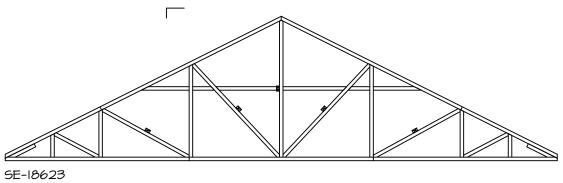


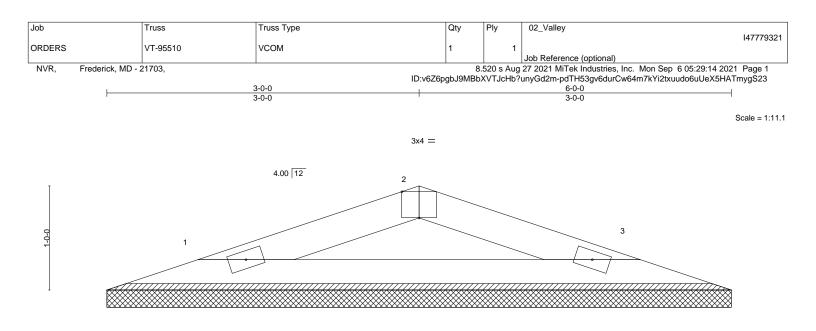


SHET NO. MOBL FALT NO. MOBL FALT NO. MOBL VERSION OI EDEN CAY VERSION OI ELEASE NO RELEASE NO. MOBL RAWIN BITLE ELEASE NO DRAWIN BITLE DRAWIN BITLE DRAWIN BITLE DRAWIN BITLE							
TRUSS BRACING DETAILS DRAWN BY ARS DRAWN BY ARS DRAW Lot DRAW-Lot DRAW-Lot DRAWN BY ARS DATE: DATE: DRAWN BY ARS N/R, Inc. Expedded in any form or manner wholes to be assigned to any influe orty, without assigned to any influe orty, without assigned to any first obtaining the expressed written consent of N/R, Inc. Erederick, MD 21703 ZT ARTESA COURT APT. SIREE T ADDRESS CITY CITY STREET ADDRESS APT.	SHEET NO.	MODEL EDEN CAY DRAWING TITLE	SET NO. EDCOO VERSION OI RELIFASE NO		DIV-COMM-LOT-UNIT	1010-	
OPTION DESCRIPTION STREET ADDRESS OPTION DESCRIPTION 5285 Westview Drive, Suite 100 first obtaining the expressed witten Prederick, MD 21703 7 ARTESA COURT CITY 27 ARTESA COURT Proderick, MD 21703 CITY	ນ 4	TRUSS BRACING DETAILS	DRAWN BY ARS		COMM-LOT KIPLING VILLAGE - OIOT		
CITY STATE ZIP FUQUAY VARINA NC NC		OPTION DESCRIPTION	OPTION	NVK, Inc. 5285 Westview Drive, Suite 100 Frederick, MD 21703	STREET ADDRESS 27 ARTESA COURT	APT. NO.	
	20				UAY VARINA	ZIP	



TRUSS BRACING NOTES:

- IF TRUSS DOES NOT APPEAR ON THIS TRUSS BRACING SHEET, NO ADDITIONAL LATERAL BRACING IS REQUIRED.
 2X4 SPF#2 LATERAL BRACES SHALL BE NAILED TO MINIMUM (3) TRUSS MEMBERS WITH MINIMUM (2) IOD
- NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING.
- 3. WEB "T" BRACE, DETAIL **3/RF-IC**, IS REQUIRED WHERE LATERAL BRACING IS NOT CONTINUOUS ACROSS THREE (3) OR MORE TRUSSES AND MAY BE USED IN LIEU OF 2X4 LATERAL BRACING.
- 4. DIAGONAL BRACING REQUIRED WHEN LATERAL BRACING IS REQUIRED (4/RF-IC)
- 5. STUDDED GABLE BRACING DETAIL (I/RF-IC) TO BE UTILIZED FOR TRUSSES 6'-9" IN HEIGHT OR GREATER.
- 6. PARTIALLY SHEATHED GABLES, SEE (5/RF-Ic) FOR "L"
- BRACING WHEN REQUIRED. 1. LATERAL BRACING CAN BE APPLIED TO EITHER SIDE
- OF THE WEB MEMBER IDENTIFIED IN THE DRAWING. 3. SHEATHING (OSB OR GYPSUM) REPLACES LATERAL
- AND DIAGONAL TRUSS BRACING.



2x4 ⋍

2x4 🗢

Plate Offsets (X,Y) [2:	0-2-0,Edge]			
LOADING (psf) TCLL 30.0 (Roof Snow=30.0) 10.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.22 BC 0.38 WB 0.00	DEFL. in (loc) I/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a	PLATES GRIP MT20 197/144
BCLL 0.0 BCDL 10.0	Code IBC2021/TPI2014	Matrix-P		Weight: 16 lb FT = 5%

BOT CHORD 2x4 SP No.3 or 2x4 SPF Stud BOT CHORD 2x4 SP No.3 or 2x4 SPF Stud
 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=6-0-0, 3=6-0-0 Max Horz 1=-16(LC 13) Max Uplift 1=-51(LC 8), 3=-51(LC 9) Max Grav 1=224(LC 18), 3=224(LC 19)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-287/114, 2-3=-287/114

 BOT CHORD
 1-3=-93/252

NOTES- (7-8)

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

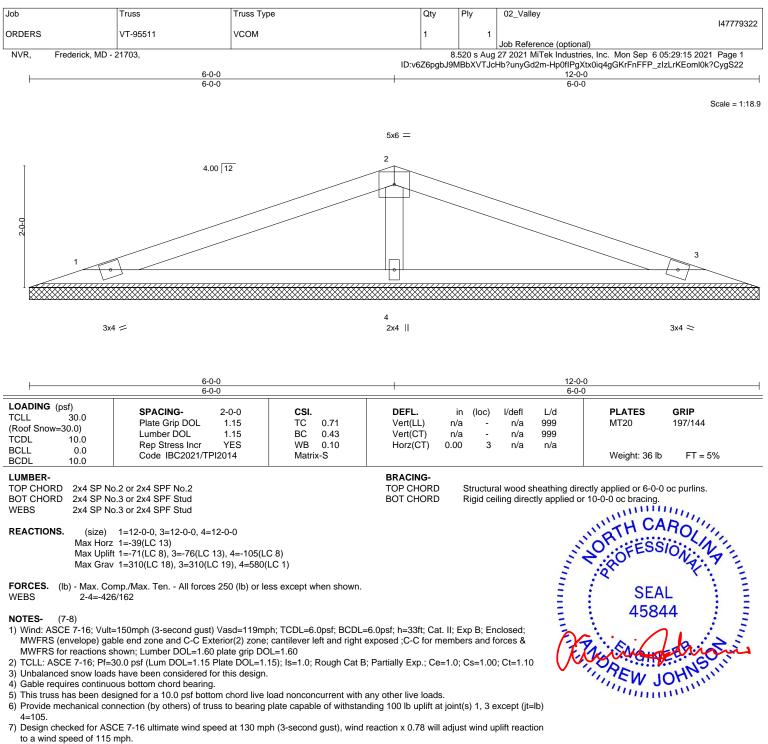
7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.

8) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.



September 16,2021

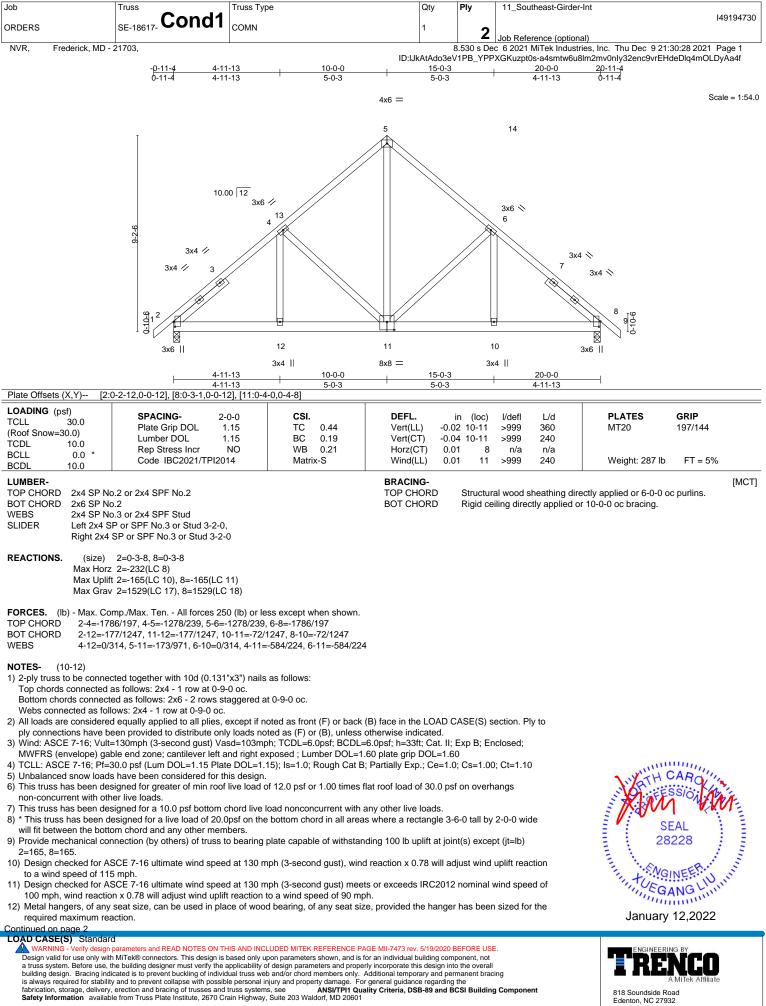




8) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

September 16,2021

A MiTek / 818 Soundside Road Edenton, NC 27932



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	11_Southeast-Girder-Int
ORDERS	SE-18617- Cond	COMN	1	2	I49194730 Job Reference (optional)
NVR,	Frederick, MD - 21703,			8.530 s De	c 6 2021 MiTek Industries, Inc. Thu Dec 9 21:30:28 2021 Page 2

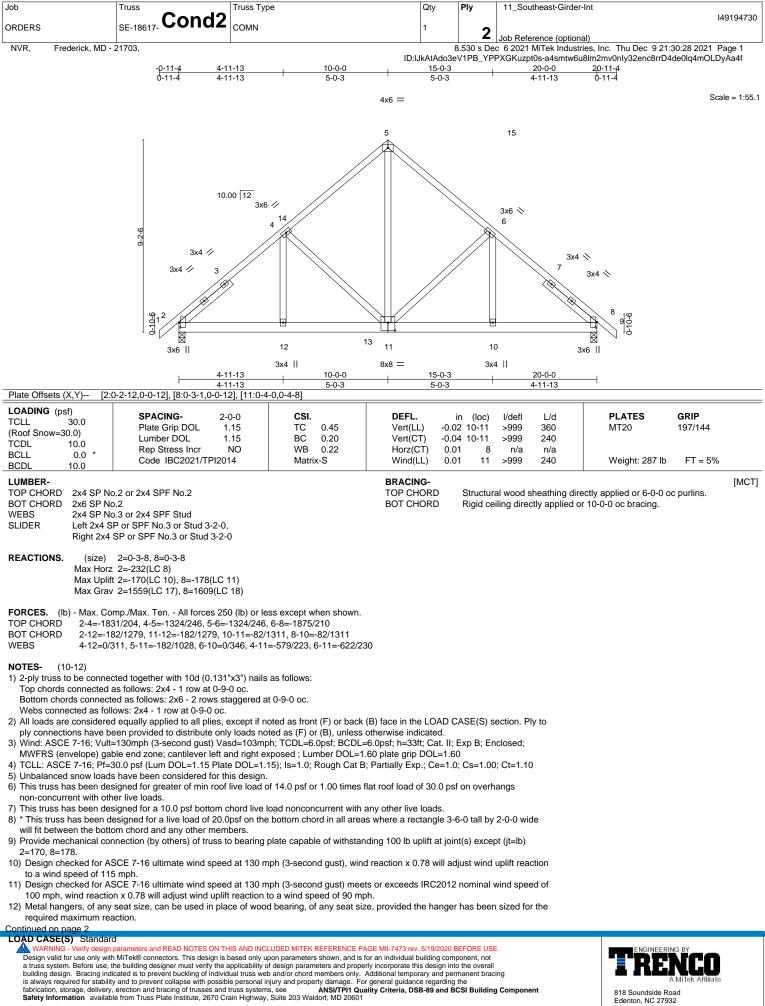
8.530 s Dec 6 2021 MiTek Industries, Inc. Thu Dec 9 21:30:28 2021 Page 2 ID:IJkAtAdo3eV1PB_YPPXGKuzpt0s-a4smtw6u8lm2mv0nly32enc9vrEHdeDlq4mOLDyAa4f

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 2-8=-60(B=-40), 1-5=-80, 5-9=-80





Job		Truss	Truss Type	Qty	Ply	11_Southeast-Girder-Int
		SE-18617- Cond2				I49194730
ORDERS		SE-18617-	COMN	1	2	Job Reference (optional)
NVR,	Frederick, MD - 2	21703,		8	3.530 s De	c 6 2021 MiTek Industries, Inc. Thu Dec 9 21:30:29 2021 Page 2

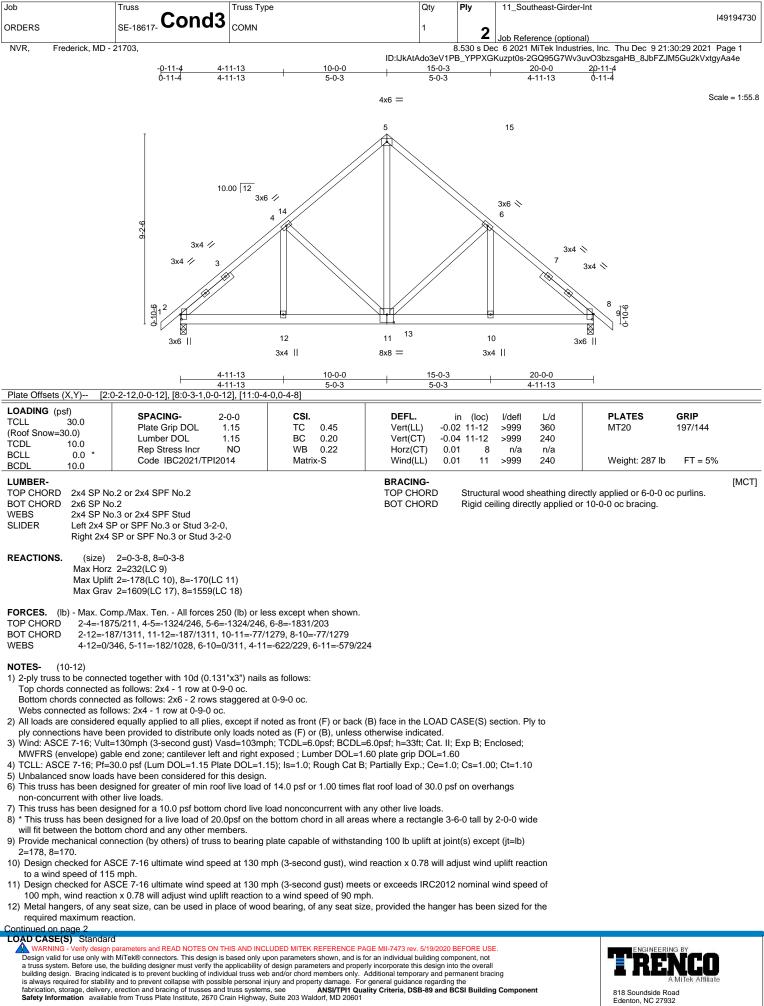
8.530 s Dec 6 2021 MiTek Industries, Inc. Thu Dec 9 21:30:29 2021 Page 2 ID:IJkAtAdo3eV1PB_YPPXGKuzpt0s-2GQ95G7Wv3uvO3bzsgaHB_8JbFZJM5Gu2kVxtgyAa4e

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 2-13=-60(B=-40), 8-13=-70(B=-50), 1-5=-80, 5-9=-80





Job		Truss	Truss Type	Qty	Ply	11_Southeast-Girder-Int
		SE-18617- Cond3				I49194730
ORDERS		SE-18617-	COMN	1	2	Job Reference (optional)
NVR,	Frederick, MD - 2	1703,		8		c 6 2021 MiTek Industries, Inc. Thu Dec 9 21:30:29 2021 Page 2

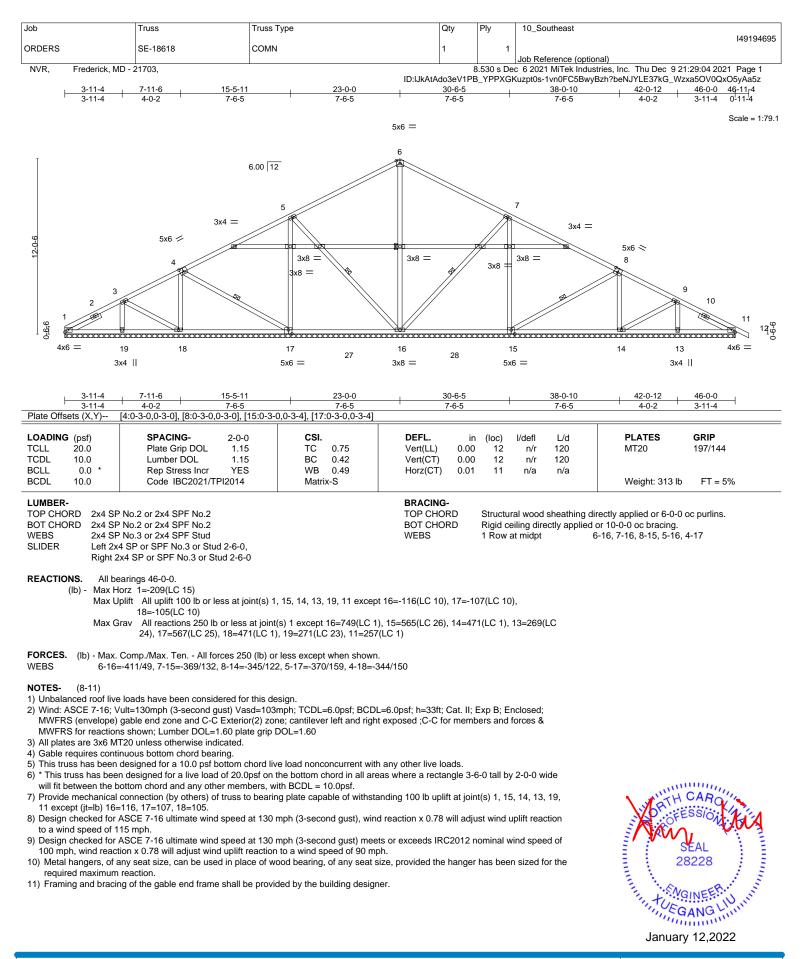
8.530 s Dec 6 2021 MiTek Industries, Inc. Thu Dec 9 21:30:29 2021 Page 2 ID:IJkAtAdo3eV1PB_YPPXGKuzpt0s-2GQ95G7Wv3uvO3bzsgaHB_8JbFZJM5Gu2kVxtgyAa4e

LOAD CASE(S) Standard

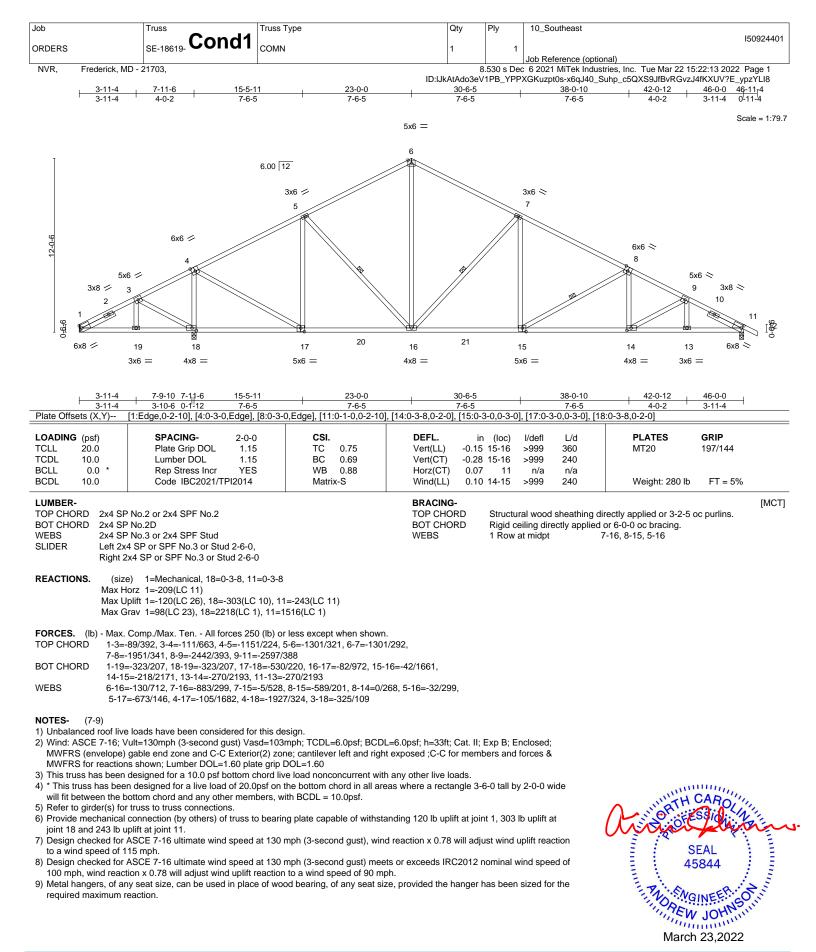
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 2-13=-70(B=-50), 8-13=-60(B=-40), 1-5=-80, 5-9=-80





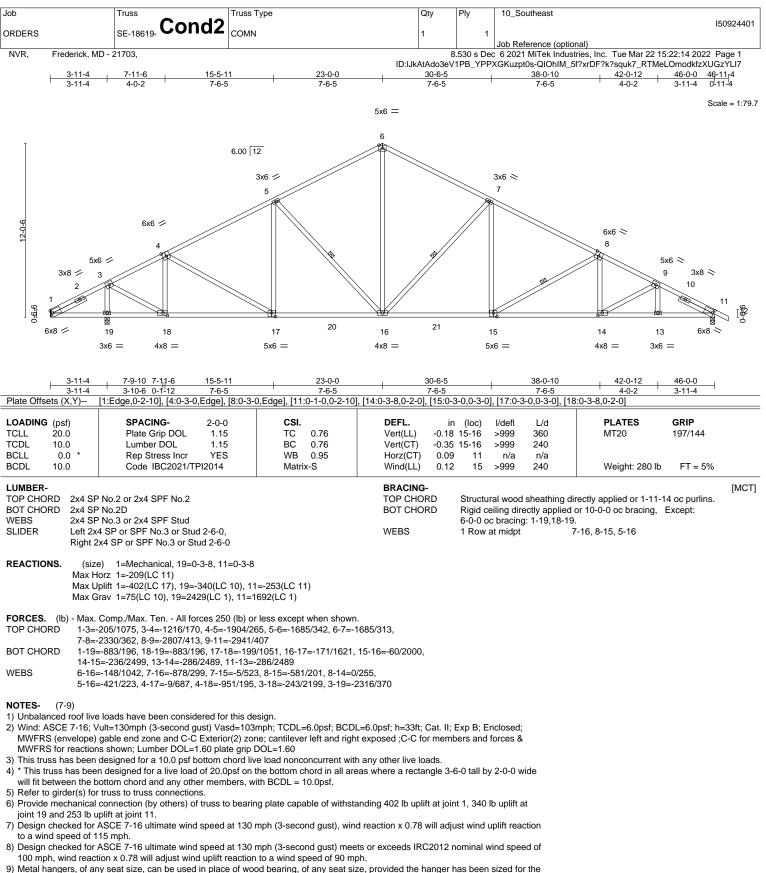
818 Soundside Road Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

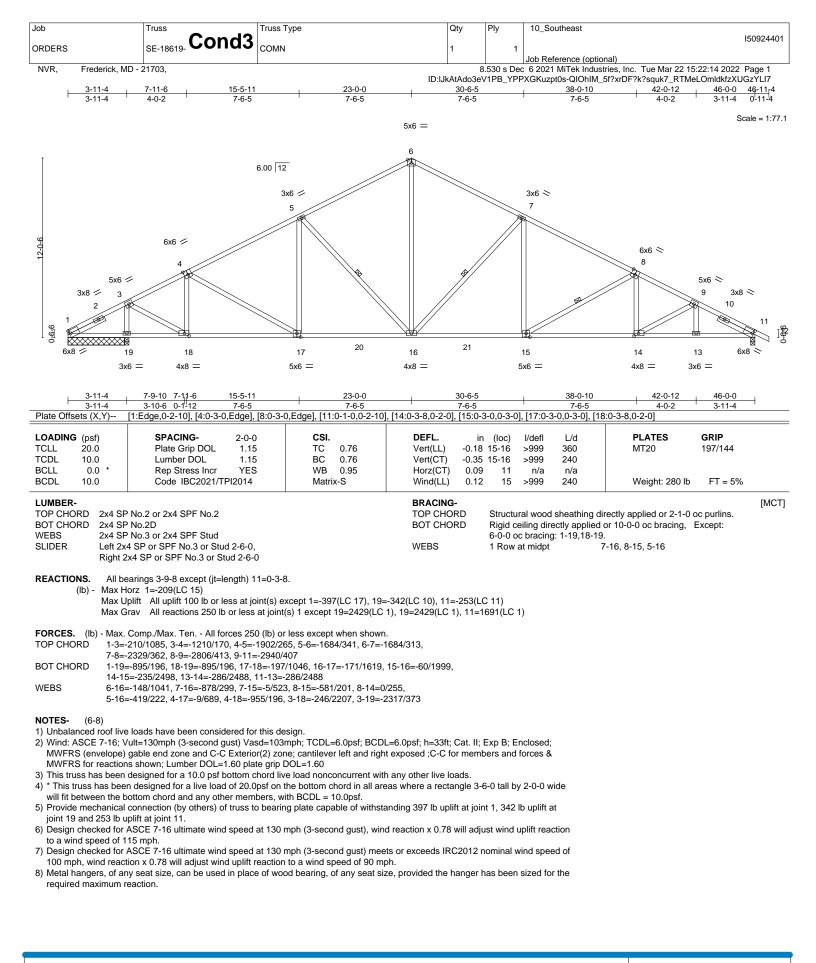
ERENCO A Mi Tek Affiliate 818 Soundside Road

Edenton, NC 27932

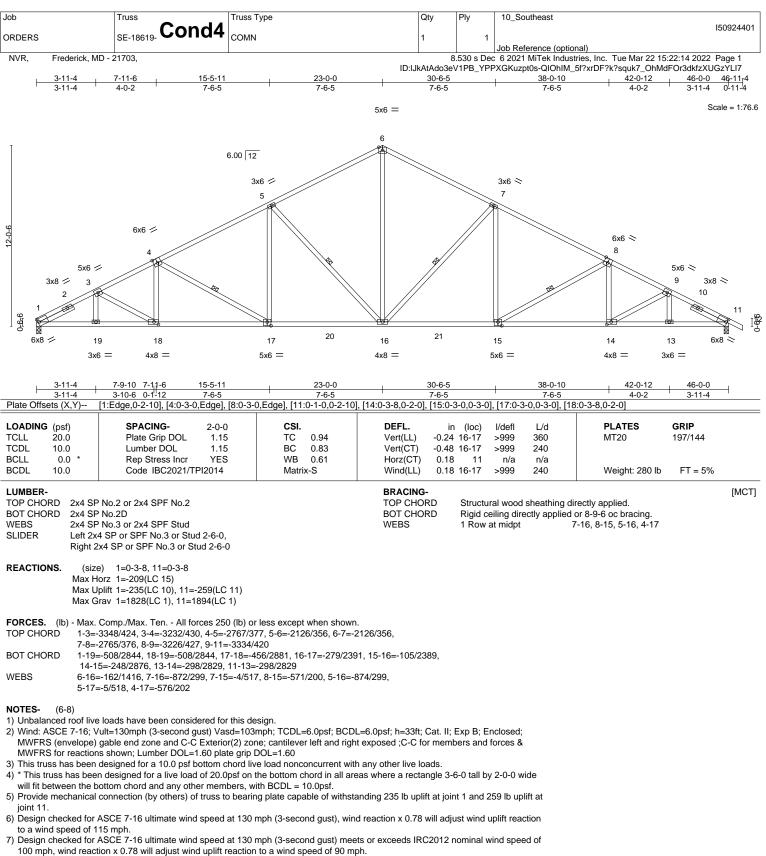


9) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



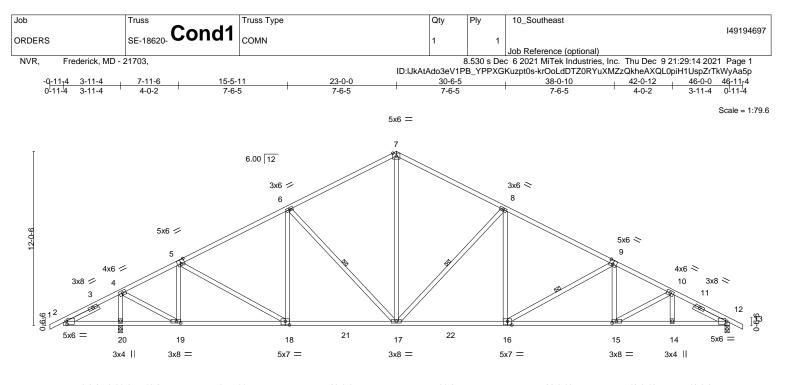






 Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.





3-9-8	3-1 ₁ 1-4 7-11-6	15-5-11	23-0-0	30-6-5	38-0-10	42-0-12	46-0-0	L
3-9-8	0-1-12 4-0-2	7-6-5	7-6-5	7-6-5	7-6-5	4-0-2	3-11-4	1
Plate Offsets (X,Y)	[2:0-0-15,0-2-8],	[5:0-3-0,0-3-4], [9:0-3-0,0-3-	-4], [12:0-0-15,0-2-8], [15:0-	3-8,0-1-8], [16:0-3-8,0-3-0],	[18:0-3-8,0-3-0], [19:0-3-8,0	0-1-8]		

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IBC2021/TPI2014	CSI. TC 0.78 BC 0.85 WB 0.81 Matrix-S	Vert(LL) -0.20		PLATES GRIP MT20 197/14 Weight: 282 lb FT =	
BOT CHORD 2x4 SF WEBS 2x4 SF SLIDER Left 2x	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3 or 2x4 SPF Stud P No.3 or SPF No.3 or Stud 2-6-0, 2x4 SP or SPF No.3 or Stud 2-6-0		BRACING- TOP CHORD BOT CHORD WEBS	6-0-0 oc bracing: 2-20,19-20	or 10-0-0 oc bracing, Except:	[MCT]

REACTIONS. (size) 20=0-3-8, 12=0-3-8 Max Horz 20=-205(LC 15) Max Uplift 20=-282(LC 10), 12=-251(LC 11) Max Grav 20=2068(LC 1), 12=1722(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-4=-105/443, 4-5=-1516/192, 5-6=-2032/276, 6-7=-1750/337, 7-8=-1750/309,

8-9=-2394/357, 9-10=-2869/409, 10-12=-3000/403

BOT CHORD 2-20=-337/135, 19-20=-337/235, 18-19=-241/1341, 17-18=-189/1735, 16-17=-59/2058, 15-16=-232/2554, 14-15=-283/2539, 12-14=-283/2539 WEBS 7-17=-144/1104, 8-17=-877/299, 8-16=-5/522, 9-16=-578/201, 9-15=0/252,

6-17=-465/235, 5-18=-22/498, 5-19=-794/171, 4-19=-194/1885, 4-20=-1971/314

NOTES-(6-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 20 and 251 lb uplift at joint 12.

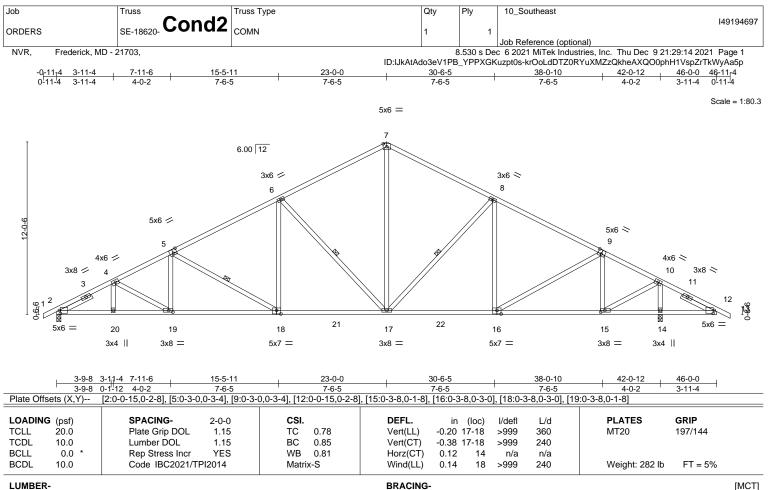
6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.

7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



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TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied.

1 Row at midpt

Rigid ceiling directly applied or 6-0-0 oc bracing.

8-17, 6-17, 5-18

- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 or 2x4 SPF Stud SLIDER Left 2x4 SP or SPF No.3 or Stud 2-6-0, Right 2x4 SP or SPF No.3 or Stud 2-6-0
- REACTIONS. (size) 2=0-3-8, 14=0-3-8 Max Horz 2=-205(LC 15) Max Uplift 2=-251(LC 10), 14=-282(LC 11) Max Grav 2=1722(LC 1), 14=2068(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2999/401, 4-5=-2870/409, 5-6=-2395/357, 6-7=-1750/309, 7-8=-1750/337,

8-9=-2031/276, 9-10=-1515/192, 10-12=-104/443

- BOT CHORD 2-20=-486/2539, 19-20=-486/2539, 18-19=-437/2557, 17-18=-262/2058, 16-17=-71/1735, 15-16=-53/1323, 14-15=-337/134, 12-14=-337/134
- WEBS 7-17=-145/1104, 8-17=-465/235, 9-16=-21/494, 9-15=-793/170, 10-15=-192/1883, 10-14=-1971/314, 6-17=-877/299, 6-18=-4/522, 5-18=-580/201, 5-19=0/252

NOTES- (6-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

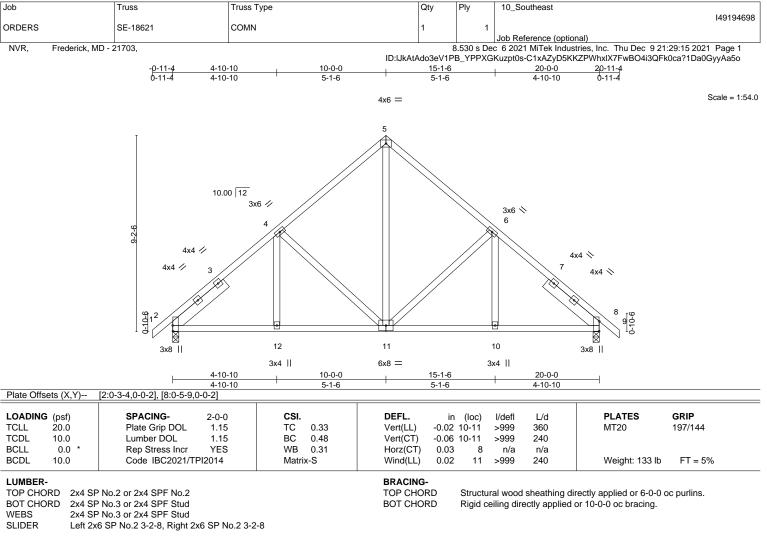
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 282 lb uplift at joint 14.

6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.

7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.





REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-232(LC 8) Max Uplift 2=-102(LC 10), 8=-102(LC 11) Max Grav 2=856(LC 1), 8=856(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-983/123, 4-5=-702/192, 5-6=-702/192, 6-8=-982/122
- BOT CHORD 2-12=-125/736, 11-12=-125/736, 10-11=-19/664, 8-10=-19/664
- WEBS 5-11=-112/482, 4-11=-323/201, 6-11=-324/202

NOTES-(6-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 2 and 102 lb uplift at joint 8.

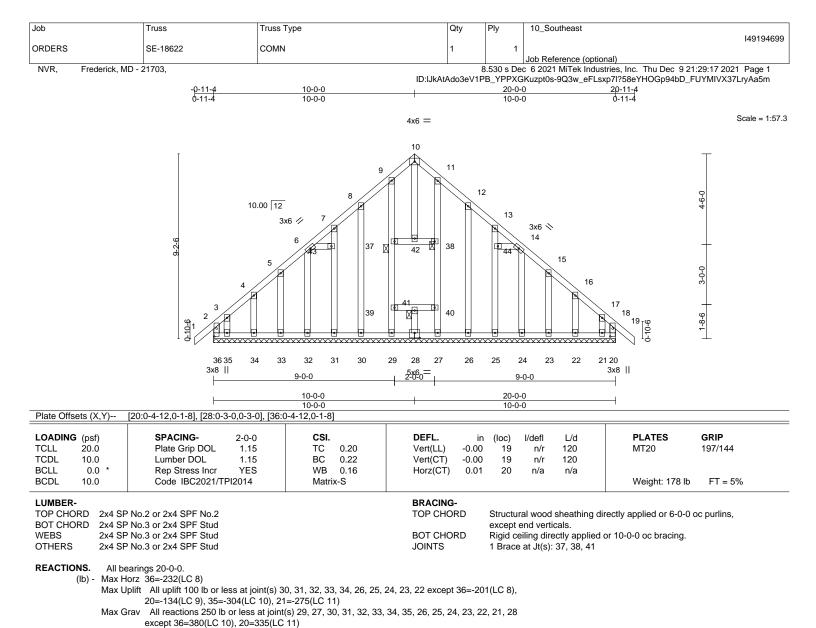
6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.

7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.







FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-309/194, 17-18=-274/142

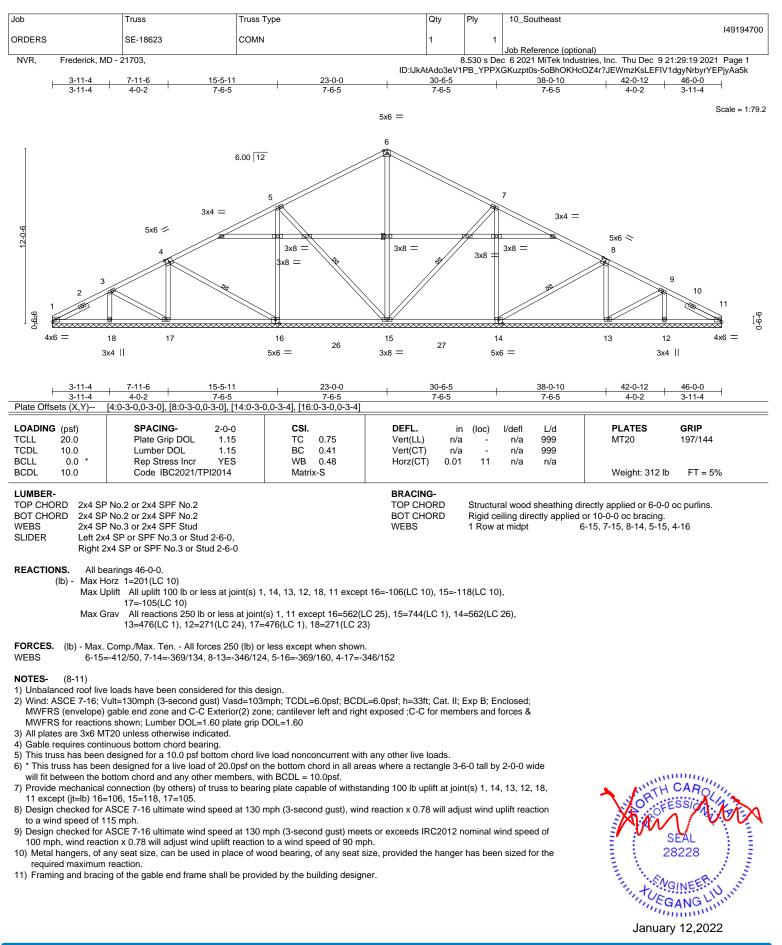
NOTES-(11-13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 3x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 1-4-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 32, 33, 34, 26, 25, 24, 23, 22 except (jt=lb) 36=201, 20=134, 35=304, 21=275.
- 11) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 12) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 13) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



January 12,2022

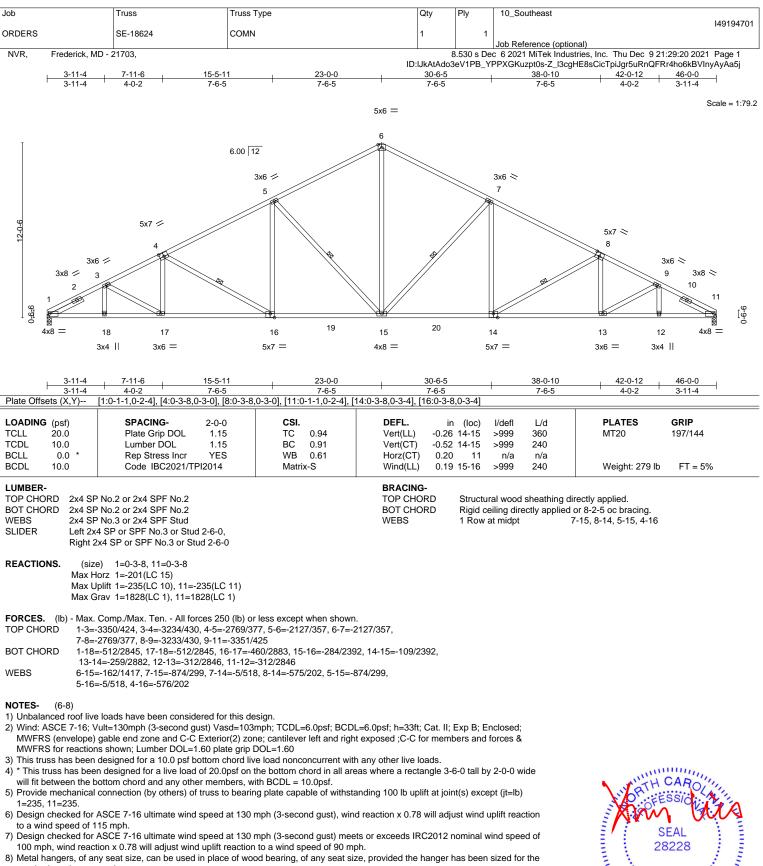




818 Soundside Road

Edenton, NC 27932

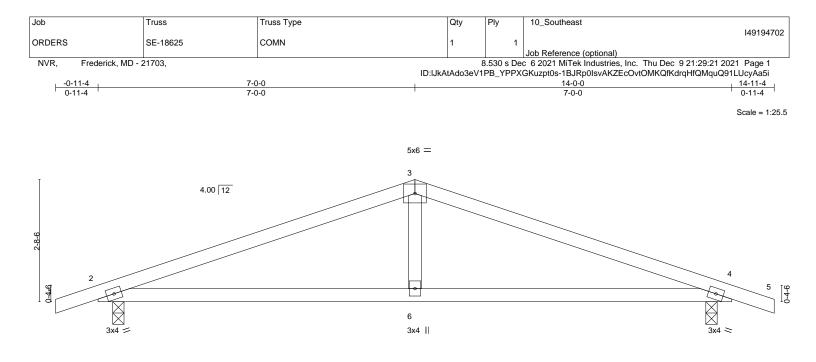
 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a duss system planteets and property incorporate dust using in the version of the system planteets and property incorporate dust using indicated is to prevent buckling of individual itruss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual itruss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual itruss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual itruss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



required maximum reaction.







0 <u>-</u> 3-14 0-3-14	7-0-0 6-8-2						13-8-2 6-8-2		<u>14-0-</u> 0 0-3-14
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.76	Vert(LL)	-0.06	2-6	>999	360	MT20	197/144
TCDL 10.0	Lumber DOL 1.15	BC 0.57	Vert(CT)	-0.14	2-6	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.13	Horz(CT)	0.02	4	n/a	n/a		
BCDL 10.0	Code IBC2021/TPI2014	Matrix-S	Wind(LL)	0.05	2-6	>999	240	Weight: 49 lb	FT = 5%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 or 2x4 SPF Stud

REACTIONS. (size) 2=0-3-0, 4=0-3-0 Max Horz 2=-45(LC 11) Max Uplift 2=-129(LC 6), 4=-129(LC 7) Max Grav 2=614(LC 1), 4=614(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1018/122, 3-4=-1018/121

BOT CHORD 2-6=-65/899, 4-6=-65/899 WEBS 3-6=0/332

NOTES-(6-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=129, 4=129,
- 6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.

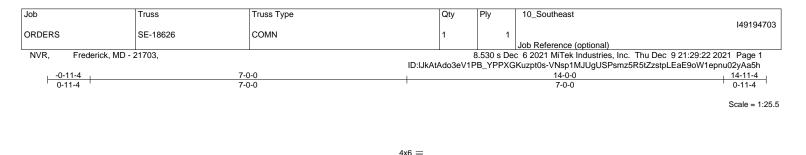


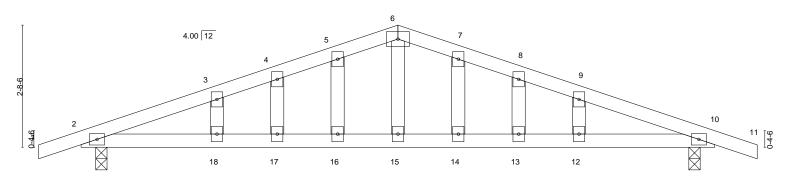
Structural wood sheathing directly applied or 3-5-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

818 Soundside Road

Edenton, NC 27932





0 <u>-3-14</u> 0-3-14	7-0-0 6-8-2			13-8-2 6-8-2		14-0-0 0-3-14
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2021/TPI2014	CSI. TC 0.65 BC 0.74 WB 0.17 Matrix-S	Vert(LL) -0.1 Vert(CT) -0.1 Horz(CT) 0.0	in (loc) l/defl L/d 1 12-13 >999 360 8 12-13 >916 240 12 10 n/a n/a 0 17-18 >999 240		RIP 7/144 FT = 5%
BOT CHORD 2x4 SF WEBS 2x4 SF	P No.3 or 2x4 SPF Stud P No.2 or 2x4 SPF No.2 P No.3 or 2x4 SPF Stud P No.3 or 2x4 SPF Stud		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin Rigid ceiling directly appl	g directly applied or 4-5-11 oc ied or 10-0-0 oc bracing.	c purlins.
Max H Max U	e) 2=0-3-0, 10=0-3-0 lorz 2=-45(LC 15) lplift 2=-129(LC 6), 10=-129(LC 7) irav 2=614(LC 1), 10=614(LC 1)					
TOP CHORD 2-3=- 7-8=- BOT CHORD 2-18: 13-14	Comp./Max. Ten All forces 250 (lb) of 1034/115, 3-4=-985/132, 4-5=-967/141, 967/140, 8-9=-985/132, 9-10=-1034/11 =-81/928, 17-18=-81/928, 16-17=-81/924 4=-81/928, 12-13=-81/928, 10-12=-81/92 =-46/390	5-6=-965/155, 6-7=-965/ 4 3, 15-16=-81/928, 14-15=	/155,			
 Wind: ASCE 7-16; MWFRS (envelope) MWFRS for reaction Truss designed for v Gable End Details a All plates are 3x4 M Gable studs spaced This truss has been * This truss has beee will fit between the b Provide mechanical 2=129, 10=129. Design checked for to a wind speed of 1 	designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on pottom chord and any other members. connection (by others) of truss to bearin ASCE 7-16 ultimate wind speed at 130	mph; TCDL=6.0psf; BCD he; cantilever left and righ DOL=1.60 For studs exposed to wi g designer as per ANSI/T re load nonconcurrent with the bottom chord in all are ng plate capable of withsta mph (3-second gust), win	It exposed ;C-C for mer nd (normal to the face), Pl 1. h any other live loads. eas where a rectangle 3 anding 100 lb uplift at jo d reaction x 0.78 will ac	nbers and forces & see Standard Industry 8-6-0 tall by 2-0-0 wide bint(s) except (jt=lb) djust wind uplift reaction	SEA 282	AROUND AL

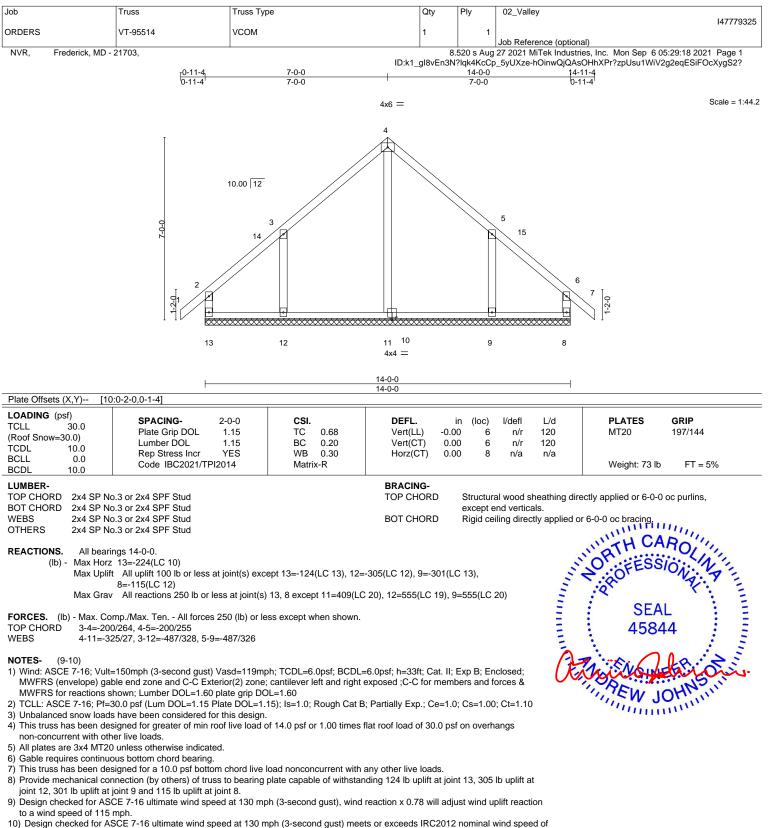
10) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



January 12,2022

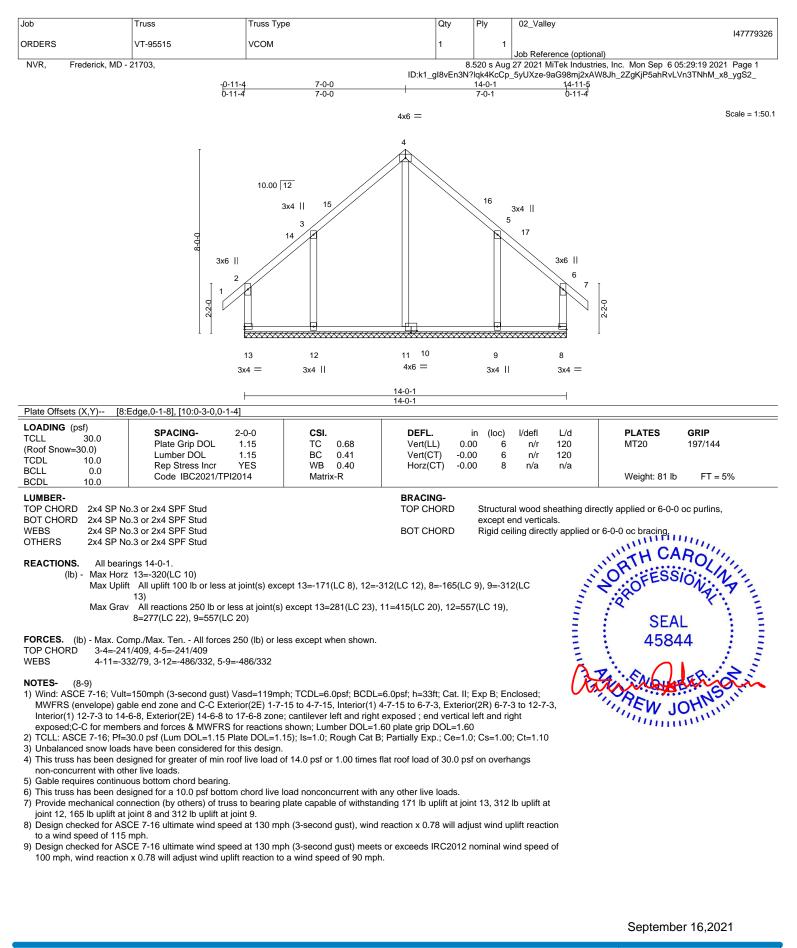




 Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

September 16,2021

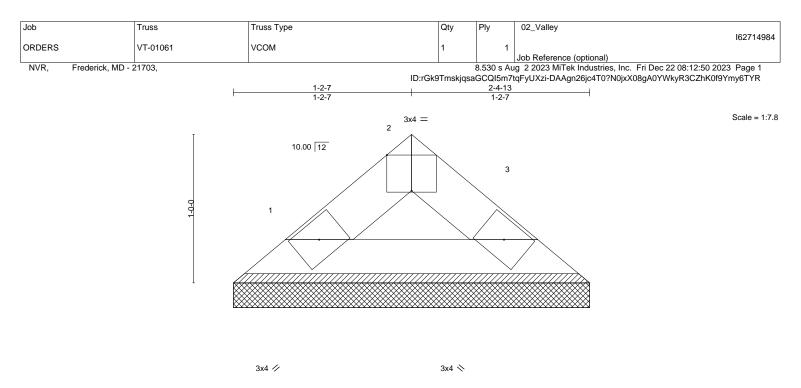
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Job	Truss	Truss Type	Qty	Ply	02_Valley]
ORDERS	VT-95516	VCOM	1	1	,	147779327
NVR, Frederick, MD -					Job Reference (optional	l) es, Inc. Mon Sep 6 05:29:21 2021 Page 1
NVK, Frederick, MD -		11-4 7-0-0 11-4 7-0-0	ID:k1_gl8vEn3i			is, inc. Mon Sep 6 05:29:21 2021 Page 1 nrY?8Qh5MBUWf14i_cFxgg8gT2CsygS1y Scale = 1:54.5
	0-07 8-8-0 1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1			⁹ 34 10	35 11 3x8 II 12 13 000-5 5 15 14 5x6 II	1-20 3-00 4-6-0
Plate Offsets (X,Y) [14	:Edge,0-3-8], [19:0-2-0,0-1-4]	7-0-0 6-070₀_0	1 1	4-0-0 7-0690-0		
LOADING (psf) TCLL 30.0 (Roof Snow=30.0) TCDL TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip DOL 1 Lumber DOL 1	D-0 CSI. 15 TC 0.67 15 BC 0.57 ES WB 0.48 4 Matrix-S	DEFL. Vert(LL) 0.0 Vert(CT) -0.0 Horz(CT) -0.0	0 12	l/defl L/d n/r 120 n/r 120 n/a n/a	PLATES GRIP MT20 197/144 Weight: 136 lb FT = 5%
BOT CHORD 2x4 SP No WEBS 2x4 SP No OTHERS 2x4 SP No REACTIONS. All bearin (lb) - Max Horz Max Uplift	24=-412(LC 9), 17=-141(LC	int(s) 23, 16 except 25=-361(LC 8), 13), 15=-405(LC 8) at joint(s) 22, 23, 17, 16, 20 except		except e Rigid ce 1 Brace	end verticals. illing directly applied or at Jt(s): 26, 27	ctly applied or 6-0-0 oc purlins, 6-0-0 oc bracing, TH CAROLINI, CFESSION, 1, 4
FORCES. (Ib) - Max. Con TOP CHORD 3-4=-130 9-10=-1 WEBS 7-30=-28 NOTES- (9-10) 1) Wind: ASCE 7-16; Vult- MWFRS (envelope) gat Interior(1) 13-4-13 to 15 exposed;C-C for memb 2) TCLL: ASCE 7-16; Pf=3 3) Unbalanced snow loads 4) This truss has been des non-concurrent with oth 5) All plates are 3x4 MT20 6) Gable requires continud 7) This truss has been des 8) Provide mechanical corr (it=lb) 25=361, 22=141, 9) Design checked for ASC to a wind speed of 115 10) Design checked for ASC	18=265(LC 20), 14=404(LC mp./Max. Ten All forces 250 J/260, 4-5=-170/329, 5-6=-24 70/329, 10-11=-130/260, 2-21 39/137 =150mph (3-second gust) Vas ble end zone and C-C Exterio 5-4-1, Exterior(2E) 15-4-1 to 1 ers and forces & MWFRS for 30.0 psf (Lum DOL=1.15 Plate s have been considered for th signed for greater of min roof ler live loads. 0 unless otherwise indicated. Dus bottom chord bearing. signed for a 10.0 psf bottom con conection (by others) of truss to 14=354, 24=412, 17=141, 15 CE 7-16 ultimate wind speed mph.	22), 24=497(LC 10), 15=490(LC 1 2) (lb) or less except when shown. 1/454, 6-7=-222/405, 7-8=-222/405 5=-279/238, 12-14=-275/238 sd=119mph; TCDL=6.0psf; BCDL=1 (2E) 2-5-9 to 5-5-9, Interior(1) 5-5- 8-4-1 zone; cantilever left and right reactions shown; Lumber DOL=1.6 e DOL=1.15); Is=1.0; Rough Cat B; is design. live load of 14.0 psf or 1.00 times fl hord live load nonconcurrent with a b bearing plate capable of withstand	1) , 8-9=-241/454, 6.0psf; h=33ft; Cat. II; 9 to 7-4-13, Exterior(2 exposed ; end vertice 10 plate grip DOL=1.61 Partially Exp.; Ce=1.0 at roof load of 30.0 ps iny other live loads. ding 100 lb uplift at join eaction x 0.78 will adji s or exceeds IRC2012	Exp B; Er R) 7-4-13 Il left and); Cs=1.0(f on overh nt(s) 23, 1 ust wind u	nclosed; to 13-4-13, right); Ct=1.10 hangs 6 except plift reaction	SEAL 45844
						September 16,2021
		IS AND INCLUDED MITEK REFERENCE PA			E.	ENGINEERING BY





2 - 4 - 132-4-13 Plate Offsets (X,Y)--[2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES GRIP TCLL 30.0 Plate Grip DOL 1.15 тс 0.06 Vert(LL) 999 197/144 n/a n/a MT20 (Roof Snow=30.0) Lumber DOL 1.15 BC 0.05 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 0.00 3 Horz(CT) n/a n/a BCLL 0.0 Code IBC2021/TPI2014 FT = 5% Matrix-P Weight: 7 lb BCDL 10.0 LUMBER-BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 or 2x4 SPF Stud BOT CHORD 2x4 SP No.3 or 2x4 SPF Stud

REACTIONS. (size) 1=2-4-13, 3=2-4-13 Max Horz 1=23(LC 9) Max Uplift 1=-16(LC 12), 3=-16(LC 13) Max Grav 1=85(LC 18), 3=85(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

 Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ;C-C for members and forces &

MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

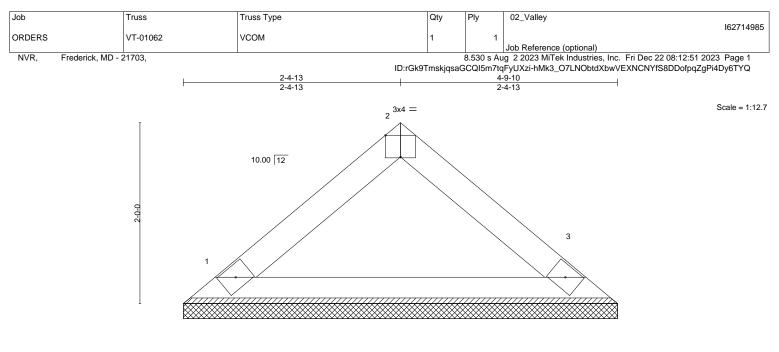


Structural wood sheathing directly applied or 2-4-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2023

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3x4 🥢

3x4 📎

			4-9-10 4-9-10	
Plate Offsets (X,Y) [2:	0-2-0,Edge]			
LOADING (psf) TCLL 30.0 (Roof Snow=30.0) TCDL TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IBC2021/TPI2014	CSI. TC 0.31 BC 0.33 WB 0.00 Matrix-P	DEFL. in (loc) I/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a	PLATES GRIP MT20 197/144 Weight: 15 lb FT = 5%
	o.3 or 2x4 SPF Stud o.3 or 2x4 SPF Stud 1=4-9-10. 3=4-9-10		BRACING- TOP CHORD Structural wood sheathing dir BOT CHORD Rigid ceiling directly applied c	ectly applied or 4-9-10 oc purlins. r 10-0-0 oc bracing.
Max Horz	$1 = 10^{-10}, 0 = 10^{-10}$ 1 = 57(LC 9) 1 = 40(LC 12), 2 = 40(LC 12)			

Max Uplift 1=-40(LC 12), 3=-40(LC 13) Max Grav 1=232(LC 18), 3=232(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (7)

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ;C-C for members and forces &
- MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



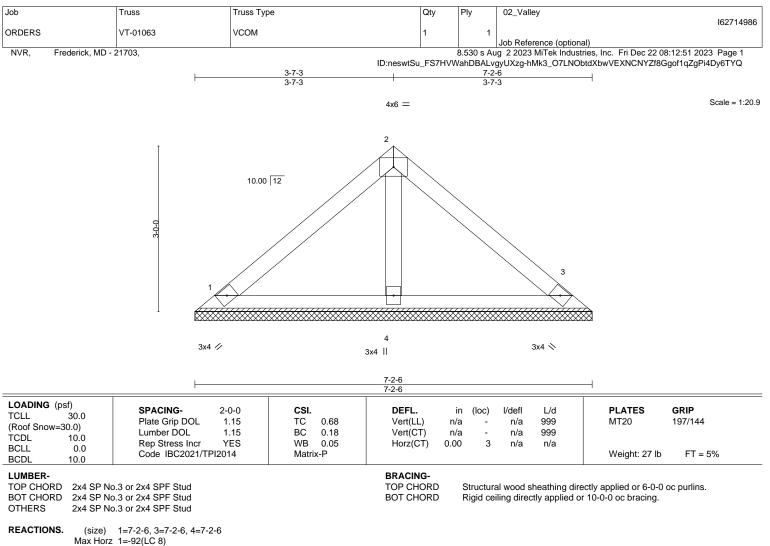
December 27,2023

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818 Soundside Road

Edenton, NC 27932



Max Uplift 1=-58(LC 13), 3=-69(LC 13), 4=-13(LC 12) Max Grav 1=257(LC 18), 3=257(LC 19), 4=271(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (7)

1) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pf=30.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



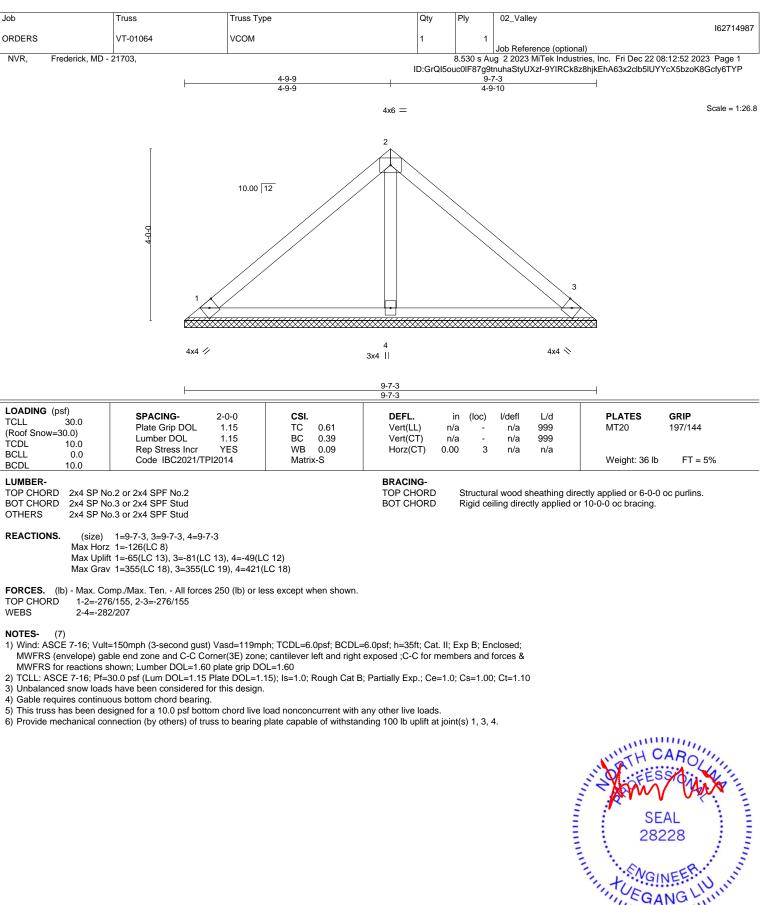
December 27,2023

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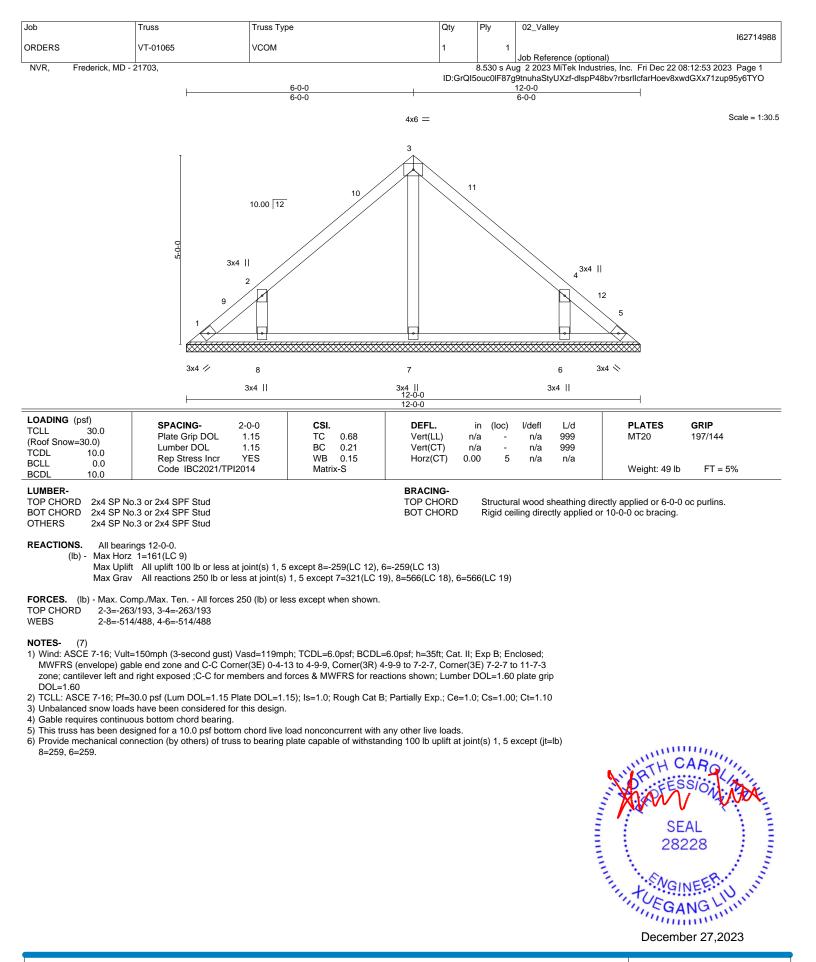
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Edenton, NC 27932



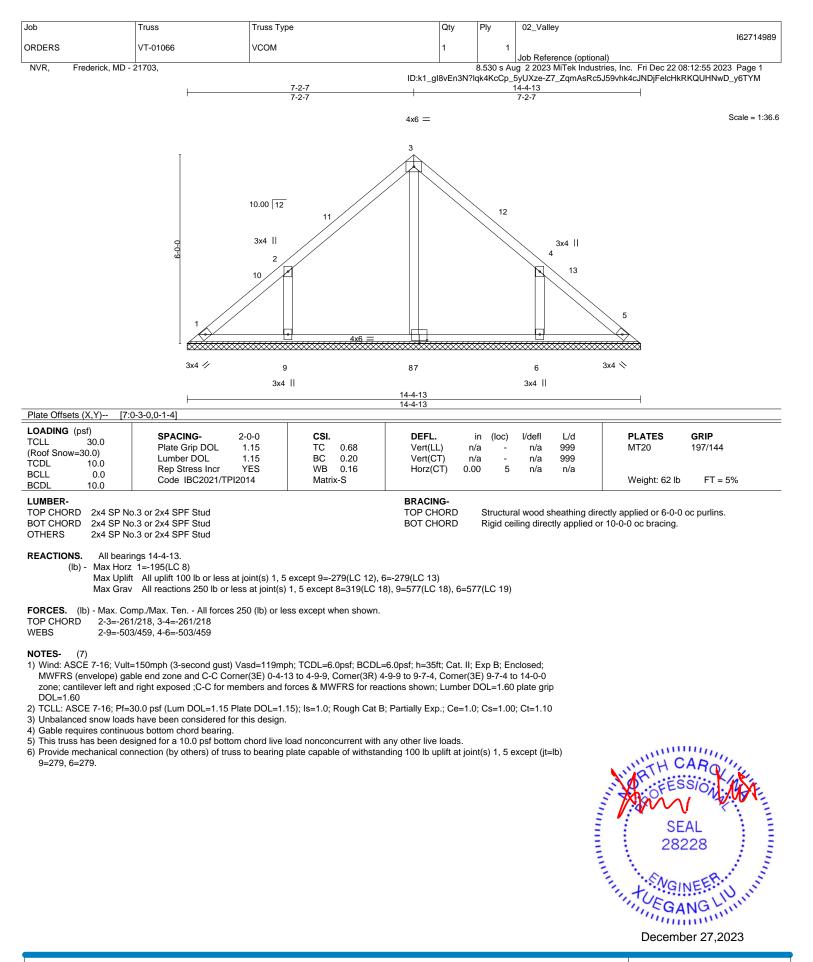


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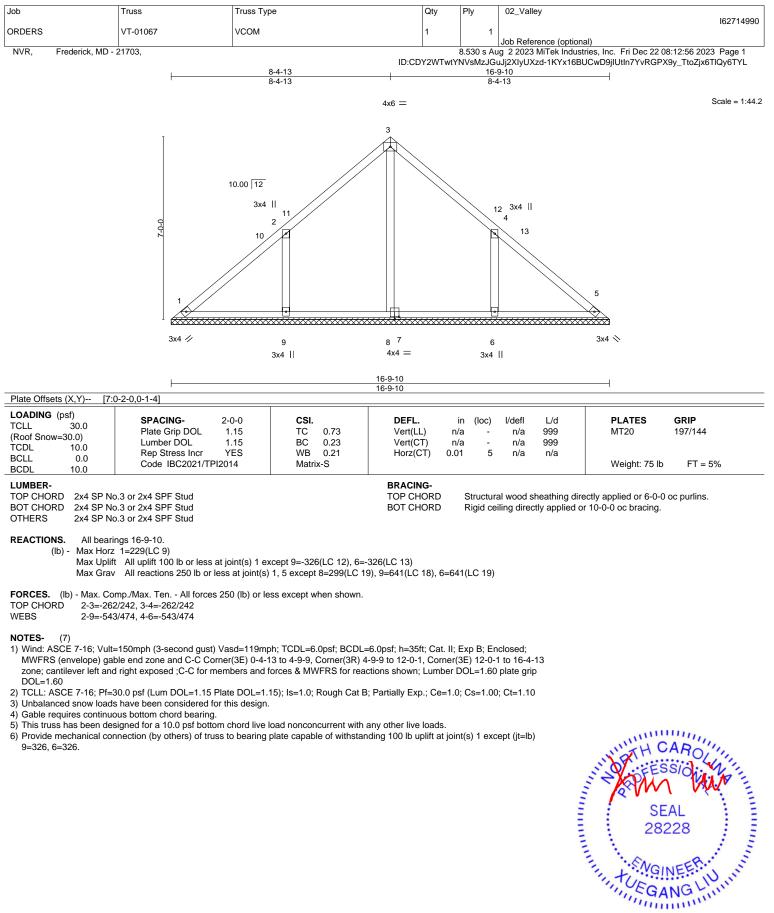
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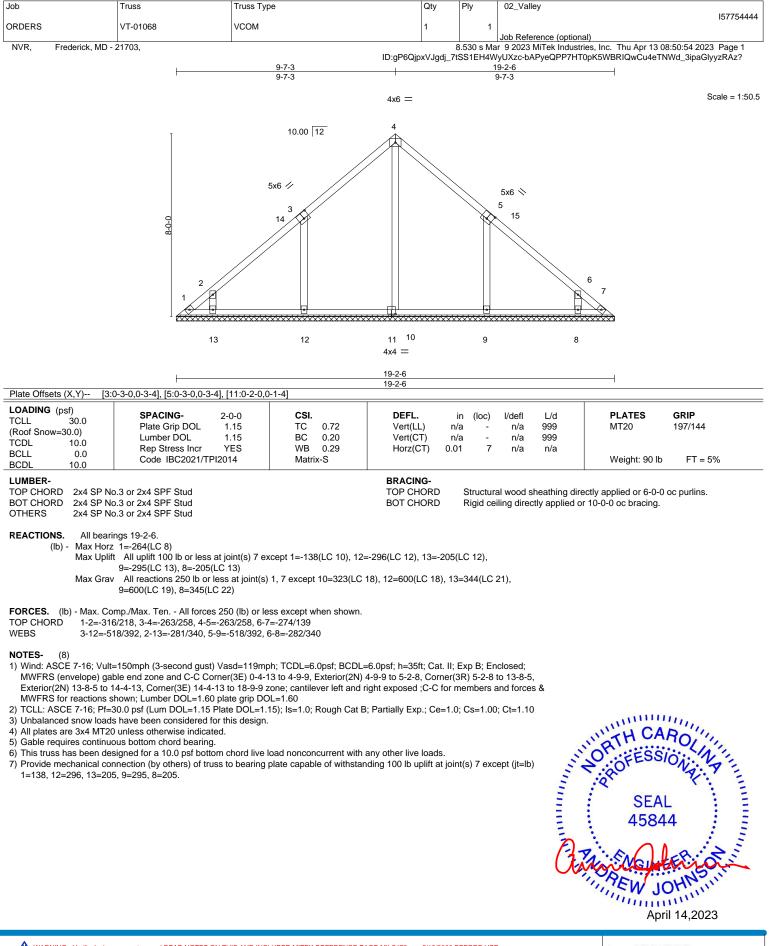
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December 27,2023

TRENCIO A MITOR AMITINA





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601