



Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: David Weekley Homes

Model: Springhill 3277 B TMB GRH

THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

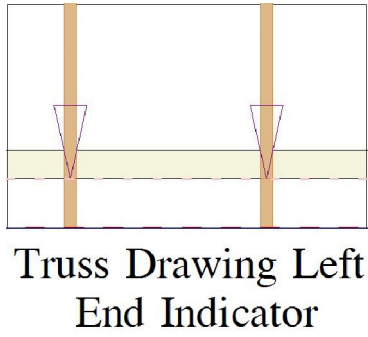
Date: _____

General Notes:

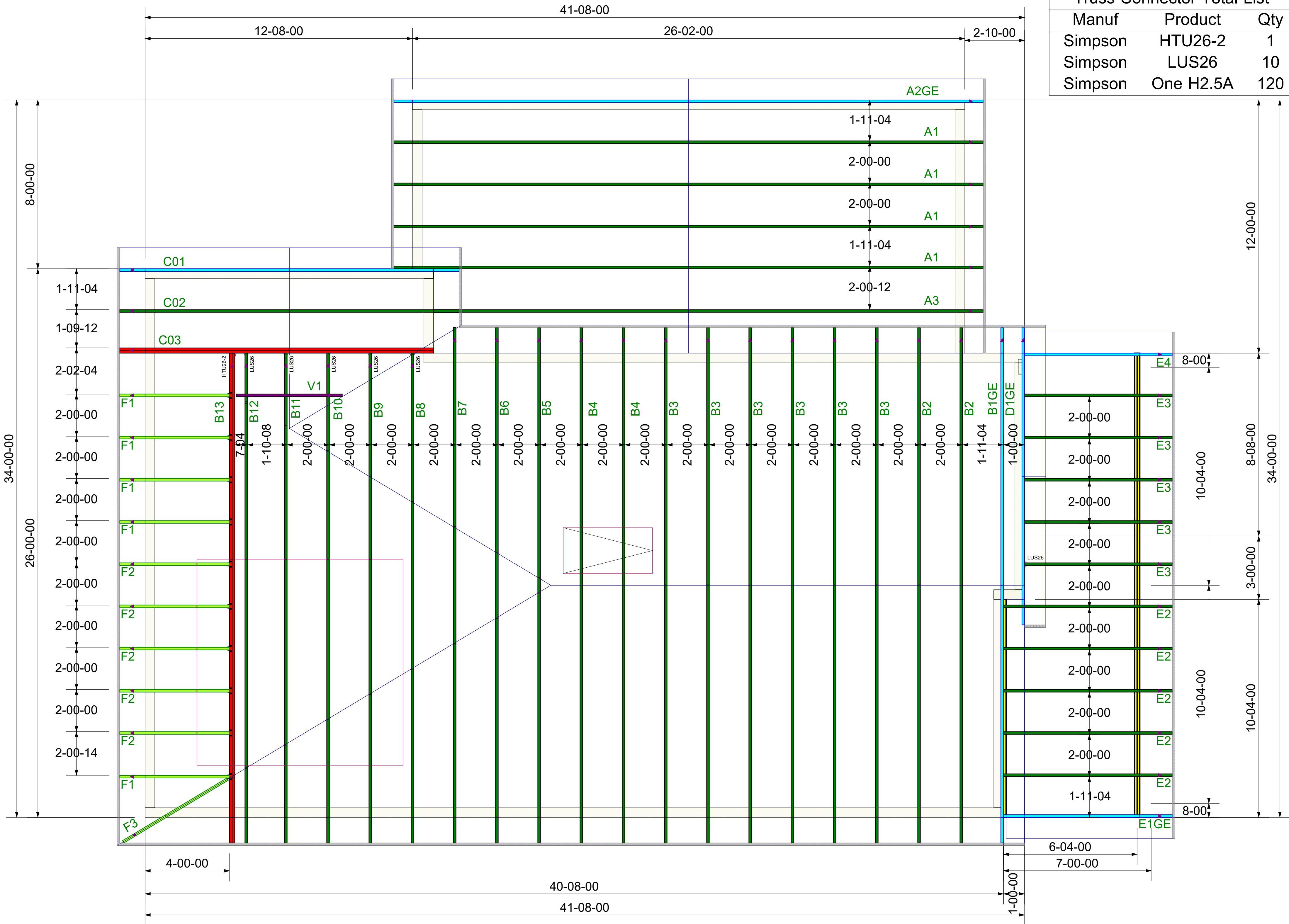
** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS. ** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT. ** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.



Truss Drawing Left End Indicator



Truss Connector Total List		
Manuf	Product	Qty
Simpson	HTU26-2	1
Simpson	LUS26	10
Simpson	One H2.5A	120

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS. ** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH. ** All uplift connectors shown within these documents are recommendations only. Per ANS/ITPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53179



David Weekley Homes	
46 Serenity-Roof-Springhill	
3277 B TMB GRH	
ROOF PLACEMENT PLAN	

Scale:	NTS
Date:	5/26/2025
Designer:	Nick Darr
Project Number:	24120148-01
Sheet Number:	1/1

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: 24120148-01
46 Serenity-Roof-Springhill 3277 B TMB GRH

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I73744812 thru I73744839

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



May 28, 2025

Gilbert, Eric

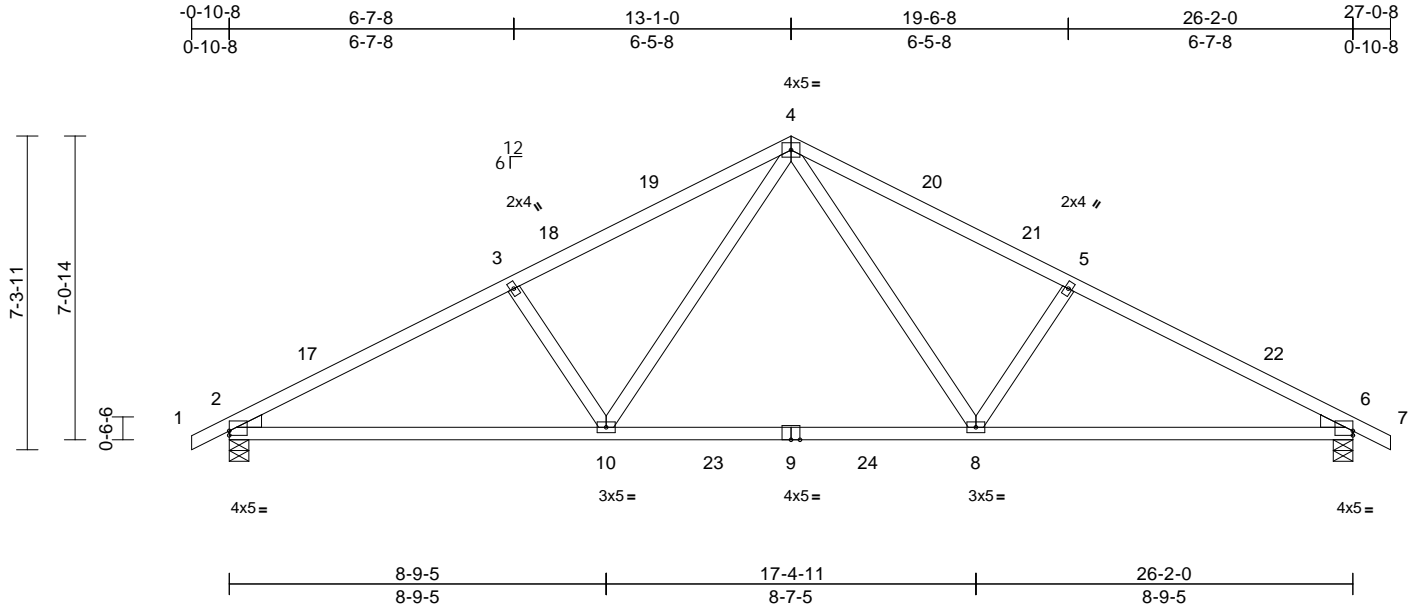
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	A1	Common	4	1	I73744812
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:13
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Page: 1



Scale = 1:53.7

Plate Offsets (X, Y): [2:Edge,0-1-4], [6:Edge,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.22	8-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.35	8-10	>899	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.06	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 122 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-5-8, 6=0-5-8
Max Horiz 2=111 (LC 18)
Max Uplift 2=-117 (LC 14), 6=-117 (LC 15)
Max Grav 2=1200 (LC 5), 6=1200 (LC 6)

FORCES

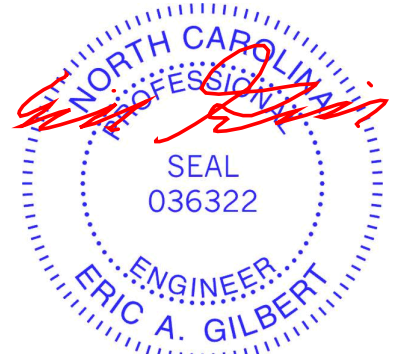
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-3=-2150/258, 3-4=-1967/276,
4-5=-1967/276, 5-6=-2150/258, 6-7=0/23
BOT CHORD 2-10=-215/1847, 8-10=-26/1218,
6-8=-135/1847
WEBS 4-8=-94/784, 5-8=-456/215, 4-10=-94/784,
3-10=-456/215

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 10-1-0, Exterior(2R) 10-1-0 to 16-1-0, Interior (1) 16-1-0 to 24-0-8, Exterior(2E) 24-0-8 to 27-0-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

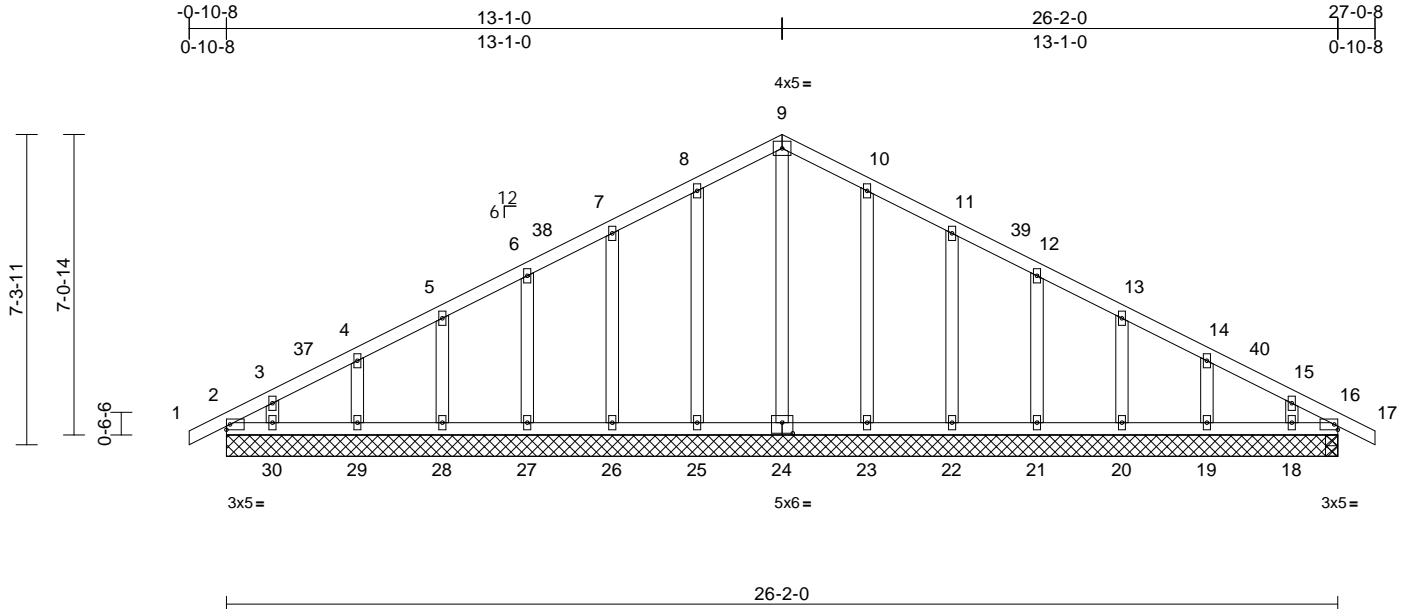
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	A2GE	Common Supported Gable	1	1	I73744813
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Nov 16 2023 Print: 8.730 S Nov 16 2023 MiTek Industries, Inc. Tue May 27 16:09:36

Page: 1

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Scale = 1:54.2

Plate Offsets (X, Y): [24:0-3-0,0-3-0]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	0.00	33	>999	240	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	29-30	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.00	16	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 149 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS

All bearings 26-2-0.
(lb) - Max Horiz 2=107 (LC 18), 31=107 (LC 18)
Max Uplift All uplift 100 (lb) or less at joint(s)
2, 16, 18, 19, 20, 21, 22, 23, 25, 26, 27, 28, 29, 30, 31, 34
Max Grav All reactions 250 (lb) or less at joint (s)
2, 16, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 34

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250

(lb) or less except when shown.

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 10-1-0, Corner(3R) 10-1-0 to 16-1-0, Exterior(2N) 16-1-0 to 24-0-8, Corner(3E) 24-0-8 to 27-0-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2'-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint (s) 2, 25, 26, 27, 28, 29, 30, 23, 22, 21, 20, 19, 18, 16, 2, 16.

LOAD CASE(S) Standard



May 28,2025

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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A MiTek Affiliate

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Edenton, NC 27932

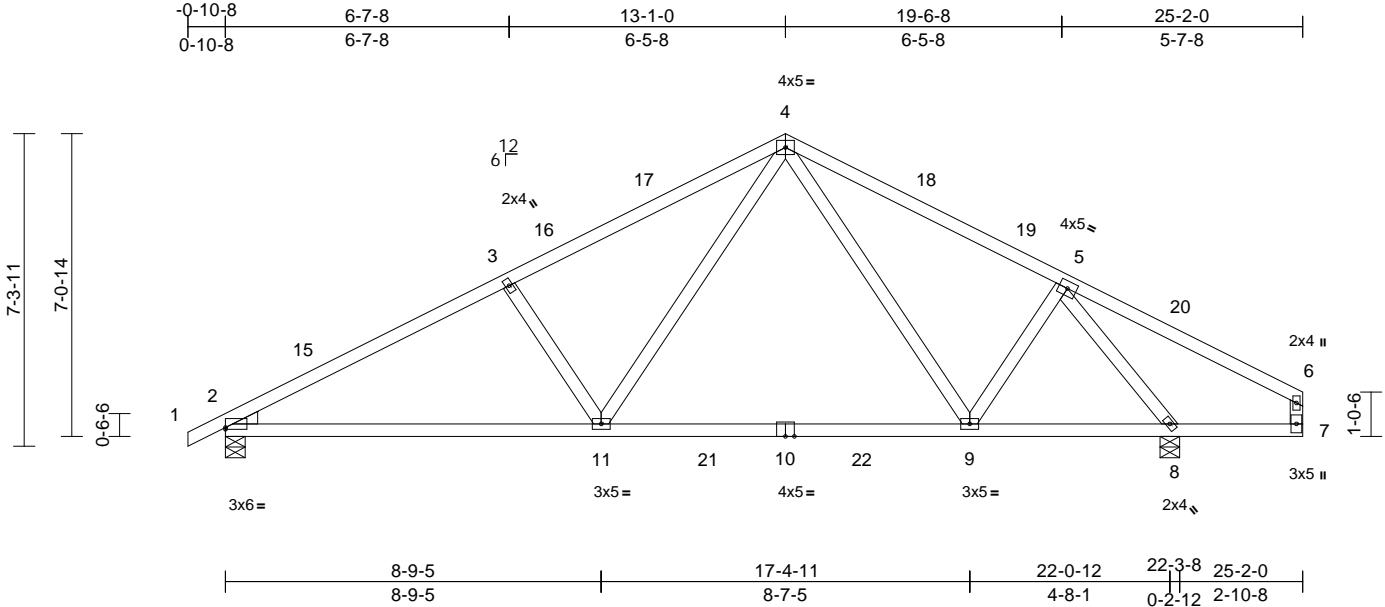
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	A3	Common	1	1	173744814
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:14

Page: 1

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Scale = 1:53.8

Plate Offsets (X, Y): [2:Edge,0-0-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.20	9-11	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.88	Vert(CT)	-0.32	9-11	>827	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(CT)	0.03	8	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 124 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 7-8.

REACTIONS

(size) 2=0-5-8, 8=0-5-8
Max Horiz 2=117 (LC 18)
Max Uplift 2=113 (LC 14), 8=102 (LC 15)
Max Grav 2=1034 (LC 5), 8=1251 (LC 6)

FORCES

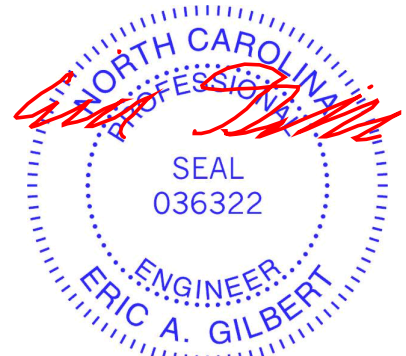
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-3=-1716/212, 3-4=-1532/229, 4-5=-1077/181, 5-6=-66/256, 6-7=-41/38
BOT CHORD 2-11=-213/1482, 9-11=-22/829, 8-9=-37/712, 7-8=-151/114
WEBS 5-8=-1453/221, 3-11=-466/216, 4-11=-90/816, 4-9=-114/118, 5-9=0/355

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 10-1-0, Exterior(2R) 10-1-0 to 16-1-0, Interior (1) 16-1-0 to 22-0-4, Exterior(2E) 22-0-4 to 25-0-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

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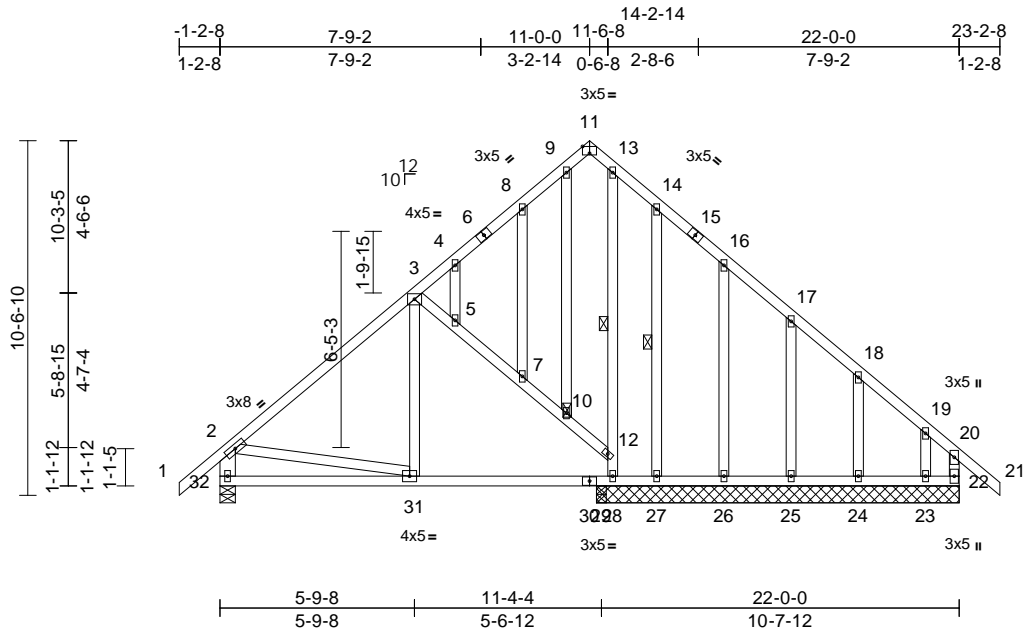
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B1GE	Common Structural Gable	1	1	173744815
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:68.6												
Plate Offsets (X, Y): [11:0-2-8,Edge]												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	0.04	31-32	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.28	Vert(CT)	-0.07	31-32	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(CT)	0.01	22	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 181 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 32-2:2x6 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 13-28, 14-27
JOINTS 1 Brace at Jt(s): 10

REACTIONS (size)
22=10-9-8, 23=10-9-8, 24=10-9-8, 25=10-9-8, 26=10-9-8, 27=10-9-8, 28=10-9-8, 29=0-3-8, 32=0-5-8
Max Horiz 32=271 (LC 12)
Max Uplift 22=119 (LC 11), 23=261 (LC 15), 24=64 (LC 15), 25=78 (LC 15), 26=79 (LC 15), 27=100 (LC 15), 28=187 (LC 7), 32=87 (LC 14)
Max Grav 22=452 (LC 25), 23=174 (LC 13), 24=183 (LC 26), 25=170 (LC 31), 26=181 (LC 26), 27=132 (LC 26), 28=90 (LC 28), 29=508 (LC 7), 32=620 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/55, 2-3=556/107, 3-4=217/126, 4-8=198/150, 8-9=197/198, 9-11=106/154, 11-13=184/139, 13-14=214/225, 14-16=227/165, 16-17=199/117, 17-18=217/78, 18-19=220/83, 19-20=315/110, 20-21=0/52, 2-32=563/143, 20-22=334/84, 3-5=341/144, 5-7=367/160, 7-10=428/204, 10-12=375/157

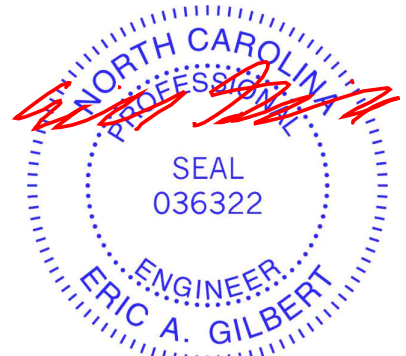
BOT CHORD 31-32=258/361, 29-31=113/440, 28-29=113/440, 27-28=100/192, 26-27=100/192, 25-26=100/192, 24-25=100/192, 23-24=100/192, 22-23=100/192
9-10=79/83, 12-28=354/30, 12-13=163/29, 7-8=95/68, 4-5=41/25, 14-27=89/124, 16-26=161/104, 17-25=147/99, 18-24=160/99, 19-23=90/164, 3-31=0/203, 2-31=18/220

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 8-0-0, Exterior(2R) 8-0-0 to 14-0-0, Interior (1) 14-0-0 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) All plates are 2x4 MT20 unless otherwise indicated.

8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
9) Gable studs spaced at 2-0-0 oc.
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
12) N/A

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 28,2025

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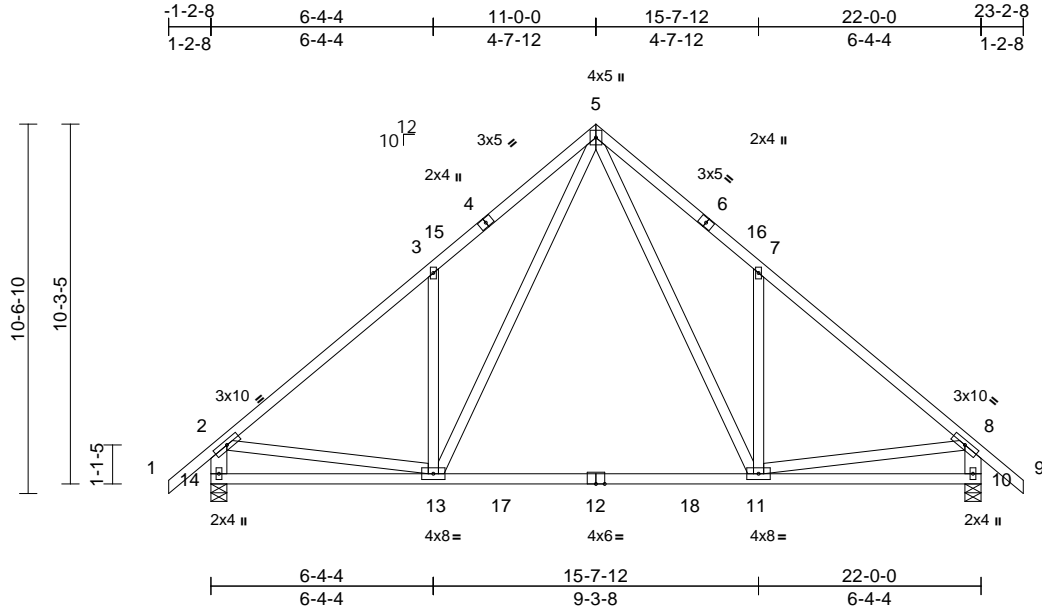
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B2	Common	2	1	173744816
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:14

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Scale = 1:65.8

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	-0.29	11-13	>884	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.84	Vert(CT)	-0.44	11-13	>582	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.01	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 151 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* 11-5,13-5:2x4 SP No.2, 14-2,10-8:2x6 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-10-6 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	10=0-5-8, 14=0-5-8
Max Horiz	14=-272 (LC 12)
Max Uplift	10=-87 (LC 15), 14=-87 (LC 14)
Max Grav	10=1057 (LC 6), 14=1057 (LC 5)

FORCES

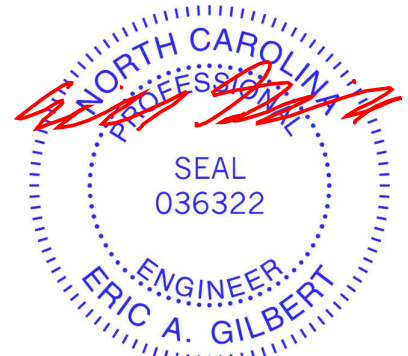
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/55, 2-3=-1266/116, 3-5=-1333/300, 5-7=-1331/299, 7-8=-1264/118, 8-9=0/55, 2-14=-1102/148, 8-10=-1104/146
BOT CHORD	13-14=-272/385, 11-13=0/613, 10-11=-108/267
WEBS	5-11=-236/735, 5-13=-236/738, 3-13=-435/288, 2-13=0/678, 8-11=0/685, 7-11=-435/288

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 8-0-0, Exterior(2R) 8-0-0 to 14-0-0, Interior (1) 14-0-0 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

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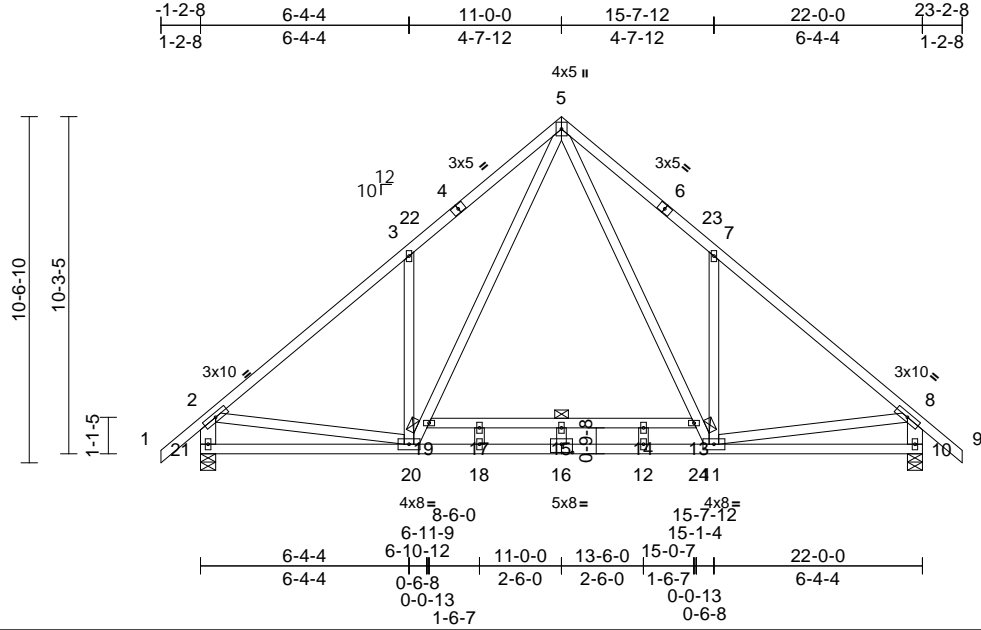
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B3	Common	6	1	I73744817
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:70.2

Plate Offsets (X, Y): [16:0-4-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	-0.35	15	>737	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.84	Vert(CT)	-0.66	15	>391	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.01	10	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 165 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3 *Except* 11-5,20-5:2x4 SP No.2, 21-2,10-8:2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-13 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 10=0-5-8, 21=0-5-8
 Max Horiz 21=272 (LC 13)
 Max Grav 10=1338 (LC 6), 21=1338 (LC 5)

FORCES

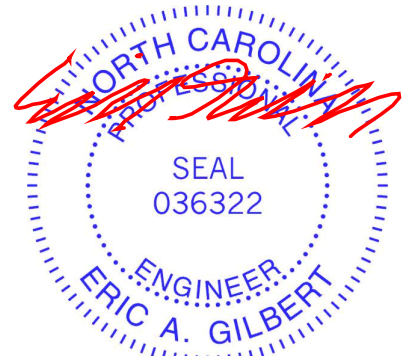
(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/55, 2-3=-1666/0, 3-5=-1730/84, 5-7=-1727/84, 7-8=-1664/0, 8-9=0/55, 2-21=-1411/0, 8-10=-1413/0
 BOT CHORD 20-21=-290/363, 18-20=0/861, 12-18=0/861, 11-12=0/861, 10-11=-126/244, 17-19=-70/0, 15-17=-70/0, 14-15=-70/0, 13-14=-70/0
 WEBS 5-13=-85/1012, 11-13=-160/842, 7-11=-432/290, 19-20=-161/846, 5-19=-85/1016, 3-20=-432/291, 2-20=0/1012, 8-11=0/1019, 15-16=-26/0, 12-14=-24/28, 17-18=-24/28

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 8-0-0, Exterior(2R) 8-0-0 to 14-0-0, Interior (1) 14-0-0 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C- C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 11-0-0 from left end, supported at two points, 5-0-0 apart.
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

LOAD CASE(S) Standard



May 28,2025

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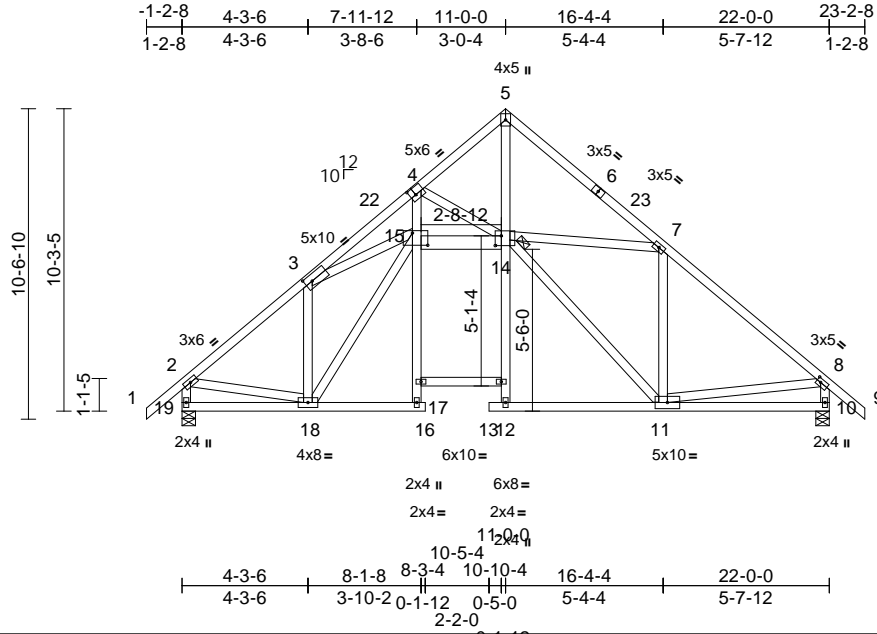
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B4	Roof Special	2	1	I73744818
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:78.3

Plate Offsets (X, Y): [3:0-3-0,0-2-8], [4:0-2-4,0-3-0], [8:0-1-12,0-1-8], [14:0-2-8,0-4-0], [15:0-6-4,0-5-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.68	Vert(LL)	-0.20	16	>999	240	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.72	Vert(CT)	-0.38	16	>690	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(CT)	0.46	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 182 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 15-14:2x6 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

10-0-0 oc bracing: 15-17
1 Brace at Jt(s): 14

REACTIONS

(size) 10=0-5-8, 19=0-5-8
Max Horiz 19=270 (LC 12)
Max Uplift 10=82 (LC 15), 19=81 (LC 14)
Max Grav 10=994 (LC 22), 19=995 (LC 21)

FORCES

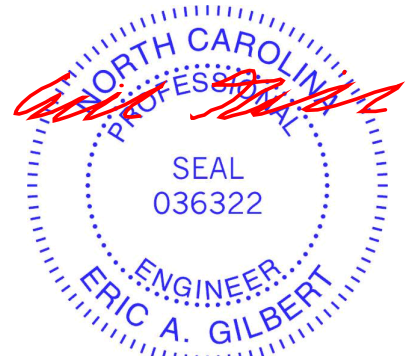
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-1035/102, 3-5=-3850/204, 5-7=-1590/97, 7-8=-1052/117, 8-9=0/52, 2-19=-950/136, 8-10=-942/141
BOT CHORD 18-19=-237/308, 17-18=-1/29, 16-17=0/0, 15-17=0/78, 4-15=-104/2426, 14-15=-77/3179, 12-13=0/0, 11-12=0/5, 10-11=-57/195
WEBS 3-15=0/2414, 4-14=-2253/225, 7-11=-723/54, 2-18=0/662, 8-11=0/640, 3-18=-1208/189, 15-18=-171/1374, 12-14=0/105, 5-14=-42/1682, 7-14=-66/523, 11-14=0/1039

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 7-10-4, Exterior(2R) 7-10-4 to 14-0-0, Interior (1) 14-0-0 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19 and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28, 2025

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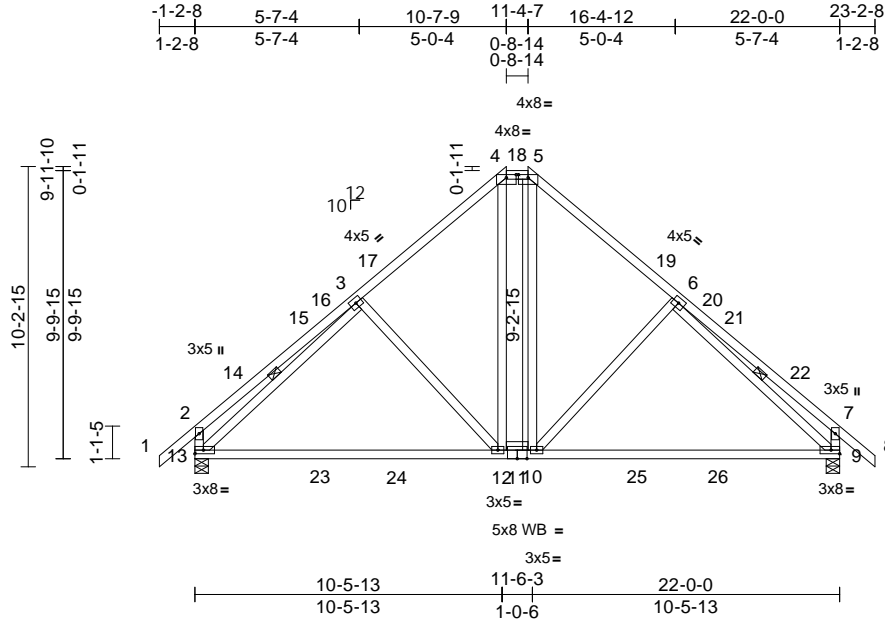
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B5	Hip	1	1	I73744819
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:78.6

Plate Offsets (X, Y): [4:0-4-0,0-1-4], [5:0-4-0,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.33	9-10	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.56	9-10	>467		
TCDL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.03	9	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 153 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-1-14 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 3-13, 6-9

REACTIONS

(size)	9=0-5-8, 13=0-5-8
Max Horiz	13=261 (LC 13)
Max Uplift	9=-89 (LC 15), 13=-89 (LC 14)
Max Grav	9=1228 (LC 53), 13=1226 (LC 51)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/52, 2-3=-551/156, 3-4=-1200/174, 4-5=-835/187, 5-6=-1200/174, 6-7=-536/156, 7-8=0/52, 2-13=-517/172, 7-9=-518/172
BOT CHORD	12-13=-102/1025, 10-12=0/808, 9-10=-5/976
WEBS	3-12=-307/230, 4-12=-44/450, 5-10=-44/452, 6-10=-307/231, 3-13=-958/73, 6-9=-958/62

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 6-4-10, Exterior(2R) 6-4-10 to 15-7-6, Interior (1) 15-7-6 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 9. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 28, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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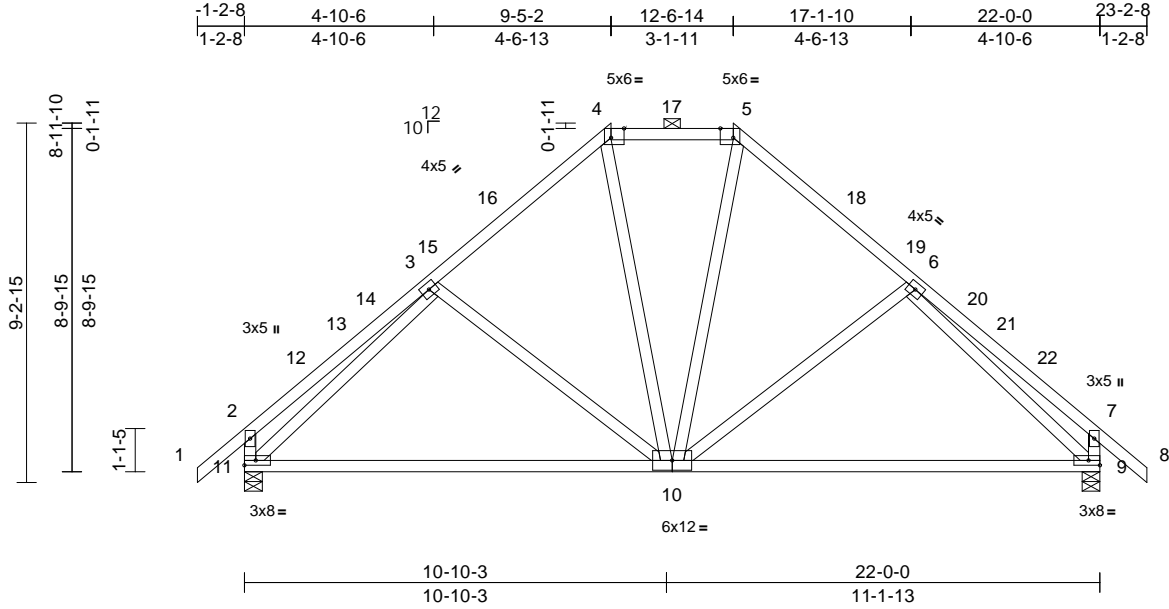
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B6	Hip	1	1	I73744820
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:15
ID:7E?FUaA5HYH69QfNUviRPozCYB0-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f

Page: 1



Scale = 1:59.3

Plate Offsets (X, Y): [4:0-4-0,Edge], [5:0-4-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.60	Vert(LL)	-0.24	10-11	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.48	10-11	>548	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.03	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 147 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-2 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 9=0-5-8, 11=0-5-8
Max Horiz 11=237 (LC 13)
Max Uplift 9=95 (LC 15), 11=95 (LC 14)
Max Grav 9=1147 (LC 41), 11=1147 (LC 41)

FORCES

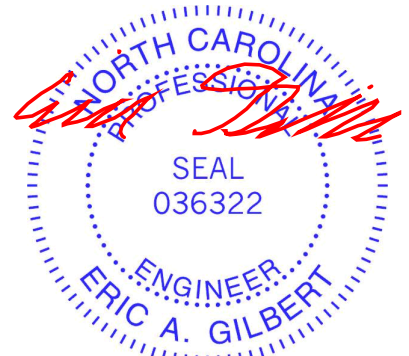
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-364/101, 3-4=-989/164, 4-5=-712/176, 5-6=-989/164, 6-7=-341/101, 7-8=0/52, 2-11=-383/131, 7-9=-382/130
BOT CHORD 9-11=-107/842
WEBS 3-11=-949/121, 6-9=-949/90, 4-10=-46/287, 5-10=-46/287, 3-10=-278/199, 6-10=-278/199

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 5-2-4, Exterior(2R) 5-2-4 to 16-9-12, Interior (1) 16-9-12 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 11 and 9. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 28, 2025

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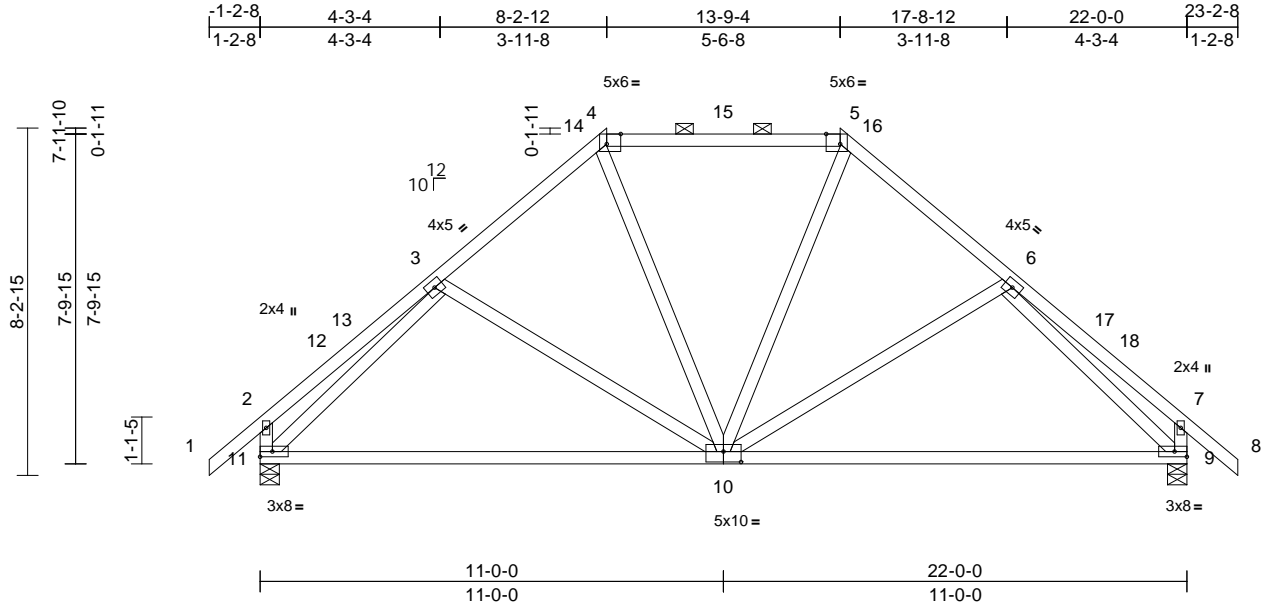
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B7	Hip	1	1	173744821
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:15

Page: 1

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Scale = 1:54.7

Plate Offsets (X, Y): [4:0-4-0,Edge], [5:0-4-0,Edge], [10:0-5-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.79	Vert(LL)	-0.24	10-11	>999	240	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.48	10-11	>548	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(CT)	0.02	9	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 143 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-1 oc purlins, except end verticals, and 2-0-0 oc purlins (4-5-7 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 9=0-5-8, 11=0-5-8
Max Horiz 11=-214 (LC 12)
Max Uplift 9=-101 (LC 15), 11=-101 (LC 14)
Max Grav 9=1108 (LC 41), 11=1108 (LC 41)

FORCES

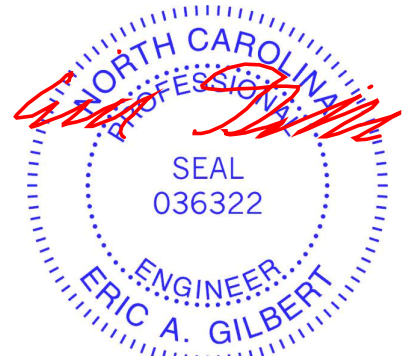
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-346/73, 3-4=-924/164, 4-5=-723/166, 5-6=-924/164, 6-7=-321/73, 7-8=0/52, 2-11=-368/108, 7-9=-365/108
BOT CHORD 9-11=-112/784
WEBS 3-11=-891/146, 6-9=-891/115, 4-10=-18/270, 5-10=-17/270, 3-10=-240/168, 6-10=-240/168

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 3-11-13, Exterior(2R) 3-11-13 to 17-10-7, Interior (1) 17-10-7 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 11 and 9. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 28, 2025

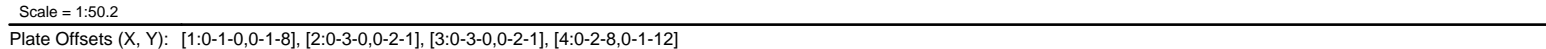
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.


Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:15 Page: 1
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<p>LUMBER</p> <p>TOP CHORD 2x4 SP No.1 *Except* 2-3:2x4 SP 2400F 2.0E, 3-5:2x4 SP No.2</p> <p>BOT CHORD 2x4 SP No.2</p> <p>WEBS 2x4 SP No.3 *Except* 7-2:2x4 SP No.2, 10-1,6-4:2x6 SP No.2</p> <p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (4-10-14 max.): 2-3.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <p>WEBS 1 Row at midpt 2-7</p> <p>REACTIONS (size) 6=0-5-8, 10= Mechanical</p> <p>Max Horiz 10=-183 (LC 10)</p> <p>Max Uplift 6=-107 (LC 15), 10=-79 (LC 14)</p> <p>Max Grav 6=1113 (LC 47), 10=1048 (LC 47)</p> <p>FORCES (lb) - Maximum Compression/Maximum Tension</p> <p>TOP CHORD 1-2=-1257/147, 2-3=-878/186, 3-4=-1249/155, 4-5=0/55, 1-10=-1066/131, 4-6=-1120/180</p> <p>BOT CHORD 9-10=-171/450, 7-9=-81/862, 6-7=-172/490</p> <p>WEBS 2-9=0/315, 2-7=-123/112, 3-7=0/300, 1-9=-85/680, 4-7=-126/700</p> <p>NOTES</p>	<p>3) TCELL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10</p> <p>4) Unbalanced snow loads have been considered for this design.</p> <p>5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.</p> <p>6) Provide adequate drainage to prevent water ponding.</p> <p>7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</p> <p>8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.</p> <p>9) Refer to girder(s) for truss to truss connections.</p> <p>10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 10.</p> <p>11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.</p> <p>12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.</p>	
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WARNING – Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEL REFERENCE PAGE MIT-TR-17-0169, 1/12/2023 BEFORE USE.

Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Components Association (www.sbcacomponents.com)

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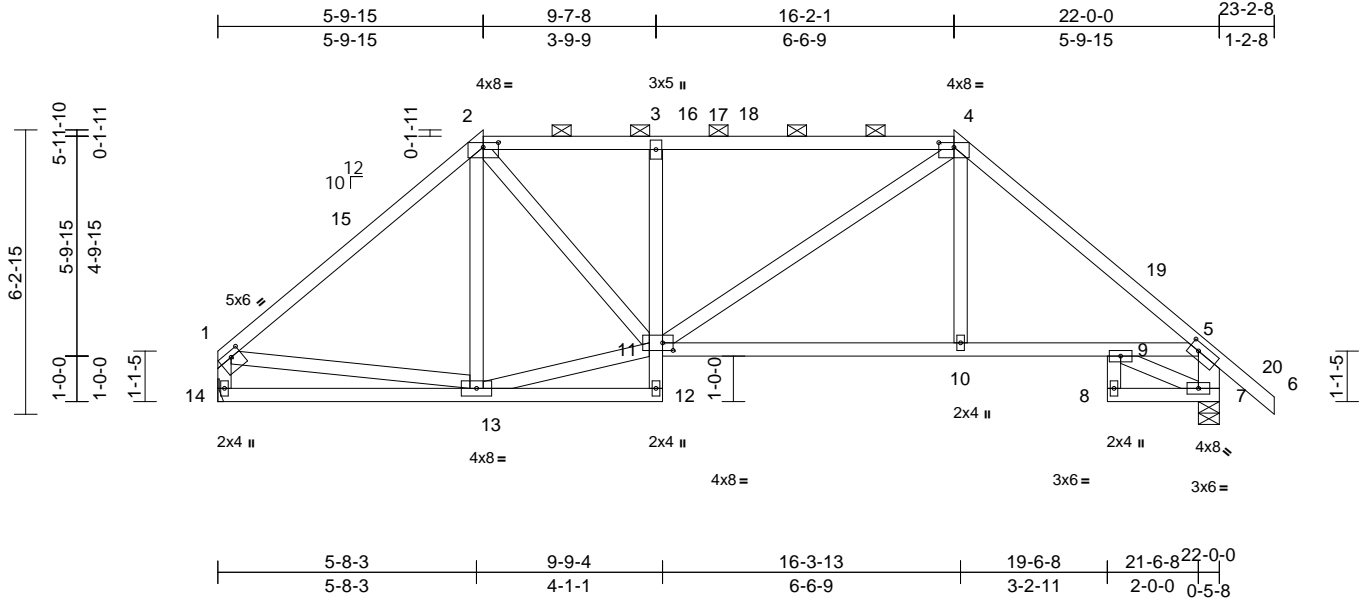
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B9	Hip	1	1	173744823
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:15

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Scale = 1:50.6

Plate Offsets (X, Y): [1:0-2-12,0-1-8], [2:0-4-0,0-1-4], [4:0-4-0,0-1-4], [5:0-2-8,0-2-0], [11:0-2-12,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.06	10-11	>999	240	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.13	10-11	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.34	Horz(CT)	0.06	7	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 139 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 12-3,9-8:2x4 SP No.3
WEBS 2x4 SP No.3 *Except* 7-5:2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 2-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 12-13.

REACTIONS

(size) 7=0-5-8, 14= Mechanical
Max Horiz 14=159 (LC 10)
Max Uplift 7=112 (LC 15), 14=84 (LC 14)
Max Grav 7=1052 (LC 41), 14=924 (LC 41)

FORCES

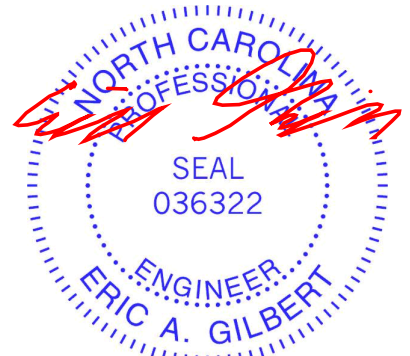
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1068/142, 2-3=-1304/177, 3-4=-1336/181, 4-5=-1297/127, 5-6=0/79, 1-14=-867/125, 5-7=-1094/177
BOT CHORD 13-14=-142/331, 12-13=-30/50, 11-12=0/46, 3-11=-612/163, 10-11=-32/960, 9-10=-29/965, 5-9=-85/903, 8-9=0/38, 7-8=-24/100
WEBS 2-13=-197/80, 11-13=-66/750, 2-11=-94/827, 4-11=-133/456, 4-10=0/294, 1-13=-75/675, 7-9=-81/167

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Exterior(2R) 3-1-12 to 10-0-14, Interior (1) 10-0-14 to 11-11-2, Exterior(2R) 11-11-2 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 14.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 28, 2025

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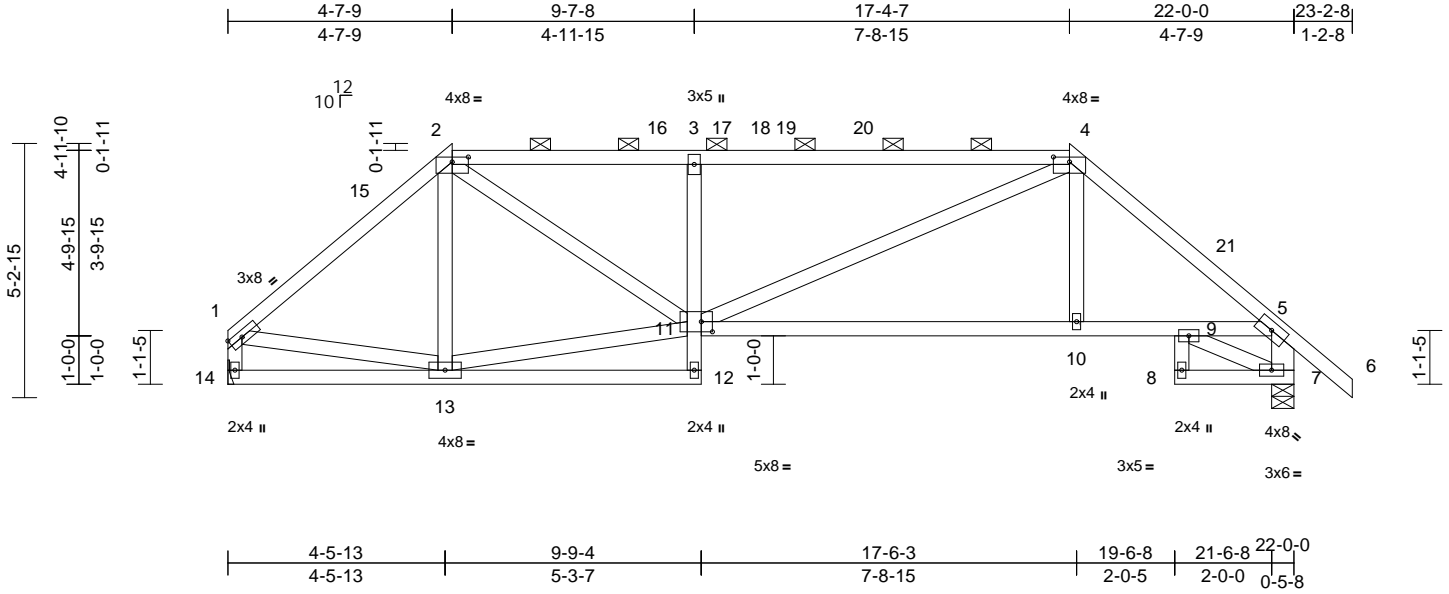
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B10	Hip	1	1	173744824
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:15

Page: 1

ID:DmVTz43yF4MoHhMxIMadL5zCYDk-RfC?PsB70Hq3NSgPqnL8w3uITxbGKwRCDoi7J4zJC?f



Scale = 1:47.6

Plate Offsets (X, Y): [2:0-4-0,0-1-4], [4:0-4-0,0-1-4], [11:0-2-12,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	-0.11	10-11	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.24	10-11	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(CT)	0.05	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 134 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* 2-4:2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP No.2 *Except* 12-3,9-8:2x4 SP No.3
WEBS	2x4 SP No.3 *Except* 7-5:2x6 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-7-5 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-15 max.): 2-4.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 7-8.

REACTIONS

(size)	7=0-5-8, 14= Mechanical
Max Horiz	14=135 (LC 10)
Max Uplift	7=116 (LC 15), 14=88 (LC 14)
Max Grav	7=1023 (LC 40), 14=971 (LC 40)

FORCES

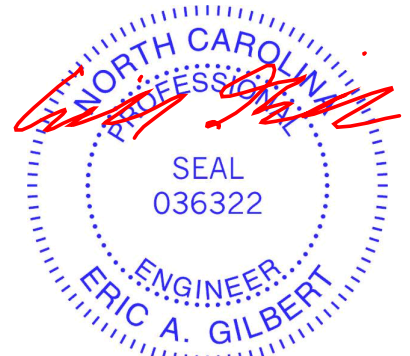
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-1147/115, 2-3=-1816/188, 3-4=-1875/197, 4-5=-1480/100, 5-6=0/89, 1-14=-931/108, 5-7=-1005/141
BOT CHORD	13-14=-114/224, 12-13=-31/84, 11-12=0/93, 3-11=-754/195, 10-11=-56/1120, 9-10=-51/1126, 5-9=-97/1103, 8-9=0/36, 7-8=-11/39
WEBS	2-13=-214/81, 11-13=-81/790, 2-11=-136/1172, 4-11=-170/829, 4-10=0/311, 1-13=-67/803, 7-9=-28/111

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Exterior(2R) 3-1-12 to 8-10-7, Interior (1) 8-10-7 to 13-1-9, Exterior (2R) 13-1-9 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 14.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



May 28,2025

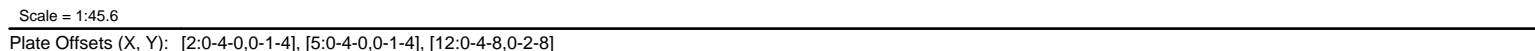
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:16 Page: 1
ID:DDRcqSfdGd4dCSIPIDzfzwcYEG-RfC?PsB70Hq3NSqPqnL8w3uTXhGKWrCDoi7J4zJC?i



LUMBER		Wind: ASCE 7-16; Vult=130mph (3-second gust)
TOP CHORD	2x4 SP No.2 *Except* 2-5:2x4 SP No.1	Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
BOT CHORD	2x4 SP No.2 *Except* 13-3,10-9:2x4 SP No.3	II; Exp B; Enclosed; MWFRS (envelope) exterior zone
WEBS	2x4 SP No.3 *Except* 8-6:2x6 SP No.2	and C-C Exterior(2E) 0-1-12 to 3-5-2, Exterior(2R) 3-5-2
BRACING		to 7-8-1, Interior (1) 7-8-1 to 14-2-1, Exterior(2R) 14-2-1
TOP CHORD	Structural wood sheathing directly applied or 4-7-10 oc purlins, except end verticals, and 2-0-0 oc purlins (2-4-4 max.): 2-5.	to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone; cantilever
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 8-9.	left and right exposed ; end vertical left and right
REACTIONS		exposed;C-C for members and forces & MWFRS for
	(size) 8=0-5-8, 15= Mechanical	reactions shown; Lumber DOL=1.60 plate grip
	Max Horiz 15=-112 (LC 10)	DOL=1.60
	Max Uplift 8=-119 (LC 15), 15=-91 (LC 14)	
	Max Grav 8=1065 (LC 40), 15=1012 (LC 40)	
FORCES		3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
	(lb) - Maximum Compression/Maximum Tension	Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate
TOP CHORD	1-2=-1132/101, 2-3=-2471/283,	DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9;
	3-4=-2477/269, 4-5=-1190/97, 5-6=-1614/87,	Cs=1.00; Ct=1.10
	6-7=0/89, 1-15=-1004/90, 6-8=-1061/128	4) Unbalanced snow loads have been considered for this
BOT CHORD	14-15=-98/133, 13-14=-16/88, 12-13=0/124,	design.
	3-12=-527/150, 11-12=-270/2203,	5) This truss has been designed for greater of min roof live
	10-11=-59/1232, 6-10=-75/1208, 9-10=0/33,	load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on
	8-9=-57/0	overhangs non-concurrent with other live loads.
WEBS	2-14=-285/102, 12-14=-108/834,	6) Provide adequate drainage to prevent water ponding.
	2-12=-196/1685, 4-12=-54/351,	7) This truss has been designed for a 10.0 psf bottom
	4-11=-1162/242, 5-11=-5/753, 1-14=-67/911,	chord live load nonconcurrent with any other live loads.
	8-10=0/94	8) * This truss has been designed for a live load of 20.0psf
NOTES		on the bottom chord in all areas where a rectangle
1)	Unbalanced roof live loads have been considered for this design.	3-06-00 tall by 2-00-00 wide will fit between the bottom
		chord and any other members.
		9) Refer to girder(s) for truss to truss connections.
		10) Provide mechanical connection (by others) of truss to
		bearing plate capable of withstanding 91 lb uplift at joint
		15.
		11) One H2.5A Simpson Strong-Tie connectors
		recommended to connect truss to bearing walls due to
		UPLIFT at jt(s) 8. This connection is for uplift only and
		does not consider lateral forces.
		12) Graphical purlin representation does not depict the size
		or the orientation of the purlin along the top and/or
		bottom chord.



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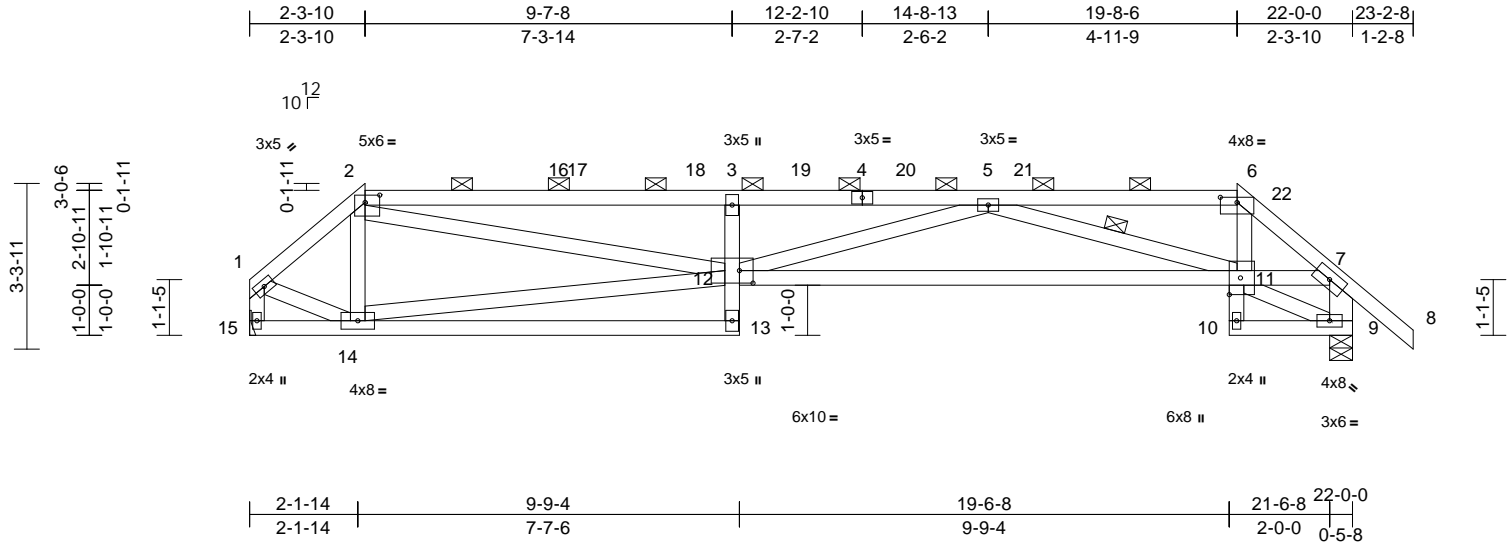
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B12	Hip	1	1	173744826
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:16
ID:Y015Lg4S6OaD_KED7C7x9LzCYGJ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCD0i7J4zJC?f

Page: 1



Scale = 1:46

Plate Offsets (X, Y): [2:0-3-8,0-1-12], [6:0-4-0,0-1-4], [11:0-4-0,0-2-11], [12:0-3-4,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.96	Vert(LL)	-0.28	11-12	>932	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.59	11-12	>436	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(CT)	0.16	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 129 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 2-4:2x4 SP 2400F
2.0E
BOT CHORD 2x4 SP No.2 *Except* 13-3,11-10:2x4 SP
No.3, 12-7:2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 12-2:2x4 SP No.2,
9-7:2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or
4-3-0 oc purlins, except end verticals, and
2-0-0 oc purlins (2-2-0 max.): 2-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc
bracing.
WEBS 1 Row at midpt 5-11

REACTIONS

(size) 9=0-5-8, 15= Mechanical
Max Horiz 15=90 (LC 10)
Max Uplift 9=121 (LC 15), 15=97 (LC 11)
Max Grav 9=1097 (LC 40), 15=1044 (LC 40)

FORCES

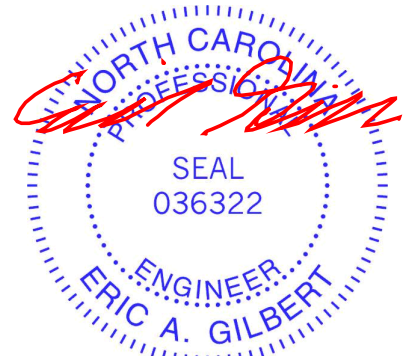
(lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=-1081/92, 2-3=-3743/484,
3-5=-3807/458, 5-6=-1424/75, 6-7=-1804/74,
7-8=0/89, 1-15=-1085/69, 7-9=-1085/115
BOT CHORD 14-15=-89/89, 13-14=-26/244, 12-13=0/140,
3-12=-542/171, 11-12=-455/3292,
7-11=-49/1466, 10-11=0/28, 9-10=-171/0
WEBS 2-14=-381/134, 12-14=-94/687,
2-12=-382/2886, 5-12=-79/564,
5-11=-1964/411, 1-14=-68/1014, 9-11=0/113,
6-11=0/826

NOTES

1) Unbalanced roof live loads have been considered for
this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
II; Exp B; Enclosed; MWFRS (envelope) exterior zone
and C-C Exterior(2E) 0-1-12 to 2-3-10, Exterior(2R)
2-3-10 to 6-6-9, Interior (1) 6-6-9 to 15-5-7, Exterior(2R)
15-5-7 to 20-2-8, Exterior(2E) 20-2-8 to 23-2-8 zone;
cantilever left and right exposed ; end vertical left and
right exposed;C-C for members and forces & MWFRS
for reactions shown; Lumber DOL=1.60 plate grip
DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate
DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9;
Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this
design.
- 5) This truss has been designed for greater of min roof live
load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on
overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom
chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 97 lb uplift at joint
15.
- 11) One H2.5A Simpson Strong-Tie connectors
recommended to connect truss to bearing walls due to
UPLIFT at jt(s) 9. This connection is for uplift only and
does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size
or the orientation of the purlin along the top and/or
bottom chord.

LOAD CASE(S) Standard



May 28,2025

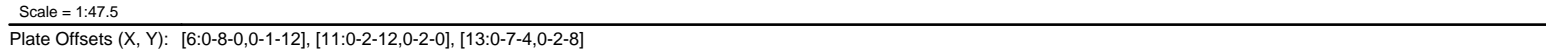
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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:16 Page: 1
ID:IOliTvW9G3RNdfSddKqWzJzCYIJ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWRcD0i7J4zJC?f



LUMBER		1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Web connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.	6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 16.
TOP CHORD	2x4 SP No.2		
BOT CHORD	2x4 SP No.2 *Except* 14-3,11-10:2x4 SP No.3, 13-7:2x4 SP No.1	2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.	12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9. This connection is for uplift only and does not consider lateral forces.
WEBS	2x4 SP No.3 *Except* 9-7,6-11:2x6 SP No.2	3) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60	13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
BRACING			14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (4-3-8 max.): 1-6.	4) TC LL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10	LOAD CASE(S) Standard
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 9-10.	5) Unbalanced snow loads have been considered for this design	1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (lb/ft) Vert: 1-6=-58, 6-7=-58, 7-8=-58, 14-16=-19, 11-13=-19, 9-10=-19 Concentrated Loads (lb)
REACTIONS	(size) 9=0-5-8, 16= Mechanical Max Horiz 16=-99 (LC 8) Max Uplift 9=-256 (LC 8), 16=-254 (LC 8) Max Grav 9=1626 (LC 33), 16=1635 (LC 33)	6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.	
FORCES	(lb) - Maximum Compression/Maximum Tension	7) Provide adequate drainage to prevent water ponding.	
TOP CHORD	1-16=-239/69, 1-2=-73/31, 2-3=-6103/1080, 3-4=-5783/1022, 4-6=-5783/1022, 6-7=-2562/437, 7-8=0/94, 7-9=-1616/262	8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	
BOT CHORD	15-16=-460/2772, 14-15=-78/506, 13-14=0/105, 3-13=-224/122, 12-13=-1152/6502, 11-12=-340/2063, 7-11=-353/2001, 10-11=0/31, 9-10=-73/12	9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.	
WEBS	2-16=-3009/488, 2-15=-330/185, 13-15=-391/2321, 2-13=-652/3473, 3-12=-790/127, 4-12=-565/166, 6-12=-712/3842, 6-11=0/278, 9-11=-11/64	10) Refer to girder(s) for truss to truss connections.	
NOTES			



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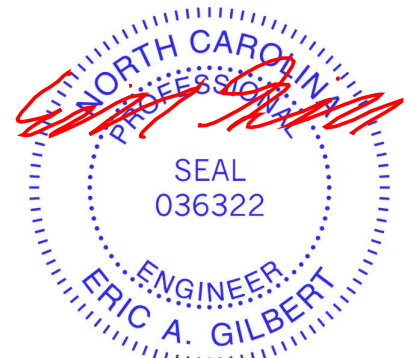
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	B13	Roof Special Girder	1	2	I73744827
					Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:16
ID:IOItVw9G3RNdfSddKqWzJzCYIJ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWRCDoi7J4zJC?f

Page: 2

Vert: 5=-67 (F), 6=-94 (F), 17=-94 (F), 18=-94 (F),
19=-94 (F), 21=-94 (F), 22=-67 (F), 23=-67 (F),
25=-67 (F), 27=-67 (F), 28=-38 (F), 29=-38 (F),
30=-38 (F), 31=-38 (F), 32=-65 (F), 33=-65 (F),
34=-65 (F), 35=-65 (F), 36=-65 (F), 37=-38 (F)



May 28, 2025

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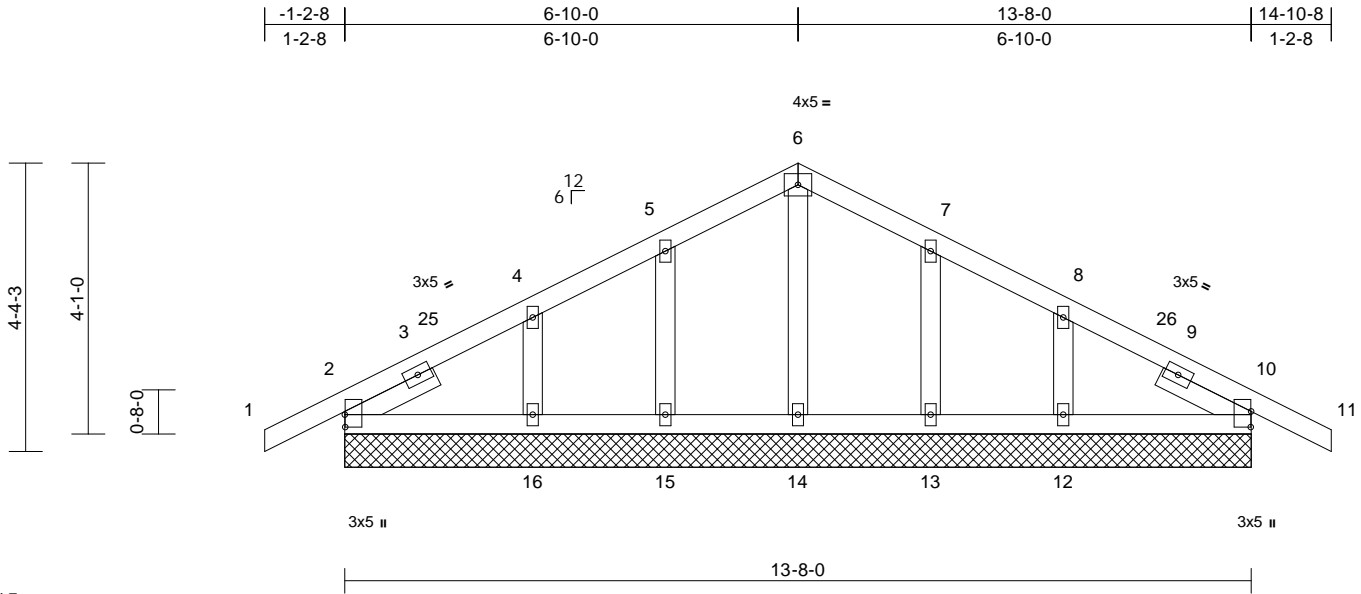
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	C01	Common Supported Gable	1	1	173744828
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17

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Plate Offsets (X, Y): [2:0-2-4,0-0-1], [10:0-2-13,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 69 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.2 -- 1-6-0, Right 2x4 SP No.2 -- 1-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	2=13-8-0, 10=13-8-0, 12=13-8-0, 13=13-8-0, 14=13-8-0, 15=13-8-0, 16=13-8-0
Max Horiz	2=64 (LC 14)
Max Uplift	2=22 (LC 15), 10=33 (LC 15), 12=63 (LC 15), 13=40 (LC 15), 15=39 (LC 14), 16=67 (LC 14)
Max Grav	2=216 (LC 21), 10=216 (LC 22), 12=276 (LC 22), 13=229 (LC 22), 14=124 (LC 21), 15=229 (LC 21), 16=276 (LC 21)

FORCES

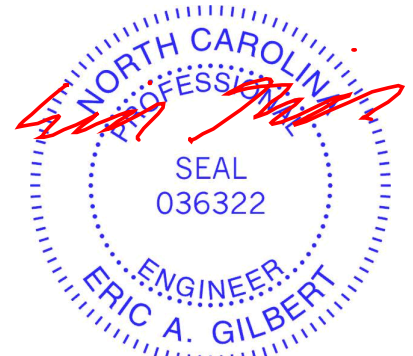
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/32, 2-4=-59/51, 4-5=-89/101, 5-6=-101/159, 6-7=-101/159, 7-8=-89/101, 8-10=-59/34, 10-11=0/32
BOT CHORD	2-16=0/63, 15-16=0/63, 14-15=0/63, 13-14=0/63, 12-13=0/63, 10-12=0/63
WEBS	6-14=-81/4, 5-15=-197/105, 4-16=-213/126, 7-13=-197/105, 8-12=-213/126

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -1-2-8 to 1-9-8, Exterior(2N) 1-9-8 to 3-10-0, Corner(3R) 3-10-0 to 9-10-0, Exterior(2N) 9-10-0 to 11-10-8, Corner(3E) 11-10-8 to 14-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 2, 33 lb uplift at joint 10, 39 lb uplift at joint 15, 67 lb uplift at joint 16, 40 lb uplift at joint 13, 63 lb uplift at joint 12, 22 lb uplift at joint 2 and 33 lb uplift at joint 10.

LOAD CASE(S) Standard



May 28, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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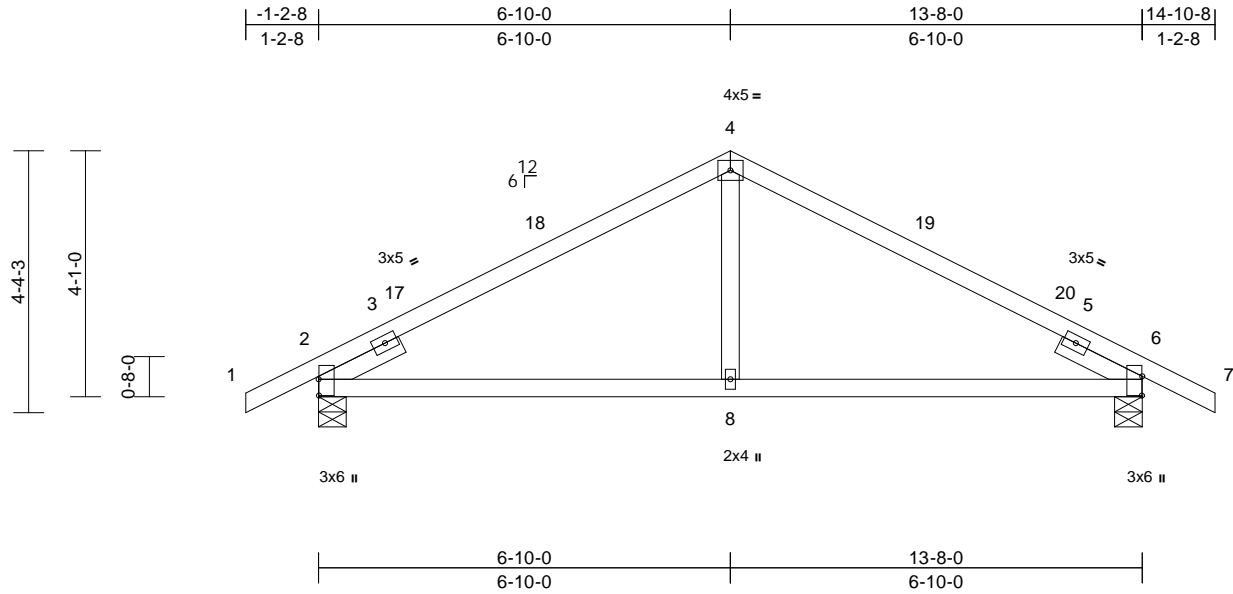
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	C02	Common	1	1	173744829
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17
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Page: 1



Scale = 1:38.2

Plate Offsets (X, Y): [2:0-3-4,0-0-1], [6:0-3-13,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.10	8-11	>999	240	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.59	Vert(CT)	-0.14	8-11	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	0.03	2	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 57 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	2=0-5-8, 6=0-5-8
Max Horiz	2=64 (LC 14)
Max Uplift	2=-76 (LC 14), 6=-76 (LC 15)
Max Grav	2=704 (LC 21), 6=704 (LC 22)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/32, 2-4=-756/230, 4-6=-756/230, 6-7=0/32
BOT CHORD	2-8=-167/576, 6-8=-157/576
WEBS	4-8=0/293

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-2-8 to 1-9-8, Interior (1) 1-9-8 to 3-10-0, Exterior(2R) 3-10-0 to 9-10-0, Interior (1) 9-10-0 to 11-10-8, Exterior(2E) 11-10-8 to 14-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

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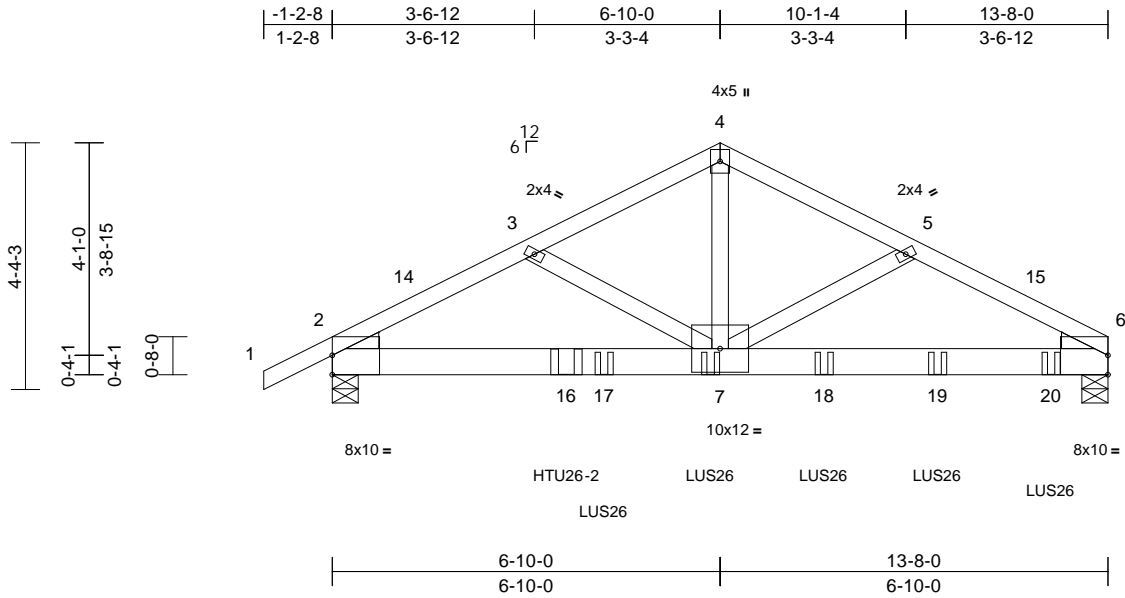
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	C03	Common Girder	1	2	I73744830
					Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:40.6

Plate Offsets (X, Y): [2:Edge,0-4-1], [6:Edge,0-4-1]

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.27	Vert(LL)	-0.11	7-13	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.72	Vert(CT)	-0.18	7-13	>924	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.93	Horz(CT)	0.02	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 149 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP 2400F 2.0E
 WEBS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-13 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-5-8, 6=0-5-8
 Max Horiz 2=71 (LC 12)
 Max Uplift 2=445 (LC 12), 6=442 (LC 13)
 Max Grav 2=3592 (LC 19), 6=4168 (LC 20)

FORCES

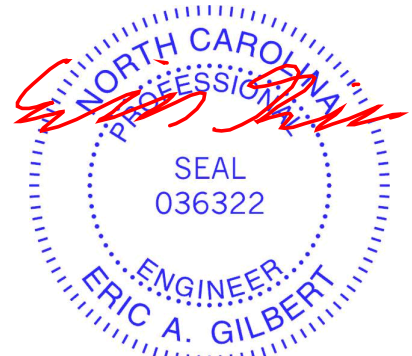
(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/31, 2-3=-5644/670, 3-4=-5359/630,
 4-5=-5363/627, 5-6=-5649/663
 BOT CHORD 2-7=-616/5015, 6-7=-552/5021
 WEBS 4-7=-495/4500, 5-7=-332/123, 3-7=-329/130

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 3 rows staggered at 0-8-0 oc.
 Web connected as follows: 2x4 - 1 row at 0-9-0 oc, Except member 4-7 2x4 - 1 row at 0-6-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.
- Use Simpson Strong-Tie HTU26-2 (20-10d Girder, 14-10d Truss) or equivalent at 4-1-8 from the left end to connect truss(es) to front face of bottom chord.
- Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss) or equivalent spaced at 6-0-0 oc max. starting at 4-9-8 from the left end to 12-8-0 to connect truss(es) to front face of bottom chord.
- Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss, Single Ply Girder) or equivalent at 8-8-0 from the left end to connect truss(es) to front face of bottom chord.

- Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss) or equivalent at 10-8-0 from the left end to connect truss(es) to front face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (lb/ft)
 Vert: 1-4=-58, 4-6=-58, 8-11=-19
 Concentrated Loads (lb)
 Vert: 7=-993 (F), 16=-1616 (F), 17=-1024 (F),
 18=-951 (F), 19=-905 (F), 20=-977 (F)



May 28,2025

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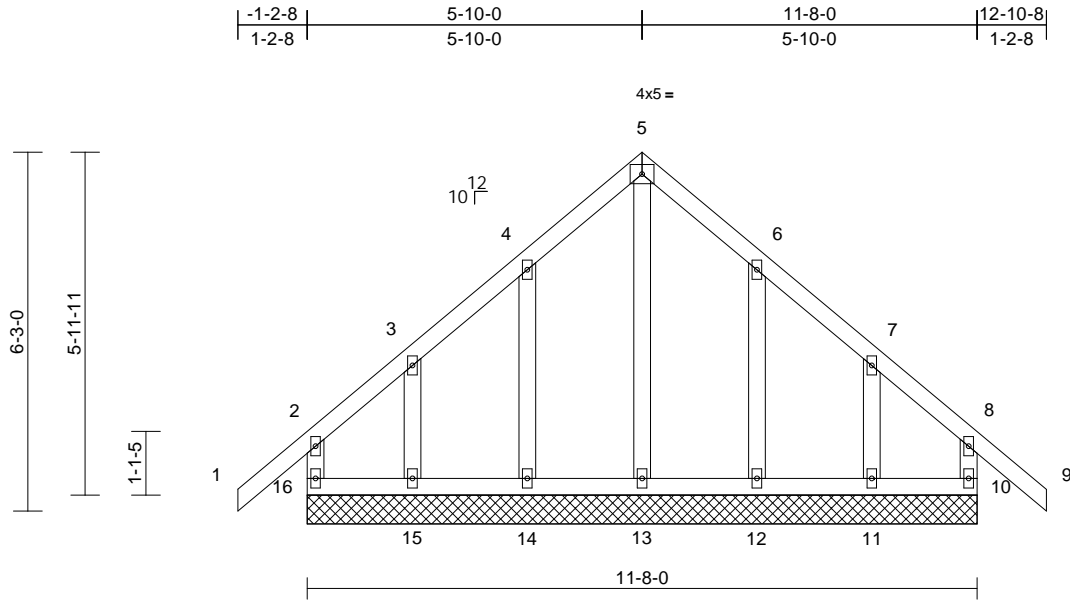
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	D1GE	Common Supported Gable	1	1	173744831
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17

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Scale = 1:40.1

Plate Offsets (X, Y): [6:0-0-0,Edge], [7:0-0-0,Edge], [8:0-0-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.00	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
Weight: 73 lb FT = 20%											

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10'-0'-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6'-0'-0 oc bracing.

REACTIONS (size)	10=11-8-0, 11=11-8-0, 12=11-8-0, 13=11-8-0, 14=11-8-0, 15=11-8-0, 16=11-8-0
Max Horiz	16=169 (LC 12)
Max Uplift	10=56 (LC 11), 11=102 (LC 15), 12=72 (LC 15), 14=72 (LC 14), 15=105 (LC 14), 16=69 (LC 10)
Max Grav	10=161 (LC 25), 11=178 (LC 31), 12=281 (LC 22), 13=202 (LC 28), 14=281 (LC 21), 15=184 (LC 25), 16=172 (LC 26)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	2-16=-142/180, 1-2=0/52, 2-3=-93/99, 3-4=-62/163, 4-5=-115/271, 5-6=-115/271, 6-7=-61/164, 7-8=-78/86, 8-9=0/52, 8-10=-142/170
BOT CHORD	15-16=-84/132, 14-15=-84/132, 13-14=-84/132, 12-13=-84/132, 11-12=-84/132, 10-11=-84/132
WEBS	5-13=-263/46, 4-14=-240/145, 3-15=-152/123, 6-12=-240/143, 7-11=-143/135

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -1-2-8 to 1-10-0, Exterior(2N) 1-10-0 to 2-10-0, Corner(3R) 2-10-0 to 8-10-0, Exterior (2N) 8-10-0 to 9-10-0, Corner(3E) 9-10-0 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2'-0'-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0'-0 tall by 2'-0'-0 wide will fit between the bottom chord and any other members.

- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 16, 56 lb uplift at joint 10, 72 lb uplift at joint 14, 105 lb uplift at joint 15, 72 lb uplift at joint 12 and 102 lb uplift at joint 11.

LOAD CASE(S) Standard



May 28, 2025

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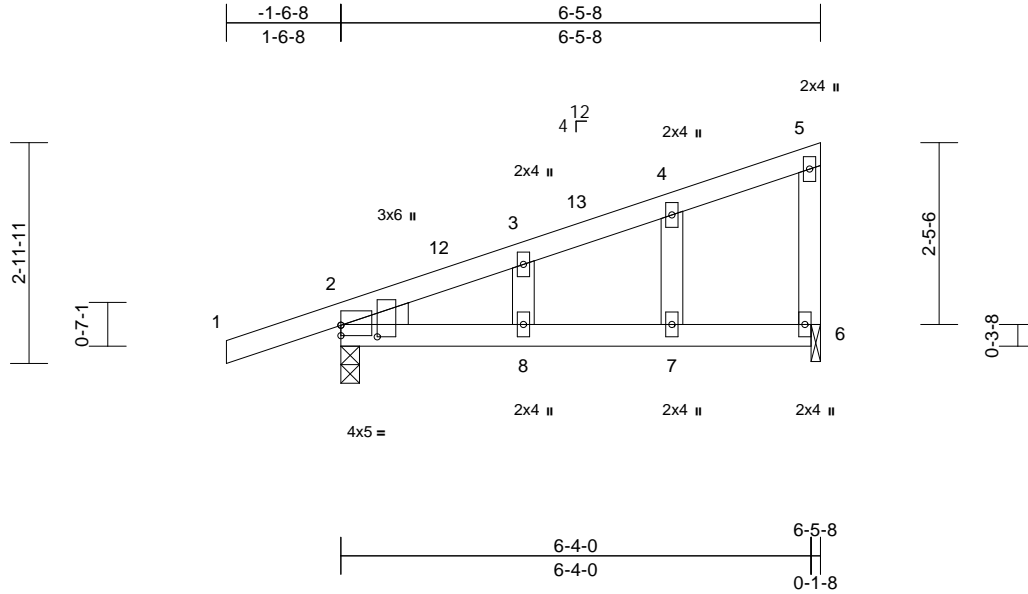
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	E1GE	Monopitch Structural Gable	1	1	I73744832
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17
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Page: 1



Scale = 1:31

Plate Offsets (X, Y): [2:Edge,0-1-11], [2:0-1-14,0-5-14]

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	0.16	7-8	>461	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.24	7-8	>322	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.03	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 30 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	2=0-3-0, 6=0-1-8
Max Horiz	2=99 (LC 13)
Max Uplift	2=-146 (LC 10), 6=-93 (LC 10)
Max Grav	2=434 (LC 21), 6=322 (LC 21)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=-0/28, 2-3=-95/66, 3-4=-50/65, 4-5=-40/64, 5-6=-182/132
BOT CHORD	2-8=-61/124, 7-8=-30/44, 6-7=-30/44
WEBS	4-7=-77/63, 3-8=-62/40

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-8 to 1-5-8, Interior (1) 1-5-8 to 3-3-12, Exterior(2E) 3-3-12 to 6-3-12 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 6 SP No.3 .
- Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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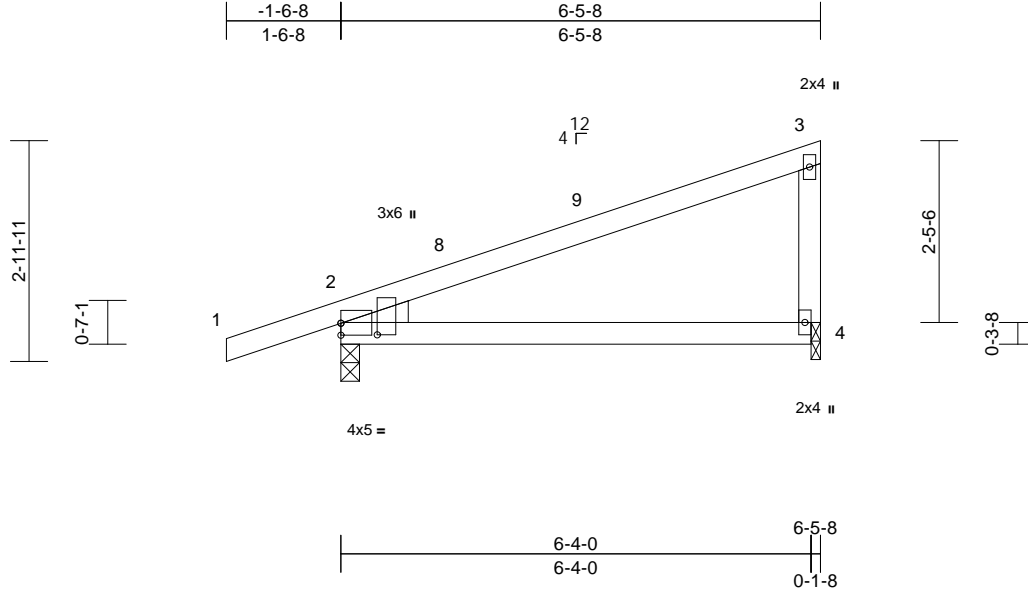
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	E2	Monopitch	5	1	I73744833
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17

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Scale = 1:31

Plate Offsets (X, Y): [2:Edge,0-1-15], [2:0-1-14,0-5-14]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	0.15	4-7	>502	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.21	4-7	>362	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.03	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
Weight: 27 lb FT = 20%												

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-0, 4=0-1-8
Max Horiz 2=102 (LC 13)
Max Uplift 2=-151 (LC 10), 4=-96 (LC 10)
Max Grav 2=448 (LC 21), 4=333 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/29, 2-3=-86/75, 3-4=-236/175
BOT CHORD 2-4=-63/128

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-8 to 1-5-8, Interior (1) 1-5-8 to 3-3-12, Exterior(2E) 3-3-12 to 6-3-12 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28, 2025

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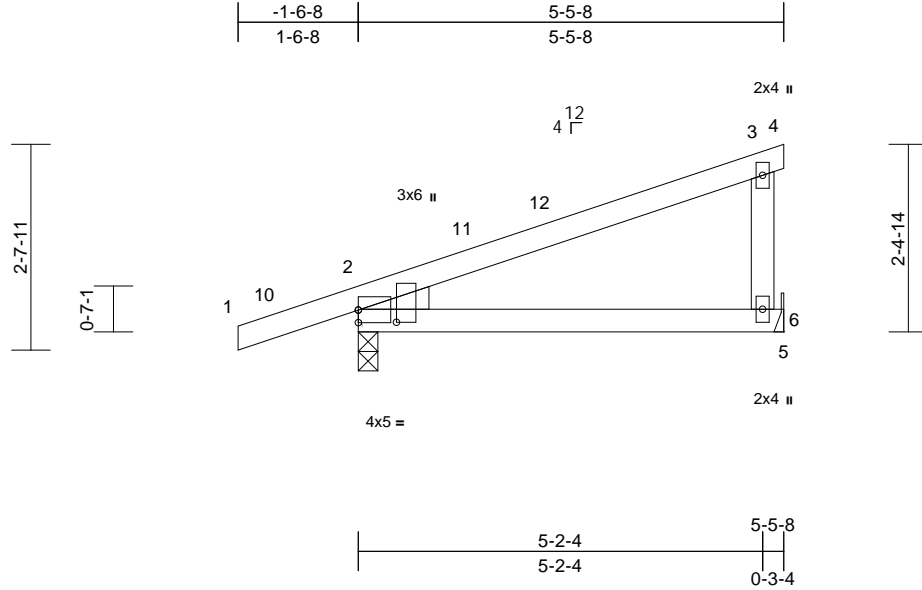
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	E3	Monopitch	5	1	I73744834
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17

Page: 1

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Scale = 1:29.6

Plate Offsets (X, Y): [2:Edge,0-1-15], [2:0-1-14,0-5-14]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	0.07	6-9	>901	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.39	Vert(CT)	-0.09	6-9	>699	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 23 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-0, 6= Mechanical
Max Horiz 2=89 (LC 13)
Max Uplift 2=-136 (LC 10), 6=-80 (LC 10)
Max Grav 2=423 (LC 21), 6=296 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-60/62, 3-4=-8/0, 3-6=-213/145
BOT CHORD 2-6=-51/107, 5-6=0/0

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-8 to 1-5-8, Interior (1) 1-5-8 to 2-5-8, Exterior(2E) 2-5-8 to 5-5-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 6.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28, 2025

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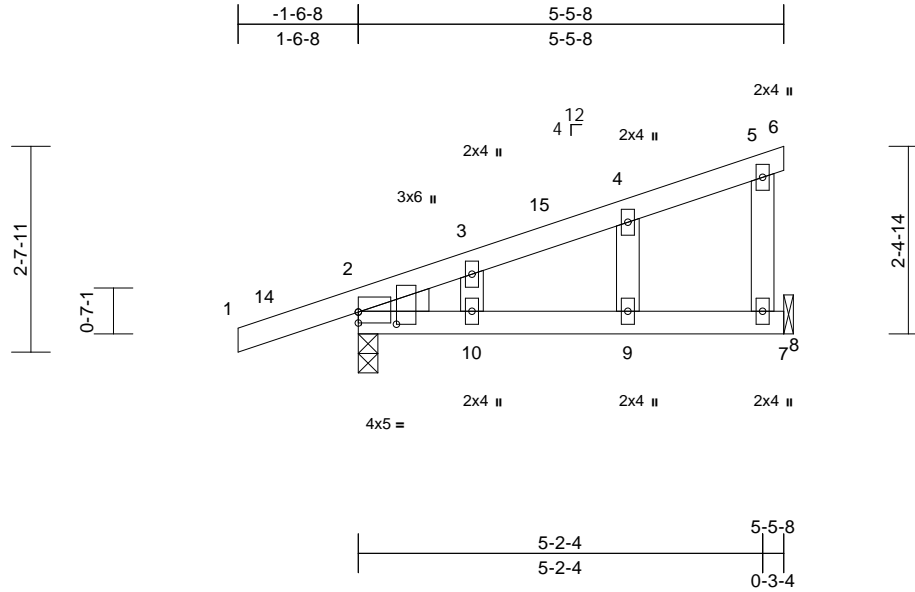
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	E4	Monopitch Structural Gable	1	1	I73744835
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:17

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Scale = 1:29.6

Plate Offsets (X, Y): [2:Edge,0-1-11], [2:0-1-14,0-5-14]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	0.08	9-10	>812	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.44	Vert(CT)	-0.10	9-10	>612	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
Weight: 26 lb											FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-0, 8= Mechanical
 Max Horiz 2=89 (LC 13)
 Max Uplift 2=-136 (LC 10), 8=-80 (LC 10)
 Max Grav 2=423 (LC 21), 8=296 (LC 21)

FORCES

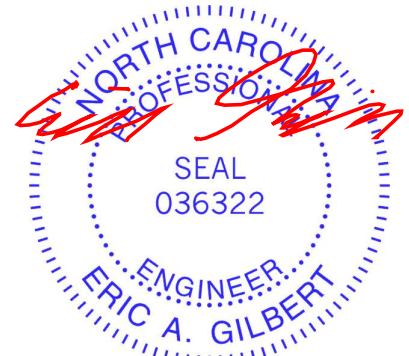
(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/39, 2-3=-55/60, 3-4=-49/58,
 4-5=-38/55, 5-6=-8/0, 5-8=-178/115
 BOT CHORD 2-10=-51/106, 9-10=-26/38, 8-9=-26/38,
 7-8=0/0
 WEBS 4-9=-76/65, 3-10=-61/41

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-6-8 to 1-5-8, Interior (1) 1-5-8 to 2-5-8, Exterior(2E) 2-5-8 to 5-5-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 8.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

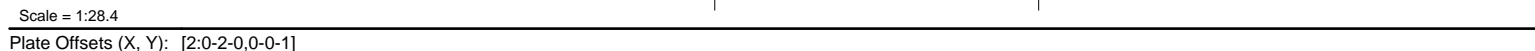
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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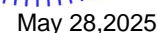
818 Soundside Road
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Cartier Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:18 Page: 1
ID:YtiqfvcqBnoGi59c1zplMGzCYN -RfC?PsB70Ha3NSaPanL8w3uITxbGKWkRCDoi7J4zJC?f



LUMBER		5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
TOP CHORD	2x4 SP No.2	6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
BOT CHORD	2x4 SP No.2	7) Bearings are assumed to be: , Joint 2 User Defined .
SLIDER	Left 2x4 SP No.2 -- 1-6-0	8) Refer to girder(s) for truss to truss connections.
BRACING		9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 4.
TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins.	10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
REACTIONS (size)		
	2=0-5-8, 4= Mechanical, 5= Mechanical	
	Max Horiz 2=91 (LC 14)	
	Max Uplift 2=-27 (LC 14), 4=-52 (LC 14)	
	Max Grav 2=361 (LC 21), 4=152 (LC 21), 5=70 (LC 7)	
FORCES (lb) - Maximum Compression/Maximum Tension		LOAD CASE(S) Standard
TOP CHORD	1-2=0/52, 2-4=-172/68	
BOT CHORD	2-5=-133/110	

- ## NOTES
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 3) Unbalanced snow loads have been considered for this design.
 - 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



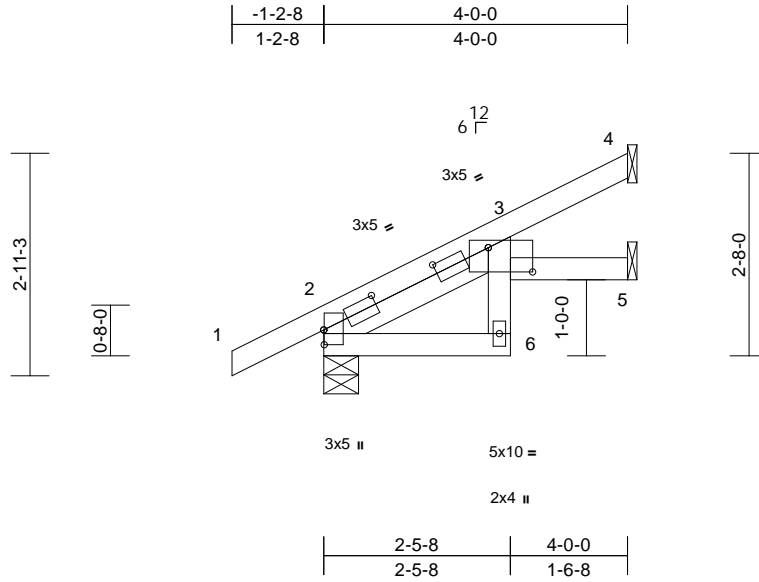
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	F2	Jack-Open	5	1	173744837
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:18

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Scale = 1:30.3

Plate Offsets (X, Y): [2:0-2-5,0-0-1], [2:1-8-0,0-1-8], [3:1-7-14,0-1-8], [3:0-7-0,0-3-13]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	-0.01	6	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.02	6	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.02	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
Weight: 20 lb											FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
SLIDER Left 2x4 SP No.2 -- 2-5-5

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 2=0-5-8, 4= Mechanical, 5= Mechanical
Max Horiz 2=91 (LC 14)
Max Uplift 2=-27 (LC 14), 4=-37 (LC 14), 5=-10 (LC 14)
Max Grav 2=361 (LC 21), 4=126 (LC 21), 5=84 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension

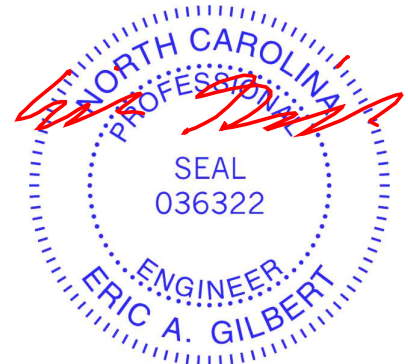
TOP CHORD 1-2=0/52, 2-3=-89/21, 3-4=-43/49
BOT CHORD 2-6=-65/76, 3-6=-13/52, 3-5=-24/16

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearings are assumed to be: , Joint 2 User Defined .
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 4 and 10 lb uplift at joint 5.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



May 28,2025

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Edenton, NC 27932

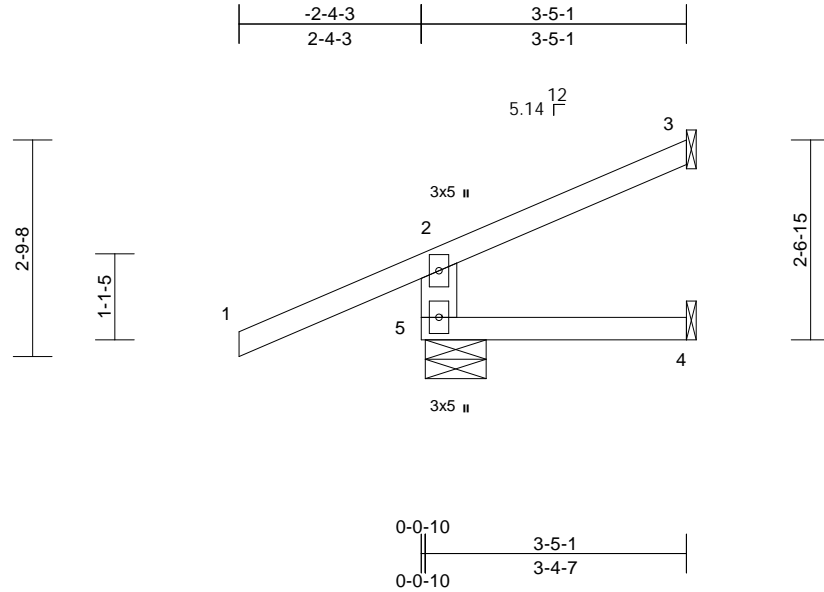
Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	F3	Jack-Open	1	1	I73744838
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Mon May 26 19:12:18

Page: 1

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Scale = 1:29.7

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.26	Vert(CT)	0.01	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.02	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 16 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x6 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-5-1 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	3= Mechanical, 4= Mechanical,
	5=0-9-7
Max Horiz	5=72 (LC 14)
Max Uplift	3=-38 (LC 14), 4=-2 (LC 21), 5=-70 (LC 10)
Max Grav	3=79 (LC 21), 4=49 (LC 7), 5=515 (LC 21)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-5=-451/310, 1-2=0/94, 2-3=-83/29
BOT CHORD	4-5=0/0

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner (3) -2-4-3 to 1-10-12, Exterior(2R) 1-10-12 to 3-4-5 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Bearings are assumed to be: , Joint 5 User Defined .
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 5, 38 lb uplift at joint 3 and 2 lb uplift at joint 4.

LOAD CASE(S) Standard



May 28,2025

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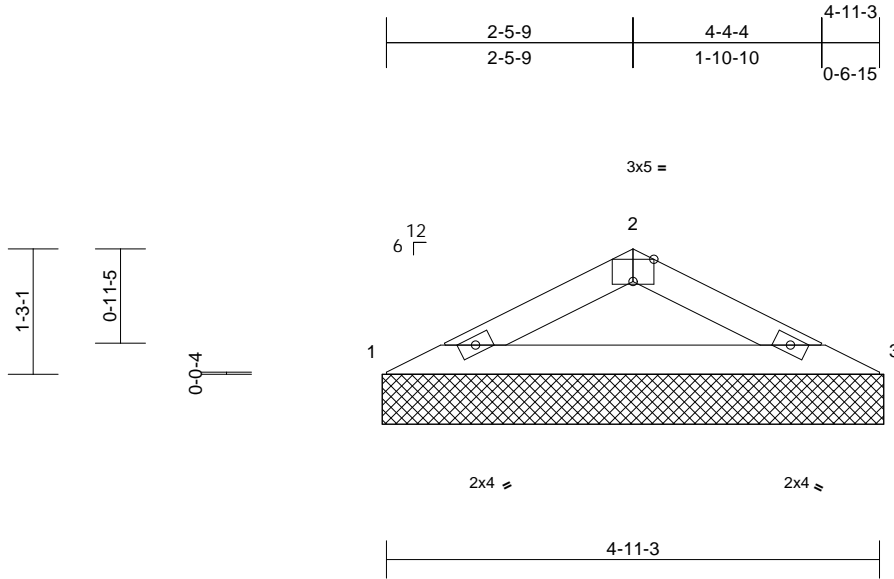
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	46 Serenity-Roof-Springhill 3277 B TMB GRH
24120148-01	V1	Valley	1	1	I73744839
Job Reference (optional)					

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Nov 16 2023 Print: 8.730 S Nov 16 2023 MiTek Industries, Inc. Tue May 27 16:16:31
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Page: 1



Scale = 1:23

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.17	Vert(TL)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.01	3	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										
Weight: 14 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=194/5-0-3, 3=194/5-0-3
Max Horiz 1=17 (LC 14)
Max Uplift 1=19 (LC 14), 3=19 (LC 15)
Max Grav 1=223 (LC 20), 3=223 (LC 21)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

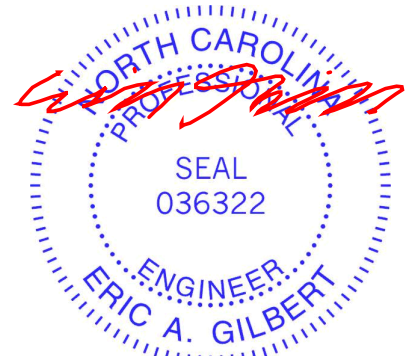
TOP CHORD 1-2=-402/178, 2-3=-251/127
BOT CHORD 1-3=-146/351

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.

- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 19 lb uplift at joint 3.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.

LOAD CASE(S) Standard



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Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

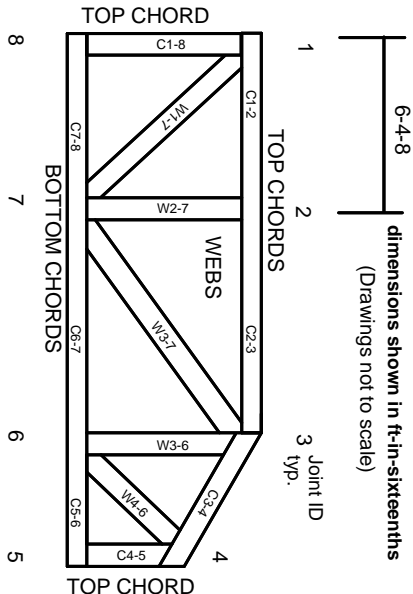


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:
ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.
Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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