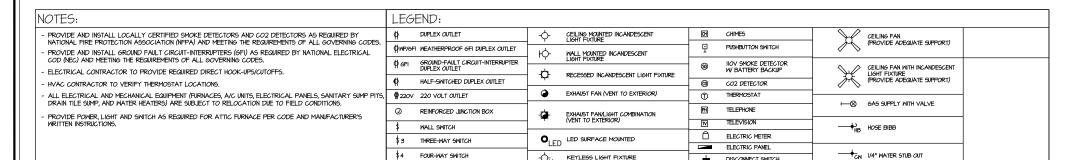
DRAWN BY:
ITS

DATE:
05/23/2025

PLAN NO. 1630



HOUSE NAME:
ARLINGTON
DRAWING TITLE
BUILDING SECTION

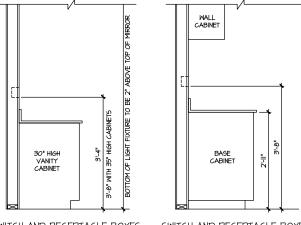


-OK KEYLESS LIGHT FIXTURE

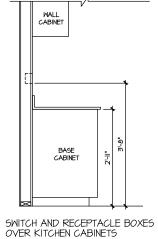
FOUR-WAY SWITCH

-]	ν –	HANGING LIGHT	FIXTURE			*
	I'-O" OUTETS / PHOME / TV RECEPTACLES	3-6" SWITCHES 4-0"	GARAGE GFI / BASEMENT WALL OUTLETS / KITCHEN REFRIGERATOR / WASHER/DRYER OUTLETS	H-6"	5-4" (BOTTOM OF FIXTURE) DINING AND BREAKFAST FIXTURE 6-8"	FRONT DOOR COACH LIGHTS	DOORBELL CHINES / 6ARAGE DOOR COACH LIGHTS

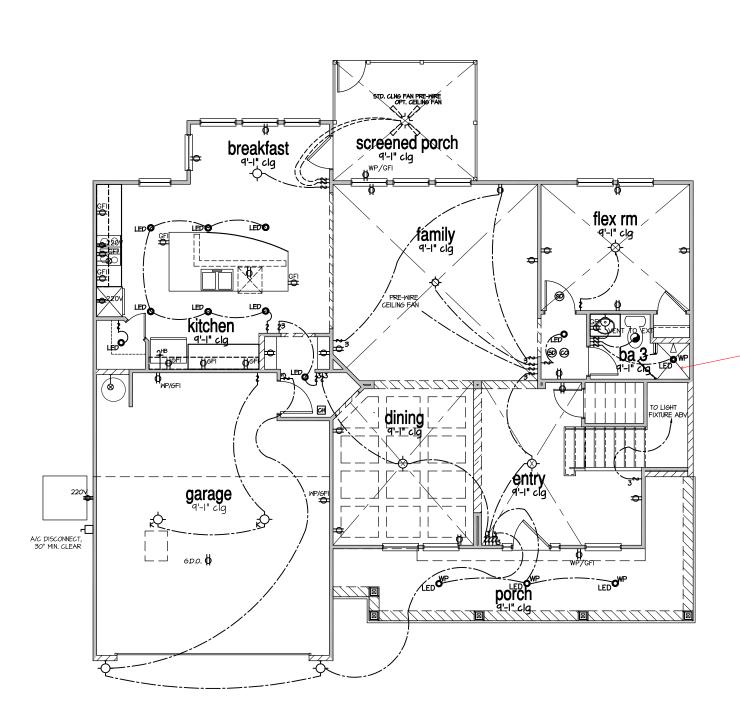
STANDARD ELECTRICAL BOX HEIGHTS



SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS



ELECTRICAL PLAN FIRST FLOOR - ELEV. 4 SCALE: 1/8" = 1'-0"



DISCONNECT SMITCH

Level 1 Tile Shower w/Fiber Glass Pan **ILO Standard**

DRAWN BY:

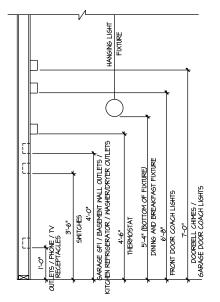
DATE: 05/23/2025 PLAN NO. 1630



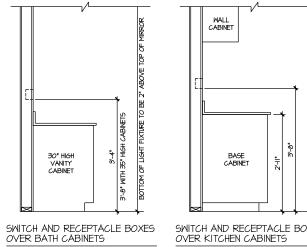
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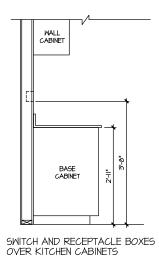
HOUSE NAME:
ARLINGTON
DRAWING TITLE

NOTES:	LEGEND:			
- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY	puplex outlet		CHI CHIMES	CEILING FAN
NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL	ØWP/GFI WEATHERPROOF GFI DUPLEX OUTLET	1	□ □	(PROVIDE ADEQUATE SUPPORT)
COD (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.	() GEL GROUND-FAULT CIRCUIT-INTERRUPTER	- WALL MOUNTED INCANDESCENT LIGHT FIXTURE	IIOV SMOKE DETECTOR	
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.	I " DUPLEX OUTLET	-Ö- RECESSED INCANDESCENT LIGHT FIXTURE	W BATTERY BACKUP	CEILING FAN WITH INCANDESCENT LIGHT FIXTURE
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.	HALF-SWITCHED DUPLEX OUTLET	T 12023525 110740235211 21011 191072	⊚ CO2 DETECTOR	(PROVIDE ADEQUATE SUPPORT)
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS. - PROVIDE POMER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S MERITTEN INSTRUCTIONS.	₱220V 220 VOLT OUTLET	EXHAUST FAN (VENT TO EXTERIOR)	① THERMOSTAT	O CICCURTY WITH VIDE
	REINFORCED JUNCTION BOX	EXHAUST FAVLIGHT COMBINATION (VENT TO EXTERIOR) OLED LED SURFACE MOUNTED	PH TELEPHONE	→ → GAS SUPPLY WITH VALVE
	\$ WALL SWITCH		TELEVISION	— ∳ _{un} Hose Bibb
			☐ ELECTRIC METER	HB 1632 3130
	\$ 3 THREE-WAY SWITCH		ELECTRIC PANEL	
	\$4 FOUR-WAY SMITCH	- ${\diamondsuit}_{\overline{k}}$ KEYLESS LIGHT FIXTURE	DISCONNECT SWITCH	CM 1/4" WATER STUB OUT

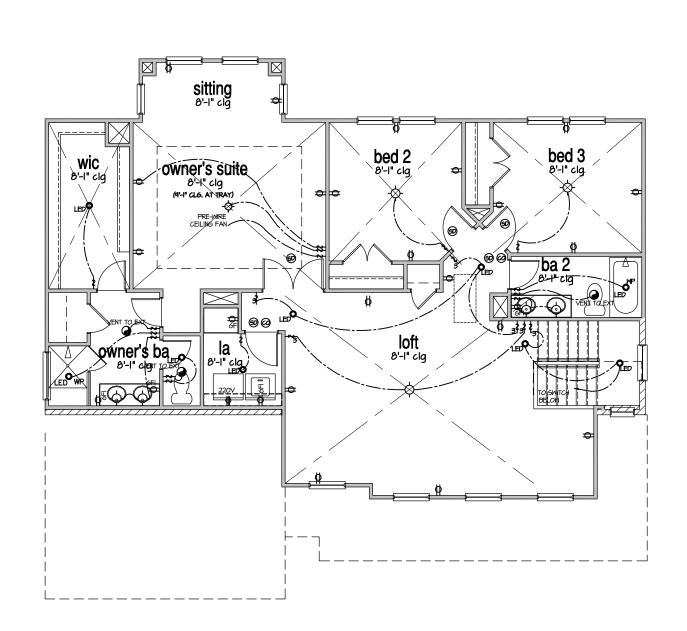


STANDARD ELECTRICAL BOX HEIGHTS





ELECTRICAL PLAN SECOND FLOOR - ELEV. 4 SCALE: 1/8" = 1'-0"

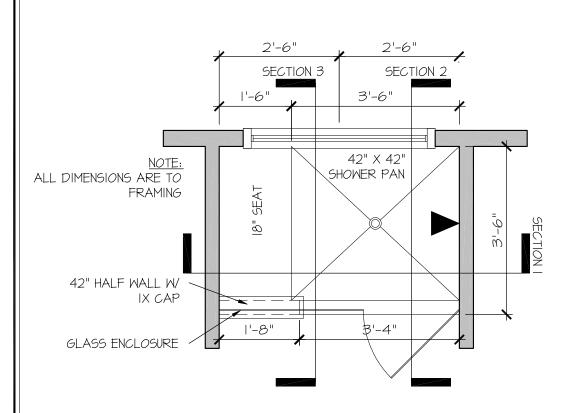


DRAWN BY:

DATE: 05/23/2025 PLAN NO. 1630

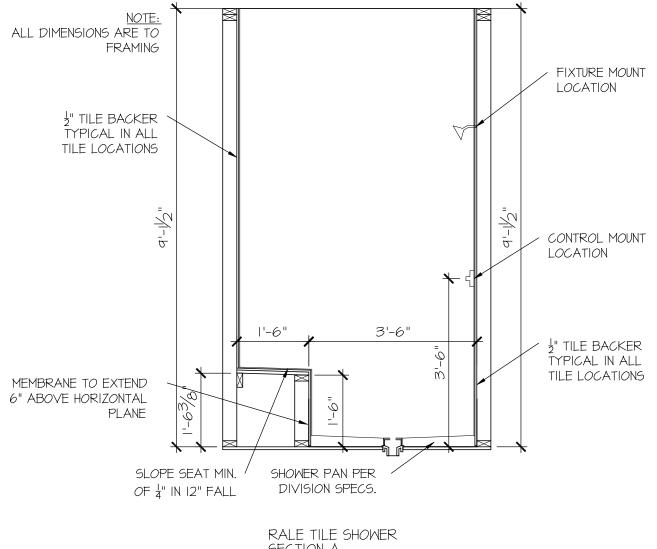


Ш ᇳ HOUSE NAME:
ARLINGTON
DRAWING TITLE
SECOND FLOOF



RALE TILE SHOWER 42" X 42" W 18" SEAT

SCALE: 1/2" = 1'-0"



SECTION A

SCALE: 1/2" = 1'-0"

CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE

24 X 36 SCALE



DETAIL SHOWER RALE



SEAL

DRAWN BY:
L. BEAVERS
DATE: 9/1/22

PLAN NO.

24 X 36 SCALE

~ "

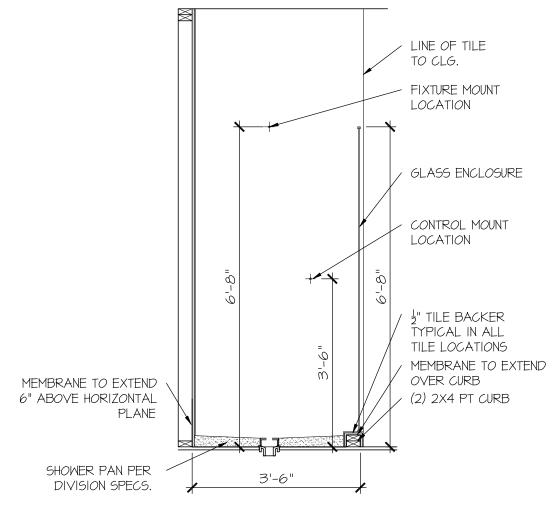


E ILE SHOWER DETAIL

OUSE NAME:

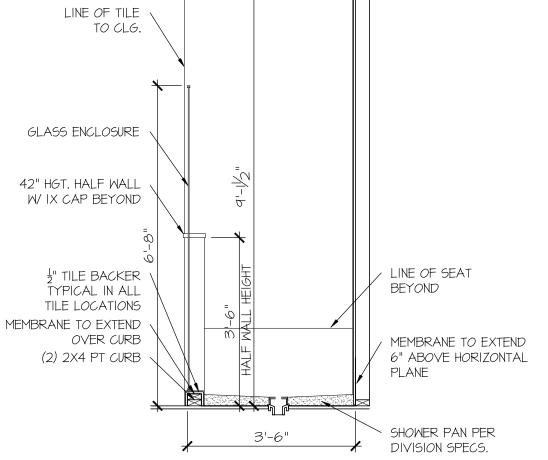
SHEET No.

P||.2



SCALE: 1/2" = 1'-0"





SCALE: 1/2" = 1'-0"

DESCRIPTION OF BLDG. ELEMENT	3"x0.131" NAIL5	3"x0.120" NAILS
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS @ 4" o.c.	(3) NAILS @ 4" o.c.
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
RIM TO TOP PLATE	TOENAILS • 8" o.c.	TOENAILS ● 6" O.C.*
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
DOUBLE STUD	NAILS @ 24" O.C.	NAILS @ 16" O.C.
DOUBLE TOP PLATE	NAILS @ 24" o.c.	NAILS • 16" o.c.
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
TOP PLATE LAP © CORNERS € INTERSECTING WALLS	(2) NAILS	(2) NAILS
* 0/1 01/2 10 11 100000000000000000000000	TTOULTH # TO 1 31 01001 CH # CO	A CINC OR HILLER OF HAILC

2½"x0.113 is an acceptable alternative to a 3"x0.120", same spacing or number of nails (only acceptable where * are shown)

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOT SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION, THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF BRACING, GUYS, AND TIE-DOWNS, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL INCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TOLERANCES.

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS OTED OTHERWISE ON PLAN

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS:
- 1/8" DEAD LOAD FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS:

LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAL LOAD, (NOT DIFFERENTIAL DEFLECTION)

LEGEND

- INTERIOR BEARING WALL
- □□□□□ BEARING WALL ABOVE
- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP.

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

ROOF DEAD = 7 PSF T.C., IO PSF B.C.

LOAD DURATION FACTOR = 1.25

LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS, ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION, ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY, NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL CONNECTIONS, TYP. U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
 I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRUCE-PINE-FIR ± 2 (SPF) OR SOUTHERN PINE ± 2 (SYP) LUMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY. CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. IUN.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (I)2x4/6 FLAT @ OPENINGS UP TO 41, (2)2x4/6 FLAT UP TO 81 ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- 'LSL' Fb=2325 psi; Fv=3I0 psi; E=I.55xI0^6 psi
- 'LVL' Fb=2600 psi; Fv=265 psi; E=2.0x10^6 psi
- 'PSL' FB=2400 PSI; FV=240 PSI; E=2.0XIO^6 PSI
- M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THE OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
- FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS \bullet 8" O/C OR 2 ROWS $\mbox{$\sc k$}$ "SIMPSON SDS SCREWS (OR 3 $\sc k$$ " TRUSSLOK SCREWS) \bullet 16" O/C, USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM EDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF $\frac{1}{4}$ "x6" SIMPSON SDS SCREWS (OR 6 $\frac{3}{4}$ " TRUSSLOK SCREWS) • 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE. A SOLID 7" BEAM IS ACCEPTABLE.
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM,
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER w/ 3"X0.I3I" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) • 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABM44Z BASE, U.N.O.

NON-BEARING HEADER SCHEDULE

SPAN	2x4 NON-BEARING	2x6 NON-BEARING
31744	PARTITION WALL	PARTITION WALL
UP TO 3'-0"	(I)2x4 FLAT	(I)2x6 FLAT
UP TO 6'-0"	(2)2x4	(3)2x4
UP TO 8'-0"	(2)2x6	(3)2×6
UP TO 12'-0"	(2)2x8	(3)2x8

ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS

SPACED @ 24" O.C. (MAX.)

FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF, TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MEK FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES \$ @ 12"o.c. FIELD. - 2 3 × 0.120" NAILS • 4" O.C. • PANEL EDGES \$ • 8" O.C. FIELD.
- 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN. SCREMS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS.
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO:
 - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS w/ (3) 3"x0.131" NAILS • 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS 6"o.c. PANEL EDGES € 12" O.C. FIELD. - w/ 2 3 × 0.120" NAILS • 4"o.c. • PANEL EDGES € • 8" O.C. FIELD.
- W/ 2 8 × 0.113" NAILS 3"o.c. PANEL EDGES \$ 6" O.C. FIELD.

VENEER LINTEL SCHEDULE

12:02:12:11:12				
SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	Steel angle size		
3'-0"	20 FT. MAX	L3"x3"x/4"		
	3 FT, MAX	L3"x3"x/4"		
6'-0"	I2 FT. MAX	L4"x3"x/4"		
	20 FT. MAX	L5"x31½"x7%"		
	3 FT, MAX	L4"x4"x½" *		
8'-O"	I2 FT. MAX	L5"x31½"x7%"		
	I6 FT. MAX	L6"x3%"x3%"		
9'-6"	I2 FT. MAX	L6"x3½"x%6"		
16'-0"	2 FT. MAX	L7"x4"x½" **		
10 -U	3 FT. MAX	L8"x4"x½" **		

SHALL SUPPORT 2 %" - 3 ½" VENEER W 40 paf MAXIMUM WEIGHT (16' SHALL HAVE 4" MIN. BEARING

- (16' SHALL HAVE 4" MIN, BEARING : 16' SHALL HAVE 8" MIN, BEARING (16' SHALL NOT BE FASTENED BACK TO HEADER, : 16' SHALL BE FASTENED BACK TO WOOD HEADER IN WALL **048**'0. W/5" DIA. x 3 %" LONG LAG SCRENG IN 2" LONG VERTICALLY SLOTTED HOLES
- MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
 WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 1/2" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT
- FINISHING.
 SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL FASTENESS SHALL MAINTAIN A 25' MINIMAN LEAR DISTANCE FROM BOTTOM OF BEAM.
- FOR QUEEN VENEER USE L4x3x FOR 3½" VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF

SD2.1 REFERS TO SD2.1A FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM:

20 MPH WIND IN 2018 NCSBC:RO (120 MPH WIND SPEED IN ASCE 7-10

WIND MAP, PER IRC R301,21,1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R301.1.3 OF THE 2018 NCSBC:RC, OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R60235& R80211

EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"XO.II3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, UN.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1/5" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6" O.C IN FIELD.

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 3/8" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 34" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C. IN FIELD, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC, ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING. IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING.
- PRE-MANUFACTURED PANELIZED WALLS:
 FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWAL OR 3" O.C. OSB SHEARWALL.

INDICATES HOLDOWN BELOW

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED, BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, I2" MAX. FROM PLATE ENDS - UTILIZING: ■ I/2" DIA. ANCHOR BOLTS @ 6'-0" O.C., 7" MIN. EMBEDMENT

 - (CONC), 15" MIN. EMBEDMENT (CMU)
 SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS 2'-8" O.C. (CMU) (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACI 318. CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: FOUNDATION WALLS 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE
- 60,000 psi BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 9' OR 10' HEIGHT (AS NOTED ON PLANS)
- TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (91/2" FOR 10" THICK WALL).
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC. ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL WITH 2" CLEAR, REINFORCEMENT
- SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.

 FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE
- DEPTH OVER OPENING OR (3)2x10 W (2)2x6 JACK STUDS, U.N.O. LARGER OPENINGS SHALL BE PER PLAN.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT
- ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP
- . JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR
- 15'-0" O.C. (MAXIMUM) JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS
- POSSIBLE (I:I RATIO), WITH A MAXIMUM OF I:15 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fin=1500 psi). MORTAR SHALL BE ASTM C270, TYPE 9. CMU DESIGN PER ACI 530 \$ 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.
- PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS. FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE.

DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

• BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES, METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

HOLD-DOWN SCHEDULE

П		
,	SYMBOL	SPECIFICATION
	► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)
	► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC66B3 ALTERNATE
	№ НD-3	SIMPSON STHDI4/STHDI4RJ

* UTILIZE THE SSTB24 ANCHOR BOLT ● ALL MONOSLAB & INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN. OOTING THICKNESS REQUIRED.

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 5" (FOR 1/8" DIA.) MIN. EMBEDMENT INTO CONCRETE.

NSTALL PER MANUF, INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D. <u>DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE</u>

TH CAR) ROFESSION O'

ENGINE SEPH T. R

± 6/6/25



I&K project numbe 126-2306

oject mgr: JTR rawn by: ssue date: 06-06-2

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RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number:

drawn by:

126-2306

JTR

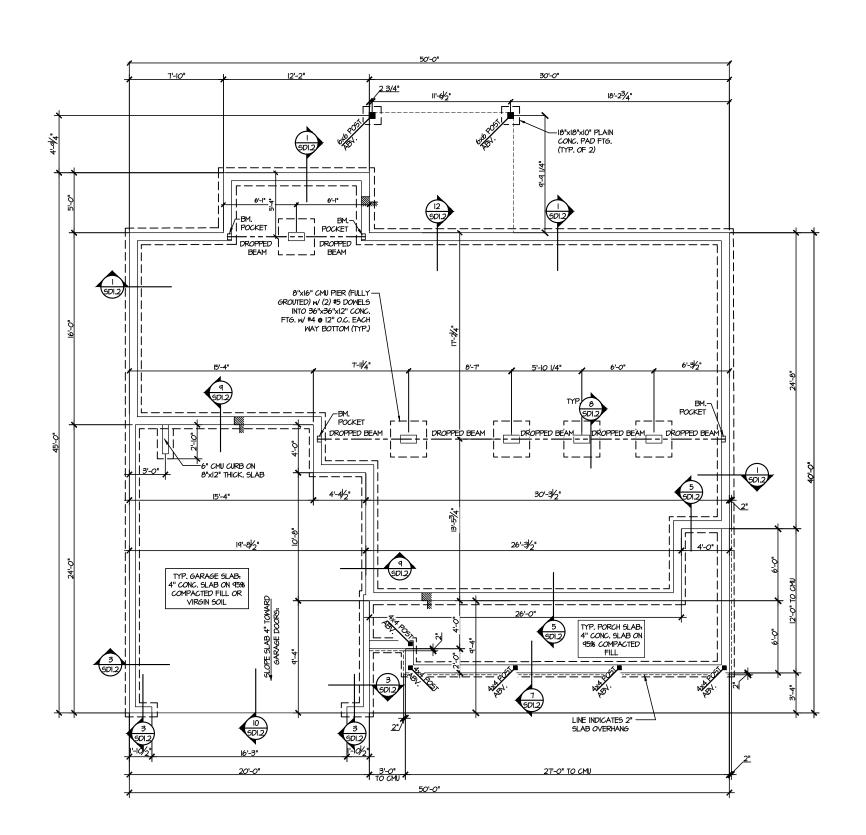
GTK issue date: 06-06-25

SEPH T. R

LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- * INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES # SCHEDULES



CRAWL SPACE FOUNDATION PLAN SCALE: 1/8"=1"-0"

BLAKE POND COMMUNITY Lot 124 - arlington 4 raleigh, nc **OUNDATION PLANS**

S1.0

project mgr: JTR drawn by: GTK issue date: 06-06-25

REVISIONS:

date: initial:

BLAKE POND COMMUNITY LOT 124 - ARLINGTON 4 RALEIGH, NC

INTERIOR BEARING WALL PEARING WALL

LEGEND

- INDICATES SHEAR WALL & EXTENT
 EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR
TYPICAL STRUCTURAL NOTES
\$ SCHEDULES

IST FLOOR FRAMING PLAN
SCALE: I/B'=1'-0'

6x6 P.T. POST w/ SIMPSON BC52-3/6 CAP & ABM66Z BASE (SEE DETAIL 3/9D3.0 (TYP.))

— 2×10 P.T LEDGER FASTENED TO RIM W (3) ¼" DIA. × 3 ½" LONG SDS SCREMS **©** 16" O.C.

_SIMPSON HUC210-2 (TYP. OF 2)

ADD'L. TRUSS BELOW-PARTITION ABOVE

____(3)2x10 DROPPED-____

2xIO P.T • I6" O.C. w/ SIMPSON LUS28-

- INDICATES KITCHEN ISLAND ABOVE

TRUSS MANUF. TO DESIGN-

FOR ADD'L. 50 PSF | DEAD LOAD BELOW KITCHEN ISLAND

> ADD'L. TRUSS-BELOW EDGE OF COUNTER ABOVE

- INTERIOR BEARING WALL
- □□□□□ BEARING WALL ABOVE
- --- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

	ENGINEERED BEAM MATERIAL SCHEDULE							
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION			
001	(3)134"x16" - FB	5 ¼"x16" - FB	N/A	(3)2xl2 + (2) ½"xll½" STEEL FLITCH PLATES - F	WI2xI4 - F			
002	(2)194"×94" - D	3½"x9¼" - D	(2)134"×94" - D	(2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATE - D	M8x10 - D			
003	(2)194"×14" - F	3½"xi4" - F	N/A	(2)2xl2 + (I) \$"xlK" STEEL FLITCH PLATE - F	WI2xI4 - F			
004	(2)13/4"x16" - H	3 ½"x16" - H	(3)13/4"×16" - H	(3)2xl2 + (2) ½"xll¼" STEEL FLITCH PLATES - F	N/A			
005	(2)134"x944" - D	3½"x4¼" - D	(2)13/4"×9/4" - D	(2)2xl0 + (I) ¼"xq¼" STEEL FLITCH PLATE - D	M8x10 - D			
006	(2)1¾"x11½" - D	3½"xII%" - D	(3)134"x 1 = D	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - D	MI0x12 - D			
001	(3)1¾"x14" - D	5 ‡"xi4" - D	(4)13/4"×14" - D	(3)2xi2 + (2層*xi以* STEEL FLITCH PLATE - D	WI0x19 - D			
008	(2)134"×944" - D	3½"x9¼" - D	(2)13/4"×9/4" - D	(2)2x10 + (1) ¼"xq¼" STEEL FLITCH PLATE - D	M8x10 - D			
P00	(2)1%/"x16" - H	3 ½"x16" - H	(3)134"×16" - H	(3)2xl2 + (2) ½"xll½" STEEL FLITCH PLATES - F	N/A			
010	(2)13/4"x117/6" - H	3½"xII%" - H	(3)1¾"x 1 ⅓" - H	(2)2xl2 + (1) ¼"xll¼" STEEL FLITCH PLATE - H	WIOx12 - H			
OII	(2)13/4"×14" - F	3½"xi4" - F	(2)13/4"×14" - F	(2)2xl2 + (l) 計以以 STEEL FLITCH PLATE - F	WI2xI4 - F			
012	¾"x 4" - F	3½"x14" - F	i¾"xi4" - F	(2)2xi2 + (i) ‡"xii¼" STEEL FLITCH PLATE - F	WI2xI4 - F			
013	(2)134"x16" - FB	3 ½"x16" - FB	(3)1¾"×16" - FB	(3)2xi2 + (2) ½"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F			
014	(2)13/4"×14" - F	3½"xl4" - F	(2)13¼"x14" - F	(2)2xi2 + (I) है"xil4" STEEL FLITCH PLATE - F	WI2xI4 - F			
010	(2)1¾"x11%" - F	3½"xII%" - F	(3)13¼"x }" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - F	WI0xI2 - F			

- BEAM NOTATION: "F" INDICATES FLUSH BEAM "FT" INDICATES FLUSH TOP BEAM
- "FB" INDICATES FLUSH BOTTOM BEAM

- "B" INDICATES FLUSH BOTTOM BEAM
 "D" INDICATES PROPPED BEAM
 "H" INDICATES DROPPED OPENING HEADER
 REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
 REFER TO DETAIL E/SD2.0 FOR TYPICAL SITEL BEAM CONNECTIONS
 FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN
- PLATES IN SUCCESSION W (2) 3"X0,120" NAILS \bullet 8" O.C. FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ!D, FASTEN PLATES IN SUCCESSION W (2) 3"x0,120" NAILS \bullet 8" O.C.

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS LEGEND

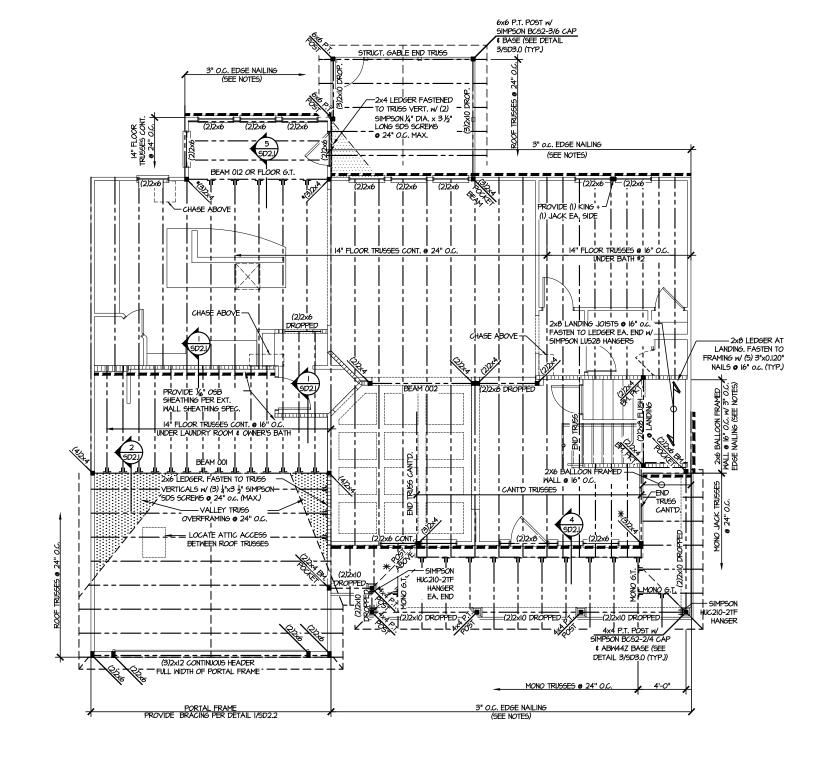
l&K project number: 126-2306

JTR rawn by: ssue date: 06-06-2

FVISIONS initial:

BLAKE POND COMMUNITY Lot 124 - arlington 4 raleigh, nc RAMING OOR

S3.0



2ND FLOOR FRAMING PLAN SCALE: 1/8°=1'-0°

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number:

126-23061 roject mgr: JTR drawn by: **GTK** issue date: 06-06-25

REVISIONS:

initial:

BLAKE POND COMMUNITY LOT 124 - ARLINGTON 4 RALEIGH, NC

ROOF

LEGEND

 INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE

• --- BEAM / HEADER

• == INDICATES SHEAR WALL & EXTENT

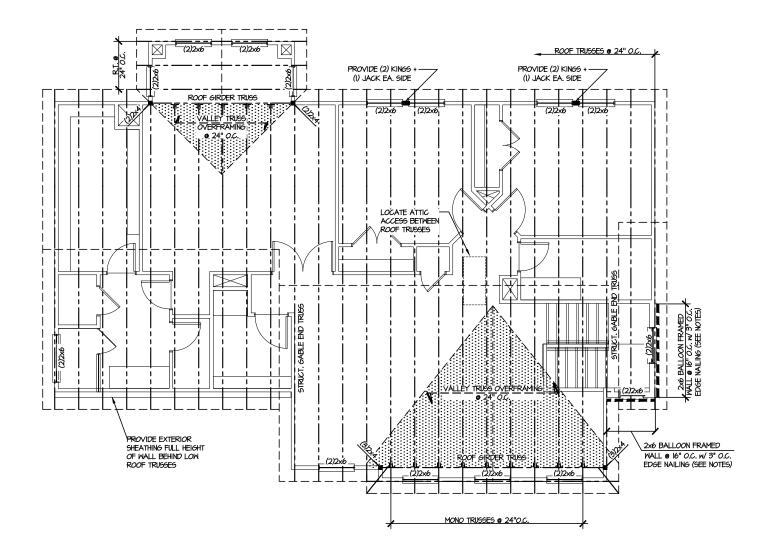
EXTENT OF OVERFRAMING

JL METAL HANGER

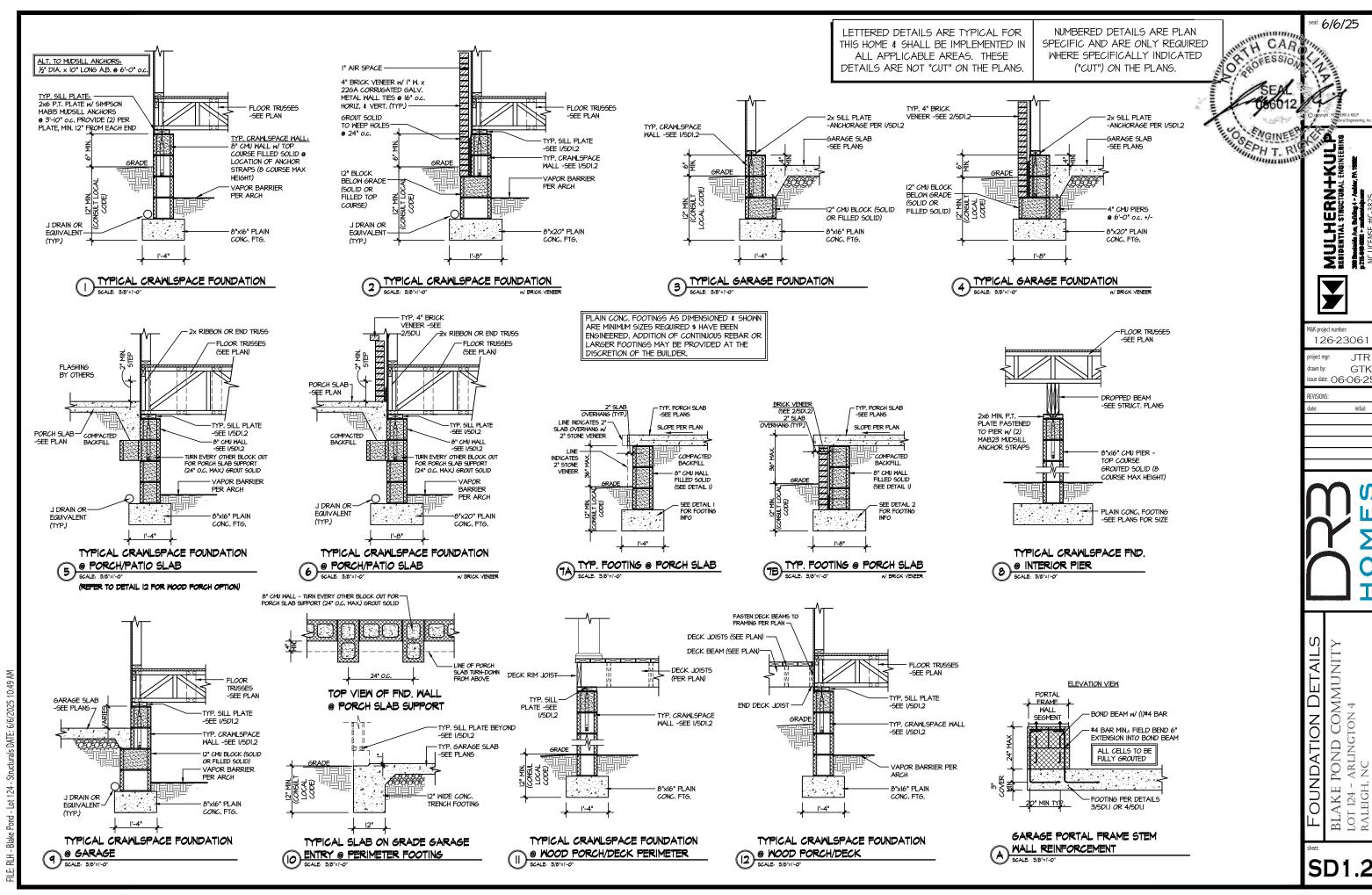
* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

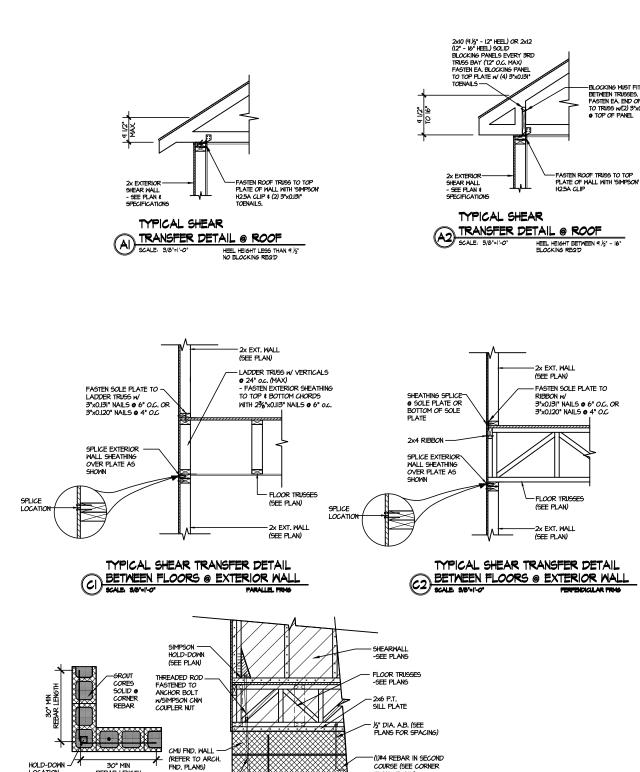
INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



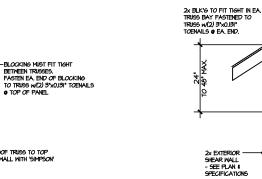
ROOF FRAMING PLAN





COURSE (SEE CORNER PLAN DETAIL) SIMPSON SSTB28 — ANCHOR BOLT w/24" MIN. EMBEDMENT -(I)#4 REBAR IN ADJACENT CELL OF HOLDOWN ANCHOR



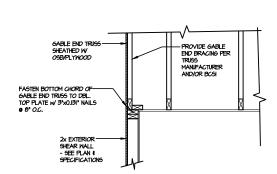


TYPICAL SHEAR TRANSFER DETAIL @ RAISED HEEL TRUSS

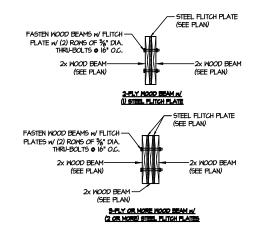
OSB/PLYWOOD SHEATHING UP VERTICALS OF ROOF TRUSSES, & FASTEN W

· 2x Bl.K'6 IN EA, TRUSS BAY FASTENED TO DBL TOP PLATE w/ (3) 3*x0.131* NAILS.

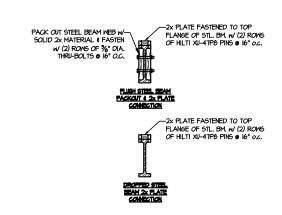
-FASTEN ROOF TRUSS TO TOP PLATE OF WALL WITH 'SIMPSON' H2.5A CLIP € (2) 3"x0.131" TOENAILS.



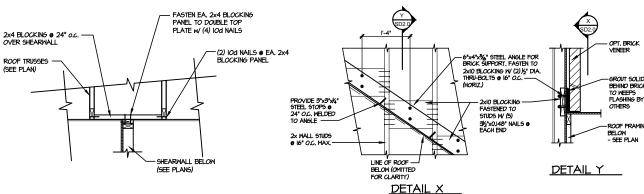
TYPICAL GABLE END DETAIL
SCALE. 3/8'=1'-0'



TYPICAL FLITCH BEAM CONNECTION DETAIL SCALE 544-1-0*



TYPICAL STEEL BEAM CONNECTION DETAIL



SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL BELOW

DETAIL SUPPORT OF BRICK VENEER

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

TH CAR SEPH T. R

MULHERN+KUL RESIDENTIAL STRECTURAL ENGINEERIN

6/6/25

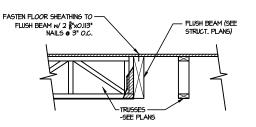
1&K project number 126-23061

JTR frawn by: GTK issue date: 06-06-2

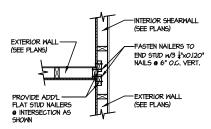
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COMMUNIT DETAILS ARLINGTON A BLAKE POND (LOT 124 - ARLINGT RALEIGH, NC Ü

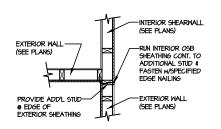
SHEAR TRANSFER DETAIL @ INTERIOR SHEAR WALL



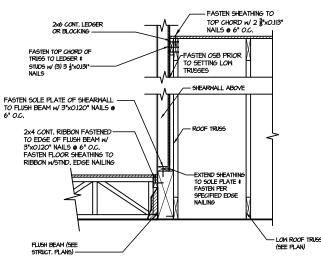
SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL ABOVE



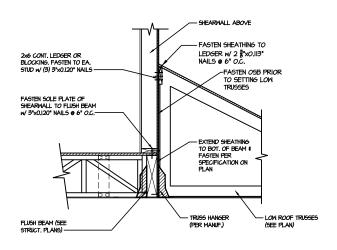
SHEAR TRANSFER DETAIL @ INTERSECTING INT. SHEARWAL SCALE 5/4'=1'-0' SHT6.



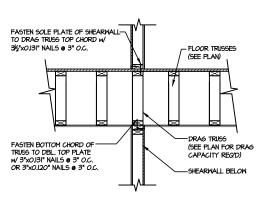
SHEAR TRANSFER DETAIL @ INTERSECTING INT. SHEARWAL SCALE SA4*=1-0" SHTG.



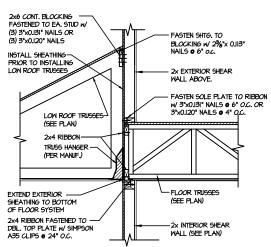
SHEAR TRANSFER DETAIL @ 2 EXTERIOR SHEARWALL ABOVE



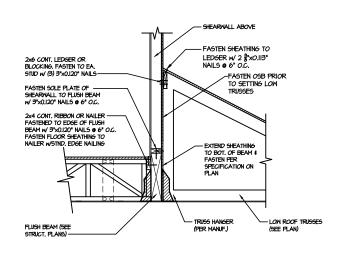
SHEAR TRANSFER DETAIL @ 6 EXTERIOR SHEARWALL ABOVE SCALE: \$44*=1"-0"



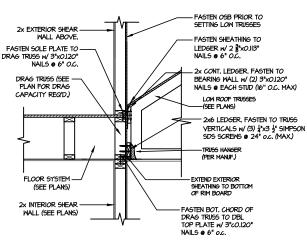
SHEAR TRANSFER DETAIL @ INT. SHEARWALL ABOVE & BELOW



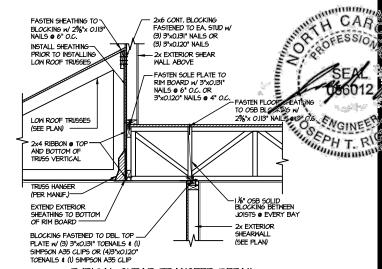
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



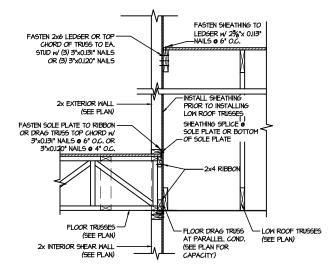
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



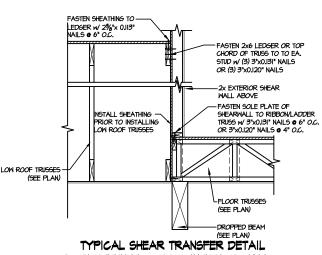
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



TYPICAL SHEAR TRANSFER DETAIL @ EXTERIOR WALL ABOVE



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



BETWEEN FLOORS @ INTERIOR WALL

COMMUNI POND ARLINC NO BLAKE LOT 124 -RALEIGH,

6/6/25

ERN+KUL

I&K project numbe

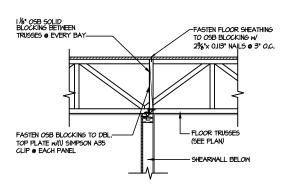
126-2306

issue date: 06-06-2

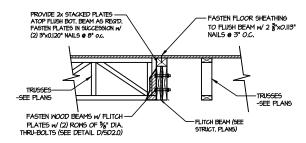
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initial:

SD2.1A

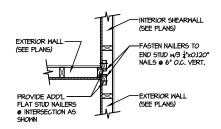


SHEAR TRANSFER DETAIL (I) @ INTERIOR SHEAR WALL SCALE \$44-1-67 PERFECICILAR FRANKS

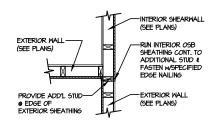


SHEAR TRANSFER DETAIL @

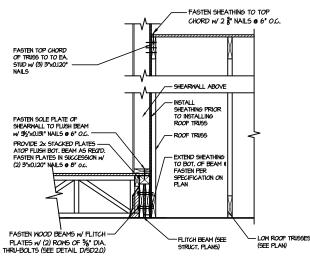
| INTERIOR SHEARWALL ABOVE | | SCALE 9/4"-1"-0" | PARALLE FRANCE |



SHEAR TRANSFER DETAIL @
INTERSECTING INT. SHEARWALL
SCALE BASTLO'
STEE GI SAME FACE



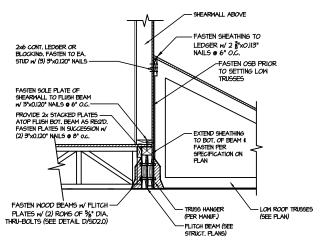
SHEAR TRANSFER DETAIL @
INTERSECTING INT. SHEARWALL
SCALE 844-1-0-1 SHEARWALL



SHEAR TRANSFER DETAIL @

EXTERIOR SHEARMALL ABOVE

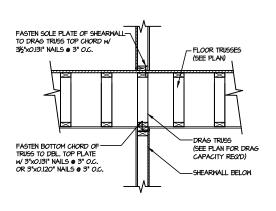
SCALE 944-11-07



SHEAR TRANSFER DETAIL @

EXTERIOR SHEARWALL ABOVE

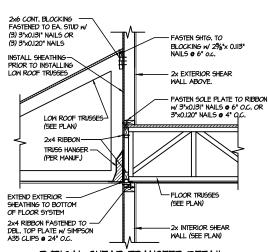
SCALE 844-1-0*



SHEAR TRANSFER DETAIL @ INT.

SHEARWALL ABOVE & BELOW

SCALE SAN-1-O' PARALE FRAME

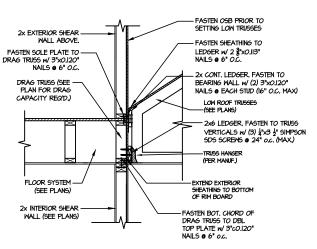


TYPICAL SHEAR TRANSFER DETAIL

BETWEEN FLOORS @ INTERIOR WALL



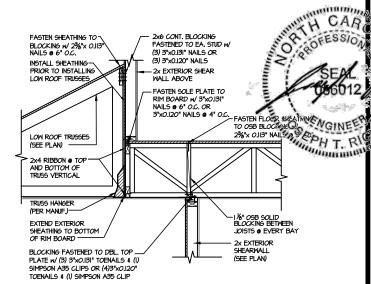
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE SCALE SATING



TYPICAL SHEAR TRANSFER DETAIL

BETWEEN FLOORS @ INTERIOR WALL

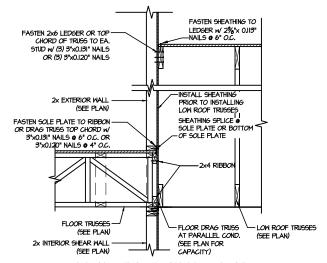
SCALE \$44-1-0



TYPICAL SHEAR TRANSFER DETAIL

(45) @ EXTERIOR WALL ABOVE

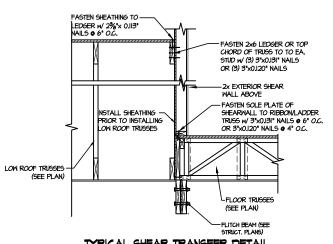
SCALE \$44-1-0*



TYPICAL SHEAR TRANSFER DETAIL

BETWEEN FLOORS @ INTERIOR WALL

SCALE 544-1-0



TYPICAL SHEAR TRANSFER DETAIL

BETWEEN FLOORS @ INTERIOR WALL

SCALE 347-1-0*

SD2.1B

POND

BLAKE LOT 124 -RALEIGH,

6/6/25

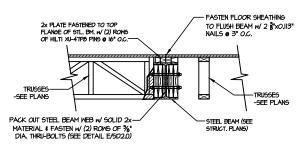
ERN+KULP STREETERAL ENGINEERING

I&K project numbe

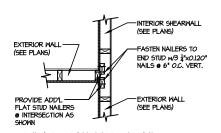
126-2306

ssue date: 06-06-2

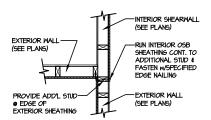
JTR



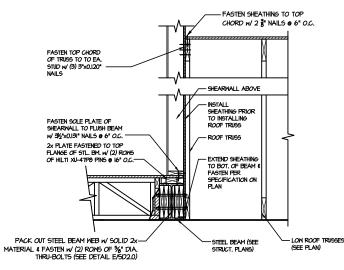
SHEAR TRANSFER DETAIL @ INTERIOR SHEARMALL ABOVE



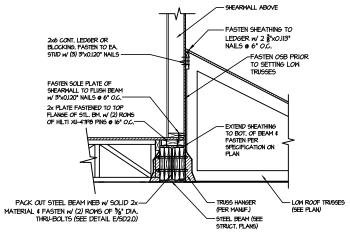
SHEAR TRANSFER DETAIL @ 9 INTERSECTING INT. SHEARWALL SHTG, ON SAME FACE



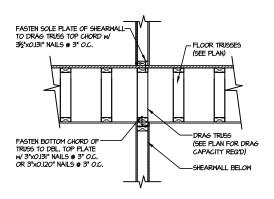
SHEAR TRANSFER DETAIL @ INTERSECTING INT. SHEARMALI SHEARMALI



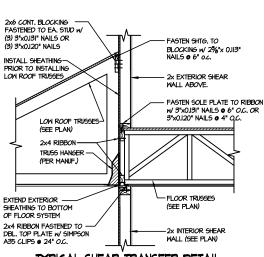
SHEAR TRANSFER DETAIL @ 2 EXTERIOR SHEARWALL ABOVE



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



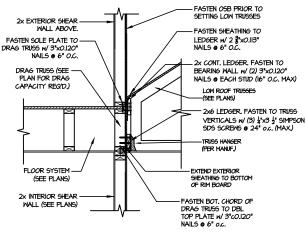
SHEAR TRANSFER DETAIL @ INT. SHEARWALL ABOVE & BELOW



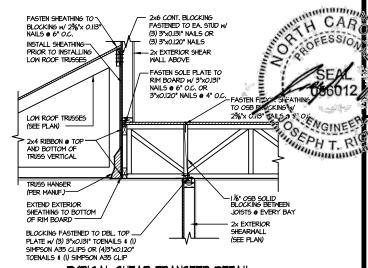
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



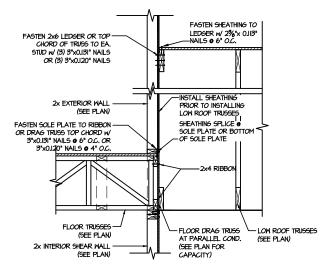
SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



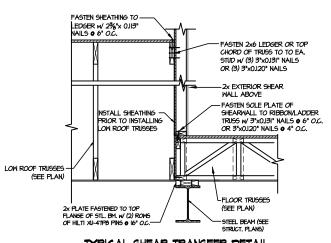
TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL
SCALE SATSICO



TYPICAL SHEAR TRANSFER DETAIL @ EXTERIOR WALL ABOVE



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL



TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL

COMMUNI POND BLAKE LOT 124 -RALEIGH,

SD2.1C

ARLIN

6/6/25

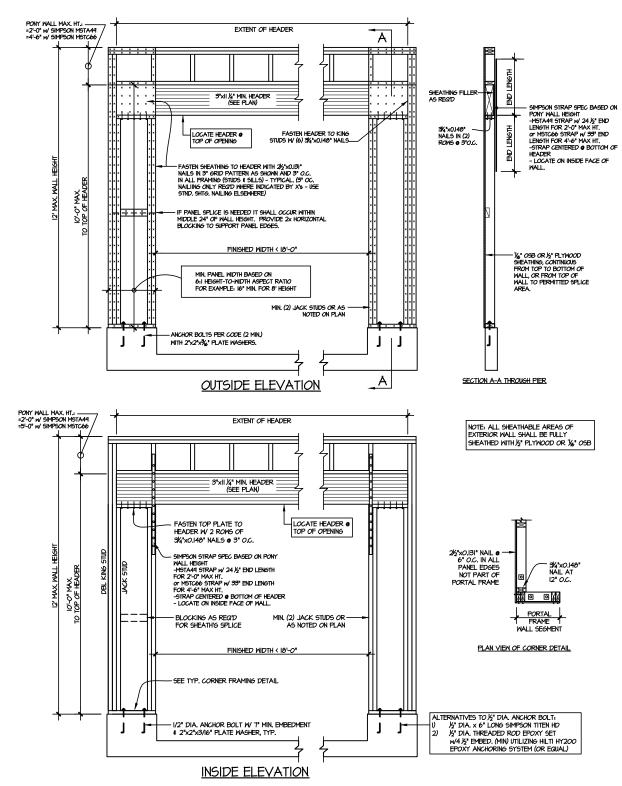
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I&K project numbe 126-2306

ssue date: 06-06-2

JTR

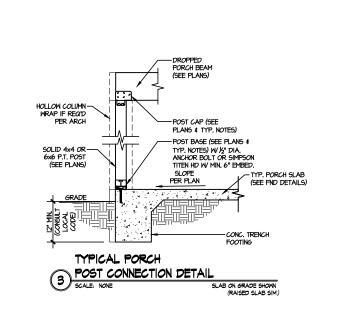




TWO SIDED GARAGE PORTAL FRAME BRACING ELEVATION ON CONCRETE STEM

JTR

GTK



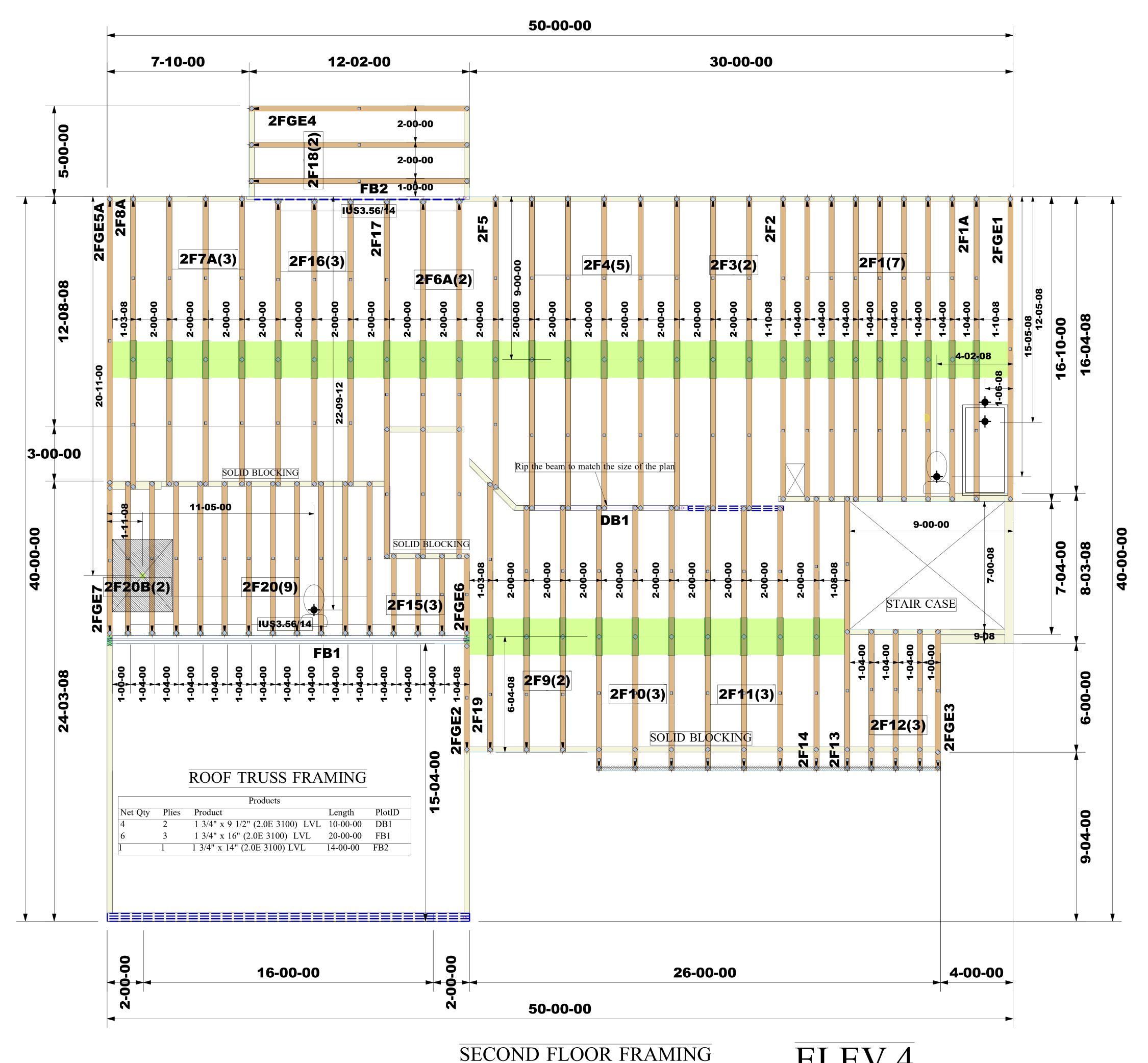
eal: 6/6/25 MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEERIN

M&K project number: 126-23061

project mgr: JTR GTK drawn by: issue date: 06-06-2

BLAKE POND COMMUNITY Lot 124 - Arlington 4 Raleigh, nc

SD3.0



ELEV.4

Truss Connector Total List Product Manuf IUS3.56/14 Simpson

*EXTERIOR DIMENSIONS ARE TO STUD. *TRUSSES @ 24" O.C. U.N.O. *INSTALLATION OF STRONGBACKS RECOMMENDED (NOT REQUIRED). *INSTALL 2X4 NAILERS ON ALL TOP RIBBON NOTCH CONDITIONS. *SEE PROFILE DWGS. FOR TRUSS

ORIENTATION BEFORE INSTALLATION.

NOTES:

BLAKE POND LOT 00.0124 PHASE SF (NC)(RAL) **ARLINGTON REV.1** EL.4 OPT.EXTENED KITCHEN/BREAKFAST GARAGE LEFT

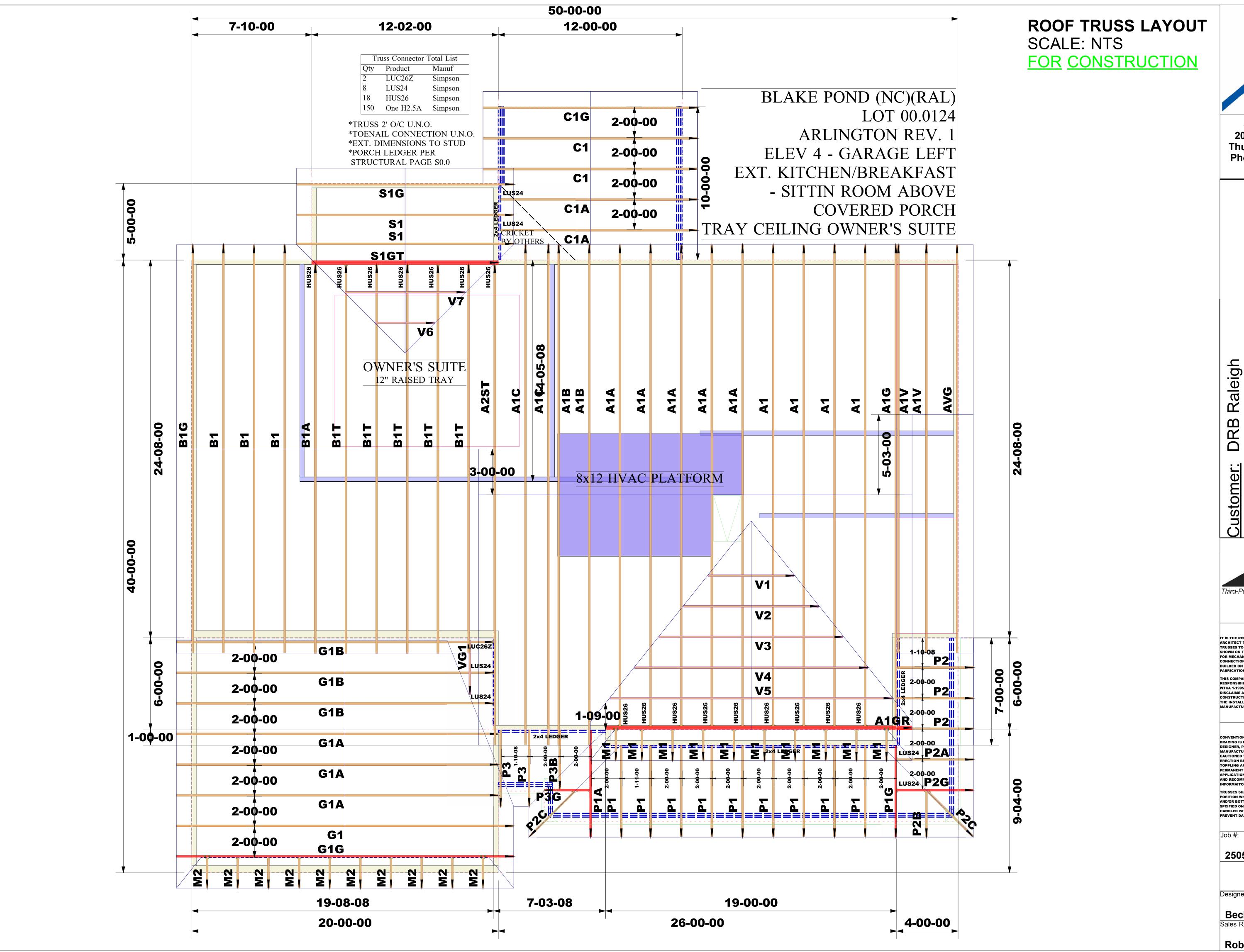
Job #: **WARNING: NOTE:** IT IS THE RESPONSIBILITY OF THE BUILDING CONVENTIONAL FRAMING, ERECTION AND/OR 2505-7719 PERMANENT BRACING IS NOT THE RESPONSIBILITY OF DESIGNER OR ARCHITECT TO PROVIDE AN THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS ADVICE REGARDING THE ERECTION BRACING WHICH IS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING ALWAYS REQUIRED TO PREVENT TOPPLING AND (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST DOMINOING DURING ERECTION; AND PERMANENT BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS **BRACING WHICH MAY BE REQUIRED IN SPECIFIC** LAYOUT PRIOR TO FABRICATION. APPLICATIONS. SEE "BRACING WOOD TRUSSES Designer: THIS COMPANY IS A TRUSS MANUFACTURER WHOSE COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND Sayan Roy Sales Rep: PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF DESIGN. TRUSSES SHALL BE HANDLED WITH TRUSSES MANUFACTURED BY THIS COMPANY. REASONABLE CARE DURING ERECTION TO PREVENT

DAMAGE OR PERSONAL INJURY.

Robbie Zarobinski

Customer: DRB Raleigh Job Name: Blake Pond Lot 00.0124 OWF <u>ot #:</u> Lot 00.0124 Model Name: ARLINGTON





Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

Roof Pond Lot 00.0124 Blake 00.0124

ARLINGTON

Model Name:

TPI Plant W974

iot Iot

NOTE:

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO

RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, I DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR TH CONSTRUCTIION DESIGN, DRAWINGS, DOCUMENTS INCLUDIN THE INSTALLATION, AND BRACING OF TRUSSES

WARNING:

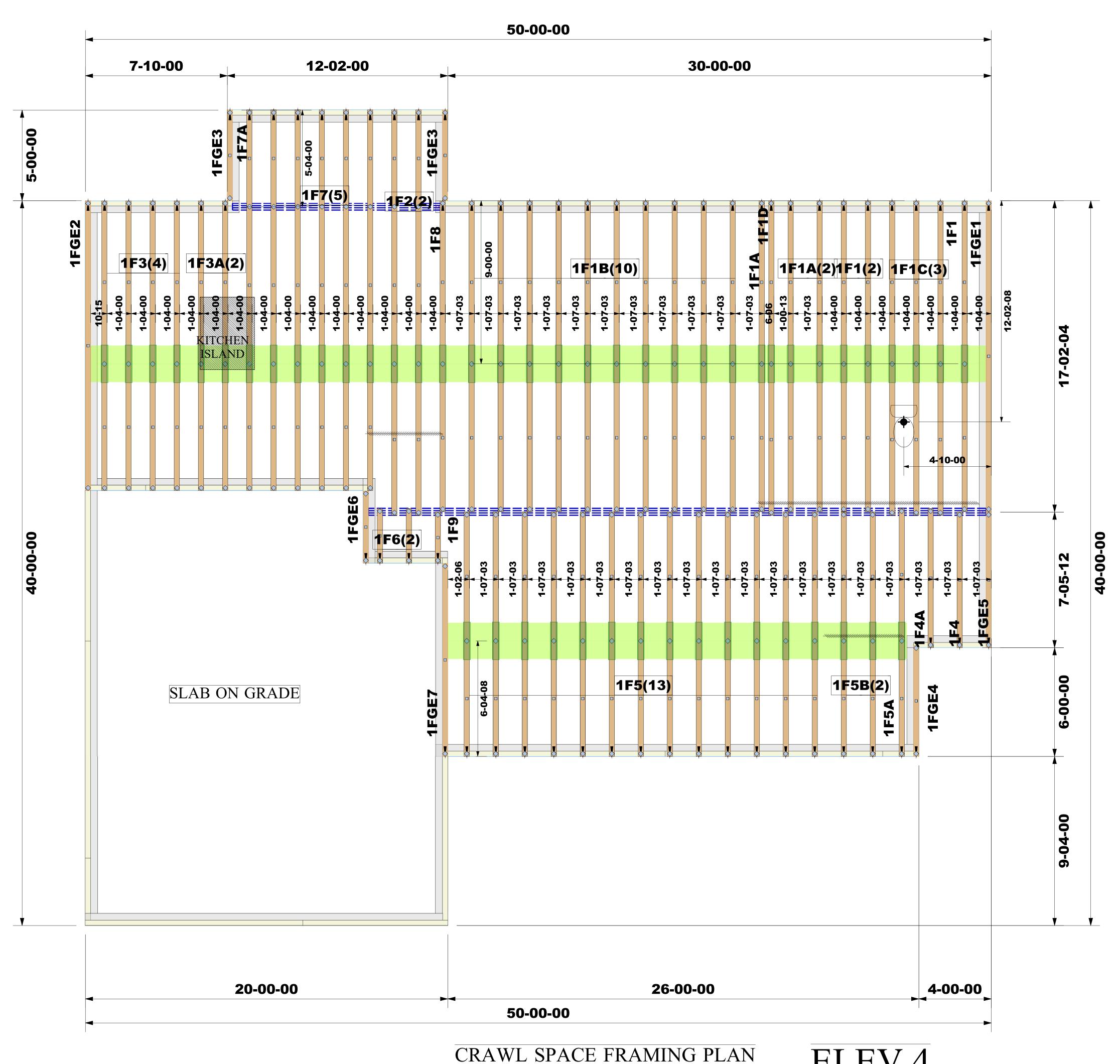
BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING T **ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN** TOPPLING AND DOMINOING DURING ERECTION; AND RMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC

POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TO SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY

2505-7720

Beckett Tayler

Robbie Zarobinski



ELEV.4

NOTES: *EXTERIOR DIMENSIONS ARE TO STUD. *TRUSSES @ 19.2"O.C. U.N.O. *INSTALLATION OF STRONGBACKS RECOMMENDED (NOT REQUIRED). *INSTALL 2X4 NAILERS ON ALL TOP RIBBON NOTCH CONDITIONS. *SEE PROFILE DWGS. FOR TRUSS ORIENTATION BEFORE INSTALLATION.

BLAKE POND LOT 00.0124 PHASE SF (NC)(RAL) ARLINGTON REV.1 EL.4 OPT.EXTENED KITCHEN/BREAKFAST GARAGE LEFT

Job #: 2505-7719	WARNING: CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE	NOTE: IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO	Customer: DRB Raleigh	H-1
	THE IRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	SUPPORTING STRUCTURE PER REACTIONS SHOWN	Job Name: Blake Pond Lot 00.0124 OWF	Th
Designer: Sayan Roy	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON. TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS. THEY	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS	Lot #: Lot 00.0124	
Sales Rep: Robbie Zarobinski	SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.	INCLUDING THE INSTALLATION, AND BRACING OF	Model Name: ARLINGTON	

