- 1. The CUSTOMER / END USER, hereafter referred to as the "FSS", obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The CUSTOMER provides at his expense all plans and specifications required to obtain a building permit. It is the CUSTOMER's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.

 2. The CUSTOMER is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the metal building system in order to insure that Building Supplier's plans comply with the applicable requirements of any governing building authorities and to obtain appropriate approvals and secure necessary permits from City, County, State, OR Federal Agencies as required.

 3. It is the responsibility of the CUSTOMER to Interpret all aspects of the END USER'S specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to Building Supplier's Specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to Building Supplier's "For Construction" drawings only. Temporary supports such as guys, braces, false work, cribbing or other elements required for the erection operation shall be determined, furnished and installed by the ERECTOR. No items should be purchosed from a preliminary set of drawing. Including anchor botts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7, Code of Standard Practice for Steel Buildings Supplier's strandard specifications apply unless stipulated otherwise in the Contract Documents. Building Supplier design, quality criteria, standards, practice, methods and tolerances shall gove

- Solution Supplier's standard specifications of payly wheels supplied of the was a fine contrary not withstanding. It is understood by both parties that the CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications. In case of discrepancies between Building Supplier's structural steel plans and plans for other trades, Building Supplier's plans shall govern. (Section 3, Code of Standard Practice for Steel Buildings —Buildings, AISC 15th Edition).

 It is the responsibility of Building Supplier, through their Engineer, to design the metal building system to meet the specifications in cluding the design criteria and design loads incorporated by the CONTRACTOR into the Order Documents. Building Supplier is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Documents.

 7. Building Supplier is responsible only for the structural design of the metal building system. The Building Supplier's Engineer is not the Design Professional or Engineer of Record for the Construction Project. The supplying of seedled engineering data and drawings for the metal building system does not imply or constitute an agreement that the Building Supplier or its design of the structural components.

 Building Supplier is responsible for the design of the another both to permit the transfer of forces of the concrete or the adequacy of the another both to permit the transfer of forces between the base plate and the anchor both in relation to the concrete. Unless otherwise provided in the Order Documents, Building Supplier steps of the design, and the anchor both in relation to the construction of the foundation or foundation embedment. The CUSTOMER should assure himself that adequate provisions are made in the foundations of the building, but the Imposed loads, and bearing capacity of the soil and other conditions of the building supplier is structural steel plans and plans for other trades, Building Supplier's sel

APPROVAL NOTES

- Approval of Building Supplier's drawings and/or calculations indicates that Building Supplier has correctly interpreted the contract requirements. This approval constitutes the CUSTOMER'S acceptance of the Building Supplier's design, concepts, assumptions, and loadings. (Section 4, Code of Practice for Steel Buildings, AISC 15th Edition and MBSM 3.3.3).
- . Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which Building Supplier will not be
- 2. Pollure to responsible.

 3. Any changes made after the CUSTOMER has signed and returned the approval drawings and/or calculations and the project is released for production shall be billied to the CUSTOMER including material, engineering, and other cost. An additional fee may be charged if the project must be moved from the engineering and/or the production/drafting schedule.

 4. It is the responsibility of the CUSTOMER to field verify all existing conditions prior to fabrication.

 5. It is imperative that any changes to these drawings:

 5.1. Be need in contrasting ink.

 5.2. Be legible and unambiguous.

 5.3. Hove all instances of changes clearly indicated.

 6. A dated signature, in the designated areas is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the CUSTOMER.

 7. Building Supplier reserves the right to resubmit drawings with extensive or complex changes required to avoid fabrication errors. This may impact the delivery schedule.

 8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between Building Supplier and its CUSTOMER are not binding on Building Supplier unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation.

- . The CUSTOMER approves of all notes and conditions on the drawings and/or calculations by signing an Approval Drawing Waiver Form.

GENERAL NOTES

- Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown
- 2. OII—canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.

- The primer for all cold-formed structural framing members contain a "wax-type" lubricant to facilitate roll-forming. Hair-line crazing which may occur during forming operations is considered normal and is not a cause for rejection.
 All other primed structural members are given one shop cost (1.0 mils) of standard red-oxide primer designed for short term field protection. This point is not intended for long term exposure to the elements.
 All botts are 1/2" x 1-1/4" A307 except at bearing frame rafter splice, endwall column to rafter and main frame connections. Refer to drawings. Note: Washers are not supplied unless noted otherwise on drawing.
 All high strength botts are A325 unless specifically noted otherwise. Structural joints with A.S.T.M. A325 high strength botts where indicated on the drawings are designed and considered to be in a Non-Silp Citical Category and therefore need only to be tightened to the snug tight condition. This condition should be attained when all surface in a joint are in firm contact and by using few impacts of an impact wrench or the full effort of a person using spud werench. Hardened washers are not required unless otherwise on the drawings.
 Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, insulation types of the substance of the full effort of a person using spud werench. Hardened washers are not required unless otherwise on the drawings.
 Fabrication shall be in accordance with Building Supplier's standard practices in compilance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH							
Hot Rolled Steel Shapes (W, S, C & L)	A572 / A529	Fy = 50 KSI							
Hot Rolled Steel Shapes (W)	A992	Fy = 50 KSI							
Round Structural Tubing (HSS)	A500	Fy = 42 KSI							
Square / Rect. Structural Tubing	A500	Fy = 46 KSI							
Structural Steel Web Plate	A572 / A1011	Fy = 55 KSI							
Structural Steel Flange Plates / Bars	A529 / A572	Fy = 55 KSI							
Cold Formed Light Gage	A653 / A1D11	Fy = 55 KSI							
Roof and Wall Sheets	A792 / A653	Fy = 50, 80 KSI							
Cable Brace	A475	Extra High Strength							
Rod Brace	A36	Fy = 36 KSI							
		MIN. TENSILE STRENGTH							
Machine Bolts & Nuts	A307	Fu = 60 KSI							
High Strength Bolts (½" diam. to 1½ diam.)	A325	Fu = 120 KSI							
Anchor Bolts	A36 / A307 / F155 Gr. 36	Fu = 58-80 KSI							
THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.									

IAS OR AISC CERTIFIED PLANT PROJECT REQUIREMENT: YES ☑ NO ☐



THIS STRUCTURE HAS BEEN I	DESIGNED IN ACCORDANC	E WITH THE FOLLOWING AS	NDICATED
DESIGN LO	ADS	FRAMING / P.	ANELS AND TRIMS
DESIGN CODE	IBC 18	FRAMING COATING	
RISK CATEGORY	II — Normal	PRIMARY & SECONDARY	RED-OXIDE
ENCLOSURE	Enclosed	ROOF PANELS	
DEAD LOAD (psf)	BUILDING STRUCTURE ONLY	GA / PANEL TYPE	24 / 24" DOUBLE-LOK
COLLATERAL LOAD (psf)	10.00	PANEL COLOR	TRUE BLACK
WIND LOAD		ROOF TRIM COLORS:	
WIND SPEED (BASIC OR ULT. PER CODE)	120	GA / EAVE COLOR	26 / TRUE BLACK
WIND IMPORTANCE FACTOR (Iw)	1.00	GA / GUTTER COLOR	26 / TRUE BLACK
WIND EXPOSURE	С	GA / GABLE COLOR	26 / TRUE BLACK
INTERNAL PRESSURE COEF., GCPI	0.18 / -0.18	WALL PANELS	
LIVE LOAD		GA. / PANEL TYPE	BOARD & BATTEN BY OTHERS
PRIMARY FRAMING (psf)	20.00	PANEL COLOR	
TRIBUTARY AREA REDUCTION	No	WALL TRIM COLORS	
SECONDARY FRAMING (psf)	20.00	GA / CORNER COLOR	BY OTHERS
SNOW LOAD		GA / OPENING COLOR	BY OTHERS
GROUND SNOW LOAD, Pg (psf)	10	GA / DOWNSPOUT COLOR	26 / TRUE BLACK
ROOF SNOW LOAD, Pf (psf)	7.00	GA / BASE TRIM COLOR	BY OTHERS
SLOPED ROOF SNOW LOAD, Ps (psf)	Pf x Cs	WAINSCOT PANELS	
SNOW EXPOSURE FACTOR, Ce	1.0000	GA / PANEL TYPE	
SNOW IMPORTANCE FACTOR, Is	1.0000	PANEL COLOR	
THERMAL FACTOR, Ct	1.00	WAINSCOT TRIM	
SLOPED FACTOR, Cs		WAINSCOT TRIM COLOR	
SEISMIC LOAD		LINER ROOF PANELS	
SEISMIC IMPORTANCE FACTOR, le	1.00	GA / PANEL TYPE	
SEISMIC OCCUPANCY CATEGORY	II – Normal	PANEL COLOR	
SITE CLASS	d	LINER WALL PANELS	
MAPPED SPECTRAL RESPONSE ACCEL.	Ss = 0.186 S1 = 0.086	GA / PANEL TYPE	
SPECTRAL RESPONSE COEFFICIENT	Sds = 0.198 Sd1 = 0.138	PANEL COLOR	
SEISMIC DESIGN CATEGORY	c	LINER TRIM	
BASIC FORCE RESISTING SYSTEMS USED	STEEL SYSTEM NOT DETAILED	LINER TRIM COLOR	
Brisio Force Resistante Stotems Coep	FOR SEISMIC RESISTANCE	PARTITION PANELS	
	RIGID FRAMES	GA / PANEL TYPE	
TOTAL DECIDI DACE CUEAD V (15-1)	BRACED FRAMES	PANEL COLOR	
TOTAL DESIGN BASE SHEAR, V (kips)	TRANSVERSE = 4.46	PARTITION TRIM	
DECENDED HODIFICATION FACTORS B	LONGITUDINAL = 3.82	PARTITION TRIM COLOR	
RESPONSE MODIFICATION FACTORS, R	RIGID FRAMES = 3	SOFFIT PANELS	
	END WALL BRACING = 3	GA / PANEL TYPE	26 / REV. PBR
05101110 050001105 005551015117 0	SIDE WALL BRACING = 3	PANEL COLOR	TRUE BLACK
SEISMIC RESPONSE COEFFICIENT, Cs	RIGID FRAMES = 0.066	SOFFIT TRIM	
	E. W. X BRACING = 0.066	SOFFIT TRIM COLOR	TRUE BLACK
	S. W. X BRACING = 0.066	FASCIA PANELS	INOE BEAGN
ANALYSIS PROCEDURE USED	EQUIV. LATERAL FORCE PROCEDURE	GA / FRONT PANEL TYPE	+
		FRONT PANEL COLOR	
RAINFALL INTENSITY (inches /Hr)	11 = 7.0600	GA / BACK PANEL TYPE	
	•		

FLOOR LOADS

BUILDING WAS DESIGN WITH ABOVE FLOOR LOADS. CONSIDERED ALL FLOOR MEMBER ARE BY OTHERS.

DEAD LOAD (psf)

LIVE LOAD (psf)

DESIGN NOTES:

PARTITION DEAD LOAD (psf)

COLLATERAL LOAD ABOVE (psf)

COLLATERAL LOAD BELOW (psf)

FLOOR

40.0000

10 0000

5.0000

5.0000

80,0000

	DEFLECTION	LIMITS:
EW	COLUMN:	180
EW	RAFTER LIVE:	180
EW	RAFTER WIND:	180
WAL	L GIRT:	90
PUR	RLIN LIVE:	180
PUR	RLIN WIND:	150
WAL	L PANEL:	60
ROC	F PANEL LIVE:	60
ROC	F PANEL WIND:	60
RF	HORIZONTAL:	60
RF	VERTICAL:	180
WIN	D BENT:	60
RF	CRANE:	100

RE SEISMIC:

WIND BENT SEIS .:

50

BACK PANEL COLOR

FASCIA TRIM COLOR

FASCIA TRIM

DELIVERY

- . Customer is responsible for verifying that the Goods listed on the Bill of Lading are received. All shortages and/or damages must be noted, in writing, on the Bill of Lading prior to Buyer signing the Bill of Lading. Failure by the Customer to document shortages of the number of packages or damages within (5) days of delivery or pickup shall waive any claim of such shortage and/or damages. It is Customer's responsibility to retain a copy of the Bill of Lading documenting any shortages and/or damages. Loss of the Bill of Lading shall also waive any right to claim any shortage and/or damage.
- Building Supplier is not obligated to send Goods by overnight air freight, direct truck line, or other expedited method unless Buyer prepays for such services. Building Supplier shall not be responsible for loss or damage to Goods that occur after tender for pick up or delivery. Self-shall have no obligation to remove or dismantile defective parts or to erect or install replacement parts. Back charges that are not accepted by Building Supplier in writing shall have no effect and Buyer's account may be placed on immediate Credit Hold until resolution. Building Supplier shall not be responsible or financially liable for delivery delays or any of Customer's costs expended on remedies unauthorized by Building Supplier, including, but not limited to, Customer's erection crew expense or rental equipment costs or liquidated or consequential damages of any kind.
- In the event that parts are damaged during transit, pictures including piece marks should be taken and reported immediately to the Buyer. A replacement part and redelivery date will be coordinated with the manufacturer.

 Any missing parts should be circled on the Bill of Lading and returned to the driver and reported to the Buyer for immediate resolution.

- RECION NOTES

 1. All bracing shown and provided by Building Supplier for this building is required and shall be installed by the ERECTOR as a permanent part of the structure ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual; Section 7.9).

 2. Temporary supports, such as guys, braces, false work, cribbing or other elements required for the erection operation shall be determined and furnished by the ERECTOR ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual; Section 7.9).

 3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, chipping, or cutting and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to Building supplier by the CUSTOMER to enable whoever is responsible either to correct the error or approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual; Section 7.12).
- 7.12).

 4. Erection tolerances are set forth in AISC Code of Standard Practice 7.11 except that individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structural steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances. 4.1. When crane support systems are part of the metal building system erection tolerances Section 9, Common industry Practices, 2018 MESM Manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The CISTOMER shall provide grout if required. The CONTRACTOR erecting the runway beams is responsible for shimming, pulming, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rall is over the runway web.

 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met. 5.1. Welders must be qualified by an independent testing agency, with suitable documentation to ARS D1.1 Structural Welding Code Steel or ARS D1.3 Structural Welding Code Sheet Steel as applicable, for the processes, positions, and materials involved. 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not pre-qualified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
- approved Welding Procedure Specification (WPS). All joints which are not pre-Record (PQR) by an independent testing agency. 6. All documentation and records shall be the responsibility of the CUSTOMER.
- Neither Building Supplier nor the CUSTOMER will out, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the CUSTOMER is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 7.13).

3. Field Modifications Policy:

- 8.1. Building Supplier will only be responsible for the field-modified parts designed and approved by the Building Supplier's Engineering Department.
 8.2. Any field modifications designed by third parties may not be approved by Building Supplier and may limit Building Supplier's warranty and liability
- 8.2. Any field modifications designed by third parties may not be approved by Building Supplier and may limit Building Supplier's warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
 9. The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim.
 10. Visible gaps between column ana/or rafter connection plates can occur as a reason to various causes without critical effect to the structural integrity. Minimal shimming, to licensistic contects of the connection plates. The purpose of shimming, besides any aesthetic benefits, is to provide resistance to the tightening procedures of high-strength botts for proper installation. The types of shim can be of a uniform thickness, full size, tapered or notched around botts to permit installation without removal of botts. Bott holes oversized by \(\frac{1}{2}\) finches are permitted in full-size shims to facilitate alignment. For further information regarding shimming, refer to the AISC publication, "Engineering for Steel Construction". In the event of connection gaps, the manufacturer must be consulted for approval and specific recommendations for proper shimming.
 12. The Building Supplier though its CS Manager, must be actified at a new texts.
- 1. The Building Supplier, through its CS Manager, must be notified at once when a condition becomes apparent that may result in a backcharge by the Erector. Notification by phone must be confirmed in writing. Some approximation of the amount of the backcharge me be established at this time and an authorization before the work is started. Building Supplier will not honor any field corrections or backcharges unless prior notice has been given and agreed upon. All discrepancies must be agreed upon, in writing. Any work which is undertaken without such notification and authorization will not be honored as a backcharge.
- 11.1 Description of nature and extent of the errors, including piece marks, quantities, photos, and measurements, where applicable.
 11.2 Description of nature and extent of proposed corrective work including estimated man-hours.
 11.3 Material to be purchased from other than the Building Supplier, including estimated quantities and cost.

- MARNING in no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper floshing, wiring, or tubing onto Calvalume should be avoided.
 It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
 Roof snow accumulations in excess of specified project design loading criteria can cause significant distress to the building structural system. It is recommended that roof be cleared of snow by the CUSTOMER Refer to A9.4 for Snow/ice Removal procedure by Metal Building Systems Manual. A copy is available upon request.

	DRAWING INDEX								
ISSUE	DESCRIPTION	PAGE							
0	PANEL ZONE LAYOUT	C2							
0	ANCHOR BOLT PLAN	F1							
0	ANCHOR BOLT BASE DETAILS	F2							
0	ANCHOR BOLT REACTIONS	F3							
0	FRAMING AND BRACING PLAN	E1							
0	ROOF FRAMING PLAN	E2							
0	ROOF SHEETING PLAN	E3-E4							
0	RIGID FRAME ELEVATION	E5-E9							
0	ENDWALL FRAMING ELEVATION	E10-E11							
0	SIDEWALL FRAMING ELEVATION	E12-E13							
0	WIND BENT ELEVATION	E14-E15							
0	STANDARD DETAILS	S1-S10							

П	BUILDING DESCRIP	TION
	WIDTH (FT)	40
П	LENGTH (FT)	90
П	BACK SIDE WALL EAVE HEIGHT (FT)	20
	FRONT SIDE WALL EAVE HEIGHT (FT)	20
	BACK SIDE WALL ROOF SLOPE	4.0:12
	FRONT SIDE WALL ROOF SLOPE	4.0:12
	BAY SPACING (FT)	SEE PLAN
	CAD	1111

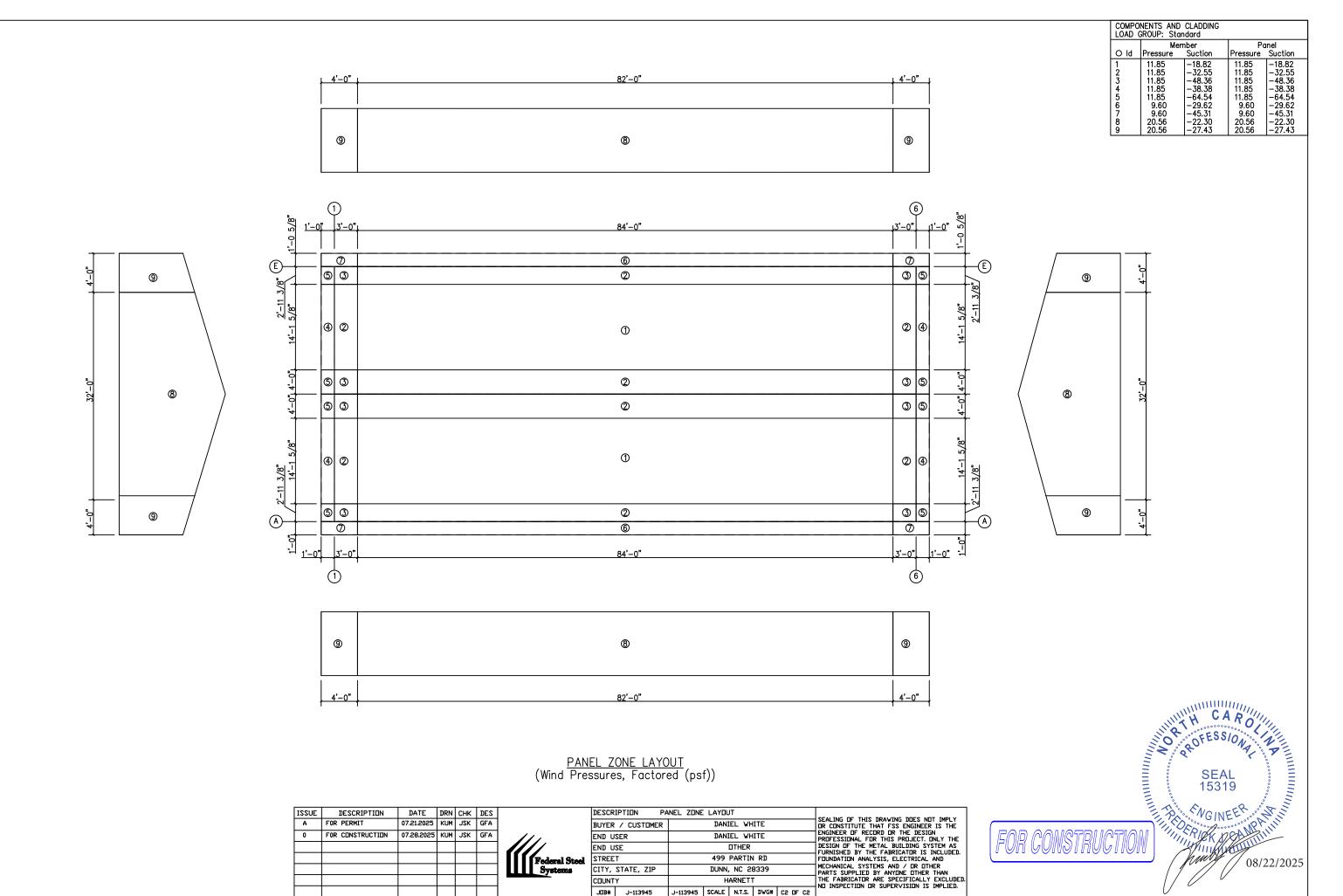
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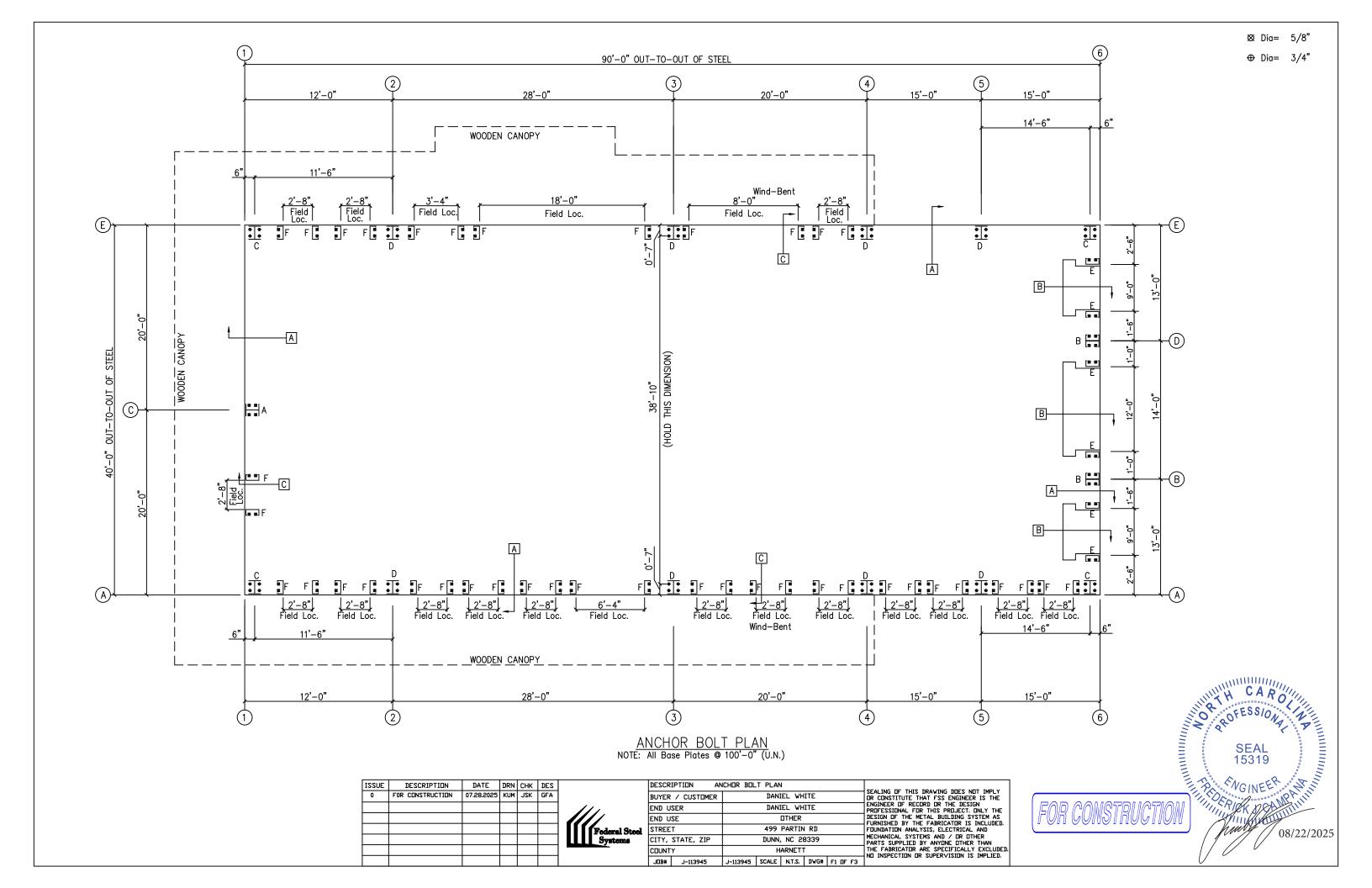
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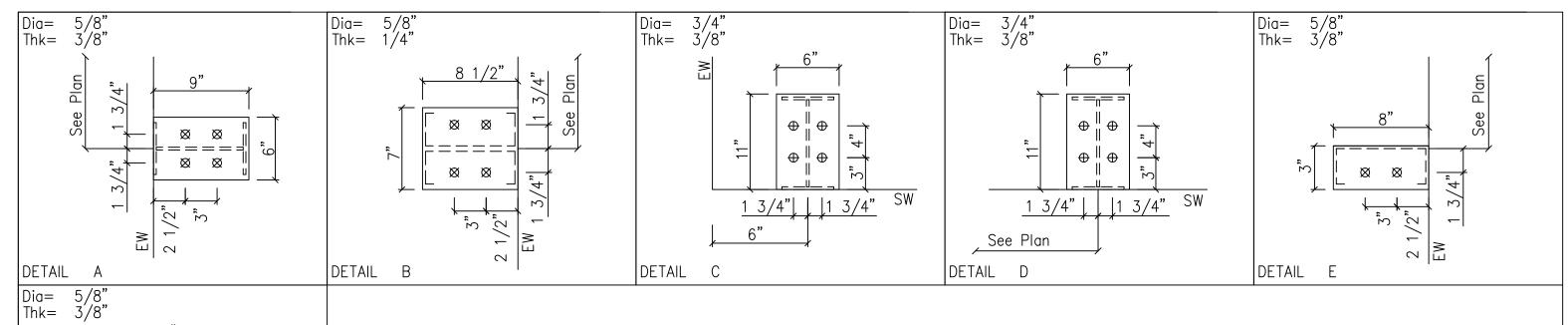
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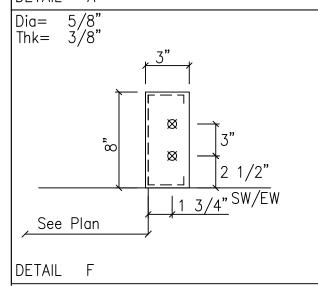


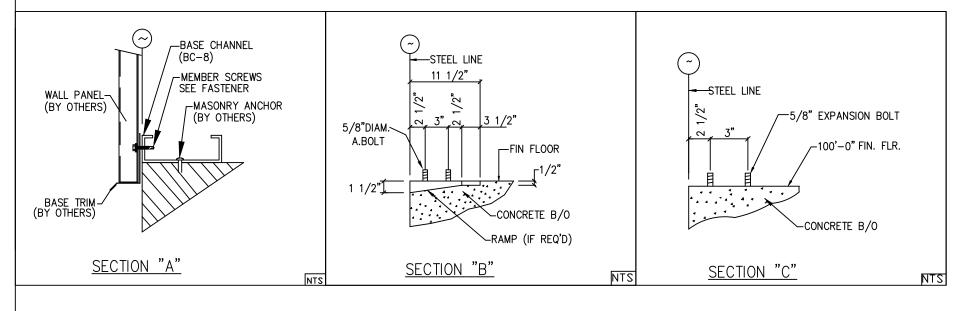
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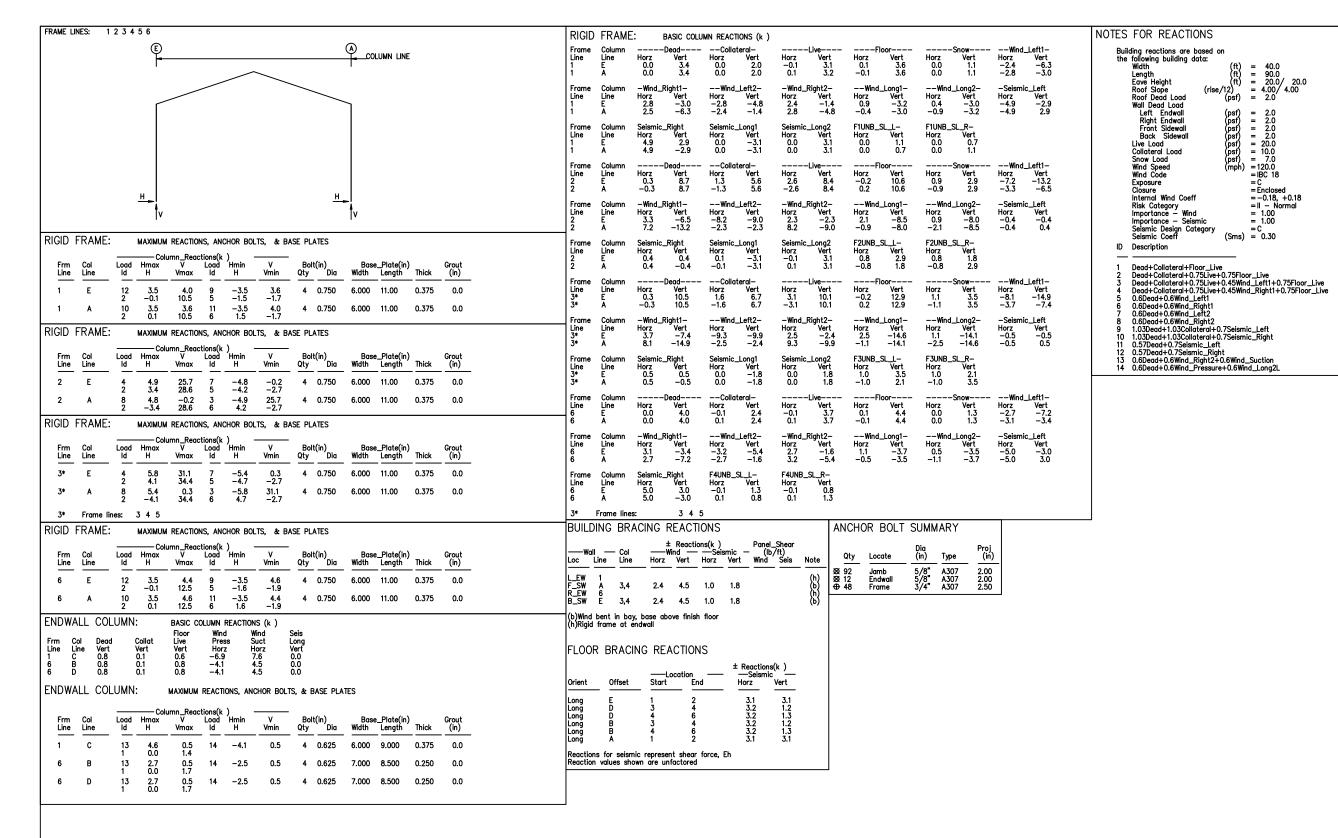


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ND USER	DAN:	IEL WHITE	ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT, ONLY THE			
ND USE		OTHER		DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY THE FABRICATOR IS INCLUDED.		
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SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT FSS ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. DINLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISSED BY THE FABRICATION IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL AND DEMONSTRATION PROFESSION. MECHANICAL SYSTEMS AND / OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN THE FABRICATOR ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

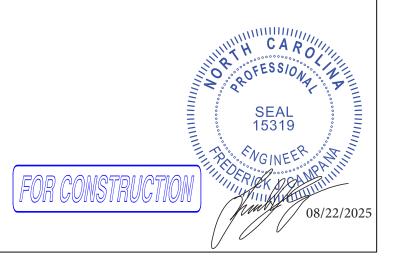
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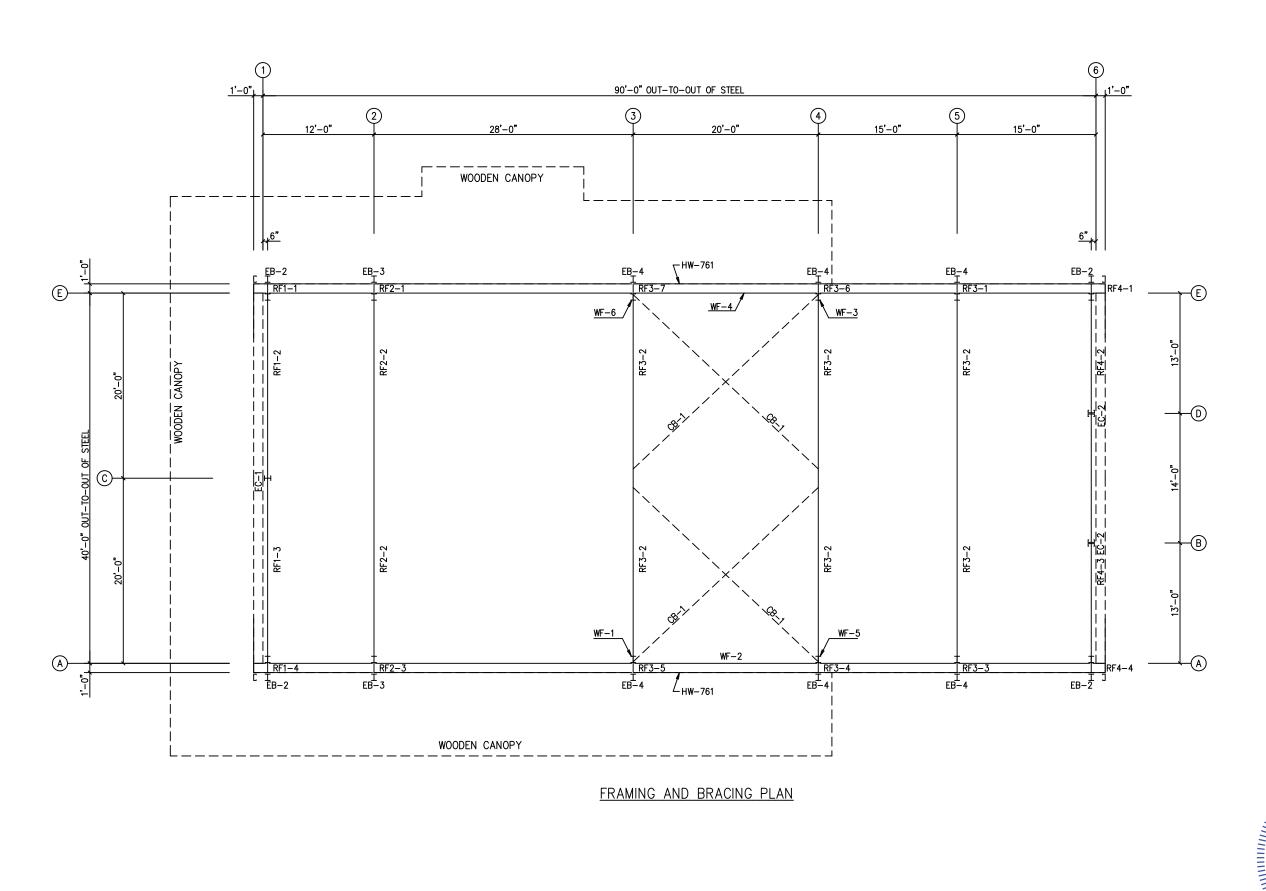
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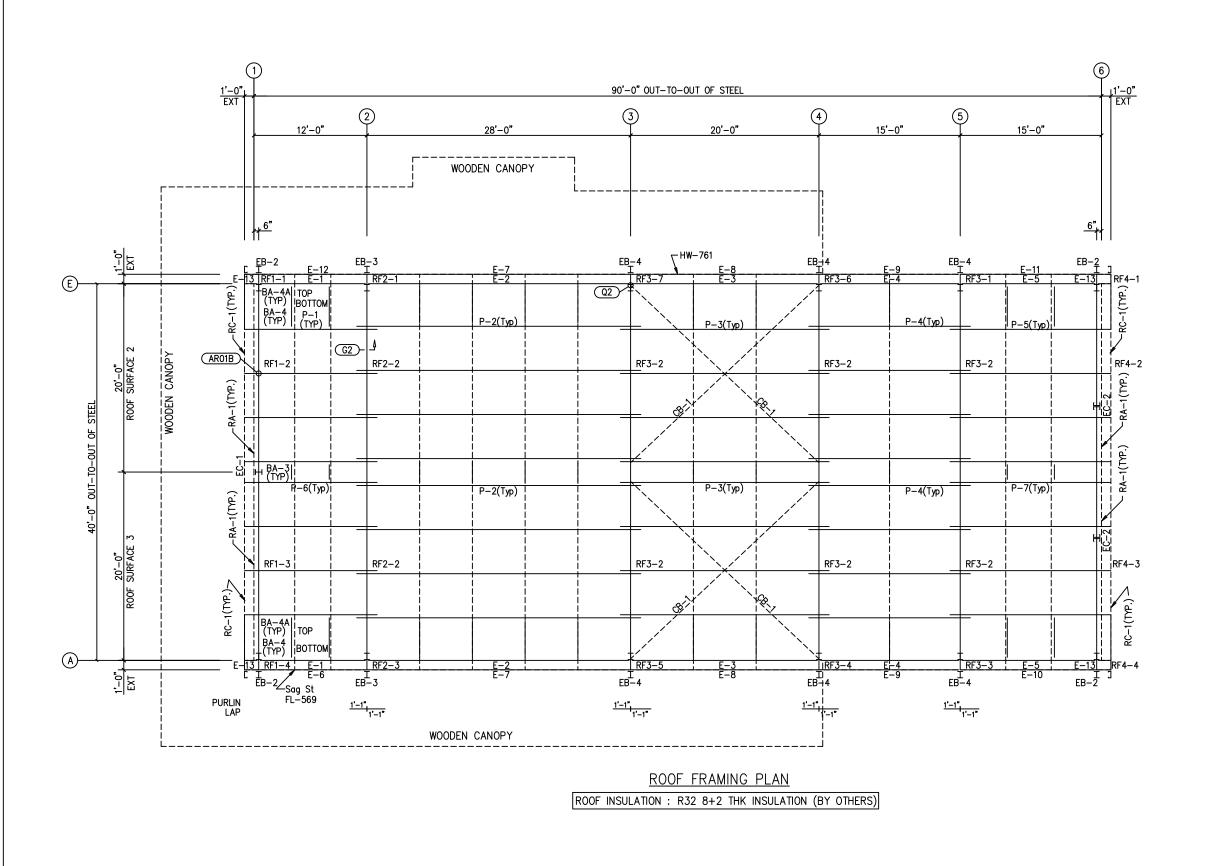






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CITY, STATE, ZIP		DUNN,	NC 2	8339			MECHANICAL SYSTEMS AND / DR DTHER PARTS SUPPLIED BY ANYONE DTHER THAN
COUNTY	HARNETT				THE FABRICATOR ARE SPECIFICALLY EXCLUDE NO INSPECTION OR SUPERVISION IS IMPLIED.		
.IDR# .I=113945	J-113945	SCALE	N.T.S.	DVG#	F1 OF	F15	THE INSTECTION OR SOLERVISION IS INCLIED.

FOR CONSTRUCTION



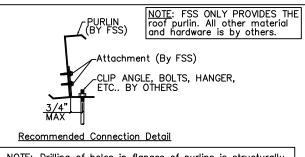
ſ	ISSUE	DESCRIPTION	DATE	DRN	CHK	DES	
ſ	Α	FOR PERMIT	07.21.2025	KUM	KKC	GFA	
ľ	0	FOR CONSTRUCTION	07.28.2025	KUM	JSK	GFA	
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ESCRIPTION RO	DDF FRAMING PLAN SEALING OF THIS DRAWING DOES NOT	TMDLV
UYER / CUSTOMER	DANIEL WHITE OR CONSTITUTE THAT FSS ENGINEER :	IS THE
ND USER	DANIEL WHITE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ON	
ND USE	OTHER DESIGN OF THE METAL BUILDING SYST	
TREET	499 PARTIN RD FOUNDATION ANALYSIS, ELECTRICAL A	ND
ITY, STATE, ZIP	DUNN, NC 28339 MECHANICAL SYSTEMS AND / OR OTHE PARTS SUPPLIED BY ANYONE OTHER T	
:DUNTY	HARNETT THE FABRICATOR ARE SPECIFICALLY E	
.I⊓R# .I=113945	I-113945 SCALE N.T.S. DWG# F2 DF F15	rii CICD.

| EXTENSION/CANOPY BOLTS | ROOF PLAN | QUAN TYPE DIA LENGTH | EB-2 | 4 A325 5/8" 1 3/4" | EB-3 | 4 A325 5/8" 1 3/4" | EB-4 | 4 A325 5/8" 1 3/4" |

MEMBER TABLE ROOF PLAN QUAN MARK PART 4 EB-2 W8x10 2 EB-3 W8x10 6 EB-4 W8x10 4 P-1 8x25Z12 8 P-2 8x25Z12 8 P-3 8x25Z12 8 P-4 8x25Z16 4 P-6 8x25Z16 4 P-6 8x25Z16 4 P-7 8x25Z16 2 E-1 E085344L 2 E-2 E085344L 2 E-2 E085344L 2 E-4 E085344L 2 E-5 E085344L 2 E-7 8x25C16 2 E-7 8x25C16 2 E-8 8x25C16 2 E-9 8x25C16 1 E-10 8x25C16 1 E-11 8x25C16 1 E-11 8x25C16 1 E-12 8x25C16 1 E-12 8x25C16 1 E-13 E085344L 4 CB-1 CBL2500 11 BA-3 2x2x14GA 22 BA-4 2x2x14GA 22 BA-4 2x2x14GA										
QUAN MARK PART 4 EB-2 W8x10 2 EB-3 W8x10 6 EB-4 W6x10 4 P-1 8x25Z12 8 P-2 8x25Z12 8 P-4 8x25Z12 8 P-4 8x25Z16 4 P-5 8x25Z12 4 P-7 8x25Z16 2 E-1 E085344L 2 E-2 E085344L 2 E-4 E085344L 2 E-4 E085344L 2 E-7 8x25C16 2 E-7 8x25C16 2 E-8 8x25C16 2 E-9 8x25C16 1 E-10 8x25C16 1 E-11 8x25C16 4 E-13 E085344L 4 E-13 E085344L 4 E-1 8x25C16 1 E-11 8x25C16 <t< td=""><td colspan="9"></td></t<>										
4 EB-2 W8x10 2 EB-3 W8x10 6 EB-4 W8x10 4 P-1 8x25Z12 8 P-2 8x25Z12 8 P-4 8x25Z12 8 P-4 8x25Z16 4 P-5 8x25Z16 4 P-6 8x25Z16 2 E-1 E085344L 2 E-2 E085344L 2 E-4 E085344L 2 E-5 E085344L 2 E-7 8x25C16 2 E-7 8x25C16 2 E-7 8x25C16 2 E-8 8x25C16 2 E-9 8x25C16 1 E-10 8x25C16 1 E-11 8x25C16 1 E-11 8x25C16 1 E-12 8x25C16 1 E-13 E085344L 4 CB-1 CBL2500 11 BA-3 2x2x14GA										
4 EB-2 W8x10 2 EB-3 W8x10 6 EB-4 W8x10 4 P-1 8x25Z12 8 P-2 8x25Z12 8 P-3 8x25Z12 8 P-4 8x25Z16 4 P-5 8x25Z16 4 P-7 8x25Z16 4 P-7 8x25Z16 2 E-1 E085344L 2 E-2 E085344L 2 E-4 E085344L 2 E-4 E085344L 2 E-5 E085344L 2 E-6 8x25C16 2 E-7 8x25C16 2 E-8 8x25C16 2 E-8 8x25C16 1 E-10 8x25C16 1 E-11 8x25C16 1 E-11 8x25C16 1 E-12 8x25C16 1 E-11 8x25C16 2 E-8 8x25C16 2 E-9 8x25C16 2 E-10 8x25C16 2 E-	QUAN									
	426488844422222111144112	EB-2 EB-3 EB-4 P-23 P-45 E-7 E-7 E-11 E-11 E-11 E-11 CB-3 CB-4	W8x10 W8x10 W8x10 W8x10 8x25Z12 8x25Z12 8x25Z16 8x25Z16 8x25Z16 8x25Z16 E085344L							



NOTE: Drilling of holes in flanges of purlins is structurally detrimental to the member. Any collateral loads, which are included in the purlin design loads, are to be attached by connection directly to the web only.

<u>Field Modifications - Warning</u>



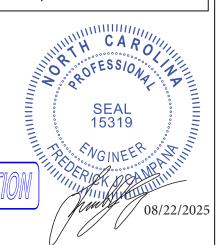
Flange C—Clamp
is not an acceptable
connection an acceptable Connection through
connection the flange is not

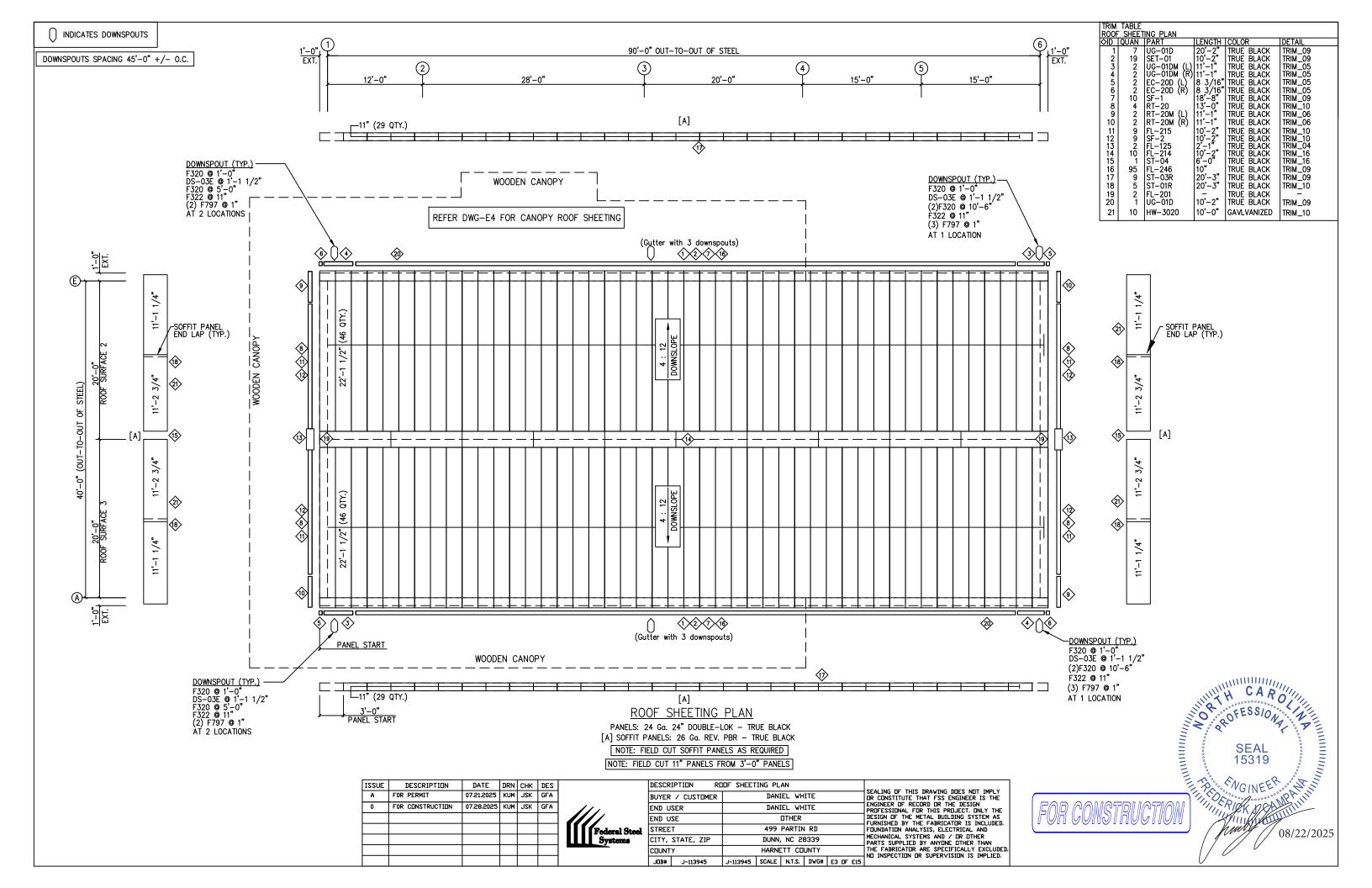


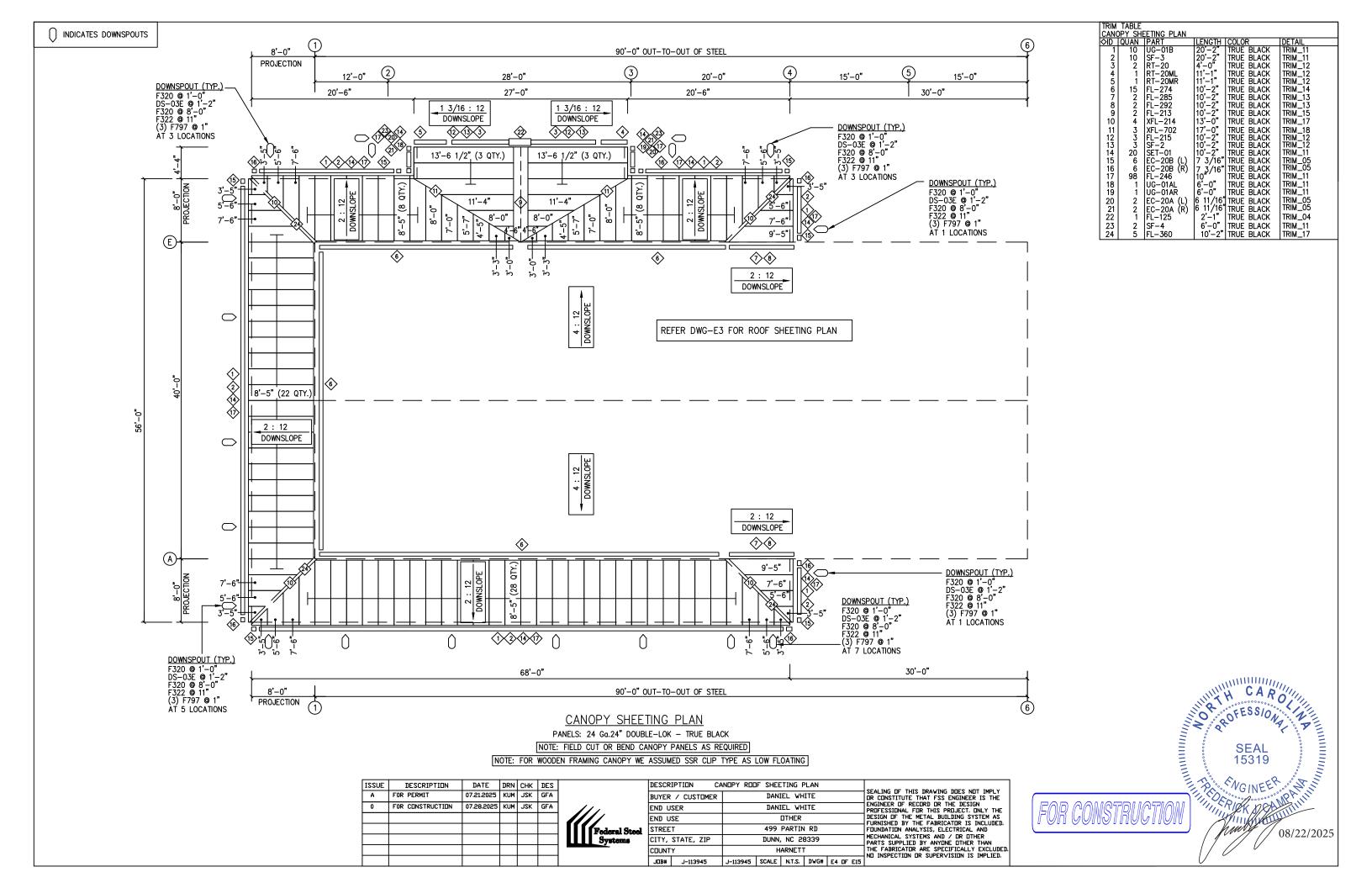
RÖD Connection through
Connection through the flange is
the flange is not not acceptable
acceptable

-PURLIN

ATTACHMENT OF SUSPENDED ITEMS (BY OTHERS)





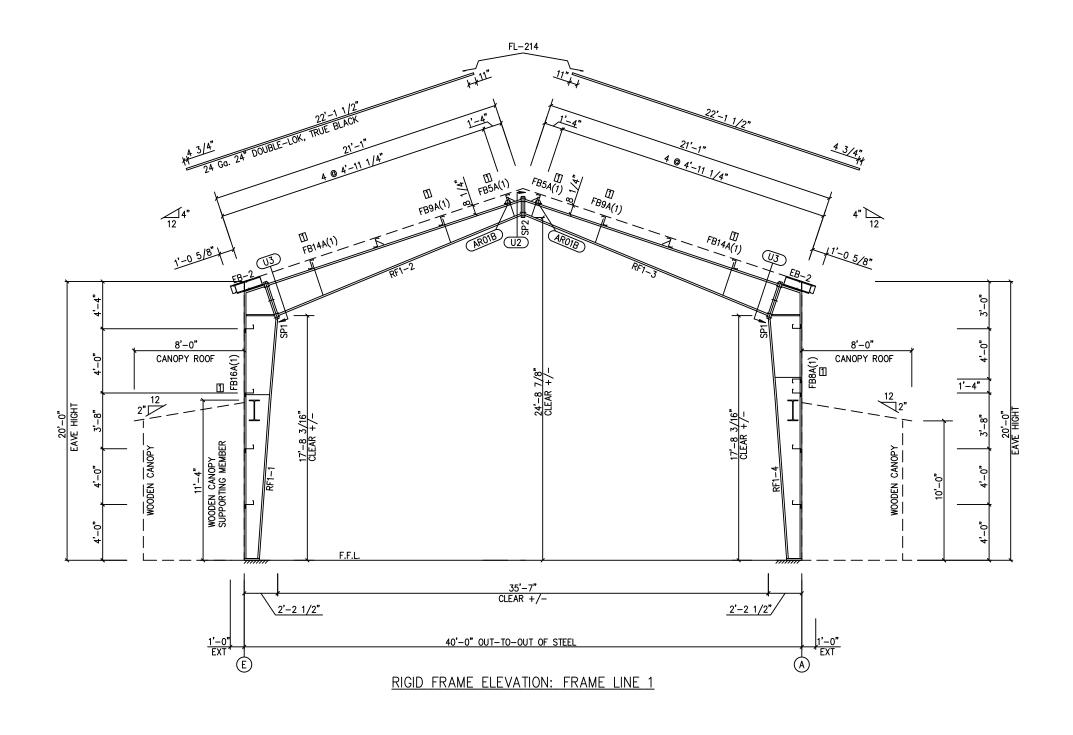


| SPLICE PLATE & BOLT TABLE | | Qty | Top | Bot | Int | Type | Dia | Length | Width | Thick | Length | SP1 | 4 | 4 | 2 | A325 | 0.625 | 2.25 | 6" | 5/8" | 2"-9 5/16" | SP2 | 4 | 4 | 0 | A325 | 0.625 | 2.25 | 6" | 1/2" | 1'-5 3/4" |

FLANGE BRACES: Both Sides(U.N.) FBxxA(1) A - 2X2X14GA

MEMBER TA	MBER TABLE											
Mark	Length	Web Depth	Web P			Inside_Flange						
	•	Start/End	Thick		W x Thk x Length	W x Thk x Length						
RF1-1	19'-3 5/16"	10.0/20.0	0.135	11'-6 3/8"	5 x 1/4" x 19'-2 11/16"	5 x 1/4" x 11'-6 11/16"						
	,	20.0/26.0	0.135	5'-9 3/4"	5 x 1/4" x 19'-2 11/16" 5 x 1/4" x 1'-5 11/16"	5 x 1/4" x 11'-6 11/16" 5 x 1/4" x 5'-10"						
		10.0/20.0 20.0/26.0 26.0/16.6 26.0/10.0 10.0/26.0 16.6/26.0 26.0/20.0	0.188	5'-9 3/4" 2'-4 1/8" 19'-5 7/16"	,	,						
RF1-2	19'-6 11/16"	26.0/10.0	0.135	19'-5 7/16"	5 x 1/4" x 19'-5 7/16"	5 x 1/4" x 19'-2 9/16" 5 x 1/4" x 19'-2 9/16" 5 x 1/4" x 5'-10" 5 x 1/4" x 11'-6 11/16"						
RF1-3	19'-6 11/16" 19'-3 5/16"	10.0/26.0	0.135	19'-5 7/16"	5 x 1/4" x 19'-5 7/16" 5 x 1/4" x 1'-5 11/16" 5 x 1/4" x 19'-2 11/16"	5 x 1/4" x 19'-2 9/16"						
RF1-4	19'-3 5 <i>/</i> 16"	16.6/26.0	0.188	2'-4 1/8"	5 x 1/4" x 1'-5 11/16"	5 x 1/4" x 5'–10"						
	,	26.0/20.0	0.135	5'-9 3'/4"	5 x 1/4" x 19'-2 11/16"	5 x 1/4" x 11'-6 11/16"						
		20.0/10.0	0.135 0.135 0.135 0.188 0.135 0.135 0.188 0.135 0.135	19'-5 7/16" 2'-4 1/8" 5'-9 3/4" 11'-6 3/8"	·	· '						
EB-2	1'-11"	W8x10		•								

CONNECT		
ID.	Qty.	MARK
1	8	CL-190

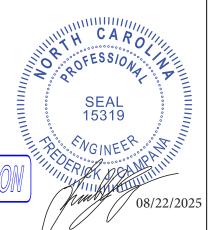


ISSUE	DESCRIPTION	DATE	DRN	CHK	DES	
Α	FOR PERMIT	07.21.2025	KUM	JSK	GFA	
0	FOR CONSTRUCTION	07.28.2025	KUM	JSK	GFA	

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Federal Steel	s
Federal Steel Systems	C
	c

	DESCR	IPTION RI	GID FRAME	ELEV	ATION					SEALING OF THIS
	BUYER	/ CUSTOMER		DAN]	EL WH	ITE				OR CONSTITUTE
	END USER DANIEL WHITE								ENGINEER OF RE	
	END USE OTHER								DESIGN OF THE FURNISHED BY T	
1	STREE	Т		499 PARTIN RD						FOUNDATION ANA
	CITY, STATE, ZIP DUNN, NC 28339								MECHANICAL SYS	
	COUNT	Υ		Н	ARNETT	Ī				THE FABRICATOR
	JDB#	J-113945	J-113945	SCALE	N.T.S.	DWG#	E5	DF	E15	NO INSTECTION

OF THIS DRAWING DOES NOT IMPLY
TITUTE THAT FSS ENGINEER IS THE
R OF RECORD OR THE DESIGN
IDNAL FOR THIS PROJECT. ONLY THE
OF THE METAL BUILDING SYSTEM AS
ED BY THE FABRICATION IS INCLUDED.
IDN ANALYSIS, ELECTRICAL AND
CAL SYSTEMS AND / OR OTHER
UPPLIED BY ANYDINE OITHER THAN
RICATOR ARE SPECIFICALLY EXCLUDED.
ECTION OR SUPERVISION IS IMPLIED.



| SPLICE PLATE & BOLT TABLE | | Qty | Top | Bot | Int | Type | Dia | Length | Width | Thick | Length | SP1 | 4 | 4 | 0 | A325 | 0.625 | 2.25 | 6" | 1/2" | 2"-3 | 1/4" | SP2 | 4 | 4 | 0 | A325 | 0.625 | 2.25 | 6" | 1/2" | 1'-3 | 11/16" |

FLANGE BRACES: Both Sides(U.N.) FBxxA(1) A - 2X2X14GA
 MEMBER TABLE

 Mark
 Length
 Web Depth
 Web Plate
 Outside Flange
 Inside Flange

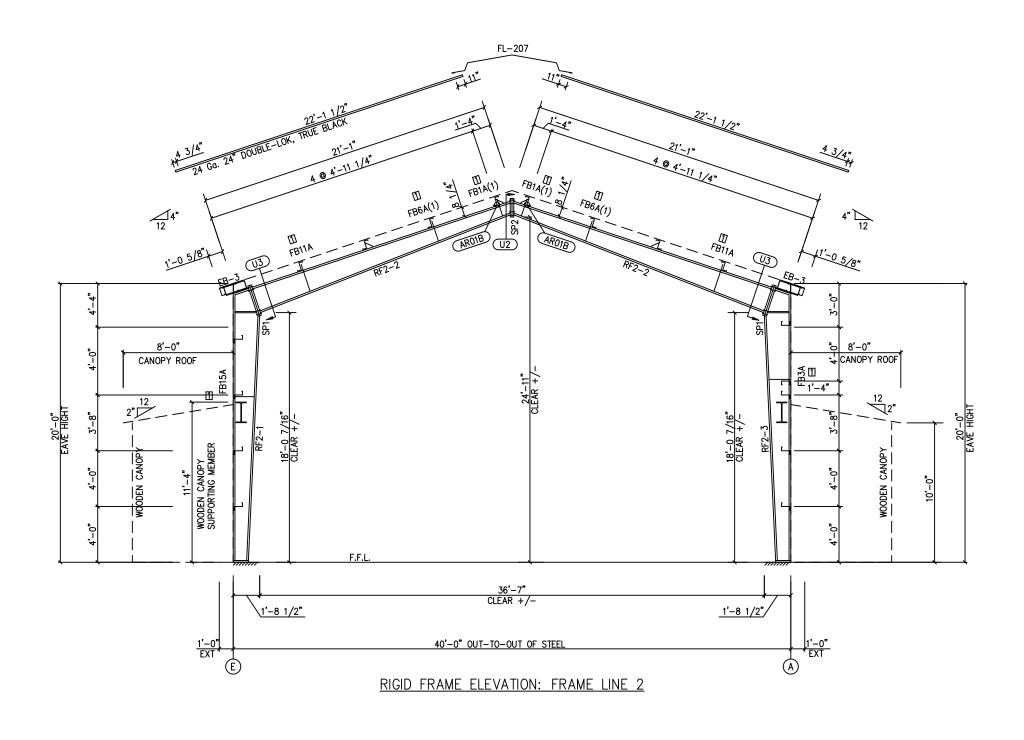
 RF2-1
 19'-3 5/16"
 10.0/20.0
 0.135
 17'-8 1/2"
 5 x 1/4" x 19'-2 11/16"
 5 x 1/4" x 17'-8 11/16"

 RF2-2
 19'-11 1/8"
 20.0/12.5
 0.188
 19'-10"
 5 x 1/4" x 19'-10"
 5 x 1/4" x 19'-10"

 RF2-3
 19'-3 5/16"
 12.5/20.0
 0.188
 1'-10 7/16"
 5 x 1/4" x 19'-10"
 5 x 1/4" x 19'-7 9/16"

 EB-3
 1'-6 5/8"
 W8x10
 0.135
 17'-8 1/2"
 5 x 1/4" x 19'-2 11/16"

| CONNECTION TABLE | ID. | Qty. | MARK | 1 | 12 | CL-190 |

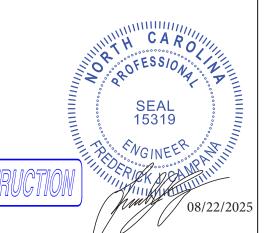


ISSUE	DESCRIPTION	DATE	DRN	CHK	DES	
Α	FOR PERMIT	07.21.2025	KUM	JSK	GFA	
0	FOR CONSTRUCTION	07.28.2025	KUM	JSK	GFA	

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	DESCR	IPTION RI	GID FRAME	ELEV	ATION				SEALIN	
	BUYER / CUSTOMER DANIEL WHITE									
	END U	SER	ER DANIEL WHITE							
	END U	SE		DTHER					DESIGN	
al	STREE	Т		499	PARTIN	I RD			FOUNDA MECHAN	
	CITY,	STATE, ZIP		DUNN, NC 28339						
	COUNT	Υ	HARNETT							
	JDB#	J-113945	J-113945	SCALE	N.T.S.	DWG#	E6 OF	E15	NO INS	

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT FSS ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY THE FABRICATIOR IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS AND / OR OTHER PARTS SUPPLIED BY ANYDINE OTHER THAN THE FABRICATIOR ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.



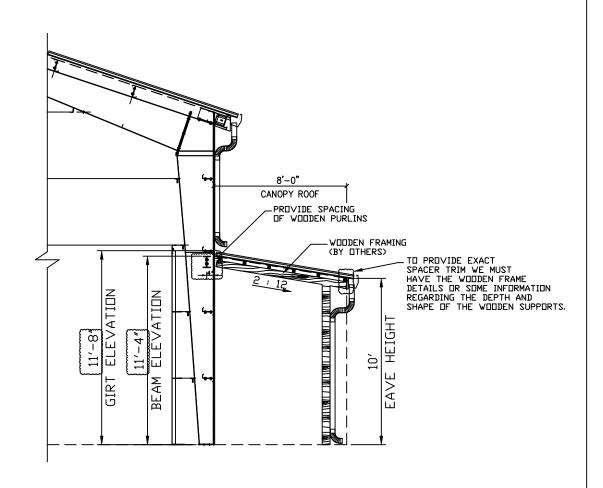
SPLICE PLATE & BOLT TABLE									
	Qty								
Mark	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP1	4	4	0	A325	0.625	2.25	6"	1/2"	2'-5 1/4"
SP2	4	4	0	A325	0.625	2.25	6"	1/2"	1'-3 11 <i>/</i> 16"

FLANGE BRACES: Both Sides(U.N.) FBxxA(1) A - 2X2X14GA

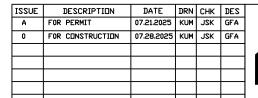
	MEMBER TA												
Γ	Mark	Length	Web Depth	Web Plate	Outside Flange	Inside_Flange							
L		,	Start/Énd	Thick Length		W x Thk x Length							
	RF3-7	19'-3 5/16"	10.0/22.0	0.135 17'-7 1/16" 0.188 2'-0 5/16" 0.188 19'-8 9/16" 0.188 2'-0 5/16" 0.135 17'-7 1/16"	5 x 1/4" x 19'-2 11/16"	5 x 1/4" x 17"-7 3/8"							
			22.0/13.9	0.188	5 x 1/4" x 1'-2 13/16"	·							
	RF3-2	19'-9 11/16" 19'-3 5/16"	22.0/ 8.0	0.188 19'–8 9 <i>/</i> 16"	5 x 1/4" x 19'-8 9'/16"	5 x 1/4" x 19'-6 3/16" 5 x 1/4" x 17'-7 3/8"							
	RF3-5	19'–3 5 <i>/</i> 16"	13.9/22.0	0.188 2'-0 5/16"	5 x 1/4" x 1'-2 13/16"	5 x 1/4" x 17'-7 3/8"							
		•	22.0/10.0	0.135 17'-7 1/16"	5 x 1/4" x 19'-2 11/16"	i i							
L	EB-4	1'-8 1/8"	W8x10	, , , , , , , , , , , , , , , , , , ,	<u> </u>								

CONNECT	ON TABLE	
ID.	Qty.	MARK
1	10	CL-190

FI-214 FI-214
36'-3" CLEAR +/-
1'-0" EXT E RIGID FRAME ELEVATION: FRAME LINE 3



CANOPY SUPPORT BEAM ELEVATION





	DESCR	IPTION	RIGID	FRAME	ELEV	ATION				SE.
	BUYER	/ CUSTOMER		DANIEL WHITE						SEA DR
	END U	SER		DANIEL WHITE						ENG PRO
	END U	SE	OTHER						DES	
ool	STREE	Т	499 PARTIN RD						FOU	
	CITY,	STATE, ZIP	DUNN, NC 28339							MEC PAR
	COUNT	Y	HARNETT						THE	
	JDB#	J-113945	J-113945	SCALE	N.T.S.	DWG#	E7	DF	E15	

EALING OF THIS DRAWING DOES NOT IMPLY IR CONSTITUTE THAT FSS ENGINEER IS THE NIGHEER OF RECORD OR THE DESIGN ROFESSIONAL FOR THIS PROJECT, ONLY THE ESIGN OF THE METAL BUILDING SYSTEM AS URNISHED BY THE FABRICATIOR IS INCLUDED. DUNDATION ANALYSIS, ELECTRICAL AND ECHANICAL SYSTEMS AND / OR OTHER ARTS SUPPLIED BY ANYONE OTHER THAN HE FABRICATIOR ARE SPECIFICALLY EXCLUDED. ID INSPECTION OR SUPERVISION IS IMPLIED.

SEAL 15319

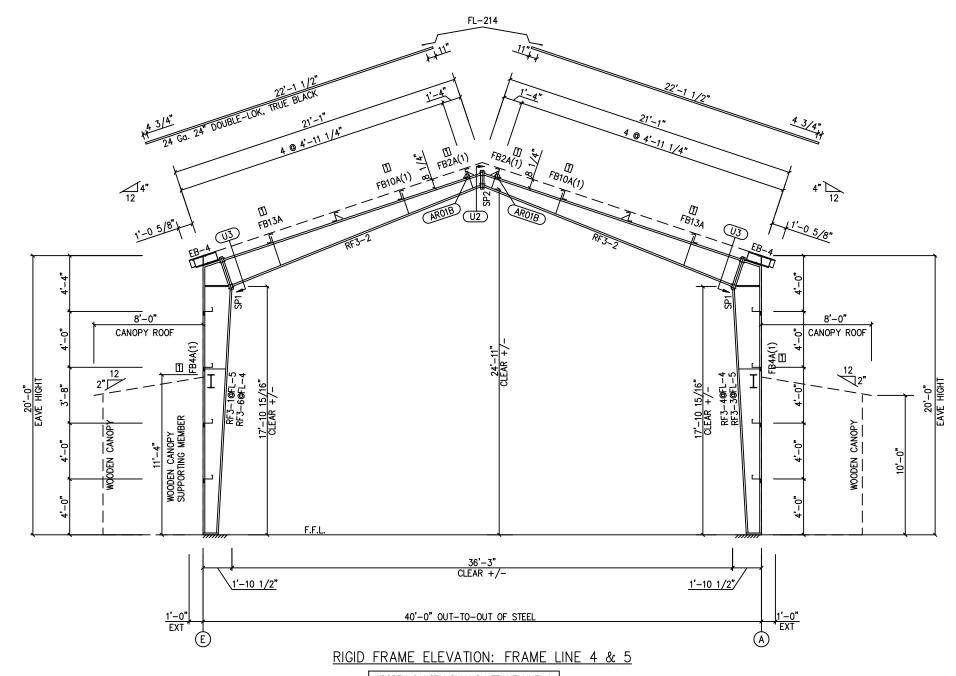
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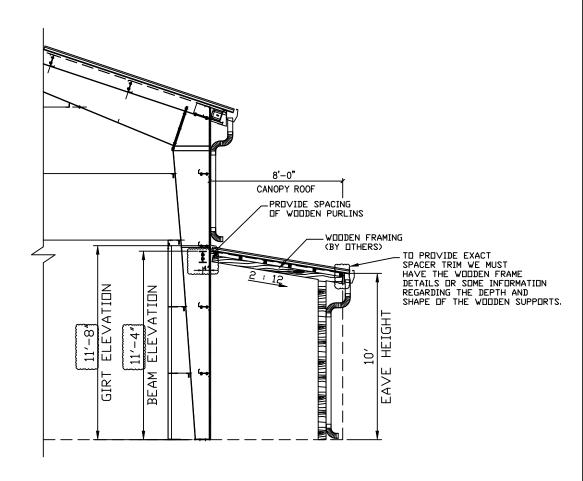
SPLICE PLA	ATE &	: BOL1	TAB	LE					
	Qty								
Mark	Top	Bot	Int	Туре	Dia	Length	Width	Thick	Length
SP1	4	4	0	A325	0.625	2.25	6"	1/2"	2'-5 1/4"
SP2	4	4	0	A325	0.625	2.25	6"	1/2"	1'-3 11/16"

FLANGE BRACES: Both Sides(U.N.) FBxxA(1) A - 2X2X14GA

MEMBER TABLE						
Manula	1	Web Depth	Web Plate			Inside Flange
Mark	Length	Start/Énd				W x Thk x Length
RF3-1/RF3-6	19'-3 5/16"	10.0/22.0	0.135 17 0.188 2'-	'-7 1/16" -0 5/16"	5 x 1/4" x 19'-2 11/16"	5 x 1/4" x 17"-7 3/8"
, ,	· '	22.0/13.9	0.188 2'-	-0 5 <i>/</i> 16"	5 x 1/4" x 1'-2 13/16"	
RF3-2	19'-9 11/16" 19'-3 5/16"	22.0/ 8.0	0.188 19' 0.188 2'- 0.135 17'	'–8 9 <i>/</i> 16"	5 x 1/4" x 19'-8 9/16"	5 x 1/4" x 19'-6 3/16" 5 x 1/4" x 17'-7 3/8"
RF3-3/RF3-4	19'-3 5 <i>/</i> 16"	13.9/22.0	0.188 2'-	'–8 9/16" –0 5/16"	l 5 x 1/4" x 1'–2 13/16"	5 x 1'/4" x 17'-7 3'/8"
,	1 '	22.0/10.0	0.135 17	'-7 1 <i>/</i> 16"	5 x 1/4" x 19'-2 11/16"	, ,
EB-4	1'-8 1/8"	W8x10		,		

CONNE	CTION TABLE	
ID.	Qty.	MARK
1	10	CI -190





CANOPY SUPPORT BEAM ELEVATION

WOODEN CANOPY ONLY ON FRAME LINE-4

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES	
Α	FOR PERMIT	07.21.2025	KUM	JSK	GFA	
0	FOR CONSTRUCTION	07.28.2025	KUM	JSK	GFA	
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	Α	A FOR PERMIT	A FOR PERMIT 07.21.2025	A FOR PERMIT 07.21.2025 KUM	A FOR PERMIT 07.21.2025 KUM JSK	A FOR PERMIT 07.21.2025 KUM JSK GFA

///
Federal Steel
Federal Steel Systems

	DESCR:	IPTION	RIGID	FRAME	ELEV	ATION				
	BUYER	/ CUSTOMER		DANIEL WHITE						SEA DR
	END U	SER		DANIEL WHITE						PRE
	END U	SE	DTHER							DES
al	STREE	Т	499 PARTIN RD							FOU
	CITY,	STATE, ZIP	DUNN, NC 28339						MEC	
	COUNT	Υ	HARNETT						THE	
	JDB#	J-113945	J-113945	SCALE	N.T.S.	DWG#	E8	DF	E15	ריין [

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT FSS ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT, ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY THE FABRICATOR IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS AND / OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN THE FABRICATOR ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

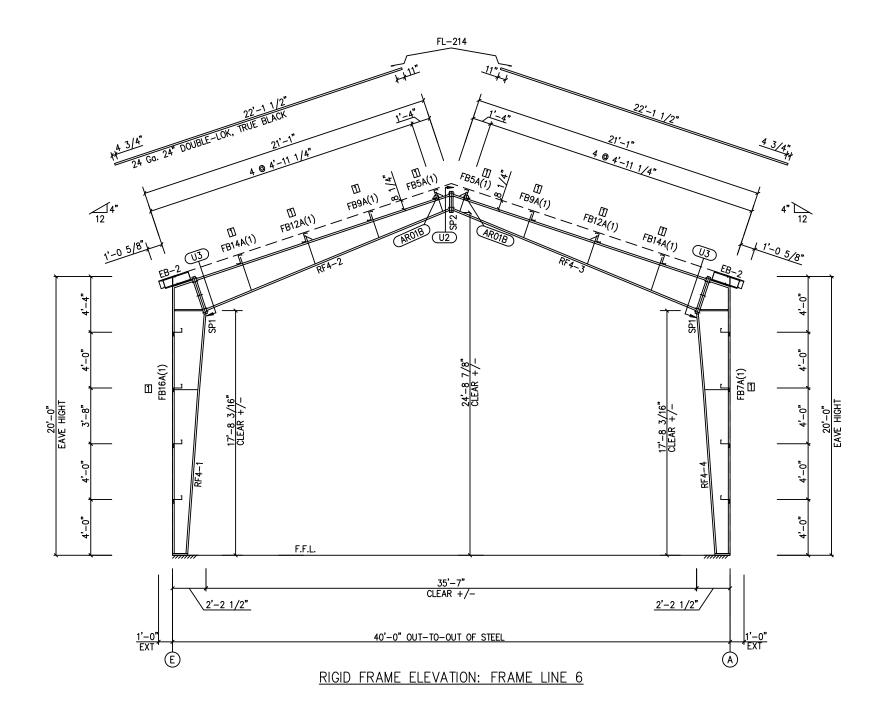
FOR CONSTRUCTION

SPLICE PLA	ATE &	BOLT	TAB	LE						Т
	Qty									1
Mark	Top	Bot	Int	Туре	Dia	Length	Width	Thick	Length	
SP1	4	4	2	A325	0.625	2.25	6"	5/8"	2'-9 5/16"	1
SP2	4	4	0	A325	0.625	2.25	6"	1/2"	1'-5 3/4"	

FLANGE BRACES: Both Sides(U.N.) FBxxA(1) A - 2X2X14GA

MEMBER TABLE							
Web Depth Web Plate O		Outside Flange	Inside Flange				
Mark	Length	Start/End	Thick Length	W x Thk x Length	W x Thk x Length		
RF4-1	19'-3 5/16"	10.0/20.0 20.0/26.0	0.135 11'-6 3/8" 0.135 5'-9 3/4"	5 x 1/4" x 19'-2 11/16" 5 x 1/4" x 1'-5 11/16"	5 x 1/4" x 11 -6 11/16" 5 x 1/4" x 5'-10"		
	,	20.0/26.0	0.135 5'-9 3/4"	5 x 1/4" x 1'-5 11/16"	5 x 1/4" x 5'-10"		
		26.0/16.6	0.188 2'-4 1/8"	'			
RF4-2	19'–6 11 <i>/</i> 16"	26.0/10.0	0.135 19'-5 7/16"	5 x 1/4" x 19'-5 7/16"	5 x 1/4" x 19'-2 9/16"		
RF4-2 RF4-3 RF4-4	19'-6 11/16" 19'-3 5/16"	10.0/26.0	0.135 19'-5 7/16"	1 5 v 1 //" v 10'-5 7 /16"	5 x 1/4" x 19'-2 9/16" 5 x 1/4" x 19'-2 9/16" 5 x 1/4" x 5'-10"		
RF4-4	19'–3 5 <i>/</i> 16"	16.6/26.0 26.0/20.0	0.188 2'-4 1/8"	I 5 x 1/4" x 1'-5 11/16"	5 x 1/4" x 5'-10" ′		
		26.0/20.0	0.135 5'-9 3/4"	5 x 1/4" x 19'-2 11/16"	5 x 1/4" x 11'-6 11/16"		
		20.0/10.0	0.188 2'-4 1/8" 0.135 19'-5 7/16" 0.135 19'-5 7/16" 0.188 2'-4 1/8" 0.135 5'-9 3/4" 0.135 11'-6 3/8"		/ /		
כם י	1'_11"	Weylo		I			

	CONNECTI	ON TABLE	
	ID.	Qty.	MARK
П	1	10	CL-190



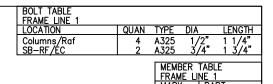
ISSUE	DESCRIPTION	DATE	DRN	СНК	DES	
Α	FOR PERMIT	07.21.2025	KUM	JSK	GFA	
0	FOR CONSTRUCTION	07.28.2025	KUM	JSK	GFA	
						L



	DESCR	IPTION RI	GID FRAME	GID FRAME ELEVATION						SEA
	BUYER	/ CUSTOMER		DANIEL WHITE						
	END U	SER		DANIEL WHITE						ENG PRO
	END U	SE	DTHER							DES FUR
	STREE	Т		499 PARTIN RD						FOU
	CITY,	STATE, ZIP	DUNN, NC 28339							MEC PAR
	COUNT	Υ		HARNETT						THE
	JOB#	J-113945	J-113945	SCALE	N.T.S.	DWG#	E9	OF	E15	IND

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT FSS ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY THE FABRICATION IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS AND / OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN THE FABRICATOR ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.



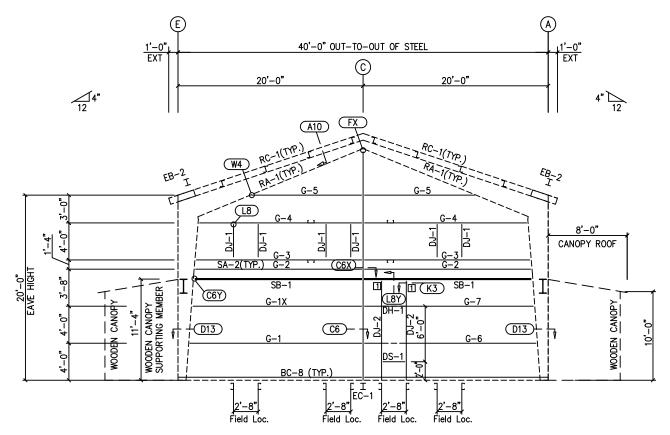


	R TABLE
	LINE 1
MARK	PART
SB-1	W8x18
EB-2	W8x10
EC-1	W8x21
DJ-1	8X25C16
DJ-2	8X25C16
DH-1	8x25C16
DS-1	8x25C16
G-1	8x25Z14
G-1X	(8x25Z14
G-2	8x25Z16
G-3	8x25C16
G-4	8x25C14
G-5	8x25Z16
G-6	8x25Z16
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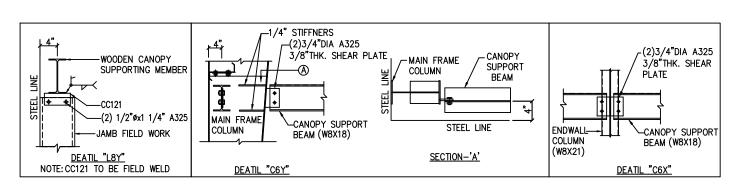
			G-7 8X23Z16
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4" ______

WOODEN CANOPY







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al	STREE	Т	499 PARTIN RD							FC
	CITY,	STATE, ZIP	DUNN, NC 28339							ME PA
	COUNT	Υ	HARNETT							TH
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12 4"

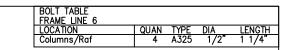
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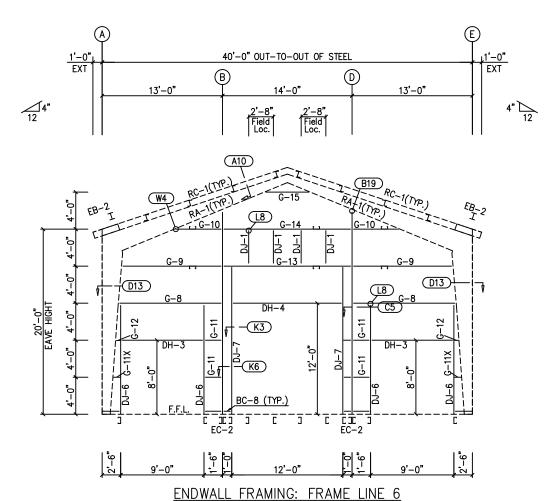
BOARD AND BATTEN PANELS BY OTHERS

ENDWALL: FRAME LINE 1

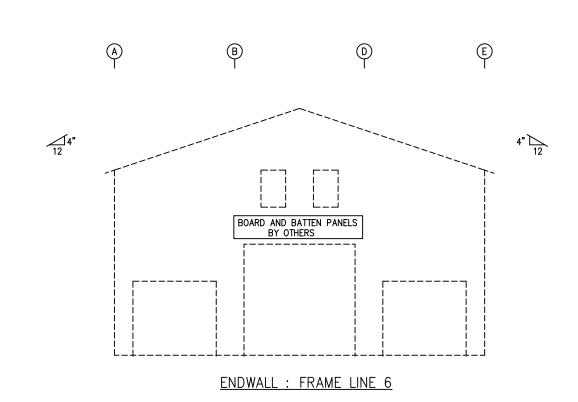
08/22/2025



MEMBER	TABLE
FRAME LI	NE 6
MARK	PART
EB-2	W8x10
EC-2	8X70D12
DJ-1	8X25C16
DJ-6	8X25C16
DJ-7	8X25C16
DH-3	8x25C16
DH-4	8x25C16
G-8	8x25Z16
G-9	8x25C16
G-10	8x25C16
G-11	8x25Z16
G-12	8x25Z16
G-11X	8x25Z16
G-13	8x25C16
G-14	8x25C16
G-15	8x25Z16



[WALL INSULATION: VRR 5" THK INSULATION (BY OTHERS)]

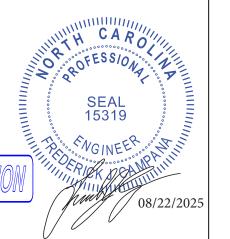


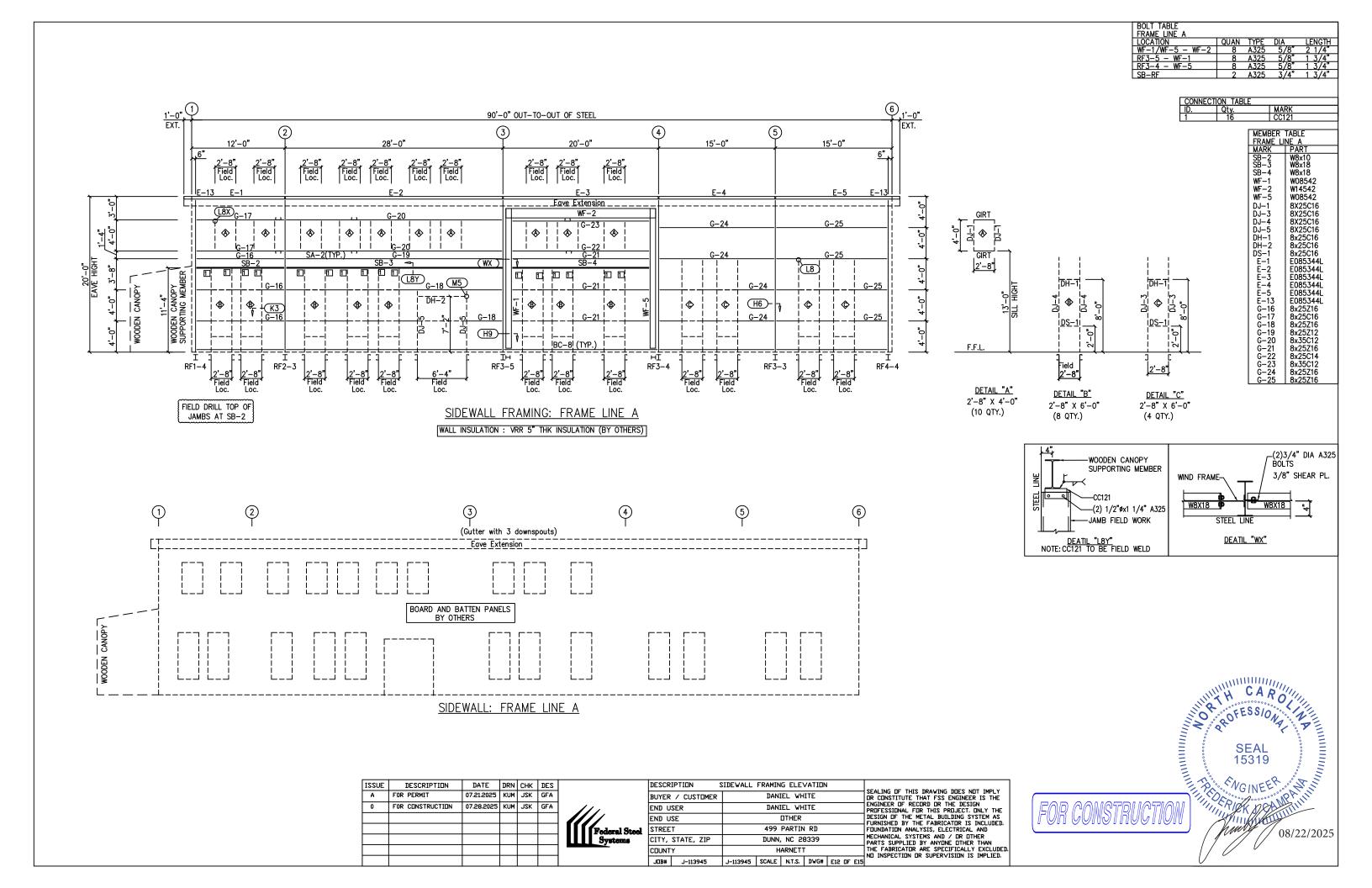
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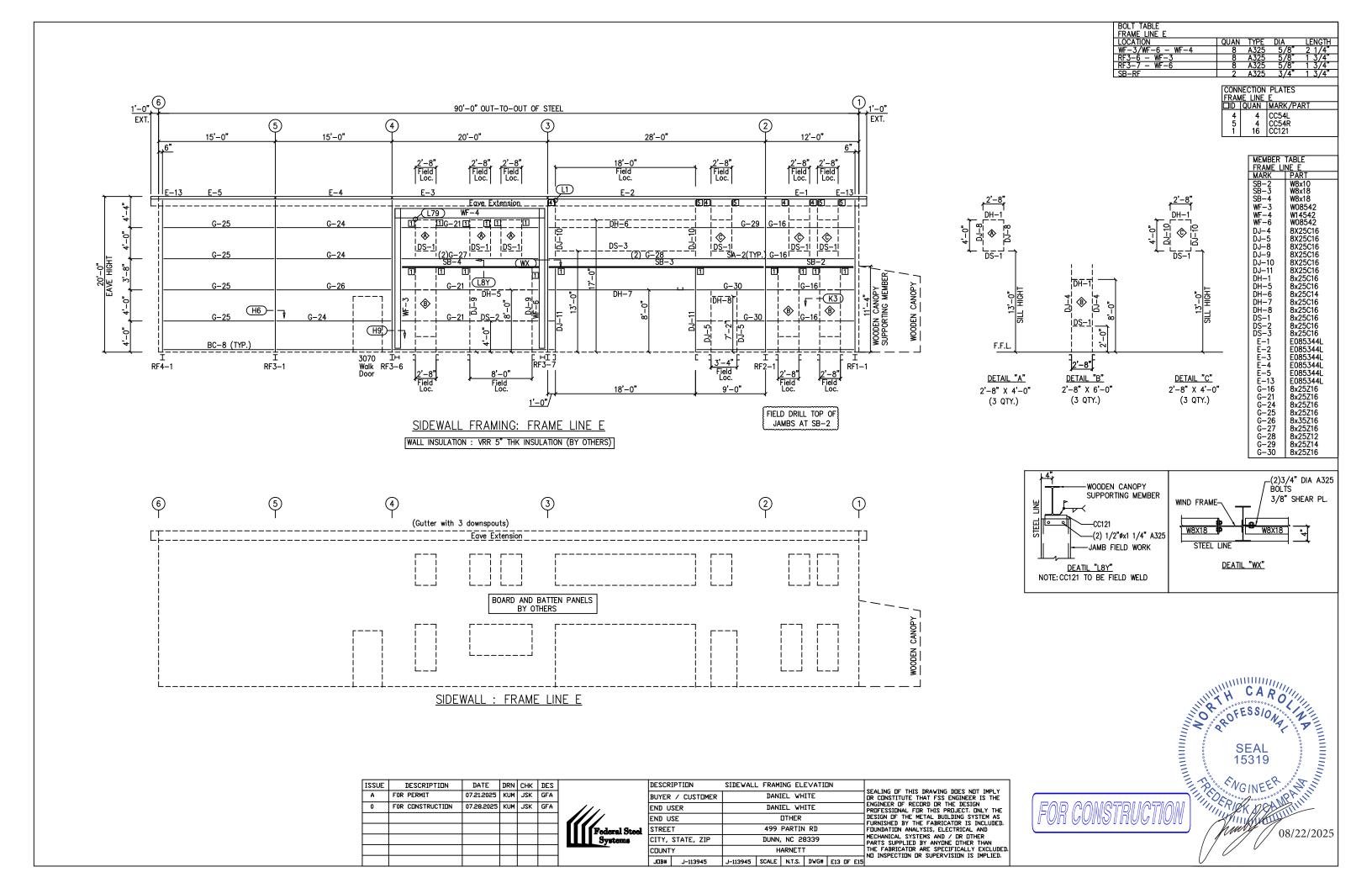


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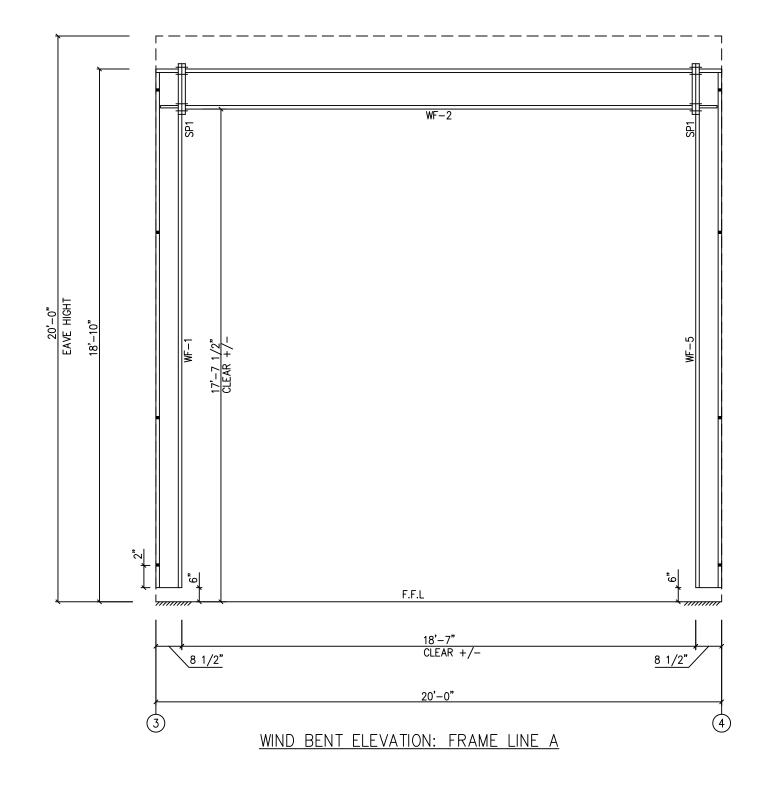






SPLICE PLATES & BOLTS								
Splice Mark	Quar	1		Bolt-		Pla	te Size	
Mark	Top/	Bot	Туре	Dia	Length	Width	Thick	Length
SP1	4	4	A325	0.625	2.25	6"	1/2"	1'-9 1/4"

MEMBER SIZE	E TABLE	
MARK	MEMBER	LENGTH
WF-2	W14542	18'-6 1/4"
WF-1	W08542	18'-4"
WF-5	W08542	18'-4"

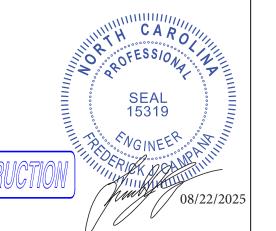


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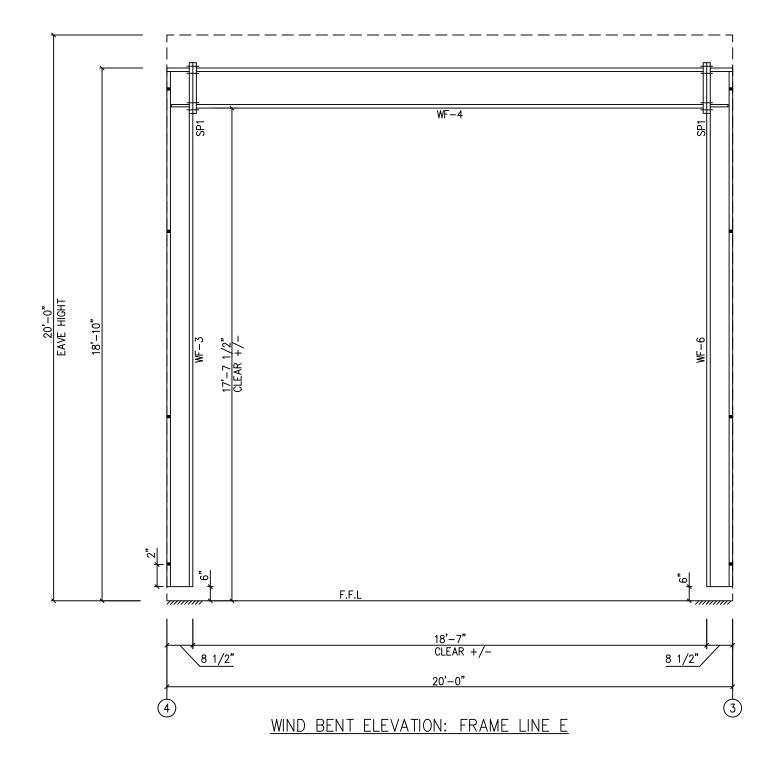
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COUNT	Y		Н	THE FABRICATOR AR					
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Splice Mark	Quan			Bolt-		Plat	te Size		
Mark	Top/	Bot	Туре	Dia	Length	Width	Thick	Length	
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MEMBER SIZI	E TABLE	
MARK	MEMBER	LENGTH
WF-4	W14542	18'-6 1/4"
WF-3	W08542	18'-4"
WF-6	W08542	18'-4"



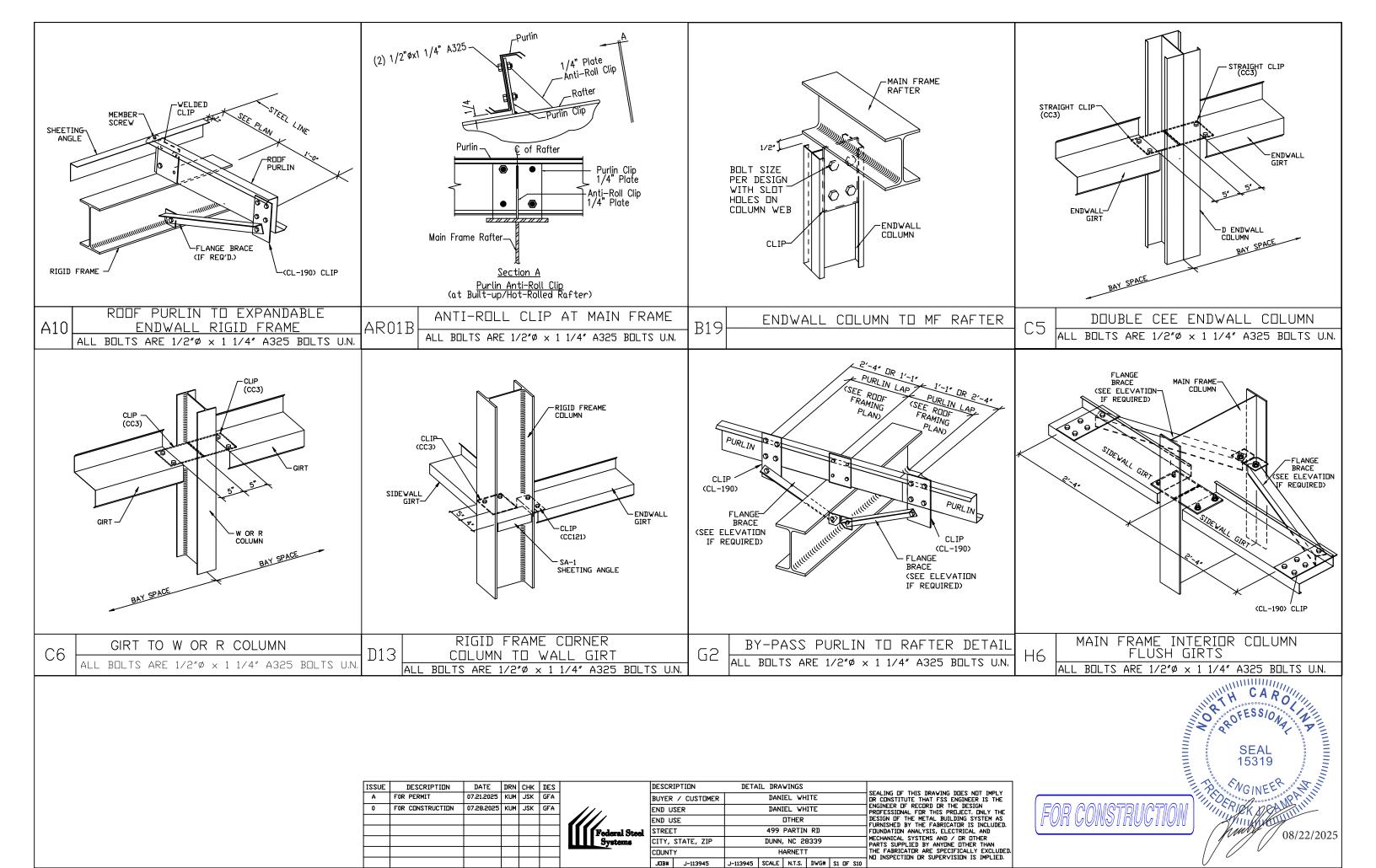
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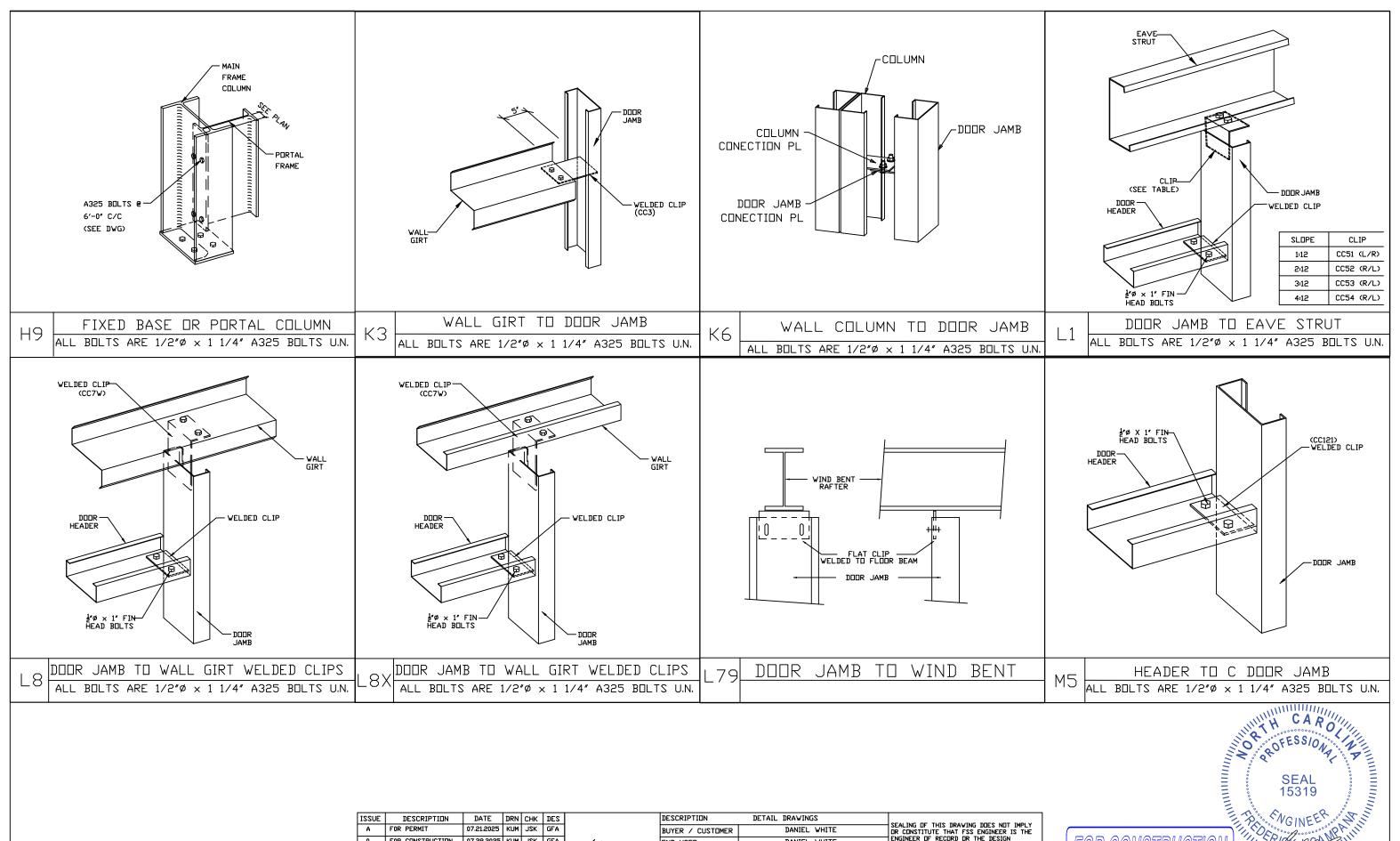


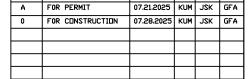
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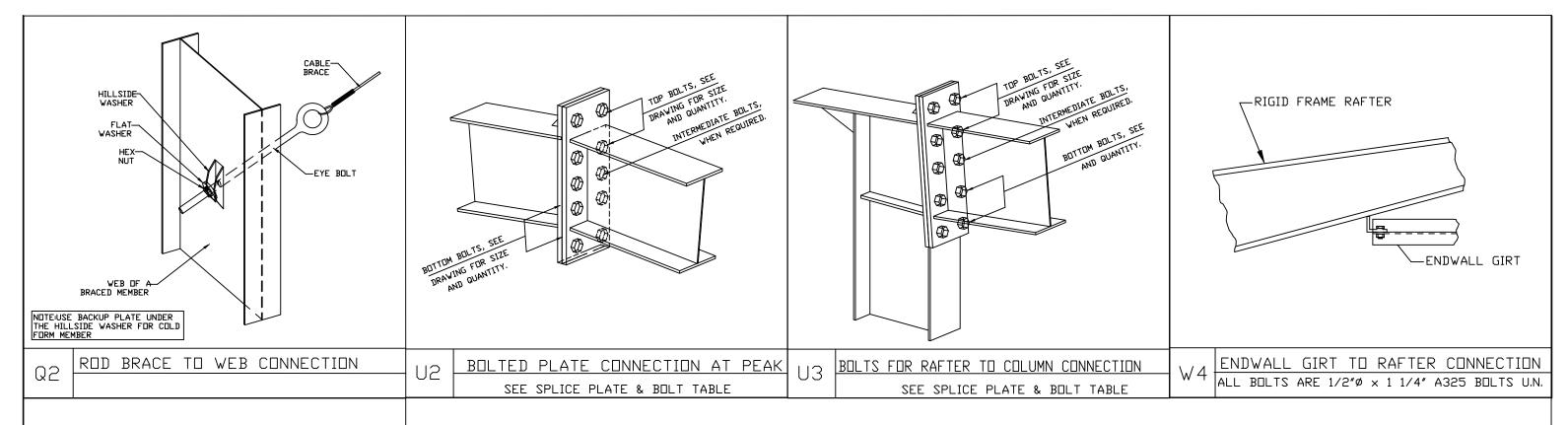


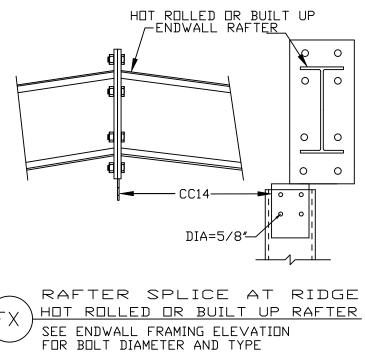






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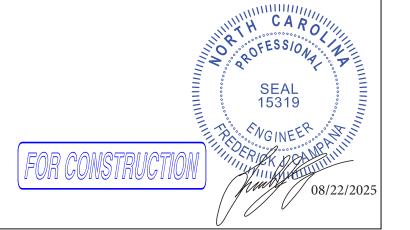


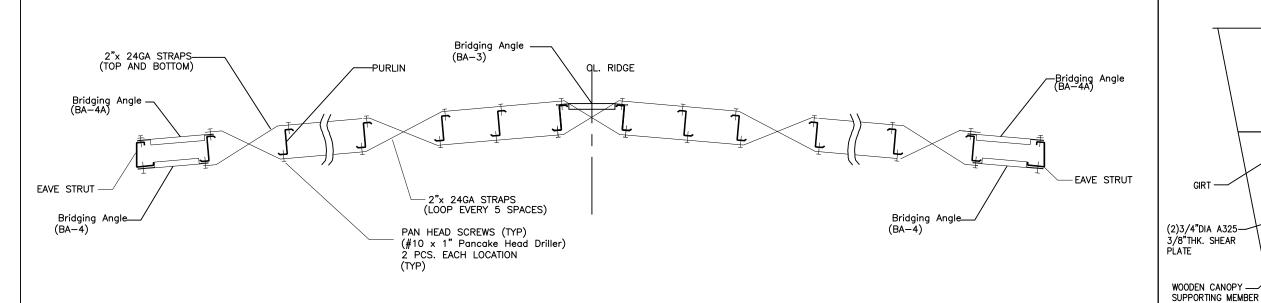


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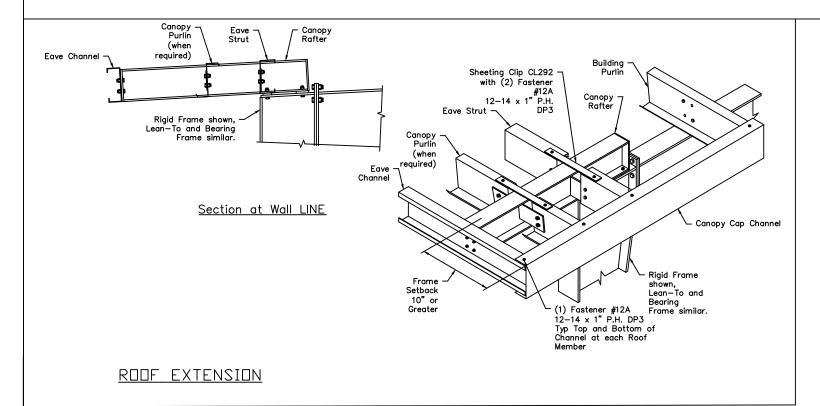


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SAG STRAP DETAIL @ ROOF W/ BRIDGING ANGLE (STANDING SEAM) (BRACED TOP & BOTTOM)



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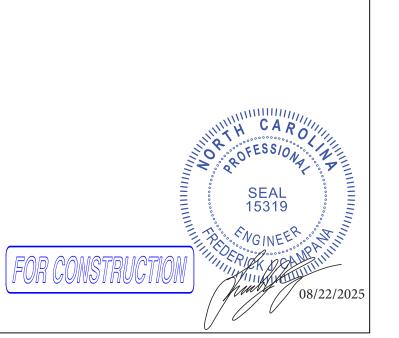


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(W8X10) (W8X18)

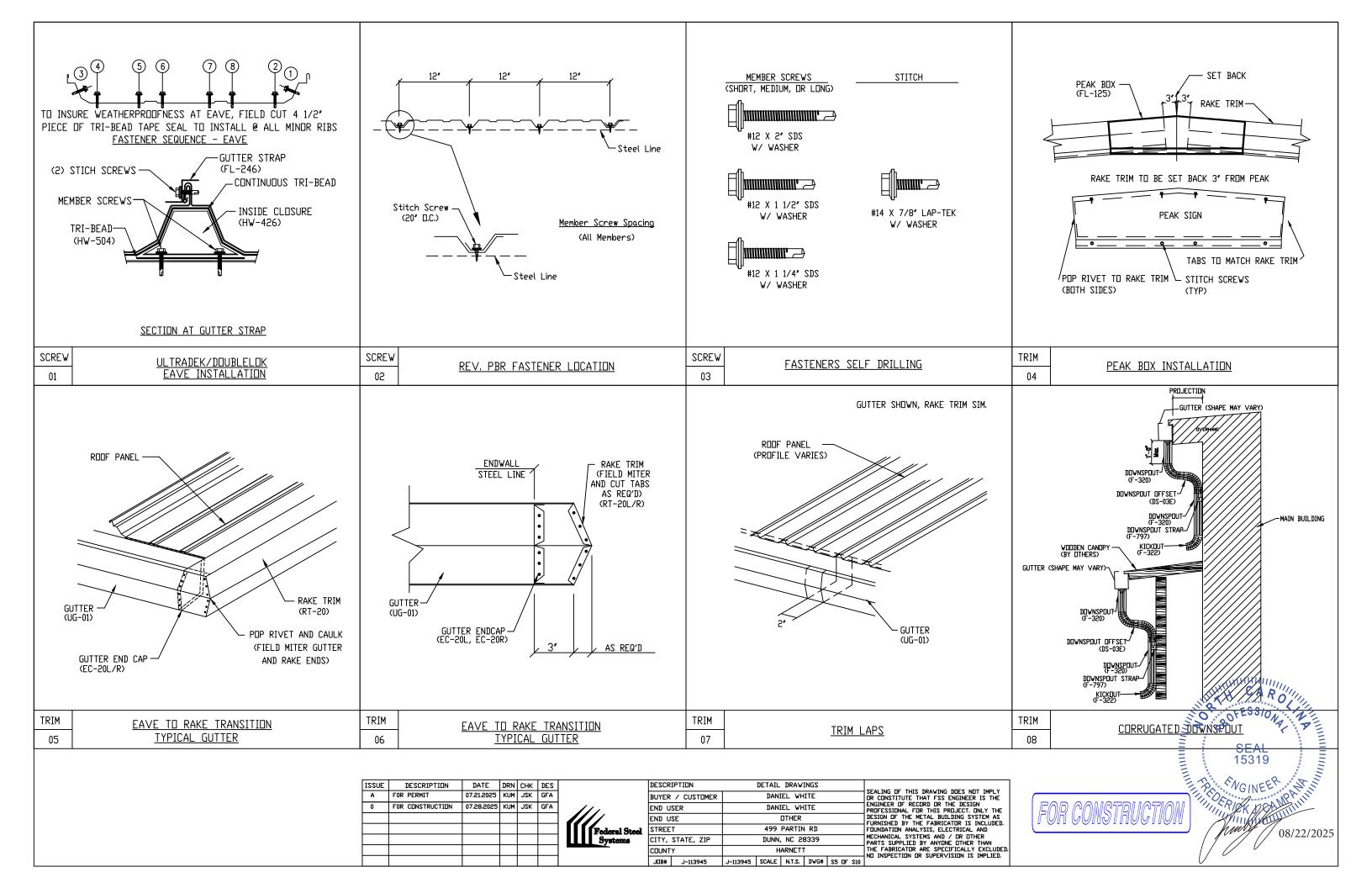


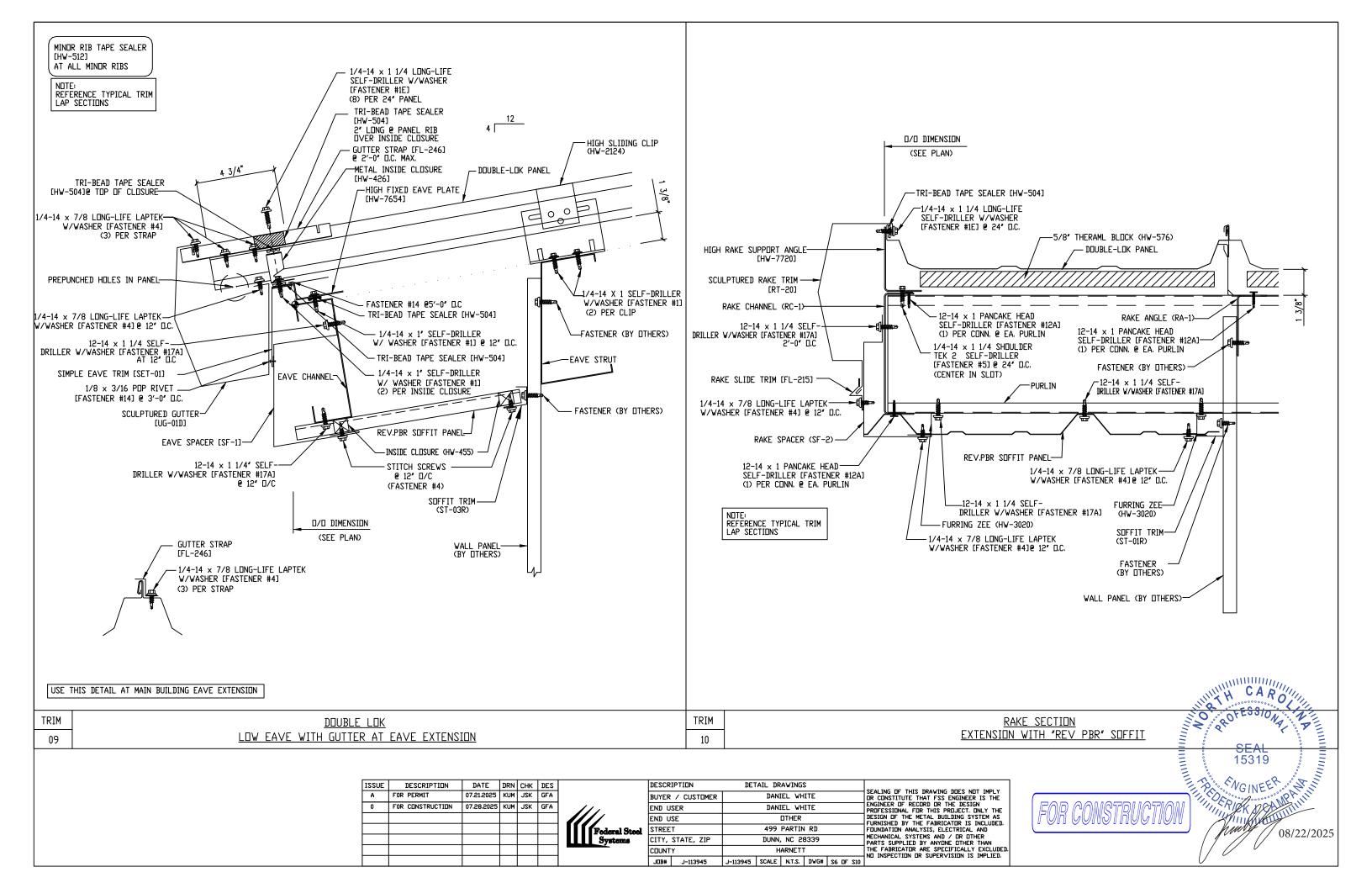
WOODEN CANOPY SUPPORTING MEMBER

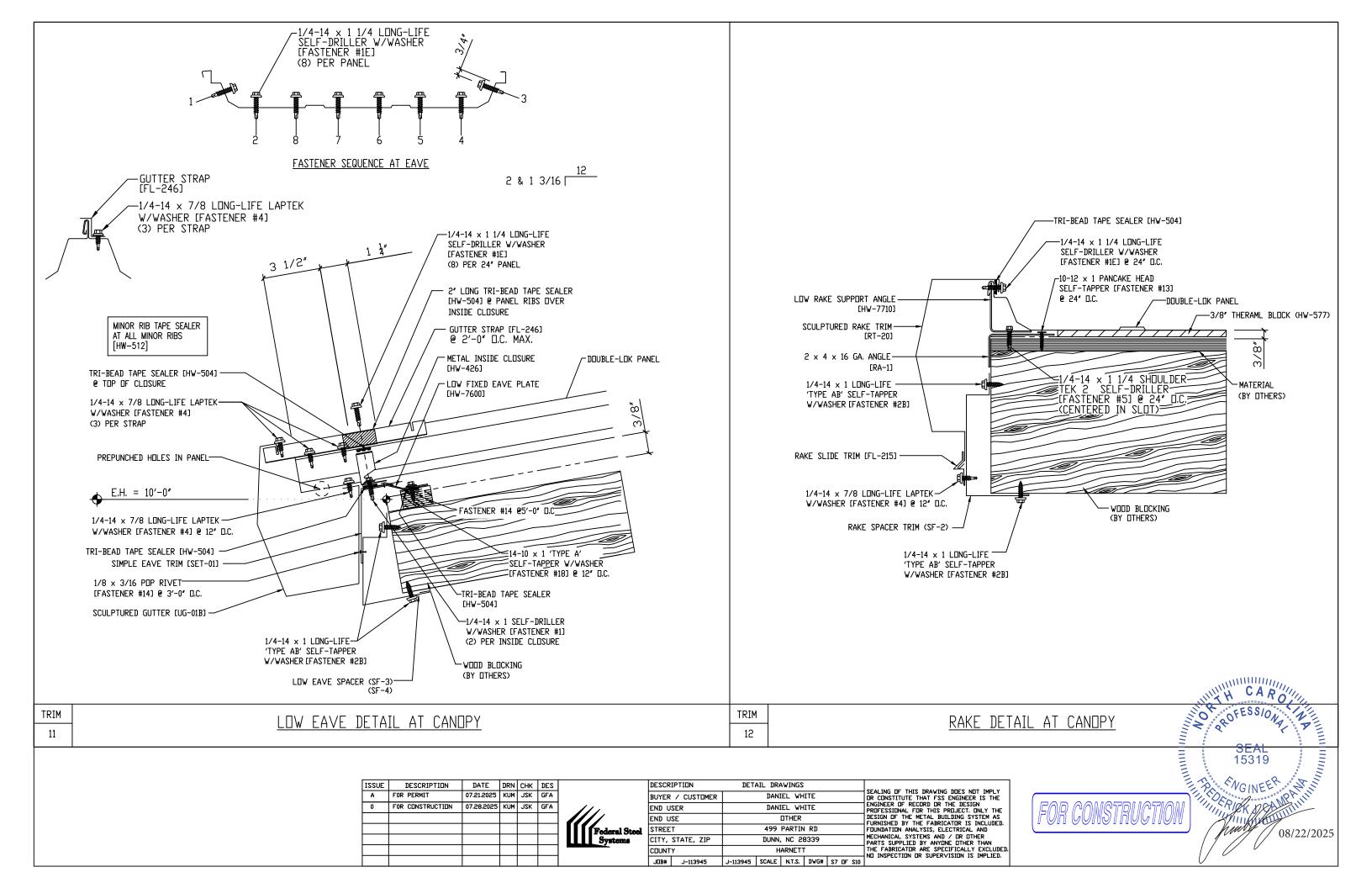
SHEETING ANGLE (SA-2)

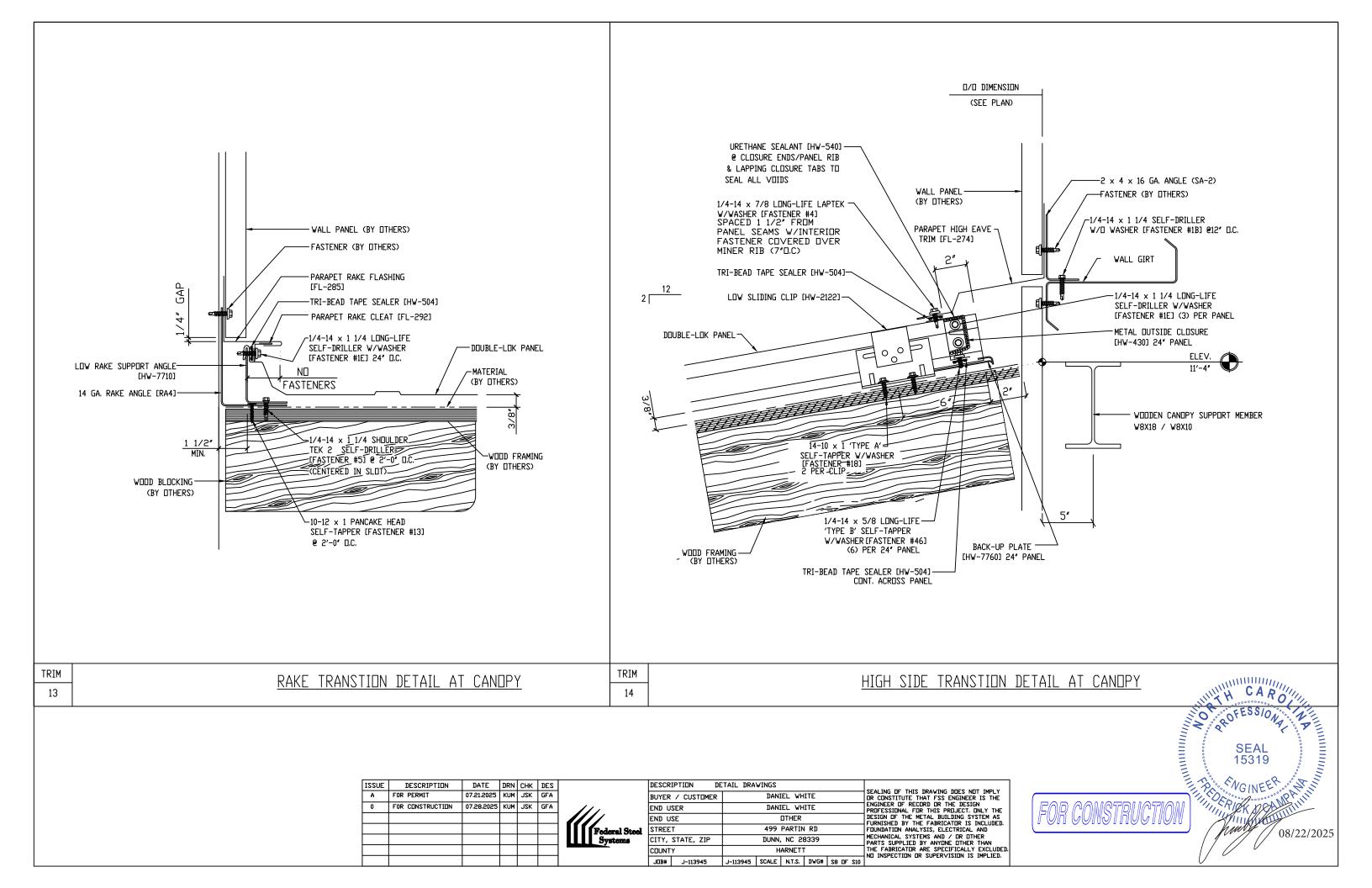
WOODEN FRAMING BY

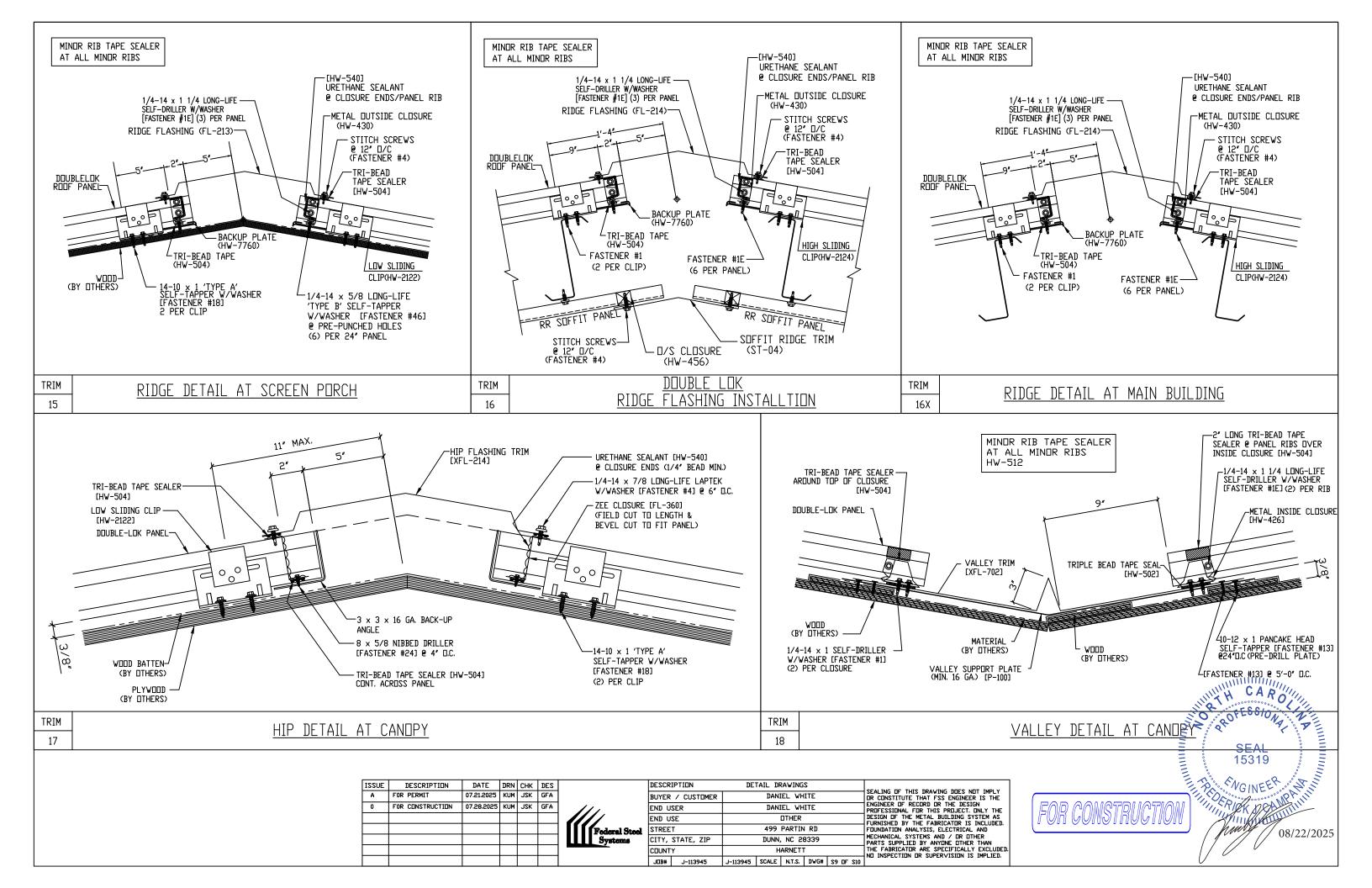
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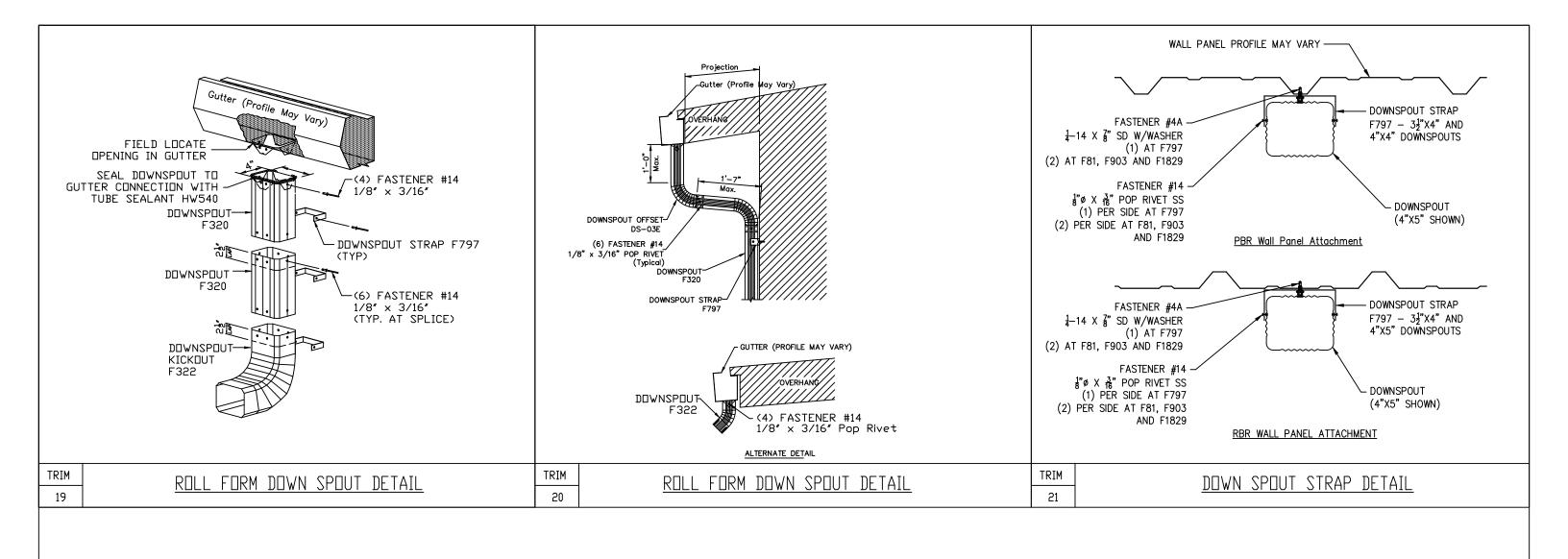












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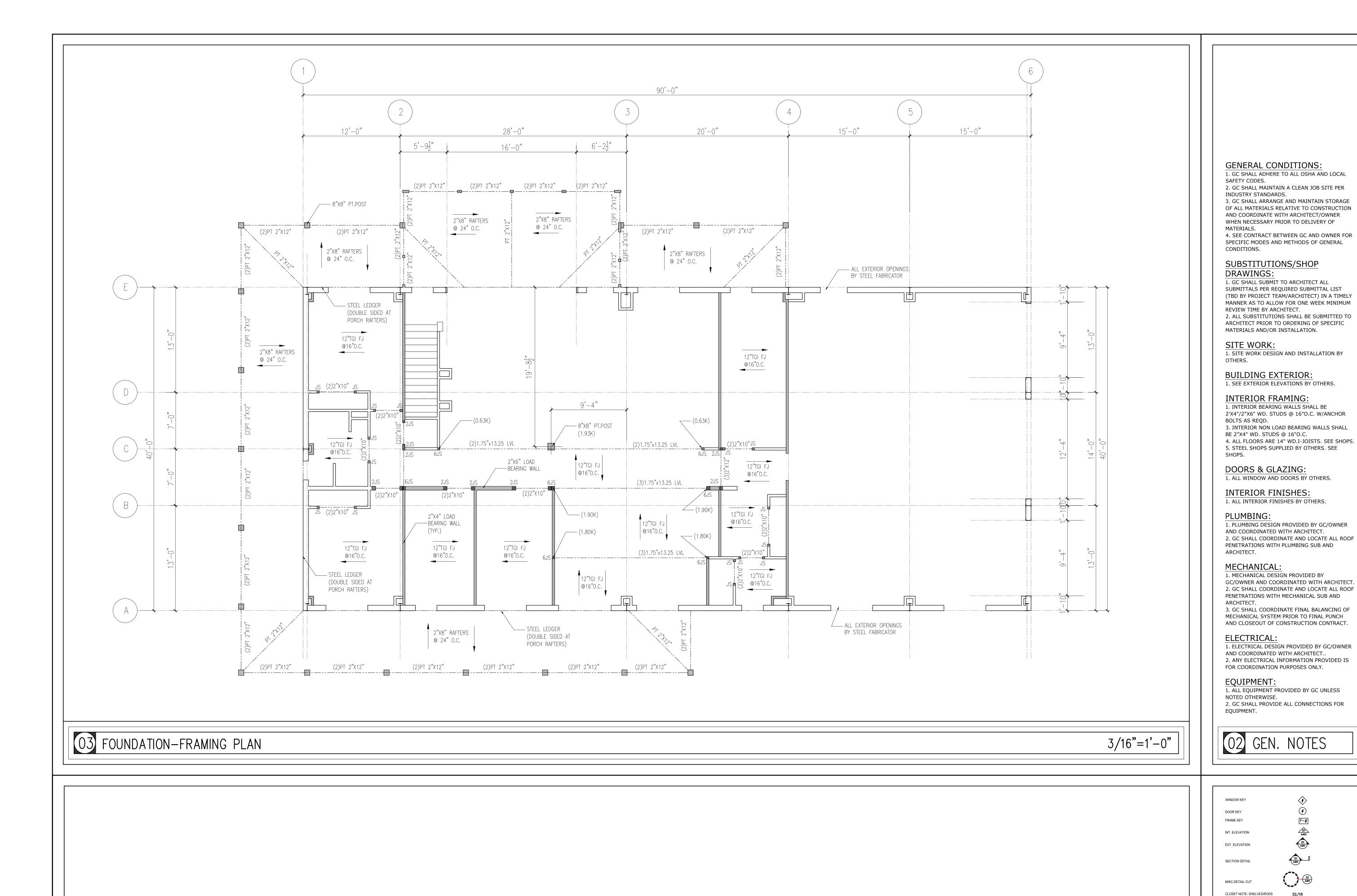
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COUNTY			HARNETT						THE FABRICATOR ARE SPECIFICALLY EXCLUDE NO INSPECTION OR SUPERVISION IS IMPLIED.			
	JDB#	J-113945	J-113945	SCALE	N.T.S.	DWG#	S10)F	S10			

SEAL 15319

FOR CONSTRUCTION

08/22/2025



Michael Morse, Architect
2310 Weymouth Court, Raleigh, NC 27612
Contact Person: Michael Morse
(p) 919-889-2305



/HITE RESIDENCE

New Single-Family House

Revisions

8-19-25

ELEVATION CHANGE

RATED ASSEMBLY. (SEE PLAN KEY FOR TYPE)

MAIN STRUCTURAL AXIS OR CL.

01 SYMBOLS KEY

UPPER FLOOR FRAMING PLAN
A001

1 of I

PERMIT

REFER TO ELEV.

nce nce

White Residence

Daniel & Carla White 895 November Lane Willow Spring, NC 27592

ELEVATIONS

Sheet Numb

Project #:

25-185

Date:
6-6-25

Drawn/Design By:
KBB

Scale:
1/4"=1'-0"

REVIEWER'S SEAL

Lent Jutters

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White Residence

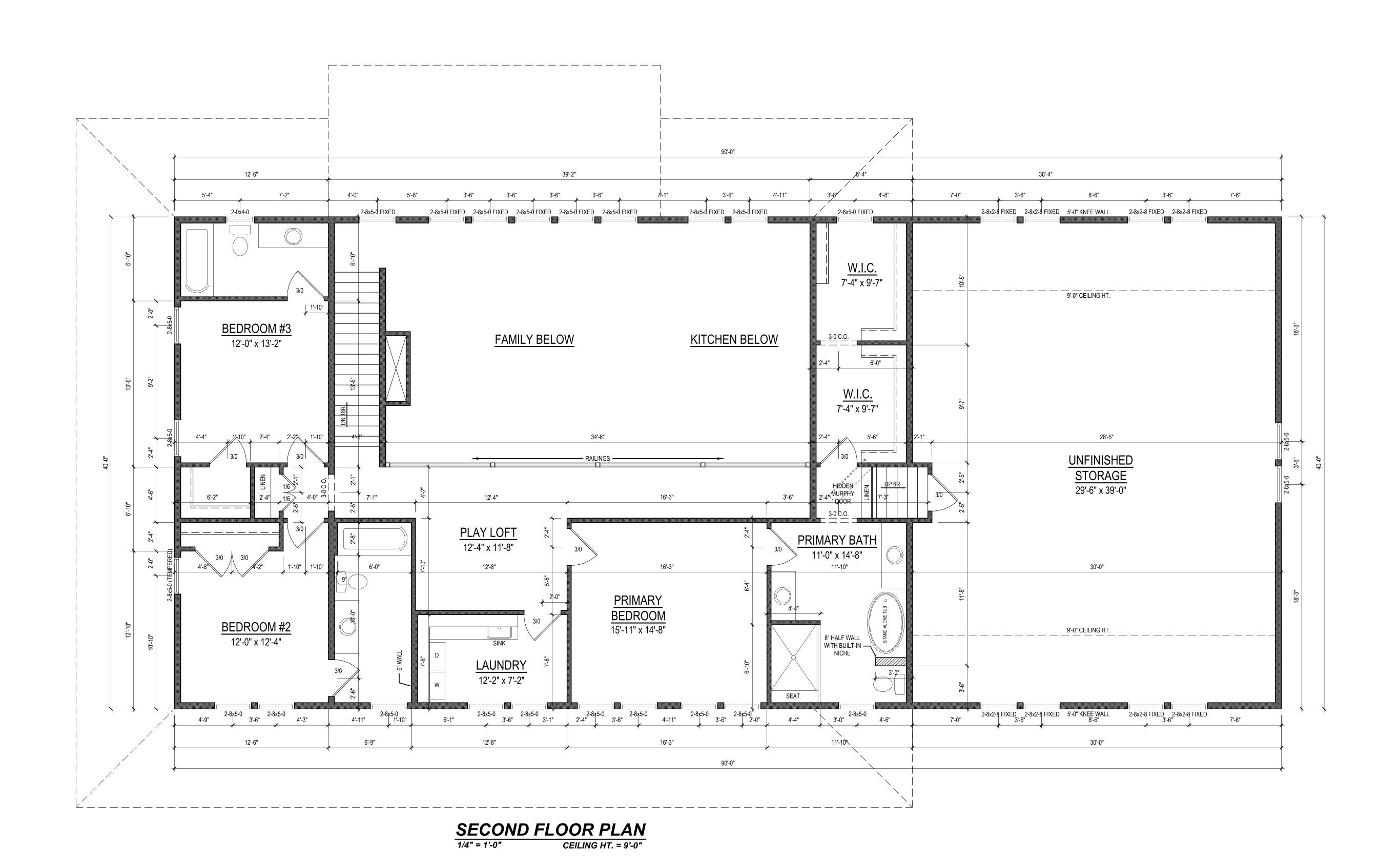
> Daniel & Carla White 895 November Lane Willow Spring, NC 27592

FIRST FLOOR

Sheet Number

REVIEWER'S SEAL

Sheet Number



2. ALL WALLS SHOWN ON THE FLOOR PLANS ARE DRAWN AT 4" UNLESS NOTED OTHERWISE.

3. ALL ANGLED WALL SHOWN ON THE PLANS ARE 45 DEGREES UNLESS NOTED

4. STUD WALL DESIGN SHALL CONFORM TO ALL NORTH CAROLINA STATE BUILDING CODE REQUIREMENTS.

5. DO NOT SCALE PLANS, DRAWING SCALE MAY BE DISTORTED DUE TO COPIER IMPERFECTIONS.

6. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH NORTH CAROLINA RESIDENTIAL STATE BUILDING CODE, 2018 EDITION.

SQUARE FOOTAGE

HEATED SQUARE F	<u>OOTAGE</u>	UNHEATED SQUARE FOOTAGE				
FIRST FLOOR=	2400	GARAGE=	1200			
SECOND FLOOR=	1640	COVERED PORCH=	1192			
THIRD FLOOR=	N/A	SCREEN PORCH=	333			
BASEMENT=	N/A	DECK=	N/A			
		STORAGE=	1192			

TOTAL UNHEATED= 3917 TOTAL HEATED= 4040

CRAWL SPACE VENTILATION CALCULATIONS

-VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON THE PLAN BUT SHOULD BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS TO PREVENT DEAD AIR POCKETS.

-100% VAPOR BARRIER MUST BE PROVIDED WITH 12" MIN. LAP JOINTS.

-THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 AS LONG AS REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS-VENTILATION OF THE SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. (COMPLY WITH NC CODE MIN. WITH REGARD TO VENT PLACEMENT FROM CORNERS)

SQ. FT. OF CRAWL SPACE/1500

N/A SQ. FT. OF REQUIRED VENTILATION

PROVIDED BY: N/A VENTS AT 0.45 SQ. FT. NET FREE

VENTILATION EACH= N/A SQ. FT. OF VENTILATION

**FOUNDATION DRAINAGE- WATERPROOFING PER SECTIONS 405 & 406.

ATTIC VENTILATION CALCULATIONS

- CALCULATIONS SHOWN BELOW ARE BASED ON VENTILATORS USED AT LEAST 3 FT. ABOVE THE CORNICE VENTS WITH THE BALANCE OF VENTIALTION PROVIDED BE EAVE VENTS.

- CATHEDRAL CEILINGS SHALL HAVE A MIN. 1" CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

5125 SQ. FT. OF ATTIC/300= 17.08

EACH OF INLET AND OUTLET REQUIRED.

*WALL AND ROOF CLADDING DESIGN VALUES

- WALL CLADDING IS DESIGNED FOR A 24.1 SQ. FT. OR GREATER POSITIVE AND NEGATIVE PRESSURE.

- ROOF VALUES BOTH POSITVE AND NEGATIVE SHALL BE AS FOLLOWS:

45.5 LBS. PER SQ. FT. FOR ROOF PITCHES OF 0/12 TO 2.25/12

34.8 LBS. PER SQ. FT. FOR ROOF PITCHES OF 2.25/12 TO 7/12

21 LBS. PER SQ. FT. FOR ROOF PITCHES OF 7/12 TO 12/12

** MEAN ROOF HEIGHT 30' OR LESS

STRUCTURAL NOTES

I) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE". IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

2) DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION (DL & LL)			
ALL FLOORS	40	10	L/360			
ATTIC (pull down access)	20	10	L/240			
ATTIC (no access)	10	5	L/240			
EXTERNAL BALCONY	60	10	L/360			
ROOF	20	10	L/180			
ROOF TRUSS	20	20	L/240			
WIND LOAD	[BASED ON 120 MPH (3-second gusts)]					

3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF

4) CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE (UNO).

5) MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC RESIDENTIAL BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT

6) ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI) UNO.

ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.

7) ALL LOAD BEARING HEADERS SHALL BE (2)2x10 (UNO). ALL WINDOW AND DOOR HEADERS SHALL BE SUPPORTED BY (1) JACK STUD AND (1) KING STUD AT EACH END UNLESS NOTED. ALL OTHER BEAMS SHALL BE SUPPORTED BY 2 STUDS OR THE AMOUNT OF STUDS REQUIRED FOR FULL BEARING AT EACH END UNLESS NOTED. POINT LOADS (STIFF KNEES, ETC.) SHALL CONSIST OF 2 STUDS UNLESS NOTED. ALL SUPPORTS OF 2 STUDS OR MORE SHALL BE TRANSFERRED THROUGH EACH FLOOR TO THE FOUNDATION.

8) ALL EXTERIOR WALLS TO BE SHEATHED WITH MIN. 7/16" WOOD STRUCTURAL PANELS FASTNED WITH 8D NAILS 6" O.C. AT EDGES AND 12" O.C. AT INT. SUPPORTS. BLOCKING SHALL BE INSTALLED IF LESS THAN 50 PERCENT OF THE WALL LENGTH IS SHEATHED. WHERE BLOCKING IS REQ'D, ALL PANELS SHALL BE FASTENED AT 3" O.C AT EDGES AND 6" O.C. AT INT. SUPPORTS.

9) ALL STRUCTURAL STEEL SHALL ASTM A-36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1\2" INCHES AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2 DIAMETER AND 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDING THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.

10) ANCHOR BOLT PLACEMENT PER SECTION R403.1.6. 1/2" DIAMETER ANCHOR BOLTS SPACED AT 6'-0" O/C AND PLACED 12" FROM THE END OF EACH PLATE SECTION

11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF 2018 NC RESIDENTIAL BUILDING CODE

12) WALL AND ROOF CLADDING VALUES: WALL CLADDING SHALL BE DESIGNED FOR A 24.1 SQ.FT. OR GREATER POSITIVE AND NEGATIVE PRESSURE

ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

45.5 LBS/SQFT FOR ROOF PITCHES OF 0/12 TO 2.25/12 34.8 LBS/SQFT FOR ROOF PITCHES OF 2.25/12 TO 7/12

21.0 LBS/SQFT FOR ROOF PITCHES OF 7/12 TO 12/12

HEARTH SLAB THICKNESS

HEARTH EXTENSION

(EACH SIDE OF OPENING)

HEARTH EXTENSION

(FRONT OF OPENING)

HEARTH REINFORCING

THICKNESS OF WALL OF FIREBOX

DISTANCE FROM TOP OF OPENING TO THROAT

SMOKE CHAMBER WALL THICKNESS **UNLINED WALLS**

CHIMNEY

VERTICAL REINFORCING

HORIZONTAL REINFORCING

BOND BEAMS

FIREPLACE LINTEL

CHIMNEY WALLS WITH FLUE LINING

DISTANCE BETWEEN ADJACENT FLUES

EFFECTIVE FLUE AREA

(BASED ON AREA OF FIREPLACE OPENING)

CLEARANCES

COMBUSTIBLE MATERIAL

MANTEL AND TRIM

ABOVE ROOF

ANCHORAGE STRAP

NUMBER

EMBEDMENT INTO CHIMNEY

FASTEN TO BOLTS

FOOTING **THICKNESS**

2) NOT REQUIRED IN SEISMIC DESIGN CATEGORY A, B, or C

** MEAN ROOF HEIGHT 30' OR LESS

13) FOR ROOF SLOPES FROM 2:12 THROUGH 4:12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER

DESIGNER IS NOT RESPONSIBLE FOR DIMENSIONING OR SQ. FTG. ERRORS ONCE CONSTRUCTION BEGINS

 \mid 14) IT IS THE CONTRACTOR'S RESPONSIBLITY TO VERIFY ALL DIMENSIONS AND SQ. FTG. ARE CORRECT PRIOR TO CONSTRUCTION.

SUMMARY OF REQUIREMENTS FOR MASONRY FIREPLACES AND CHIMNEYS

REQUIREMENTS

8" FIREPLACE OPENING < 6 SQUARE FOOT

12" FIREPLACE OPENING < 6 SQUARE FOOT

16" FIREPLACE OPENING < 6 SQUARE FOOT

20" FIREPLACE OPENING < 6 SQUARE FOOT

REINFORCED TO CARRY ITS OWN WEIGHT AND ALL IMPOSED LOADS

10" SOLID BRICK or 8" WHERE A FIREBRICK LINING IS USED

JOINTS IN FIREBRICK 1/4" MAXIMUM

FOUR NO. 4 FULL-LENGTH BARS FOR CHIMNEY UP TO 40" WIDE

ADD TWO NO. 4 BARS FOR EACH ADDITIONAL 40" or FRACTION OF

WIDTH or EACH ADDITIONAL FLUE.

1/4" TIES AT 18" AND TWO TIES AT EACH BEND IN VERTICAL STEEL

NO SPECIFIED REQUIREMENTS

NONCOMBUSTIBLE MATERIAL

SOLID MASONRY UNITS or HOLLOW MASONRY UNITS GROUTED

SOLID WITH NOT LESS THAN 4-INCH NOMINAL THICKNESS

SEE SECTION R1003.13

SEE SECTION R1003.15

SEE SECTION R1001.11 AND R1003.18

SEE SECTION R1001.11, EXCEPTION 4

3' AT ROOFLINE AND 2' AT 10'

12" HOOKED AROUND OUTER BAR WITH 6" EXTENSION

4 JOISTS

THREE 1/2" DIAMETER

12" EACH SIDE OF FIREPLACE WALL

LETTER

K

NOTE: THIS TABLE PROVIDES A SUMMARY OF MAJOR REQUIREMENTS FOR THE CONSTRUCTION OF MASONARY CHIMNEYS

AND FIREPLACES. LETTER REFERENCES ARE TO FIGURE R1001.1(NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE), WHICH SHOWS EXAMPLES OF TYPICAL CONSTRUCTION. THIS TABLE DOES NOT COVER ALL REQUIREMENTS, NOR DOES IT COVER ALL ASPECTS OF THE INDICATED REQUIREMENTS. FOR THE ACTUAL MANDATORY REQUIREMENTS OF THE CODE, SEE

THE INDICATED SECTION OF TEXT.

1) THE LETTERS REFER TO FIGURE R1001.1 OF THE NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE

R308.4.5 GLAZING & WET SURFACES:

GLAZING IN WALLS, ENCLOSURES OR FENCES CONTAINING or FACING HOT TUBS, SPAS, WHIRLPOOLS, SAUNAS, STEAM ROOMS, BATHTUBS, SHOWERS AND INDOOR or OUTDOOR SWIMMING POOLS WHERE THE BOTTOM EXPOSED EDGE OF THE GLAZING IS LESS THAN 60 INCHES, MEASURED VERTICALLY ABOVE ANY STANDING or WALKING SURFACE SHALL BE CONSIDERED TO BE A HAZARDOUS LOCATION. THIS SHALL APPLY TO SINGLE GLAZING AND EACH PANE IN MULTIPLE GLAZING.

EXCEPTION: GLAZING THAT IS MORE THAN 60 INCHES, MEASURED HORIZONTALLY AND IN A STRAIGHT LINE, FROM THE WATER'S EDGE OF A BATHTUB, HOT TUB, SPA, WHIRLPOOL or SWIMMING POOL or FROM THE EDGE OF A SHOWER, SAUNA or STEAM ROOM

AN ATTIC ACCESS OPENING SHALL BE PROVIDED TO ATTIC AREAS THAT EXCEED 400 SQUARE FEET (37.16 M²) AND HAVE A VERTICAL HEIGHT OF 60 INCHES (1524 MM) OR GREATER. THE NET CLEAR OPENING SHALL NOT BE LESS THAN 20 INCHES (508 MM BY 762 MM) AND SHALL BE LOCATED IN A HALLWAY OF OTHER READILY ACCESSIBLE LOCATION. A 30-INCH (762 MM) MINIMUM UNOBSTRUCTED HEADROOM IN THE ATTIC SPACE SHALL BE PROVIDED AT SOME POINT ABOVE THE ACCESS OPENING. SEE SECTION M1305.1.3 FOR ACCESS REQUIREMENTS WHERE MECHANICAL EQUIPMENT IS LOCATED IN ATTICS.

EXCEPTION:1) CONCEALED AREAS NOT LOCATED OVER THE MAIN STRUCTURE INCLUDING PORCHES, AREAS BEHIND KNEE WALLS, DORMERS, BAY WINDOWS, ETC. ARE NOT REQUIRED TO

2) PULL DOWN STAIR TREADS, STRINGERS, HANDRAILS, AND HARDWARE MAY PROTRUDE INTO THE NET CLEAR OPENING

DWELLING / GARAGE SEPARATION (SECTION R302.5, R302.6 and R302.7):

WALLS - A MINIMUM 1/2" GYPSUM BOARD MUST BE INSTALLED ON ALL WALLS SUPPORTING FLOOR/CEILING ASSEMBLIES USED FOR SEPARATION REQUIRED BY THIS SECTION. <u>OPENING PROTECTION -</u> OPENINGS FROM A PRIVATE GARAGE DIRECTLY INTO A ROOM USED FOR SLEEPING PURPOSES SHALL NOT BE PERMITTED. OTHER OPENINGS BETWEEN THE GARAGE AND RESIDENCE SHALL BE EQUIPPED WITH SOLID WOOD DOORS NOT LESS THAT 1 % INCHES (35MM) IN THICKNESS, SOLID or HONEYCOMB-CORE STEEL DOORS NOT LESS THAN 1 1/2 INCHES (35MM) THICK, or 20-MINUTE FIRE-RATED DOORS.

DUCT PENETRATION - DUCTS IN THE GARAGE AND DUCTS PENETRATING THE WALLS or CEILINGS SEPARATING THE DWELLING FROM THE GARAGE SHALL BE CONSTRUCTED OF A MINIMUM NO. 26 GAUGE (0.48MM) SHEET STEEL or OTHER APPROVED MATERIAL AND SHALL NOT HAVE OPENINGS INTO THE GARAGE

CEILINGS - GARAGE TO BE SEPARATED FROM HABITABLE ROOMS ABOVE BY NOT LESS THAN 5/8-INCH TYPE X GYPSUM BOARD OR EQUIVALENT PER NCRC SECTION R302.6N **STAIRS -** ENCLOSED ACCESSIBLE SPACE UNDER STAIRS SHALL HAVE WALLS, UNDER-STAIR SURFACE AND ANY SOFFITS PROTECTED ON THE ENCLOSED SIDE WITH 1/2 INCH (12.7MM) GYPSUM BOARD.

OTHER PENETRATIONS - PENETRATIONS THROUGH THE SEPARATION REQUIRED IN SECTION R302.6 SHALL BE PROTECTED AS REQUIRED BY SECTION R302.11, ITEM 4.

R609.1 EXTERIOR WINDOWS & DOORS:

THIS SECTION PRESCRIBES PERFORMANCE AND CONSTRUCTION REQUIREMENTS FOR EXTERIOR WINDOWS AND DOORS INSTALLED IN WALLS. WINDOWS AND DOORS SHALL BE INSTALLED AND FLASHED IN ACCORDANCE WITH THE FENESTRATION MANUFACTURER'S WRITTEN INSTRUCTIONS. WINDOW AND DOOR OPENINGS SHALL BE FLASHED IN ACCORDANCE WITH SECTION R703.4. WRITTEN INSTALLATION INSTRUCTIONS SHALL BE PROVIDED BY THE FENESTRATION MANUFACTURER FOR EACH WINDOW OR DOOR.

TABLE N1102.1 INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT

	MAXIMUM	MINIMUM INSULATION R-VALUE						
CLIMATE ZONE	GLAZING U-FACTOR	CEILINGS	WALLS	FLOORS	BASEMENT WALLS	SLAB PERIMETER	CRAWL SPACE WALLS	
3	.35	R-38 or R-30	R-15	R-19	R-5/13	R-0	R-5/13	
4	.35	R-38 or R-30	R-15	R-19	R-10/15	R-10	R-10/15	

STAIRWAYS & GAURDS REQUIREMENTS PER 311.7 & R312

EACH TREAD AND RISER MUST BE UNIFORM.

THE GREATEST RISER HEIGHT SHALL NOT

EXCEED THE SMALLEST BY MORE THAN 3/8".

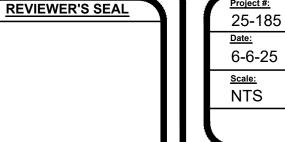
THE GREATEST TREAD DEPTH SHALL NOT EXCEED

THE SMALLEST BY MORE THAN 3/8".

THE TOP AND BOTTOM RISER OF INTERIOR

STAIRS SHALL NOT EXCEED THE SMALLEST RISER

BY MORE THAN 3/4".









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GENERAL NOTES

ALL DOORS LEADING TO DWELLING FROM THE GARAGE TO BE 20-MINUTE FIRE RATED DOOR

CARBON MONOXIDE ALARMS ARE REQUIRED TO BE INSTALLED OUTSIDE <u>ALL</u> SLEEPING AREAS

PER NCRC SECTION R310

<u>UL FIRESTOP SYSTEMS:</u> **W-L-1001:** FOR SMALL PIPE PENETRATIONS IN WOOD-STUD WALLS. C-AJ-1202: FOR PENETRATIONS THROUGH CEILINGS AND FLOOR ASSEMBLIES.

EXTERIOR WALL CAVITY INSULATION SHALL BE ENCLOSED ON ALL SIDES WITH RIGID OR AN AIR BARRIER MATERIAL: BEHIND TUBS, SHOWERS, TAIRS, FIRE PLACES AND KNEE WALLS. PER NCRC SECTION N1102.2.12

CRAWLSPACE ACCESS NEEDS TO BE A MINIMUM DPENING MEASURING 18 INCHES BY 24 INCHES PER NCRC SECTION R408.8

R905.2 references TABLE R905.1.1(2) which states that "For roof slopes from two units vertical in 12 units horizontal (2:12), up to four units vertical in 12 units horizontal (4:12), underlayment shall be two layers applied in the following manner: apply a 19-inch strip of underlayment felt parallel to an starting at the eves. Starting at the eave, apply 36-inch-wide sheets of underlayment, overlapping successive sheets 19 inches. Distortions in the underlayment shall not interfere with the ability of the shingles to seal.

R301.5 is a table titled "MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (in pounds per square foot)". It states that the LIVE LOAD for Stairs is 40, with a note that states "Individual stair treads shall be designed for the uniformly distributed live load or a 300-pound concentrated load acting over an area of 4 square inches, whichever produces the greatest stress".

PER NCRC R302.5.1

GARAGE TO BE SEPARATED FROM HABITABLE ROOMS ABOVE BY NOT LESS THAN 5/8-INCH TYPE X GYPSUM BOARD OR PER NCRC SECTION R302.6N

<u>WINDOW FALL PROTECTION</u>, PER NCRC SECTION R312.2

PER NCRC SECTION R315

EMERGENCY ESCAPE AND RESCUE OPENINGS AS

PENETRATION SEALING: SEAL **ALL** PENETRATIONS IN FIRE-RATED WALLS, CEILINGS, OR FLOORS WITH UL-RATED FIRESTOP MATERIALS.