

RE: 2411-0099-I - Elmhurst Rev 3-Elev 5-Roof Site Information: Project Customer: DRB Raleigh Project Name: DRE Lot/Block: Subdivision Model: Elmhurst Rev 3 Address: City: State: NC General Truss Engineering Criteria & Design Loads Drawings Show Special Loading Conditions): Design Code: IRC2021/TPI2014 Wind Code: ASCE 7-16 Wind Speed: 120 mph Roof Load: 40.0 psf Mean Roof Height (feet): 25	B Raleigh Model Track n: 6 (Individual Truss Design Design Program: MiTek 20/2 Design Method: MWFRS (E Floor Load: N/A psf Exposure Category: B	Trenco 818 Soundside Rd Edenton, NC 27932 20 25.2 nvelope)/C-C hybrid Wind ASCE 7-16
No.Seal#Truss NameDateNo.Seal#1173750757P2 $5/28/25$ 35 173750772173750758P1G $5/28/25$ 36 17375073173750760B1G $5/28/25$ 37 173750774173750761B1 $5/28/25$ 38 1737507675173750761B1 $5/28/25$ 39 173750766173750763C1 $5/28/25$ 39 173750767173750765A1GE $5/28/25$ 39 1737507610173750767H2G $5/28/25$ $5/28/25$ 11 11173750767H2G $5/28/25$ $5/28/25$ 11 11173750768H4 $5/28/25$ $5/28/25$ 13173750772H3G $5/28/25$ $5/28/25$ 14173750775A1A $5/28/25$ 15173750776H1 $5/28/25$ 16173750776H1 $5/28/25$ 17173750776H1 $5/28/25$ 18173750776H1 $5/28/25$ 20173750778H1G $5/28/25$ 21173750781V3 $5/28/25$ 22173750781V3 $5/28/25$ 23173750785A2T $5/28/25$ 24173750786H3 $5/28/25$ 25173750786H3 $5/28/25$ 26173750786H3 $5/28/25$ 27173750786H3 $5/28/25$ 28173750	Truss Name Date A2AT 5/28/25 B1GE 5/28/25 B2 5/28/25 A B2A 5/28/25 5 V7 5/28/25 D5 V7 5/28/25	
The truss drawing(s) referenced above have been prepared the Truss Engineering Co. under my direct supervision based on provided by Structural, LLC. Truss Design Engineer's Name: Gilbert, Eric My license renewal date for the state of North Carolina is D IMPORTANT NOTE: The seal on these truss component designs that the engineer named is licensed in the jurisdiction(s) identified and the designs comply with ANSI/TPI 1. These designs are based upon parameters on the or MiTek or TRENCO. Any project specific information included is TRENCO's customers file reference purpose only, and was not taken interpreparation of these designs. MiTek or TRENCO has not independently applicability of the design parameters and incorporate these designs into the overall building design per ANSI/TPI 1.	by the parameters ecember 31, 2025 is a certification at the the terrs ich were for MiTek's or o account in the verified the ilding. Before use, properly , Chapter 2.	SEAL SINEFRICTION GILBERTINI
1 of 1	Gilbert, Eric	

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	P2	Monopitch Supported Gable	1	1	Job Reference (optional)	173750757

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 ID:MrAI?V5gQQf5epwenXEPqZzSRcp-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

1-7-0







Scale = 1:38.6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	0.00	4-9	>999	360	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.20	Vert(CT)	0.00	4-9	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP		Wind(LL)	0.00	4-9	>999	240		
BCDL	10.0										Weight: 7 lb	FT = 20%
6) Plates checked for a plus or minus 5 degree rotation												

TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
BRACING		
TOP CHORD	Structura	wood sheathing directly applied or
	1-7-0 oc p	ourlins.
BOT CHORD	10-0-0 oc	bracing.
REACTIONS	(size)	2=0-3-0, 4= Mechanical
	Max Horiz	2=24 (LC 16)
	Max Uplift	2=-5 (LC 12), 4=-46 (LC 36)
	Max Grav	2=382 (LC 36), 4=265 (LC 47)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	
TOP CHORD	1-2=0/36,	2-3=-96/180
BOT CHORD	2-4=-164/	145
NOTES		

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 2) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 4) Unbalanced snow loads have been considered for this desian.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- about its center.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc. 8)
- This truss has been designed for a 10.0 psf bottom 9) chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Refer to girder(s) for truss to truss connections.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 4.
- 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads. LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design and the second way the approximation of design and the property incorporate this design into the overall building design. Bracing indicated is to prevent building of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	P1G	Monopitch Supported Gable	1	1	Job Reference (optional)	173750758

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 ID:QHdHHGhPtUq80L929KcuJuzSRc2-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:38

2)

Plate Offsets (X, Y): [2:0-4-4,0-0-4]

Loading FCLL (roof) Snow (Pf/Pg) FCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-AS	0.46 0.48 0.05	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.05 -0.08 0.00 0.04	(loc) 8-13 8-13 2 8-13	l/defl >999 >752 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 23 lb	GRIP 244/190 FT = 20%	
LUMBER TOP CHORD SOT CHORD WEBS DTHERS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3		4) 5) 6)	Unbalanced design. This truss ha load of 12.0 p overhangs no Plates check	snow loads have to s been designed fo psf or 2.00 times fl pn-concurrent with ed for a plus or mi	or great or great at roof le other li nus 5 de	nsidered for t er of min root oad of 15.4 p ve loads. egree rotation	his f live osf on n						
 RACING OP CHORD Structural wood sheathing directly applied, except end verticals. OT CHORD Rigid ceiling directly applied. EACTIONS (size) 2=0-3-0, 7= Mechanical Max Horiz 2=68 (LC 15) Max Uplift 2=-6 (LC 12), 7=-29 (LC 13) Max Grav 2=395 (LC 40), 7=351 (LC 44) 6) Plates checked for a plus or r about its center. 7) Gable studs spaced at 2-0-0 about its center. 7) Gable studs spaced at 2-0-0 about its center. 8) This truss has been designed chord live load nonconcurrent 9) * This truss has been designed chord live load nonconcurrent 90 * This truss has been designed chord live load nonconcurrent 90 * This truss has been designed chord live load nonconcurrent 90 * This truss has been designed chord in all are 3-06-00 tall by 2-00-00 wide hord and any other member 						c. or a 10. vith any for a liv s where Il fit betv) psf bottom other live loa e load of 20. a rectangle veen the bott	ads. Opsf						
FORCES	(lb) - Maximum Com Tension 1-2=0/36, 2-3=-119/ ⁻ 4-5=-10/0_4-7=-300	pression/Maximum 181, 3-4=-83/86, /114	10) 11)	Refer to girde Provide mech bearing plate	er(s) for truss to tru hanical connection capable of withst	uss conr i (by oth anding 2	nections. ers) of truss 9 lb uplift at	to joint						
BOT CHORD WEBS NOTES	2-8=-158/143, 7-8=- 3-8=-132/119	36/39, 6-7=0/0	12)	One H2.5A S recommende UPLIFT at jt(Simpson Strong-Tie ed to connect truss s) 2. This connect	e conne to bear ion is fo	ctors ing walls due · uplift only a	e to nd						
Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 5-1-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown;				does not con This truss ha load of 250.0 panels and a Bottom Chor This truss de structural wo	sider lateral forces s been designed f lb live and 3.0lb d t all panel points a d, nonconcurrent sign requires that od sheathing be a	s. or a mo ead loca long the vith any a minim pplied d	ving concentu ted at all mice Top Chord a other live loa um of 7/16" irectly to the	rated d and ads. top		4		OR FESS	ROLLIN	

chord and 1/2" gypsum sheetrock be applied directly to

- Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- the bottom chord. LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	P1	Monopitch	8	1	Job Reference (optional)	173750759

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 ID:JJODhRxaxxU91aG5uEUpgJzSRbj-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:33.9

Plate Offsets (X	, Y):	[2:0-0-4,Edge]
------------------	-------	----------------

Loading TCLL (roof) Snow (Pf/Pg) TCDL 3CLL 3CDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-AS	0.63 0.70 0.00	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.09 -0.12 0.00 0.04	(loc) 6-9 6-9 2 6-9	l/defl >673 >496 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 21 lb	GRIP 244/190 FT = 20%	_
LUMBER TOP CHORD SOT CHORD WEBS BRACING TOP CHORD SOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she except end verticals Rigid ceiling directly (size) 2=0-3-0, 6 Max Horiz 2=68 (LC Max Uplift 2=-5 (LC Max Grav 2=371 (LC	athing directly applied applied. 5= Mechanical 15) 12), 6=-28 (LC 13) 2 40), 6=358 (LC 42)	 6) This truss ha chord live lo chord live lo on the botto 3-06-00 tall chord and at at at a given bearing plate 6. 10) One H2.5A 3 recommend UPLIFT at jt 	as been designed fr ad nonconcurrent w has been designed m chord in all areas by 2-00-00 wide wil hy other members. er(s) for truss to tru- thanical connection e capable of withsta Simpson Strong-Tie ed to connect truss (s) 2. This connecti	or a 10.0 vith any for a liv s where Il fit betw uss conre (by oth anding 2 e connee to beari on is for	D psf bottom other live loa e load of 20.1 a rectangle veen the bott nections. ers) of truss t 8 lb uplift at j ctors ng walls due uplift only at	ads. Opsf om to joint e to nd						
TOP CHORD BOT CHORD NOTES 1) Wind: ASC Vasd=95m II; Exp B; I Exterior(2E zone; cant	(ib) - Maximum Com Tension 1-2=0/36, 2-3=-145/ 3-6=-306/140 2-6=-64/109, 5-6=0// CE 7-16; Vult=120mph nph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) -1-0-0 to 2-0-0, Inter ilever left and right exp	pression/Maximum 78, 3-4=-10/0, 0 (3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C rior (1) 2-0-0 to 5-1-0 posed ; end vertical le	does not cor 11) This truss ha load of 250.(panels and a Bottom Choi 12) This truss de structural we at. chord and 1, the bottom co LOAD CASE(S) off	sider lateral forces as been designed fo Dib live and 3.0lb da at all panel points a rd, nonconcurrent v asign requires that yod sheathing be a (2" gypsum sheetro hord. Standard	s. or a move ad loca long the vith any a minim pplied di ock be a	ving concentr ted at all mid Top Chord a other live load um of 7/16" rectly to the i oplied directly	rated and ads. top y to				WTH CA	ROI	

Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown;

- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

A MiTek Aff 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	C1G	Monopitch Supported Gable	1	1	Job Reference (optional)	173750762

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:26 ID:nveuljeU0mAtex94eWIMkLzSPYQ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:37.4

Plate Offsets (X, Y): [2:0)-0-5,0-2-0], [7:Edge,0-3-8]
----------------------------	------------------------------

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-AS	0.87 0.70 0.06	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.10 -0.17 0.02 0.09	(loc) 9-13 9-13 2 9-13	l/defl >911 >555 n/a >983	L/d 360 240 n/a 240	PLATES MT20 Weight: 36 lb	GRIP 244/190	
LUMBER TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING TOP CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E zone; cant and right e members a Lumber DC 2) Truss desis orly. For s see Stand or consult 3) TCLL: ASC Plate DOL 1.15 Plate Exp.; Ce=	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 Left 2x4 SP No.3 1 Structural wood sheat except end verticals. 10-0 oc bracing. (size) 2=0-4-8, 1 Max Horiz 2=41 (LC Max Uplift 2=-68 (LC Max Uplift 2=-68 (LC Max Grav 2=432 (LC (lb) - Maximum Com Tension 1-2=0/24, 2-4=-449/7 5-6=-273/161, 7-10= 2-9=-241/385, 8-9=-13 5-8=-99/150, 4-9=-13 CE 7-16; Vult=120mph ph; TCDL=6.0psf; BC Enclosed; MWFRS (en E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed; porch left and and forces & MWFRS (on E) -1-0-0 to 2-0-0, Inter ilever left and right exp exposed to wind loads in studs exposed to wind loads in s	I-6-0 athing directly applied (15=0-1-8 12) (12), 15=-48 (LC 12) (14), 15=378 (LC 45) (15), 15=328 (LC 45) (15), 1	4) 5) 6) 7) 8) 9) 10) 11] 38 99 12] 38 11] 38 12] 5 12] 5 12] 5 12] 5 12] 6 13] 5 14] 5 14] 5 12] 12] 12] 12] 12] 12] 12] 12]	Unbalanced a design. This truss ha load of 12.0 tp overhangs no Plates check about its cen Gable studs a This truss ha chord live loa * This truss ha on the botton 3-06-00 tall b chord and an Bearing at joi using ANSI/T designer sho Provide mech bearing plate One H2.5A S recommende UPLIFT at jt(and does not This truss ha load of 250.0 panels and a Bottom Chord This truss de structural wo chord. AD CASE(S)	snow loads have b s been designed for on-concurrent with ed for a plus or mir ter. spaced at 2-0-0 oc s been designed for d nonconcurrent w as been designed for d nonconcurrent w as been designed for the consider as the second p other members. If 1 angle to grain uld verify capacity nanical connection at joint(s) 15. Simpson Strong-Tie d connect truss (s) 2 and 15. This c consider lateral for s been designed for b live and 3.0lb det t all panel points al d, nonconcurrent w sign requires that a od sheathing be ap Standard	een cor or great at roof k other liv ous 5 de cor a 10.0 vith any for a liv where l fit betw parallel formula of bear onnection onnection onnection on a minim oplied di	sidered for t er of min rood and of 15.4 p re loads. ggree rotation other live loa e load of 20.1 a rectangle reen the bott to grain valu a. Building ng surface. ers) of truss i ctors ng walls due on is for uplif ring concentri ted at all mic Top Chord a other live loa other live loa tother live tothe	his f live sef on n ads. Opsf aom Je to to to to s to d and ads. top		M. M		SEA 0363	RO(1) 22 E.F.F.F.F.F.F.F.F.F.F.F.F.F.F.F.F.F.F.F	

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	C1	Monopitch	5	1	Job Reference (optional)	173750763

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:25 ID:3jVhdUJYNTmivobuZLm08fzSPYr-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





7-11-8

Scale = 1:34.8

2)

3)

design.

Plate Offsets (X, Y): [2:0-0-5,0-2-0], [5:0-3-8, Edge]

Lumber DOL=1.60 plate grip DOL=1.60

Exp.; Ce=1.0; Cs=1.00; Ct=1.10

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially

Unbalanced snow loads have been considered for this

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL	(psf) 20.0 15.4/20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-AS	0.66 0.86 0.00	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.13 -0.19 0.02 0.08	(loc) 5-9 5-9 2 5-9	l/defl >717 >488 n/a >999	L/d 360 240 n/a 240	PLATES M18AHS MT20	GRIP 186/179 244/190
BCDL	10.0											Weight: 32 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x4 SP No.3 1 Structural wood shea except end verticals. 10-0-0 oc bracing. (size) 2=0-4-8, 1 Max Horiz 2=41 (LC Max Uplift 2=-68 (LC Max Grav 2=432 (LC (lb) - Maximum Com Tension	1-6-0 athing directly applied 1=0-1-8 12) 12), 11=-48 (LC 12) 2 44), 11=378 (LC 43 pression/Maximum	4) 5) 6) 7) d, 8) 9) 9)	This truss ha load of 12.0 µ overhangs nu All plates are Plates check about its cen This truss ha chord live loa * This truss h on the botton 3-06-00 tall b chord and ar Bearing at jo using ANSI/T designer sho Provide mect	s been designed for posf or 2.00 times file on-concurrent with MT20 plates unlet ed for a plus or mit ter. is been designed for ad nonconcurrent w has been designed in chord in all areas by 2-00-00 wide wil yo other members. int(s) 11 considers TPI 1 angle to grain uld verify capacity hanical connection at joint(s) 11.	or greate at roof lo other lix ss other lix ss other nus 5 de or a 10.0 vith any for a liv s where l fit betw parallel of beari (by othe	er of min root aad of 15.4 p ve loads. wise indicate egree rotation of psf bottom other live load e load of 20. a rectangle veen the bott to grain valu a. Building ng surface. ers) of truss	f live livsf on ed. n ads. Opsf to					
TOP CHORD	1-2=0/24, 2-4=-480/*	155, 5-6=-70/334,	11)	One H2.5A S	Simpson Strong-Tie	conne	ctors						
BOT CHORD WEBS NOTES	4-b=-268/163 2-5=-268/371 4-11=-236/65	(0)))	12)	UPLIFT at jt(and does not This truss ha	to connect truss s) 2 and 11. This c t consider lateral fo s been designed fo	to beari connection prces. or a move	ng walls due on is for uplif ving concenti	e to it only rated				mmm	1999. 1. 1997.
 Wind: AS0 Vasd=95r II; Exp B; Exterior(2 zone; can and right of members 	CE 7-16; Vult=120mph mph; TCDL=6.0psf; BC Enclosed; MWFRS (en E) -1-0-0 to 2-0-0, Inter tilever left and right exp exposed; porch left and and forces & MWFRS	(3-second gust) DL=6.0psf; h=25ft; C velope) and C-C rior (1) 2-0-0 to 7-6-4 posed ; end vertical le right exposed;C-C f for reactions shown;	at. 13) eft or LO	panels and a Bottom Chor This truss de structural wo chord. AD CASE(S)	t all panel points a d, nonconcurrent v sign requires that a od sheathing be ap Standard	long the vith any a minim oplied di	Top Chord a other live loa um of 7/16" rectly to the	and ads. top		Winn		OR FESS	

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

⁸¹⁸ Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	A1GE	Hip Supported Gable	1	1	Job Reference (optional)	173750765

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:23 ID:yhD1aBWmZ2AKWm94h02xSYzEGIM-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Plate Offsets (X, Y): [2:0-2-8,0-0-1], [14:0-4-1,0-0-1]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-AS	0.21 0.19 0.08	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 2	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 103 lb	GRIP 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS SLIDER BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x4 SP No.3 1-6-0 Structural wood she except 2-0-0 oc purlins (6-C Rigid ceiling directly (size) 2=19-11- 16=19-11 22=19-11 22=19-11 24=19-11 Max Horiz 2=-31 (LC 17=-7 (LC 24=-16 (L Max Grav 2=314 (LC 16=332 (L)	1-6-0, Right 2x4 SP N athing directly applied -0 max.): 6-10. applied. 0, 14=19-11-0, -0, 19=19-11-0, -0, 21=19-11-0, -0, 23=19-11-0, -0 2 14) 12), 16=-13 (LC 17), 2 17), 23=-7 (LC 16), C 16) C 93), 14=314 (LC 11 -C 115), 17=336 (LC	WE lo.3 NC 1) d, 2) 3) 7), 4) 114),	 WEBS 8-20=-282/49, 7-21=-283/45, 6-22=-277/14, 5-23=-289/71, 4-24=-285/81, 9-19=-283/46, 10-18=-277/14, 11-17=-289/70, 12-16=-285/79 NOTES NOTES 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) 1-10-0 to 1-11-8, Exterior(2N) 1-11-8 to 6-0-8, Corner(3R) 13-10-8 to 16-10-8, Exterior(2N) 9-0-8 to 13-10-8, Corner(3R) 13-10-8 to 16-10, Exte								een designed for rd in all areas w 0-00 wide will fit er members. al connection (b bible of withstance 3, 16 lb uplift at t at joint 16 and en designed for a e and 3.0lb deace anel points alon neconcurrent with requires that an ueathing be appl osum sheetrock presentation do of the purlin alon ndard	a live load of here a rectan between the y others) of tr ing 1 lb uplift joint 24, 7 lb 1 lb uplift at ju noving cond located at al g the Top Ch any other liv inimum of 7/ ed directly to be applied di es not depict g the top anc	f 20.0psf gle bottom uss to at joint 2, uplift at oint 2. centrated I mid ord and e loads. 16" the top rectly to the size I/or
FORCES TOP CHORD BOT CHORD	18=328 (I 20=333 (I 22=328 (I 108), 24= (Ib) - Maximum Com Tension 1-2=0/39, 2-4=-149/ 5-6=-81/92, 6-7=-45 8-9=-45/96, 9-10=-4 11-12=-64/67, 12-14 2-24=-13/56, 9-10=-4 11-12=-64/67, 12-14 2-24=-13/56, 16-17	LC 113), 19=333 (LC LC 111), 21=333 (LC LC 109), 23=336 (LC 332 (LC 107) hpression/Maximum 43, 4-5=-64/67, /96, 7-8=-45/96, 5/96, 10-11=-81/92, 4=-149/43, 14-15=0/3 s-13/56, 22-23=-13/56 r=-13/56, 14-16=-13/5	$\begin{array}{cccc} 112), \\ 110), \\ 5) \\ 6) \\ 7) \\ 8) \\ 9 \\ 6, \\ 9 \\ 9, \\ 55, \\ 10) \\ 56 \\ 11) \end{array}$	1.15 Plate DO Exp.; Ce=1.0 Unbalanced s design. This truss har load of 12.0 p overhangs no Provide adeq Plates check about its cent Gable require Gable studs s This truss har chord live loa	DL = 1.15); Is=1.0; I ; Cs=1.00; Ct=1.10 snow loads have be s been designed for osf or 2.00 times fla on-concurrent with o uate drainage to pr ed for a plus or min er. es continuous botto spaced at 2-0-0 oc. s been designed for d nonconcurrent with	Rough , Lu=50 een cor r greate t roof lo other liv event v us 5 de m chor r a 10.0	Cat B; Partial)-0-0 isidered for th er of min roof 1 bad of 15.4 ps re loads. water ponding egree rotation d bearing. D psf bottom other live load	ly is f on		W UTTINK,		SEA 0363	L 22 EER HLBE	A STATE AND

May 28,2025

ERENCIO A MITEK Affiliate

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	A3	Common	6	1	Job Reference (optional)	173750766

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:25 ID:UmB4xfioozB3REN9dNLi5vzEGI6-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale =	1:49.5
---------	--------

Plate Offsets (X, Y): [2:0-3-8,Edge], [8:0-5-1,Edge]

Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15		CSI TC	0.61	DEFL Vert(LL)	in -0.38	(loc) 10-13	l/defl >637	L/d 360	PLATES MT20	GRIP 244/190	
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.66	Vert(CT)	-0.49	10-17	>491	240			
TCDL	10.0	Rep Stress Incr	YES		WB	0.21	Horz(CT)	0.02	8	n/a	n/a			
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-AS		Wind(LL)	0.02	10	>999	240			
BCDL	10.0											Weight: 95 lb	FI = 20%	
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD WEBS NOTES 1) Unbalance this design	2x4 SP No.2 2x4 SP SS 2x4 SP No.3 Left 2x4 SP No.3 1-6-0 Structural wood she Rigid ceiling directly (size) 2=0-3-8, 8 Max Horiz 2=-48 (LC Max Grav 2=857 (LC (lb) - Maximum Corr Tension 1-2=0/36, 2-4=-1213 5-6=-926/127, 6-8=- 2-8=-100/1007 5-10=0/522, 6-10=-3 ed roof live loads have 1.	This truss ha load of 12.0 g overhangs no Plates check about its cem This truss ha chord live loa * This truss h on the bottom 3-06-00 tall b chord and an This truss ha load of 250.0 panels and a Bottom Chord) This truss de structural wo chord and 1/2 the bottom ch DAD CASE(S)	s been designed for basis or 2.00 times fit on-concurrent with ed for a plus or mi- ter. s been designed fit d nonconcurrent v as been designed n chord in all areas y 2-00-00 wide wi y other members. s been designed fi- lb live and 3.01b di- t all panel points a d, nonconcurrent v sign requires that of sheathing be a 2" gypsum sheetro nord. Standard	or greate at roof k other lin nus 5 de or a 10.0 with any l for a liv s where ll fit betw or a mov ead loca llong the with any a minim pplied di ock be ap	er of min rool aad of 15.4 p re loads. egree rotation other live loa e load of 20.1 a rectangle veen the bott ving concentri ted at all mic Top Chord a other live loa other live loa other live the polied directly	f live sf on n ads. 0psf om rated and ads. top y to								
 Wind: ASC Vasd=95m II; Exp B; E Exterior(2E 20-11-0 zc vertical left forces & M DOL=1.60 TCLL: ASC Plate DOL 1.15 Plate Exp.; Ce=' Unbalance 	CE 7-16; Vult=120mph pph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) -1-0-0 to 2-0-0, Inte R) 9-11-8 to 12-11-8, I ne; cantilever left and t and right exposed;C- IWFRS for reactions s plate grip DOL=1.60 CE 7-16; Pr=20.0 psf (E) 1.15; Pg=20.0 psf, F DOL = 1.15); Is=1.0; I 1.0; Cs=1.00; Ct=1.10 ed snow loads have be	(3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C rior (1) 2-0-0 to 9-11- interior (1) 12-11-8 to right exposed ; end C for members and hown; Lumber roof LL: Lum DOL=1. 2f=15.4 psf (Lum DOL Rough Cat B; Partiall een considered for thi	at. 8, 15 L = y							Contraction of the second		SEA 0363	ROCI 1	7

- Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



G

mmm May 28,2025



Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:28 ID:IcLX?fvIoEc4CypCE7yn8IzeeaM-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:62.3

Plate Offsets (X, Y): [7:0-2-0,0-2-5], [15:0-2-0,0-2-5]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TI	PI2014	CSI TC BC WB Matrix-AS	0.20 0.19 0.19	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(lo	oc) l/de - n/ - n/ 22 n/	i L/ a 99 a 99 a n/	d PLAT 9 MT20 9 a Weigl	' ES ht: 200 lb	GRIP 244/190 FT = 20%	%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 Structural wood she except end verticals (6-0-0 max): 7-15	athing directly applied , and 2-0-0 oc purlins	top (CHORD 2 3 6 1 1 1 1 1	2-38=-293/45, 1-2=(3-4=-70/78, 4-5=-76 5-7=-117/131, 7-8=- 3-10=-72/128, 10-1 12-13=-72/128, 13-1 12-13=-72/128, 15-1 16-17=-75/112, 17-1 9-20=-85/58, 20-2	0/52, 2- 6/71, 5-(-72/128 1=-72/1 14=-72/ 16=-117 18=-76/ 1=0/52,	3=-85/77, 6=-75/112, , 8-9=-72/128 28, 11-12=-7 128, 7/131, 71, 18-19=-7 20-22=-293/	3, '2/128, '0/78, '40	4) 5) 6)	TCLL: AS Plate DO 1.15 Plate Exp.; Ce= Unbalanc design. This truss load of 12 overhang	CE 7- =1.15 DOL 1.0; C ed sno has b 0 psf s non-	16; Pr=20); Pg=20. = 1.15); I s=1.00; C ww loads I een desig or 2.00 til concurrer	.0 psf (roo 0 psf; Pf= s=1.0; Ro t=1.10, L have been gned for g mes flat ro t with oth	of LL: Lum 20.4 psf (L ugh Cat B; u=50-0-0 considere reater of m oof load of her live loac	DOL=1.15 .um DOL = Partially d for this in roof live 15.4 psf on ds.
BOT CHORD REACTIONS	(of of max), r=10. Rigid ceiling directly (size) 22=31-5-(25=31-5-(28=31-5-(34=31-5-(34=31-5-(37=31-5-(Max Horiz 38=111 (L Max Uplift 22=-12 (L 24=-5 (LC) 35=-17 (L 37=-41 (L	applied.), 23=31-5-0, 24=31-4), 26=31-5-0, 30=31-4), 32=31-5-0, 33=31-4), 35=31-5-0, 36=31-4), 38=31-5-0 LC 15) C 13), 23=-35 (LC 17 C 16), 36=-4 (LC 16) C 13), 38=-36 (LC 12)	BOT (5-0, 5-0, 5-0, 5-0, WEB: (), (),	CHORD 3	37-38=-49/71, 36-3; 34-35=-49/71, 33-3; 31-32=-49/71, 29-3; 27-28=-49/71, 26-2; 24-25=-49/71, 26-2; 24-25=-49/71, 23-2; 24-25=-49/71, 23-2; 24-25=-276/35, 10-3; 3-32=-276/35, 10-3; 3-32=-276/34, 13-3; 2-29=-276/34, 13-3; 2-29=-275/2, 16-2(7-25=-281/64, 18-3; 9-23=-277/63	7=-49/7 4=-49/7 1=-49/7 7=-49/7 4=-49/7 31=-27(=-275/1 =-286/5 28=-27(8 5=-278/ 24=-28(1, 35-36=-49 1, 32-33=-49 1, 28-29=-49 1, 22-26=-49 1, 22-23=-49 5/34, 0, 6-34=-278 5, 3-37=-277 5/43, 23, 5/55,	//71, //71, //71, //71, //71 //71 //23, //65,	7) 8) 10) 11) 12) 13)	Provide a Plates ch about its Gable red Truss to I braced ag Gable stu This truss chord live * This tru on the bo 3-06-00 t	dequa ecked center. juires o e fully jainst l ds spa has b load r s has ttom cl all by 2	te drainag for a plus continuou sheathed ateral mo aced at 2- een desig nonconcu been desig hord in all -00-00 w	je to prev or minus s bottom d from one vement (i 0-0 oc. gned for a rrent with signed for l areas wi ide will fit	ent water p 5 degree n chord beari e face or se .e. diagona 10.0 psf bi any other I a live load here a recta between th	ionding. otation ing. courely il web). ottom live loads. of 20.0psf angle he bottom
FORCES	Max Grav 22=309 (L 136), 24= (LC 134), 27=333 (L 131), 29= (LC 129), 32=333 (L 126), 34= (LC 124), 37=320 (L (lb) - Maximum Com Tension	LC 137), 23=320 (LC 336 (LC 135), 25=33: 26=333 (LC 133), C 132), 28=333 (LC 333 (LC 130), 30=33: 31=333 (LC 128), C 127), 33=333 (LC 333 (LC 125), 35=33: 336=336 (LC 123), C 122), 38=309 (LC pression/Maximum	NOTE 2 1) U 2) W th 2) W V 3) U (3) 121) 2 121) 2 121) 6 0 5 3) T 0 3) O 5	ES Inbalanced his design. Vind: ASCE (asd=95mpf 3E) -1-0-0 to borner(3R) 8 2-5-15, Cor 5-8-8 to 32- nd vertical h orces & MW POL=1.60 pl russ design nly. For stu ee Standarc r consult qu	roof live loads have 7-16; Vult=120mph 1; TCDL=6.0psf; BC closed; MWFRS (er 0 2-1-11, Exterior(2) 1-11 to 12-0-12, E ner(3R) 22-5-15 to 5-0 zone; cantileve eft and right expose FRS for reactions s ate grip DOL=1.60 ed for wind loads ir ds exposed to wind d Industry Gable En alified building desi	been of CDL=6. CDL=6. Invelope N) 2-1- ixterior(25-8-8, r left ar ed;C-C shown; n the pla d (norm id Detai gner as	considered fo ond gust))psf; h=25ft; () and C-C Cc) and C-C Cc T to 8-11-1, 2N) 12-0-12 : Exterior(2N) dr ight expos for members Lumber ane of the tru: al to the face is as applical s per ANSI/TF	r Cat. porner to sed ; and ss), ble, PI 1.					H CA FESS SEA 0363 NGIN A. C	L 22 EEER.	

Continued on page 2



May 28,2025

Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE WARNING Design valid for use only with MTek connectors. This design is based only upon parameters and property incorporate this design is based only upon parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	H2G	Hip Supported Gable	1	1	Job Reference (optional)	1/3/50/6/

- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 38, 12 lb uplift at joint 22, 17 lb uplift at joint 35, 4 lb uplift at joint 36, 41 lb uplift at joint 37, 17 lb uplift at joint 25, 5 lb uplift at joint 24 and 35 lb uplift at joint 23.
- 15) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 16) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:28 Page: 2 ID:IcLX?fvloEc4CypCE7yn8IzeeaM-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:30 ID:N6dgvB2z8dhvAb2vTNOVZ7zeeqz-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

-1-0-0 32-5-0 8-0-0 12-4-3 19-0-13 23-5-0 31-5-0 \vdash +____ 8-0-0 4-4-3 6-8-10 4-4-3 8-0-0 1-0-0 1-0-0 4x6= 6x6= 24 0-2-2 H 21 4 23 25 5 -H 0-2-2 22 8-2-7 30 16 3x6 🞜 3x6 12 71 3 6 29 17 8-2-7 8-0-5 8-0-5 28 18 4x8 💋 ¹⁹20 26²⁷ 4x8 👟 2 7 8 0-0 15 9 ₿ 31 14 32 13 12 11 33 10 34 3x6= 3x6= 4x4= MT20HS 3x8 = 4x4= 3x4= MT18HS 3x16 = 8-0-0 12-2-7 19-2-9 23-5-0 31-5-0 8-0-0 4-2-7 7-0-2 4-2-7 8-0-0 31-5-0

Scale = 1:64.2

Plate Offsets (X, Y): [2:0-2-12,0-1-8], [7:0-2-12,0-1-8], [9:Edge,0-1-8]

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.86	Vert(LL)	-0.19	9-10	>999	360	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15		BC	0.51	Vert(CT)	-0.28	9-10	>999	240	MT18HS	244/190
TCDL	10.0	Rep Stress Incr	YES		WB	0.45	Horz(CT)	-0.04	15	n/a	n/a	MT20HS	187/143
BCLL	0.0*	Code	IRC2021	I/TPI2014	Matrix-AS		Wind(LL)	0.02	11	>999	240		
BCDL	10.0											Weight: 200 lb	FT = 20%
BCLL BCDL LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD WEBS REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E 12-4-3, Ex to 19-0-13, 23-5-0 to 3 end vertica forces & M DOL=1.60	0.0* 10.0 2x4 SP No.2 *Except 2x4 SP SS 2x4 SP No.3 *Except Structural wood shea except end verticals, (5-0-15 max.): 4-5. Rigid ceiling directly 1 Row at midpt (size) 9=0-3-8, 1 Max Horiz 9=142 (LC Max Grav 9=1582 (L (lb) - Maximum Com Tension 5-6=-1900/46, 6-7=-2 4-5=-1559/65, 1-2=0 3-4=-1889/46, 7-9=-7 14-15=-27/763, 13-1 10-11=0/1839, 9-10= 6-10=0/293, 6-11=-5 5-13=-131/132, 4-13 3-14=0/292, 7-10=0/ ed roof live loads have b. E 7-16; Vult=120mph nph; TCDL=6.0psf; BCI Enclosed; MWFRS (en E) -1-0-0 to 2-1-11, Inte terior(2R) 12-4-3 to 16 , Exterior(2R) 19-0-13 32-5-0 zone; cantilever al left and right exposed WFRS for reactions sf plate grip DOL=1.60	Code t* 5-4:2x4 SP SS t* 9-7,15-2:2x6 SP N athing directly applied, and 2-0-0 oc purlins applied. 5-13 5=0-3-8 215) C 59), 15=1576 (LC pression/Maximum 2279/0, 7-8=0/51, /51, 2-3=-2269/0, 1561/43, 2-15=-1555 4=0/1828, 11-13=0/1 -96/823 19/57, 5-11=0/652, =0/622, 3-13=-521/5 1077, 2-14=0/1071 been considered for (3-second gust) DL=6.0psf; h=25ft; C velope) and C-C erior (1) 2-1-11 to -9-8, Interior (1) left and right expose d;C-C for members a hown; Lumber	IRC2021 3) (o.2 4) d, 5) 6) 7) 100 6/43 1515, 11 7, 12 13 (at. LC (at.) (at.)	I/TPI2014 TCLL: ASCE Plate DOL=1 1.15 Plate DO Exp.; Ce=1.0 Unbalanced design. This truss ha load of 12.0 j overhangs ne Provide adec All plates are Plates check about its cen This truss ha chord live loa) * This truss ha load of 250.0 panels and a Bottom Chor) This truss de structural wo chord and 1/2 the bottom chorc DAD CASE(S)	Matrix-AS 7-16; Pr=20.0 psf; 15); Pg=20.0 psf; DL = 1.15); Is=1.0; ; Cs=1.00; Ct=1.10; snow loads have b s been designed for the formed state of the formed state of the formed state s been designed formed formed state of the formed state and for a plus or mini- ter. s been designed formed formed state of the formed state and nonconcurrent with uate drainage to p MT20 plates unless ed for a plus or mini- ter. s been designed formed formed state and nonconcurrent with y other members, s been designed formed state blive and 3.0lb det t all panel points all d, nonconcurrent with sign requires that a ord sheathing be ap 2" gypsum sheetro- nord. Standard	(roof LL Pf=20.4 Rough O, Lu=50 een cor or great at roof k other liv revent v ss other hus 5 de or a 10.0 vith any for a liv s where l fit betw with BC or a monim oplied d ck be a does no long the	Wind(LL) .: Lum DOL= 9 psf (Lum DC Cat B; Partia 0-0-0 Isidered for the er of min roof bad of 15.4 pr vater ponding wise indicate agree rotation 0 psf bottom other live load e load of 20.1 a rectangle veen the bottom DL = 10.0psf ving concentr ted at all mid Top Chord a other live load other live load other live load e load of 71.6" rectly to the top popled directly ot depict the se top and/or	0.02 1.15 DL = illy his f live sf on g. dd. n dds. Opsf om f. rated i and stated i size	11	>999	240	Weight: 200 Ib WHTH CA SEA 0363 NGINI A. G	FT = 20%

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road Edenton, NC 27932

May 28,2025

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	A1G	Hip Supported Gable	1	1	Job Reference (optional)	173750770

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:23

Page: 1

Structural, LLC, Thurmont, MD - 21788.



Scale = 1:57.3

Plate Offsets (X, Y): [5:0-1	I-8,Edge],	[9:0-1-8,Edge]														
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	20	(psf) 20.0 .4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC20)21/TPI2	2014	CSI TC BC WB Matrix-MR	0.22 0.22 0.19	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 14	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 112	GRIP 244/190 Ib FT = 205	%	
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	 D 2x4 SP No.2 D 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3 D Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-9. D Rigid ceiling directly applied or 6-0-0 oc bracing. S (size) 14=16-5-0, 15=16-5-0, 16=16-5-0, 21=16-5-0, 22=16-5-0, 22=16-5-0, 23=16-5-0 Max Horiz 23=-121 (LC 14) Max Uplift 14=-42 (LC 13), 15=-79 (LC 17), 19=-2 (LC 12), 22=-80 (LC 16), 23=-49 (LC 12) Max Grav 14=318 (LC 105), 15=335 (LC 104), 16=332 (LC 103), 17=333 (LC 102), 19=333 (LC 101), 20=333 (LC 100), 21=332 (LC 99), 23=234 (L 0 9), 23=234 (L 0 9) 				 Unb. this Wind Vase Wind Vase (3E) (3F) (3R) Corr 17-5 verti force DOL Trus only see or cc DOL Plate 1.15 Exp. TCL desis (5) This 	alanced design. d: ASCE d=95mpf xp B; En- -1-0-0 tc 4-8-13 t 4-8-13 t es & MW =1.60 pl ss design . For stu Standarc onsult qu L: ASCE e DOL=1 ; Plate DC ; Ce=1.0 alanced gn. truss ha	roof live loads ha 7-16; Vult=120m n; TCDL=6.0psf; I closed; MWFRS 0 2-2-8, Exterior(1-8-3 to 14-8-3, I cantilever left an nd right exposed; FRS for reactions ate grip DOL=1.6 ed for wind loads ds exposed to wid 1 Industry Gable I alified building de 7-16; Pr=20.0 ps OL = 1.15); Is=1.0; (; Cs=1.00; Ct=1. snow loads have s been designed	ve been of ph (3-sec 3CDL=6.0 (envelope 2N) 2-2-8 r(2N) 7-8-8 Exterior(2 d right ex C-C for n s shown; 0 in the pla nd (norm End Deta esigner as of (roof LL f; Pf=20.4 D; Rough 10, Lu=50 been cor for greate	considered fo cond gust) Dpsf; h=25ft; () and C-C Cc to 4-8-13, Cc 13 to 11-8-3, 10 posed; end nembers and Lumber ane of the true al to the face) ils as applicat s per ANSI/TF t psf (Lum DC Cat B; Partial)-0-0 isidered for th er of min roof	r Dat. prmer prmer ss), ble, I.15 bL = Ily live	 14, 14 of the deal and contract of the deal o						
FORCES	(lb) - Maxiı Tension	22=335 (L mum Com	pression/Maximum	n n	load over 7) Prov	of 12.0 p hangs no ide adec	psf or 2.00 times on-concurrent wit quate drainage to	flat roof lo h other liv prevent v	oad of 15.4 ps /e loads. water ponding	sf on 1.			and a	ORTHO	C C C	Ville .	
TOP CHORD	2-23=-298/131, 1-2=0/83, 2-3=-125/104, 3-4=-108/206, 4-5=-170/169, 5-6=-77/186, 6-7=-77/186, 7-8=-77/186, 8-9=-77/186, 9-10=-170/168, 10-11=-110/207, 11-12=-125/99, 12-13=0/83, 12-14=-298/129 22-23=-62/67, 21-22=-62/67				 Plates checked for a plus or minus 5 degree rotation about its center. Gable requires continuous bottom chord bearing. Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web). 								SEAL 036322				
WEBS	22-23=-62/67, 21-22=-62/67, 20-21=-62/67, 19-20=-62/67, 17-19=-62/67, 16-17=-62/67, 15-16=-62/67, 14-15=-62/67 7-19=-276/80, 6-20=-276/9, 4-21=-279/5, 3-22=-285/163, 8-17=-276/7, 10-16=-279/5, 11-15=-285/164				 11) Gable studs spaced at 2-0-0 oc. 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 2 0.0 0.0 trull w 2.00.00 uid a will be bottom to be for the bottom. 							10.	A A A A A A A A A A A A A A A A A A A		NEER.		

3-06-00 tall by 2-00-00 wide will fit between the bottom

chord and any other members.

NOTES

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road Edenton, NC 27932

GI 11111111

May 28,2025

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	170750774
	A1GR	Hip Girder	2	2	Job Reference (optional)	173750771

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:24 Page: 1 ID:Z8YEsqlXPkWihCTmczEGF2zeebs-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



Scale = 1:51.8

Plate Offsets (X, Y): [2:0-6-4,0-1-12], [4:0-6-4,0-1-12], [8:0-3-8,0-6-4], [10:0-3-8,0-6-4]

Loa TCI	ading LL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15		CSI TC	0.73	DEFL Vert(LL)	in -0.05	(loc) 6-8	l/defl >999	L/d 360	PLATES MT20	GRIP 244/190	
Sho	w (Pt/Pg)	20.4/20.0	Lumber DOL Rop Stross Incr	1.15 NO		BC	0.31		-0.09	6-8	>999	240 p/p	MIBAHS	186/179	
RCI		0.0*	Code	IRC202	1/TPI2014	Matrix-MS	0.00	Wind(LL)	0.01	10-11	-000	240			
BC		10.0	Code	11(0202	1/11/12/014	WIGUIX-IVIO			0.00	10-11	2999	240	Weight [,] 292 lb	FT = 20%	
													11019111 202 10	2070	
LUI TOI BO WE	MBER P CHORD T CHORD BS	2x4 SP No.2 2x8 SP DSS 2x4 SP No.3 *Excep	t* 11-1,6-5:2x6 SP N	4) lo.2	Wind: ASCE Vasd=95mph II; Exp B; End and right exp Lumber DOL	7-16; Vult=120mp n; TCDL=6.0psf; Be closed; MWFRS (e osed ; end vertica =1.60 plate grip D	h (3-sec CDL=6.0 envelope I left and OL=1.60	ond gust) Dpsf; h=25ft; (e); cantilever I right expose)	Cat. left ed;	Co	oncentra Vert: 20 23=-132 (B), 28=	ted Loa =-1335 28 (B), -1579	ads (lb) 5 (B), 21=-1328 (E 24=-1328 (B), 26 (B)	8), 22=-1328 (B =-1201 (B), 27=	9), =-1634
BRACING TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 2-4. TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15) SOT CHORD Rigid ceiling directly applied or 10-0 oc TOL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15) Plate DOL=1.15); Is=1.0; Rough Cat B; Partially TSP Plate DOL=1.10; Cs=1.00; Ct=1.10, Lu=50-0-0															
BO	I CHORD	bracing.	applied or 10-0-0 oc	; 6)	Unbalanced	snow loads have b	peen cor	sidered for th	nis						
bracing. 6=0-3-8, 11=0-3-8 design. REACTIONS (size) 6=0-3-8, 11=0-3-8 7) Provide adequate drainage to prevent water ponding. Max Horiz 11=109 (LC 9) 8) All plates are MT20 plates unless otherwise indicated. Max Grav 6=6125 (LC 47), 11=6985 (LC 2) 9) Plates deeked for a plus of a place or plus of a plus															
FO	RCES	(lb) - Maximum Com	pression/Maximum	9)	about its cen	ter.	nus 5 ue	gree rotation	I						
то		Tension 1-25348/0 2-34	116/0 3-44116/0	10) This truss ha	s been designed f	or a 10.0) psf bottom	d a						
101		4-5=-5263/0, 1-11=-4	4478/0. 5-6=-4402/0	11	1) * This truss has been designed for a live load of 20.0psf										
BO	T CHORD	10-11=0/960, 9-10=0	0/3740, 8-9=0/3713,		on the bottom chord in all areas where a rectangle										
	DC	6-8=0/1041	1179 2 0 206/45		3-06-00 tall b	y 2-00-00 wide wi	ll fit betv	veen the botto	om						
VVE	50	2-10=0/2860, 2-9=0/ 4-9=0/1136, 4-8=0/2	969.1-10=0/2801.	12	chord and an	y other members.	or a mov	ing concentr	ated						
		5-8=0/2801	,	12	load of 250.0lb live and 3.0lb dead located at all mid										
NO	TES				panels and at all panel points along the Top Chord and										
1)	2-ply truss	to be connected toget	her with 10d		Bottom Chord, nonconcurrent with any other live loads.									AN'	
	(0.131"x3") nails as follows:		13) Graphical pu	rlin representation	does no	ot depict the s	size			82		Not T	7
	Top chord	s connected as follows	: 2x4 - 1 row at 0-9-	0	bottom chord	alion of the punin a	along the	top and/or			-		14 / Ja	4:	-
	Bottom ch	ords connected as follo	-0 00. 2x8 - 2 rows	14) Use Simpsor	n Strong-Tie HUS2	P6 (14-1))d Girder 4-1	b0						
	staggered	at 0-5-0 oc.			Truss) or equ	vivalent spaced at	2-0-0 00	max. starting	gat		=	1	SEAL		Ξ.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc. 0-6-12 from the left end to 14-6-12 to connect truss(es)									1	:	03632	22 :	-		
2) All loads are considered equally applied to all plies, to back face						ack face of bottom chord.									3
	except if n	oted as front (F) or bac	ck (B) face in the LO	AD 15) Fill all nail ho	les where hanger	is in cor	tact with lum	ber.		-	-	1. A.	- · ·	3
	CASE(S) s	section. Ply to ply conn	ections have been	LC	DAD CASE(S)	Standard						2.0	NGINE	EH	~
	unless oth	o uistribute only 1080S l erwise indicated	noted as (F) of (B),	1)	Dead + Sno	w (balanced): Lun	nber Inc	rease=1.15, I	Plate			24	20 GIN	Et N	
3)	Unbalance	ed roof live loads have	been considered for		Increase=1.	. ID ade (Ib/ft)							AG	LBUIN	
<i>.</i> ,	this design).			Vert: 1-2:	=-51, 2-4=-61, 4-5:	=-51, 6-	11=-20					111111	min	
						, , , , , ,	- , -								

- unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design.
- Increase=1.15 Uniform Loads (lb/ft)
 - Vert: 1-2=-51, 2-4=-61, 4-5=-51, 6-11=-20

818 Soundside Road Edenton, NC 27932

May 28,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	A1B	Common	1	1	Job Reference (optional)	3750773

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:22 ID:U72Srl8_YTGyjXutGMSmGPzeelh-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:82.3

Plate Offsets (X, Y): [15:0-2-9,Edge]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-AS	0.74 0.64 0.83	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.36 -0.89 0.12 0.06	(loc) 17 15-17 12 18-19	l/defl >999 >473 n/a >999	L/d 360 240 n/a 240	PLATES MT20 M18AHS Weight: 233 lb	GRIP 244/190 186/179 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.2 *Excep 2x4 SP SS 2x4 SP No.3 *Excep Left 2x6 SP No.2 1 1-6-0 Structural wood shee Rigid ceiling directly 1 Row at midpt (size) 2=0-3-8, 1 Max Horiz 2=-172 (L Max Grav 2=1582 (L (b) - Maximum Com	t* 5-1,9-13:2x4 SP S t* 20-21:2x4 SP SS I-6-0, Right 2x6 SP I athing directly applied applied. 20-21 I2=0-3-8 C 14) C 2), 12=1582 (LC : pression/Maximum	2) SS No.2 ed. 3) 2) 4)	Wind: ASCE Vasd=95mpl II; Exp B; En Exterior(2E) 17-5-8, Exte 20-11-6 to 3: exposed ; er members an Lumber DOL TCLL: ASCE Plate DOL=1 1.15 Plate D Exp.; Ce=1.0 Unbalanced	7-16; Vult=120m, 1; TCDL=6.0psf; E closed; MWFRS (1-0-0 to 2-5-14, I rior(2R) 17-5-8 to 5-11-0 zone; canti d vertical left and d forces & MWFR =1.60 plate grip D 5 7-16; Pr=20.0 psf OL = 1.15); Is=1.0 Cs=1.00; Ct=1.1 snow loads have	oh (3-sec 3CDL=6.0 envelope nterior (1 20-11-6, lever left right exp S for rea 00L=1.60 f (roof LL ; Pf=15.4); Rough I0 been cor	ond gust))psf; h=25ft; i) and C-C) 2-5-14 to Interior (1) and right osed;C-C for ctions shown) : Lum DOL= psf (Lum DC Cat B; Partia asidered for th	Cat. n; 1.15 DL = Illy	LOAD	CASE(S)	Star	ndard		
TOP CHORD	(10) - Maximum Com Tension 1-2=0/41, 2-4=-2287 6-7=-2176/0, 7-8=-2 10-122287/151 12	//152, 4-6=-2130/0, 195/0, 8-10=-2130/0 2-13-0/41	5) ,	In this truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.										
BOT CHORD	2-19=-39/1851, 17-1	9=0/1719, 15-17=0/	1288,	from left end	, supported at two	points, t	5-0-0 apart.	-5-8						
WEBS	4-13=//1724, 12-12 4-19=-211/113, 6-19 6-18=-491/219, 8-15 8-14=-170/237, 10-1 18-20=0/951, 7-20=(15-21=0/964, 20-22= 17-22=0/169	+=-0/1852 ==-150/242, ==-489/227, 4=-211/114, D/1004, 7-21=0/1016 =-44/0, 21-22=-44/0,	7) 8) 9) 5, 10	All plates are Plates check about its cen This truss ha chord live loa) * This truss h	M I 20 plates unle led for a plus or m ter. Is been designed ad nonconcurrent has been designed in chord in all area	ess other inus 5 de for a 10.0 with any d for a liv us where	wise indicate egree rotation) psf bottom other live loa e load of 20.0 a rectangle	id. n Ids. Opsf		4		ORTH CA	RO	_
NOTES				3-06-00 tall b	y 2-00-00 wide w	ill fit betv	veen the bott	om				2 -	- que	
 Unbalance this design 	ed roof live loads have n.	been considered for	11 12	chord and ar) This truss ha load of 250.0 panels and a Bottom Chor	ny other members is been designed blb live and 3.0lb d it all panel points d, nonconcurrent isign requires that	for a mov lead loca along the with any a minim	ving concentr ted at all mid Top Chord a other live loa	ated I and ids.		TITLIN AND		SEA 0363	L 22	

structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

13) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road Edenton, NC 27932

GILBE

May 28,2025

GIL

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof					
	A1	Common	5	1	Job Reference (optional)	173750774				

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:21 ID:wus_xnleLm7WUAGfMRj75Mzeeo3-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Plate Offsets (X, Y): [15:0-2-4,0-3-0]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	I/TPI2014	CSI TC BC WB Matrix-AS	0.75 0.63 0.81	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.35 -0.85 0.07 0.05	(loc) 17 17 13 18-19	l/defl >999 >483 n/a >999	L/d 360 240 n/a 240	PLATES MT20 M18AHS Weight: 239 lb	GRIP 244/190 186/179 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.2 *Excep 2x4 SP SS 2x4 SP No.3 *Excep 20-21:2x4 SP SS Left 2x6 SP No.2 1 Structural wood shere except end verticals. Rigid ceiling directly 1 Row at midpt (size) 2=0-3-8, 1 Max Horiz 2=1855 (LC Max Grav 2=1556 (L	t* 5-1:2x4 SP SS t* 13-11:2x6 SP No.2, I-6-0 athing directly applied applied. 10-13, 20-21 I3= Mechanical C 15) C 2), 13=1506 (LC 2)	2)	Wind: ASCE Vasd=95mpl II; Exp B; En Exterior(2E) 17-5-8, Exter 20-11-2 to 3/ exposed ; er members an Lumber DOL TCLL: ASCE Plate DOL=1 1.15 Plate D Exp.; Ce=1.0 Unbalanced design	7-16; Vult=120mp 1; TCDL=6.0psf; B closed; MWFRS (e 1-0-0 to 2-5-10, Ir ior(2R) 17-5-8 to 2 4-3-12 zone; cantil d vertical left and d forces & MWFR: =1.60 plate grip D 7-16; Pr=20.0 psf; OL = 1.15); Is=1.0 b; Cs=1.00; Ct=1.1 snow loads have t	h (3-sec CDL=6.0 envelope nterior (1 20-11-2, ever left right exp S for rea OL=1.60 f (roof LL Pf=15.4 ; Rough 0 been cor	ond gust) Dpsf; h=25ft; () and C-C) 2-5-10 to Interior (1) and right osed;C-C for ctions shown) : Lum DOL= psf (Lum DC Cat B; Partia	Cat. ; 1.15 DL = Ily nis	14) Atti	c room c CASE(S)	hecke	d for L/360 deflec ndard	tion.	
FORCES	(lb) - Maximum Com Tension	pression/Maximum	5)	This truss ha	s been designed f	or great	er of min roof	live sf on						
TOP CHORD	1-2=0/41, 2-4=-2242 6-7=-2127/0, 7-8=-2 10-11=-572/1, 11-13	2/143, 4-6=-2085/0, 091/0, 8-10=-2058/0, 5=-431/21	6)	overhangs n 250.0lb AC u from left end	on-concurrent with init load placed on	the bott	ve loads. om chord, 17	'-5-8						
BOT CHORD	2-19=-57/1832, 17-1 14-15=0/1652, 13-14	9=0/1694, 15-17=0/12 4=0/1721, 12-13=0/0	252, 7) 8)	All plates are Plates check	e MT20 plates unle ed for a plus or mi	ess other inus 5 de	wise indicate egree rotation	d. 1				,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1111	
WEBS	10-13=-1752/0, 4-19 6-19=-147/245, 6-18 8-15=-442/232, 8-14 10-14=-149/120, 18- 7-21=0/921, 15-21=(21-22=-46/0, 17-22=	H=-214/113, H=-493/219, H=-177/198, H=-177	9) 10) 07,	This truss ha chord live loa) * This truss h on the bottor 3-06-00 tall b chord and ar	is been designed f ad nonconcurrent v nas been designed n chord in all area by 2-00-00 wide wi y other members.	or a 10.0 with any I for a liv s where Il fit betw) psf bottom other live loa e load of 20.0 a rectangle veen the botto	ds. Opsf om		1	r'i	OP. ESS	ROUN	,
NOTES 1) Unbalance this design	ed roof live loads have n.	been considered for	11) 12) 13)	 Refer to gird This truss had load of 250.0 panels and a Bottom Chor This truss de structural wood tructural wood tructura wood tructu	er(s) for truss to trus s been designed f llb live and 3.0lb d t all panel points a d, nonconcurrent sign requires that od sheathing be a	uss conr or a mov ead loca along the with any a minim pplied d	ections. ving concentr ted at all mid Top Chord a other live loa um of 7/16" rectly to the	ated and ds.		1111111			ER KIN	

May 28,2025

Page: 1

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

the bottom chord.



Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof					
	A1A	Common	1	1	Job Reference (optional)	3750775				

Structural LLC Thurmont MD - 21788

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:22

Page: 1



NOTES

WEBS

Loading

TCDL

BCLL

BCDL

WFBS

WEBS

SI IDER

Unbalanced roof live loads have been considered for 1) this design.

4-18=-221/99, 10-13=-1962/66

- Wind: ASCE 7-16; Vult=120mph (3-second gust) 2) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-10, Interior (1) 2-5-10 to 17-5-8, Exterior(2R) 17-5-8 to 20-11-2, Interior (1) 20-11-2 to 34-3-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to
- the bottom chord 13) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	H1	Нір	1	1	Job Reference (optional)	173750776

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:26 ID:T?pRuy59_RwnvYEgHTovttzeefH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Weight: 224 lb FT = 20%

14) Graphical purlin representation does not depict the size

or the orientation of the purlin along the top and/or

bottom chord.

LOAD CASE(S) Standard

Page: 1

LUMBER

Scale = 1:88.2

TCLL (roof)

TCDL

BCLL

BCDL

Snow (Pf/Pg)

TOP CHORD	2v4 SP No 2 *Except* 1-5:2v4 SP SS
BOT CHORD	2x4 SP SS
WEBS	2x4 SP No 3 *Except* 14-12:2x6 SP No 2
SLIDER	Left 2x6 SP No.2 2-0-0
BRACING	
TOP CHORD	Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (4-2-10 may): 7-8
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 6-18, 11-14, 9-17
REACTIONS	(size) 2=0-3-8, 14= Mechanical Max Horiz 2=181 (LC 15) Max Cray 2, 181 (LC 57) 14, 1783 (LC 50)
	Wax Glav 2=1010 (LC 57), 14=1783 (LC 59)
FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/41, 2-4=-2846/302, 4-6=-2726/106, 6-7=-2136/150, 7-8=-1764/153, 8-9=-2127/151, 9-11=-2679/106, 11-12=-546/40, 12-14=-405/42
BOT CHORD	2-20=-105/2345, 18-20=0/2112, 17-18=0/1659, 15-17=0/2059, 14-15=-36/2207, 13-14=0/0
	4 20_ 219/00 6 20_0/442 6 19_ 767/09
WEBS	7-18=-31/842, 8-17=-28/725, 11-14=-2328/68, 9-17=-680/99, 9-15=0/403, 11-15=-167/99

10.0

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-0 to 2-5-10, Interior (1) 2-5-10 to 17-2-1, Exterior(2E) 17-2-1 to 17-8-15, Exterior(2R) 17-8-15 to 21-2-9, Interior (1) 21-2-9 to 34-3-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- Plates checked for a plus or minus 5 degree rotation
- about its center.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Refer to girder(s) for truss to truss connections.
- 12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



Job	Truss	Truss Type	Qty Ply Elmhurst Rev 3-Elev 5-Roof			
	H2	Нір	1	1	Job Reference (optional)	173750777

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:27 ID:xaDGhQtomPC6I00GtmUDHZzeegs-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:72

Plate Offsets (X, Y): [6:0-6-6,Edge], [7:0-4-6,Edge]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-AS	0.96 0.63 0.65	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.24 -0.36 0.07 0.04	(loc) 15-16 15-16 12 16-18	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS M18AHS Weight: 205 lb	GRIP 244/190 187/143 186/179 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD	2x4 SP No.2 *Except 2x4 SP SS 2x4 SP No.3 *Except Left 2x6 SP No.2 - 2 Structural wood shea except end verticals, (4-5-6 max.): 6-7.	* 6-7,1-5:2x4 SP SS * 12-9:2x6 SP No.2 -0-0 athing directly appliec and 2-0-0 oc purlins	2) I,	Wind: ASCE Vasd=95mph II; Exp B; End Exterior(2E) - 13-8-14, Exter 17-2-8 to 21- (1) 24-7-11 to exposed ; en members and	7-16; Vult=120mp ; TCDL=6.0psf; B closed; MWFRS (¢ -1-0-0 to 2-5-10, Ir rior(2R) 13-8-14 t 2-2, Exterior(2R) 2 0 34-8-0 zone; car d vertical left and d forces & MWFRS	h (3-sec CDL=6.0 envelope nterior (1 o 17-2-8 21-2-2 to tillever le right exp S for rea	cond gust) Opsf; h=25ft; (e) and C-C) 2-5-10 to b, Interior (1) 24-7-11, Inte eft and right bosed;C-C for ctions shown	Cat. erior ;	14) Gra or th botti LOAD C	phical pu ne orient om chor CASE(S)	urlin re ation c d. Star	presentation doe f the purlin along ndard	s not depict the size the top and/or
BOT CHORD WEBS REACTIONS	Rigid ceiling directly 1 Row at midpt (size) 2=0-3-8, 1 Max Horiz 2=150 (LC Max Grav 2=1699 (L	applied. 4-16, 6-15, 8-15 2= Mechanical : 15) C 57), 12=1683 (LC 5	3) 59) 4)	members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0									
FORCES	(lb) - Maximum Com	pression/Maximum	4)	design.	Jesign.								
TOP CHORD	Tension 1-2=0/41, 2-4=-2701 6-7=-1804/147, 7-8=	/199, 4-6=-2237/131, -2194/128,	5)	This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.									
BOT CHORD	8-9=-2541/82, 9-10= 2-18=-87/2222, 16-1 15-16=0/1770, 13-15 12-13=-28/387, 11-1	0/16, 9-12=-1751/75 8=-37/2222, 5=-20/2116, 2=0/0	6) 7) 8)	 Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated. Plates checked for a plus or minus 5 degree rotation 									11111
WEBS	4-18=0/326, 4-16=-5 6-15=-184/121, 7-15 8-13=-146/218, 9-13	39/79, 6-16=0/650, =0/603, 8-15=-471/75 =0/1754	9) 5, 10)	This truss has chord live loa	s been designed f d nonconcurrent v as been designed	or a 10.0 vith any) psf bottom other live load e load of 20 0	ds. Insf			A.	ORTHCA	QLI III.
NOTES 1) Unbalanced roof live loads have been considered for this design. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				on the bottom 3-06-00 tall b chord and an Refer to girde This truss ha load of 250.0 panels and a Bottom Chorr This truss de structural wo	 This truss has been designed for a live load of 20.0psf This truss has been designed for a live load of 20.0psf This truss has been designed for a live load of 20.0psf Sefer to girder(s) for truss to truss connections. This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads. This truss design requires that a minimum of 7/16" 							L22 EEREKTUU	

- 11) Refer to girder(s) for truss to truss connections. 12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



G minim

May 28,2025

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof		
	H1G	Hip Supported Gable	1	1	Job Reference (optional)	173750778	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:26 ID:0RQd5UKWCTfSX3S4iB6bTAzeedi-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:73.7 Plate Offsets (X, Y): [8:0-2-0,0-2-5], [16:0-2-0,0-2-5]

Loading TCLL (roof) Snow (Pf/Pg)	2	(psf) 20.0 0.4/20.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 VES		CSI TC BC WB	0.20 0.19 0.26	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a	(lo	oc) l - - 23	/defl n/a n/a n/a	L/d 999 999 p/a	PLATES MT20	GRIP 244/1	90
BCLL BCDL		0.0* 10.0	Code	IRC2	021/TPI2014	Matrix-AS	0.20	1012(01)	0.00		20	170	n/a	Weight: 235	b FT=	20%
3CLL 3CDL LUMBER TOP CHORD 3OT CHORD WEBS DTHERS BRACING TOP CHORD 3OT CHORD BOT CHORD REACTIONS	2x4 SP N 2x4 SP N 2x4 SP N 2x4 SP N Structura except er (6-0-0 ma Rigid ceil (size) Max Horiz Max Uplift Max Grav	0.0* 10.0 0.2 0.2 0.3 0.3 1 wood sheat od verticals, 1x.): 8-16. ing directly 23=34-8-C 29=34-8-C 29=34-8-C 29=34-8-C 33=34-8-C 43=32-6 (LC 14=4), 25=4 (LC 142), 28=332 (LC 139), 31=5 (LC 137), 34=333 (LC	Code athing directly applied and 2-0-0 oc purlins applied.), 24=34-8-0, 25=34-), 31=34-8-0, 32=34-), 31=34-8-0, 32=34-), 34=34-8-0, 32=34-), 34=34-8-0, 32=34-), 34=34-8-0, 32=34-), 34=34-8-0, 32=34-), 34=34-8-0, 32=34-), 35=-13 (LC 17) C 15), 24=-53 (LC 17) C 17), 35=-1 (LC 12) C 16), 40=-11 (LC 16) (15), 24=-52 (LC 13) C 12) C 145), 24=320 (LC 145), 24=320 (LC 333 (LC 143), 26=33 27=333 (LC 136), 32=33 33=333 (LC 136), C C 135), 35=333 (LC	IRC2 d, 8-0, 8-0, 8-0, 8-0, 8-0, 7), , , , , , , , , , , , , , , , , ,	O21/TPI2014 FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalanced this design.	Matrix-AS (lb) - Maximum Co Tension 2-43=-293/43, 1-2= 3-4=-88/79, 4-5=-8 6-7=-101/135, 7-8 9-10=-88/148, 10-1 12-13=-88/148, 13 14-15=-88/148, 15 16-17=-117/151, 1 18-19=-81/92, 19-2 21-22=-74/65, 22-2 42-43=-48/66, 41-4 39-40=-48/66, 38-5 32-33=-48/66, 31-5 28-29=-48/66, 31-5 32-33=-48/66, 31-5 32-32-32, 32-6 32-32-32, 32-6 32-32-32, 32-6 32-32-42-27/70, 31-1 32-32-273/64, 5-40 32-32-273/64, 5-40 3	mpressi =0/47, 2 6/70, 5- 117/15 1=-88/1 14=-88 -16=-88, 10=-77/7 23=-257, 12=-48/6 35=-48/6 35=-48/6 25=-48/6 25=-48/6 25=-48/6 32=-27 -29=-27 -29=-27 -27=-27 -27=-27 -27=-27 -27=-27 -29=-29 -29	on/Maximum 3=-100/88, 6=-81/91, 1, 8-9=-88/14! 48, 11-12=-88 148,	8, 3/148, 72, 66, 66, 66, 66, 66, 66	2) 3) 4) 5)	Wind: Vasd= II; Exp. (3E) -7-5-8 end vc DOL= Truss only. see SI Plate I 1.15 F Exp.; Unbal desigr	ASCE 95mp 9 B; Er 4(3R) 15, Co 3 to 34 4ertical 1.60 p design For st tandar Sult qu ASCE DOL= 2 Plate D Ce=1. anced 1.6 1.6 1.6 1.60 p 1.60 p 1	E 7-16 h; TCl hclose to 2-5- lleft ar hcd fo uds ep d Indu ualifie E 7-16 1.15);)OL = 0; CS= snow	Weight: 235 I ; Vult=120mph DL=6.0psf; BC: d; MWFRS (en 10, Exterior(2N to 14-1-10, E) R) 24-2-15 to 2 one; cantilever id right expose for reactions sl rip DOL=1.60 r wind loads in sposed to wind ustry Gable End d building desig ; Pr=20.0 psf (I Pg=20.0 psf; F 1.15; Is=1.0; F 1.00; Ct=1.10, Ioads have be SE 036	b FT = : (3-second DL=6.0ps velope) a 1) 2-5-10 t tterior(2N ?7-5-8, Ex left and r d;C-C for town; Lur the plane (normal tr 1 Details a nor as per soof LL: Li 'f=20.4 ps Rough Ca Lu=50-0- en consid	20% d gust) f; h=25ft; Cat. nd C-C Corner to 10-8-1,) 14-1-10 to tterior(2N) ight exposed ; members and nber o for the truss o the face), as applicable, er ANSI/TPI 1. um DOL=1.15 if (Lum DOL = t B; Partially 0 lered for this
		134), 37=3 (LC 132), 40=332 (L 129), 42=3 (LC 127)	333 (LC 133), 38=33 39=333 (LC 131), C 130), 41=337 (LC 312 (LC 128), 43=30	8											VEER GILB	Runna

May 28,2025

Page: 1



Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, recetion and bracing of trusses and truss systems, see **ANSI/TPI Quility Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	H1G	Hip Supported Gable	1	1	Job Reference (optional)	173750778
Structural, LLC, Thurmont, MD -	21788,	Run: 25.20 S May 13	3 2025 Print:	25.2.0 S May	/ 13 2025 MiTek Industries, Inc. Tue May 27 08:52:20	6 Page: 2

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding. 7) 8) Plates checked for a plus or minus 5 degree rotation about its center.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc. 12) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. 13) * This truss has been designed for a live load of 20.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 14) N/A
- 15) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 16) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S) Standard

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:26 ID:0RQd5UKWCTfSX3S4iB6bTAzeedi-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	V1	Valley	1	1	Job Reference (optional)	173750779

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 Page: 1 ID:GhRTS_jTBWc6G2IUBQtPIkzeeq6-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:50.2

Plate Offsets ()	(, Y):	[3:0-4-1,0-3-0],	[4:0-4-1,0-3-0]
------------------	--------	------------------	-----------------

Loading		(psf) 20.0	Spacing Plate Grip DOI	2-0-0 1.15		CSI TC	0.47	DEFL Vert(LL)	in n/a	(loc)	l/defl n/a	L/d 999	PLATES MT20	GRIP 244/190
Snow (Pf/Pa)	2	0.4/20.0	Lumber DOL	1.15		BC	0.42	Vert(TL)	n/a	-	n/a	999	11120	210,100
TCDL		10.0	Rep Stress Incr	YES		WB	0.15	Horiz(TL)	0.00	6	n/a	n/a		
BCLL		0.0*	Code	IRC202	21/TPI2014	Matrix-MS								
BCDL		10.0											Weight: 67 lb	FT = 20%
LUMBER				2) Wind: ASCE	7-16; Vult=120mp	oh (3-seo	cond gust)		14) Thi	s truss h	as bee	en designed for a	moving concentrated
TOP CHORD	2x4 SP N	lo.2			Vasd=95mpl	h; TCDL=6.0psf; B	CDL=6.	0psf; h=25ft; 0	Cat.	load	d of 250.	0lb live	e and 3.0lb dead	located at all mid
BOT CHORD	2x4 SP N	0.2			II; Exp B; En	closed; MWFRS (envelope	e) and C-C	-	par	nels and	at all p	anel points along	the Top Chord and
OTHERS	2x4 SP N	0.3			Exterior(2E)	0-0-0 to 3-0-0, Inte	erior (1)	3-0-0 to 5-7-1	5,	Bot	tom Cho	rd, noi	nconcurrent with	any other live loads.
BRACING					EXTERIOR(2E)	5-7-15 to 9-1-5, E	xterior(2	R) 9-1-5 to	loft	15) Gra	apnicai p	uriin re	epresentation doe	e the ten and/or
TOP CHORD	Structura	I wood she	athing directly applie	d or	and right evr	lor (1) 13-3-1 to 14	l left and	right expose	d.C-	bot	tom chor	alion (d	or the putiin along	J the top and/or
	6-0-0 oc	purlins, exc	ept		C for membe	ers and forces & M	WFRS f	or reactions	u,U		CASEIS	u. Stai	ndard	
	2-0-0 0C Digid coil	ing directly	-0 max.): 3-4.		shown; Lum	ber DOL=1.60 plat	e grip D	OL=1.60		LOAD		014	naara	
BOTCHORD	bracing	ing unectly	applied of 10-0-0 oc											
REACTIONS	(size)	1-14-9-4	6-14-9-4 7-14-9-4	3) Truss desigr	ned for wind loads	in the pl	ane of the true	SS					
	(0120)	8=14-9-4.	9=14-9-4, 10=14-9-4	, 4	only. For stu	uds exposed to wir	nd (norm	al to the face)),					
	Max Horiz	1=-87 (LC	: 14)	-	see Standar	d Industry Gable E	nd Deta	ils as applicat	ole,					
	Max Uplift	1=-56 (LC	69), 6=-56 (LC 71),	1		aillied building de	signer a:		11. 115					
		7=-70 (LC	17), 10=-72 (LC 16)) 7	Plate DOI =1	1 15): Pa=20.0 psf	· Pf=20.4	Losf (Lum DC) =					
	Max Grav	1=265 (LC	C 74), 6=265 (LC 81)	,	1.15 Plate D	OL = 1.15); $Is=1.0$: Rouah	Cat B: Partial						
		7=503 (LC	C 58), 8=385 (LC 92)	,	Exp.; Ce=1.0	0; Cs=1.00; Ct=1.1	0, Lu=5	D-0-0	.,					
		9=385 (LC	91), 10=506 (LC 56	5) 5) Unbalanced	snow loads have l	been cor	sidered for th	nis					
FORCES	(lb) - Max	umum Com	pression/Maximum		design.									
	1 2- 205	121 22-	250/110 2 4- 101/1	10 6) Provide adeo	quate drainage to	prevent	water ponding	j .					
	4-5=-259	/102 5-6=-1	205/110, 3-4=-101/1	13, 7) Plates check	ked for a plus or mi	inus 5 de	egree rotation					munn	11111
BOT CHORD	1-10=-57	/125. 9-10=	-19/76. 8-9=-19/80.	0	about its cen	iter.	om ohou	dhooring					W'TH CA	Roll
	7-8=-21/7	7, 6-7=-57	/125	o Q	Gable requir	spaced at 4-0-0 or		u bearing.				S'	R	ALINI,
WEBS	3-9=-286	/45, 4-8=-28	86/23, 2-10=-484/25	3, 1	0) This truss ha	as been designed f	ora 10) psf bottom				1	OFESS	Strain 1
	5-7=-484	/254			chord live loa	ad nonconcurrent	with anv	other live load	ds.			UB		Jan 4
NOTES				1	1) * This truss h	has been designed	for a liv	e load of 20.0)psf		1		2	
1) Unbalanc	ed roof live	loads have	been considered for		on the bottor	m chord in all area	s where	a rectangle	-		-	:	SEA	1 : =
this desig	n.				3-06-00 tall b	by 2-00-00 wide wi	II fit betw	veen the botto	om		Ξ			
					chord and ar	ny other members,	with BC	DL = 10.0psf			Ξ.		0363	22 : 5
				1	 Provide mec 	hanical connection	n (by oth	ers) of truss to	0		-			1 2
					1 56 lb uplife	e capable of withst	anoing t	nt 10 and 70	unt			5	1. A.	all S
					uplift at joint	7.	Jint at ju					25	NGIN	EETAN
				1	 Beveled plat 	e or shim required	to provi	de full bearing	3			11,	710	allin
				-	surface with	truss chord at join	t(s) 1, 6.						IN A G	ILDIN

- bearing plate capable of withstanding 56 lb uplift at joint 1, 56 lb uplift at joint 6, 72 lb uplift at joint 10 and 70 lb uplift at joint 7.
- 13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 6.



GI 11111111 May 28,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	V2	Valley	1	1	Job Reference (optional)	173750780

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 ID:C3ZDsgkjj8sqWLvsJrvtN9zeeq4-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:57.5

Plate Offsets (X, Y):	[3:0-4-1,0-3-0], [4:0-4-1,0-3-0]
-----------------------	----------------------------------

		-												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2027	I/TPI2014	CSI TC BC WB Matrix-MS	0.44 0.67 0.14	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 14	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 60 lb	GRIP 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3 Structural wood she 6-0-0 oc purlins, exc 2-0-0 oc purlins (6-0 Rigid ceiling directly bracing. (size) 1=12-5-4, 8=12-5-4, Max Horiz 1=-83 (LC Max Uplift 1=-130 (LC 7=-87 (LC 10=-88 (L 	athing directly applied pot -0 max.): 3-4. applied or 10-0-0 oc 6=12-5-4, 7=12-5-4, 9=12-5-4, 10=12-5-4 (12) C 70), 6=-130 (LC 72) (17), 9=-16 (LC 13), C 16)	2) d or , 3) 4 2), 4)	Wind: ASCE Vasd=95mph II; Exp B; En Exterior(2E) 5-7-15, Exterior left and right exposed;C-C reactions sho DOL=1.60 Truss design only. For stu see Standard or consult qu TCLL: ASCE Plate DOL=	7-16; Vult=120m r; TCDL=6.0psf; closed; MWFRS 0-4-15 to 3-4-15, ior(2E) 5-7-15 to iterior (1) 10-115 exposed ; end vec for members an pown; Lumber DOI ed for wind loads ds exposed to w d Industry Gable alified building dv 7-16; Pr=20.0 ps .15); Pg=20.0 ps .15); Pg=20.0 ps .15); Pg=20.0 ps	ph (3-sec BCDL=6.0 (envelope Interior (1 6-9-5, Ex 2 to 12-0-1 errical left d forces & L=1.60 pla s in the pla ind (norm: End Detai esigner as sf (roof LL f; Pf=20.4 0. Pough	ond gust))psf; h=25ft; () and C-C) 3-4-15 to terior(2R) 6-5 5 zone; cantil and right k MWFRS for ate grip ane of the trus ate of the trus ate of the trus ate of the trus ate of the trus the face) Is as applicat per ANSI/TF : Lum DOL=1 psf (Lum DC cat P: Partial	Cat. 	13) This load pan Bott 14) Gra or th bott LOAD (truss h l of 250. els and om Cho phical p he orient om chor CASE(S)	as bee Olb live at all p rd, nor urlin re tation c d. Star	n designed for a e and 3.0lb dead anel points along neconcurrent with presentation doe of the purlin along ndard	moving conce located at all i the Top Cho any other live is not depict th the top and/o	entrated mid rd and loads. ne size or
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalanc this desig	Max Grav 1=237 (LC 7=496 (LC 9=345 (LC (lb) - Maximum Com Tension 1-2=-194/159, 2-3=- 4-5=-229/89, 5-6=-1 1-10=-40/77, 9-10=- 7-8=-28/78, 6-7=-40 3-9=-262/47, 4-8=-2 5-7=-480/307 ced roof live loads have gn.	C 83), 6=164 (LC 89) C 45), 8=345 (LC 94) C 93), 10=496 (LC 45 pression/Maximum 229/102, 3-4=-79/11 81/139 25/77, 8-9=-26/78, /78 62/28, 2-10=-480/300 been considered for	, 5) 5) 6) 7) 7, 8) 9) 6, 10 6, 11	 1.15 Plate DU Exp.; Ce=1.0 Unbalanced design. Provide adec Plates check about its cen Gable studs This truss ha chord live loa * This truss ha chord live loa * This truss ha chord live loa * This truss ha chord and ar Provide mec bearing plate 1, 30 lb upli at joint 10, 8 and 130 lb upli 	UL = 1.15); (s=1.); Cs=1.00; Ct=1. snow loads have quate drainage to ed for a plus or n ter. es continuous bo spaced at 4-0-0 (s been designed ad nonconcurrent has been designed ad nonconcurrent in chord in all are: y 2-00-00 wide v by other members hanical connectio c capable of withs ft at joint 6, 16 lb plift at joint 6.	u) Rough 10, Lu=50 been con prevent whinus 5 de ttom chore to. for a 10.0 with any d for a live as where vill fit betw s. on (by oth tsanding 1 uplift at jc 7, 130 lb u	Cat B; Partial I-0-0 sidered for th vater ponding gree rotation d bearing. 0 psf bottom other live load e load of 20.0 a rectangle reen the botto sers) of truss tr 30 lb uplift at joint 9, 88 lb up uplift at joint 1	nis g. ds. opsf om joint plift		M. CONTINUE.	The second secon	SEA 0363	ROW IN L 22 EERER	and an

- this design.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 1, 130 lb uplift at joint 6, 16 lb uplift at joint 9, 88 lb uplift at joint 10, 87 lb uplift at joint 7, 130 lb uplift at joint 1 and 130 lb uplift at joint 6.



G 11111111 May 28,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	V3	Valley	1	1	Job Reference (optional)	173750781

Structural LLC Thurmont MD - 21788

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 ID:KIKi1JhCfvMO1kb540qxCJzeeq8-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

5-0-10



Scale = 1:52													
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.65	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.98	Vert(TL)	n/a	-	n/a	999			
TCDL	10.0	Rep Stress Incr	YES	WB	0.35	Horiz(TL)	-0.01	3	n/a	n/a			
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MS									
BCDL	10.0										Weight: 41 lb	FT = 20%	
	2x4 SP No 2		4) TCLL: ASC Plate DOL=	E 7-16; Pr=20.0 =1.15): Pa=20.0 p	psf (roof LL sf: Pf=15.4	L: Lum DOL=	1.15 DL =						

BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3 BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. REACTIONS (size) 1=10-1-4, 3=10-1-4, 4=10-1-4 Max Horiz 1=-78 (LC 14) Max Uplift 1=-253 (LC 51), 3=-1 (LC 17) Max Grav 1=252 (LC 47), 3=6 (LC 54), 4=910 (LC 51) FORCES (Ib) - Maximum Compression/Maximum

Tension TOP CHORD 1-2=-249/545, 2-3=-244/546 BOT CHORD 1-4=-366/180, 3-4=-393/175 WEBS 2-4=-810/159

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 5-0-10, Exterior(2R) 5-0-10 to 8-0-10, Interior (1) 8-0-10 to 10-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed:C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable. or consult qualified building designer as per ANSI/TPI 1.

- 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this desian.
- Plates checked for a plus or minus 5 degree rotation 6) about its center.
- Gable requires continuous bottom chord bearing. 7)
- 8) Gable studs spaced at 4-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 1, 1 lb uplift at joint 3 and 1 lb uplift at joint 3
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3, 9.
- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads. LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	V4	Valley	1	1	Job Reference (optional)	173750782

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:31 ID:oUt4EeiqQDUFfuAHdjMAIWzeeq7-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

3-10-10



Scale = 1:40.6

		1												
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.50	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.67	Vert(TL)	n/a	-	n/a	999			
TCDL	10.0	Rep Stress Incr	YES		WB	0.12	Horiz(TL)	0.00	4	n/a	n/a			
BCLL	0.0*	Code	IRC2021	/TPI2014	Matrix-MP									
BCDL	10.0											Weight: 31 lb	FT = 20%	
LUMBER TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.3		4)	TCLL: ASCE Plate DOL=1 1.15 Plate D	: 7-16; Pr=20.0 l.15); Pg=20.0 p OL = 1.15); Is=	psf (roof LL osf; Pf=15.4 1.0; Rough	.: Lum DOL=′ ⊧psf (Lum DC Cat B; Partia	1.15 DL = Ily						
OTHERS	2x4 SP No.3		-	Exp.; Ce=1.0); Cs=1.00; Ct=	1.10								
BRACING			5)	Unbalanced	snow loads hav	e been cor	isidered for th	าเร						
TOP CHORD	Structural wood sheat 7-8-12 oc purlins.	athing directly applie	ed or 6)	Plates check	ed for a plus or	minus 5 de	egree rotation	n						
BOT CHORD	Rigid ceiling directly bracing.	applied or 6-0-0 oc	7)	Gable requir	iter. es continuous b	ottom chor	d bearing.							

 REACTIONS
 (size)
 1=7-9-4, 3=7-9-4, 4=7-9-4
 8)
 Gable studs

 Max Horiz
 1=-59 (LC 14)
 9)
 This truss

 Max Uplift
 1=-59 (LC 45), 3=-59 (LC 44), 4=-6 (LC 16)
 10) * This truss

 Max Grav
 1=268 (LC 47), 3=268 (LC 51),
 3-06-00 tall

	4=578 (LC 45)
FORCES	(Ib) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-2=-203/274, 2-3=-203/274
BOT CHORD	1-4=-184/192, 3-4=-184/192
WEBS	2-4=-482/249

NOTES

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust)
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 3-10-10, Exterior(2R) 3-10-10 to 7-0-13, Interior (1) 7-0-13 to 7-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 8) Gable studs spaced at 4-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads.
 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 1, 59 lb uplift at joint 3 and 6 lb uplift at joint 4.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.
- 13) This truss has been designed for a moving concentrated load of 250 0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
 LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof		
	V5	Valley	1	1	Job Reference (optional)	173750783	

Structural, LLC, Thurmont, MD - 21788

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:32 ID:oUt4EeiqQDUFfuAHdjMAIWzeeq7-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:36

. .

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.39	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 21 lb	FT = 20%
			5) Unbalanced	snow loads hav	a boon cor	sidered for th	aic					

LOWIDER		
TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.3
OTHERS	2x4 SP N	0.3
BRACING		
TOP CHORD	Structural 5-4-12 oc	l wood sheathing directly applied or purlins.
BOT CHORD	Rigid ceili bracing.	ing directly applied or 6-0-0 oc
REACTIONS	(size)	1=5-5-4, 3=5-5-4, 4=5-5-4
	Max Horiz	1=-40 (LC 14)
	Max Uplift	1=-30 (LC 50), 3=-30 (LC 48)
	Max Grav	1=276 (LC 47), 3=276 (LC 51), 4=428 (LC 54)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	·
TOP CHORD	1-2=-211/	(186, 2-3=-211/186
BOT CHORD	1-4=-95/1	34, 3-4=-95/134
WEBS	2-4=-281/	/142
NOTES		
-		

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- 5) is have been considered for this design.
- 6) Plates checked for a plus or minus 5 degree rotation
- about its center.
- Gable requires continuous bottom chord bearing. 7)
- Gable studs spaced at 4-0-0 oc. 8)
- This truss has been designed for a 10.0 psf bottom 9) chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1 and 30 lb uplift at joint 3.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 3.
- 13) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty Ply Elmhurst R		Elmhurst Rev 3-Elev 5-Roof	
	V6	Valley	1	1	Job Reference (optional)	173750784

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:32 ID:oUt4EeiqQDUFfuAHdjMAIWzeeq7-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:21.6

Plate Offsets (X, Y): [2:0-2-0,Edge]

_oading FCLL (roof) Snow (Pf/Pg)	(psf) 20.0 15.4/20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI TC BC	0.18 0.39	DEFL Vert(LL) Vert(TL)	in n/a n/a	(loc) - -	l/defl n/a n/a	L/d 999 999	PLATES MT20	GRIP 244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 10 lb	FT = 20%
UMBER OP CHORD 30T CHORD 3RACING TOP CHORD 30T CHORD REACTIONS	2x4 SP No.2 2x4 SP No.3 Structural wood shea 3-0-12 oc purlins. Rigid ceiling directly bracing. (size) 1=3-1-4, 3 Max Horiz 1=22 (LC Max Grav 1=315 (LC	athing directly applie applied or 10-0-0 oc 3=3-1-4 13) 2 47), 3=315 (LC 51)	 7) Gable requir 8) Gable studs 9) This truss ha chord live loa d or 10) * This truss h on the bottor 3-06-00 tall b chord and ar 11) Beveled plats surface with 12) This truss ha load of 250 (250) 	es continuous botton spaced at 4-0-0 oc. s been designed for ad nonconcurrent wit nas been designed for n chord in all areas v by 2-00-00 wide will f yo other members. e or shim required to truss chord at joint(s s been designed for lb live and 3 0lb des	n chor a 10.0 h any or a liv where it betv provi) 1, 3. a mov d loca	d bearing.) psf bottom other live loa e load of 20.0 a rectangle veen the botto de full bearing ving concentrr. ted at all mid	ds.)psf om g ated					
ORCES	(lb) - Maximum Com Tension	pression/Maximum	panels and a Bottom Chor	t all panel points alo d. nonconcurrent wit	ng the	Top Chord a other live loa	and ds.					
FOP CHORD BOT CHORD	1-2=-343/70, 2-3=-3- 1-3=-49/242	43/71	LOAD CASE(S)	Standard								
NOTES												
I) Unbalance	ed roof live loads have	been considered for										
this design 2) Wind: AS(Vasd=95n II; Exp B; Exterior(2 vertical lef forces & M	n. CE 7-16; Vult=120mph nph; TCDL=6.0psf; BC Enclosed; MWFRS (en E) zone; cantilever left t and right exposed;C- IWFRS for reactions sl	(3-second gust) DL=6.0psf; h=25ft; C velope) and C-C and right exposed ; d C for members and hown; Lumber	at. end							and the second se	NITH CA	ROLIN

- DOL=1.60 plate grip DOL=1.60
 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP11.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL= 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

A MiTek Affili 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	A2T	Roof Special	4	1	Job Reference (optional)	173750785

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:24 ID:bgSfFIWI2bhwR4rVXCcjwLzeesy-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:78.8

Plate Offsets (X, Y): [2:0-2-14,0-2-0], [9:0-4-15,0-2-8], [11:Edge,0-1-8], [16:0-4-8,0-2-0]

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		тс	0.73	Vert(LL)	-0.19	11-12	>999	360	MT20HS	187/143
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.84	Vert(CT)	-0.31	16-17	>999	240	MT20	244/190
TCDL	10.0	Rep Stress Incr	YES		WB	0.95	Horz(CT)	0.12	11	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-AS		Wind(LL)	0.05	17-18	>999	240		
BCDL	10.0											Weight: 202 lb	FT = 20%
LUMBER			2	Wind: ASCE	7-16; Vult=120m	nph (3-sec	cond qust)						
TOP CHORD	2x4 SP SS *Except	* 1-5.7-10:2x4 SP No	o.2	Vasd=95mp	h; TCDL=6.0psf;	BCDL=6.0	.) Dpsf; h=25ft;	Cat.					
BOT CHORD	2x4 SP No.2 *Exce	ot* 19-3.16-15:2x4 S	P	II; Exp B; En	closed; MWFRS	(envelope	and C-C						
	No.3, 18-16,13-11:2	2x4 SP SS		Exterior(2E)	-1-0-0 to 2-1-12,	Interior (1) 2-1-12 to						
WEBS	2x4 SP No.3 *Exce	ot* 11-9:2x6 SP No.2	2	15-8-8, Exte	rior(2R) 15-8-8 to	o 18-10-3,	Interior (1)						
BRACING				18-10-3 to 3	2-5-0 zone; canti	lever left a	and right						
TOP CHORD	Structural wood she	eathing directly applie	ed	exposed ; er	nd vertical left and	d right exp	osed;C-C fo	r					
	except end verticals	3.	ou,	members an	d forces & MWFI	RS for rea	ctions show	n;					
BOT CHORD	Rigid ceiling directly	/ applied.		Lumber DOL	_=1.60 plate grip	DOL=1.60)						
REACTIONS	(size) 11=0-3-8	. 20=0-3-8	3	TCLL: ASCE	7-16; Pr=20.0 p	sf (roof LL	: Lum DOL=	1.15					
	Max Horiz 20=175 (LC 15)		Plate DOL=1	1.15); Pg=20.0 ps	st; Pt=15.4	pst (Lum D	OL =					
	Max Grav 11=1316	(LC 2), 20=1310 (LC	C 2)	1.15 Plate D	OL = 1.15; IS=1.	.0; Rough	Cat B; Partia	ally					
FORCES	(lb) - Maximum Cor	noression/Maximum	,	Exp.; Ce=1.0	J; CS=1.00; Ct=1	.10 boon oor	oidorod for t	hio					
IONOLO	Tension	npression/maximum	4	dooign	show loads have	e been cor	isidered for i	nis					
TOP CHORD	1-2=0/47 2-3=-273	7/46 3-4=-2147/64	5	This trues ha	e haan dasignag	t for areat	ar of min roo	flivo					
	4-6=-1522/113.6-8	=-1525/143.	5	load of 12 0	nsf or 2 00 times	flat roof lo	ad of 15.4 r	sf on					
	8-9=-1801/70, 9-10	=0/51, 2-20=-1302/5	0.	overhands n	on-concurrent wi	ith other liv	/e loads	51 011					
	9-11=-1274/99	,	6	All plates are	MT20 plates un	less other	wise indicate	ьq					
BOT CHORD	19-20=-53/212, 18-	19=-28/256, 3-18=0/	471, 7	Plates check	ed for a plus or r	minus 5 de	earee rotatio	n					
	17-18=-31/2573, 16	6-17=0/1795, 15-16=	0/175,	about its cer	iter.		- <u>-</u>						
	14-15=-163/101, 12	2-14=0/1447,	8	This truss ha	as been designed	d for a 10.0) psf bottom						
	11-12=-28/563			chord live loa	ad nonconcurren	t with any	other live loa	ads.				N''L CA	E III
WEBS	3-17=-788/59, 4-17	=0/466, 4-16=-767/8	1, 9	* This truss I	has been designe	ed for a liv	e load of 20.	0psf				TH UA	Dille
	8-12=0/301, 2-18=-	8/2140, 9-12=0/956,		on the bottor	m chord in all are	as where	a rectangle				2	A SECO	in VIA'L
	6-14=-65/323, 8-14	=-485/89, 6-16=0/76	4,	3-06-00 tall I	oy 2-00-00 wide v	will fit betw	veen the both	tom		/	52	OFEND	Ningin
	14-16=0/1099			chord and a	ny other member	s.				9		the second	
NOTES			1	This truss has	as been designed	d for a mov	ing concent	rated			1	:• / •	· · · · ·
1) Unbalance	ed roof live loads have	e been considered fo	r	load of 250.0	Ib live and 3.0lb	dead loca	ited at all mid	b		=		SEA	L 1 3
this desig	n.			panels and a	at all panel points	along the	Top Chord	and		=		0000	
				Bottom Chor	d, nonconcurren	t with any	other live loa	ads.		=		0363	22
			1	I) This truss de	esign requires the	at a minim	um of 7/16"				i (•	1 E -
				structural wo	od sheathing be	applied d	rectly to the	top			-	1. A.	- 1 S
				chord and 1/	2" gypsum sheet	trock be a	pplied direct	y to			- 0	N. SNOW	FRIAN
				the bottom c	nord.						1	Q. GIN	E. A.N

LOAD CASE(S) Standard



Page: 1

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



C

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	170750707	
	A1C	Common	1	1	Job Reference (optional)	173750787	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:22 ID:U72Srl8_YTGyjXutGMSmGPzeelh-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:82.3

Plate Offsets (X, Y): [15:0-2-9,Edge]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(pst) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2027	1/TPI2014	TC BC WB Matrix-AS	0.70 0.64 0.83	Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.36 -0.88 0.12 0.06	(loc) 16 14-16 11 17-18	l/defi >999 >477 n/a >999	L/d 360 240 n/a 240	MT20 M18AHS Weight: 232 lb	GRIP 244/190 186/179 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD WEBS	2x4 SP SS 2x4 SP SS 2x4 SP No.3 *Excep Left 2x6 SP No.2 1-6-0 Structural wood she Rigid ceiling directly 1 Row at midpt (size) 1=0-3-8, 1 Max Horiz 1=-169 (L Max Grav 1=1521 (L (lb) - Maximum Com Tension 1-3=-2291/98, 3-5=- 6-7=-2199/0, 7-9=-2 11-12=0/41 1-18=-34/1857, 16-1 13-14=0/1727, 11-13 9-13=-210/113, 3-18 5-17=-496/219, 17-1	t* 19-20:2x4 SP SS 1-6-0, Right 2x6 SP N athing directly applied applied. 19-20 11=0-3-8 C 14) C 2), 11=1583 (LC 2 pression/Maximum 2134/0, 5-6=-2180/0, 131/0, 9-11=-2288/15 8=0/1723, 14-16=0/1 3=-8/1853 i=-212/114, 9=0/955, 6-19=0/100 1=0/1019, 14-20=0/96	3) lo.2 4) 5) d. 6) 7) 8) 9) 52, 10 52, 11 18, 57,	TCLL: ASCE Plate DOL=1 1.15 Plate DO Exp.; Ce=1.0 Unbalanced design. This truss ha load of 12.0 µ overhangs no 250.0lb AC u from left end, All plates areck about its cen This truss ha chord live loa) * This truss ha on the bottom 3-06-00 tall b chord and ar) This truss ha load of 250.0 panels and a Bottom Chor	7-16; Pr=20.0 ps .15); Pg=20.0 ps DL = 1.15); Is=1.0; ; Cs=1.00; Ct=1.1 snow loads have s been designed d osf or 2.00 times f on-concurrent with nit load placed or supported at two MT20 plates unle ed for a plus or m ter. s been designed d nonconcurrent as been designed n chord in all area s been designed b live and 3.0lb c t all panel points a d nonconcurrent	f (roof LL ; Pf=15.4 ; Rough 10 been cor for great ilat roof kn n other lim n other lim n other lim n the bott points, { ess other inus 5 de for a 10.0 with any d for a liv is where ill fit betw for a movies along the with any	.: Lum DOL= psf (Lum DC Cat B; Partia asidered for th er of min roof pad of 15.4 p; ve loads. om chord, 17 5-0-0 apart. wise indicate agree rotation 0 psf bottom other live loa e load of 20.0 a rectangle veen the bottow ving concentri ted at all mid Top Chord a other live loa	1.15 DL = IIV IIV sf on f-5-8 d. ds. Dpsf om ated and ds.				Marth CA	Rovin	
NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; I Exterior(21 S5-11-0 zc vertical lef	19-21=-44/0, 20-21= 7-13=-171/236, 16-2 ad roof live loads have b. CE 7-16; Vult=120mph ph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) 0-0-0 to 3-5-14, Inte R) 17-5-8 to 20-11-6, In ne; cantilever left and t and right exposed;C-	44/0, 5-18=-149/24; 21=0/169 been considered for (3-second gust) DL=6.0psf; h=25ft; C ivelope) and C-C rior (1) 3-5-14 to 17- interior (1) 20-11-6 to right exposed ; end C for members and	13, 12 13 LC at. 5-8,	 This trusts de structural wo chord and 1/2 the bottom cl Attic room ch DAD CASE(S) 	a, nonconcurrent od sheathing be a 2" gypsum sheetr oord. ecked for L/360 c Standard	applied di applied di ock be a	um of 7/16" rectly to the toplied directly	iop / to		Manna and and and and and and and and and		SEA 0363	22 EREALING	Ammin

II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-5-14, Interior (1) 3-5-14 to 17-5-8, Exterior(2R) 17-5-8 to 20-11-6, Interior (1) 20-11-6 to 35-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof		
	H3AG	Roof Special Structural Gable	1	1	Job Reference (optional)	173750788	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:29 ID:Falt9NpfWfjANha8kplqKizeeWd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:100.9

Continued on page 2

Plate Offsets (X, Y): [8:0-5-8,0-2-0]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202 ⁻	1/TPI2014	CSI TC BC WB Matrix-AS	0.77 0.96 0.70	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.25 -0.34 0.04 0.03	(loc) 25-26 25-26 18 23-25	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20HS MT20 Weight: 256 lb	GRIP 187/143 244/190 p FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 *Excep 2x4 SP No.3 *Excep No.2 2x4 SP No.3 Structural wood shear except end verticals (4-7-3 max.): 6-8.	t* 24-27:2x4 SP SS t* 20-12,26-1:2x4 SP athing directly applied , and 2-0-0 oc purlins	, , 1)	EBS 5 2 2 1 4 4 1 1 1 1 5 7 ES	82, 6-22=-563 34, 8-20=-314 70, 29-30=0/1 /76, 107, 4-25=0/4 5/0, 49, 5/132, 61	 9) Prates criecked for a plus or minus 5 degree rotation about its center. 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web). 11) Gable studs spaced at 2-0-0 oc. 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 								
WEBS JOINTS	Rigid ceiling directly 1 Row at midpt 1 Brace at Jt(s): 28, 29, 31 (size) 16=3-9-8,	applied. 6-23, 2-26, 4-23 17=3-9-8, 18=0-3-8, 1	1) 2) 26=	Unbalanced this design. Wind: ASCE Vasd=95mph II; Exp B; End	considered for cond gust) Dpsf; h=25ft; C e) and C-C	Cat.	 14) Refer to girder(s) for truss to truss connections. 15) N/A 							
	Mechanic Max Horiz 26=-172 (Max Uplift 16=-104 (Max Grav 16=234 (L 18=2132 (al LC 12) LC 70), 17=-372 (LC 9 .C 115), 17=175 (LC (LC 63), 26=1370 (LC	93) 114), 34)	Exterior(2E) 0-3-4 to 3-9-2, Interior (1) 3-9-2 to 15-5-8, 16) 7 Exterior(2R) 15-5-8 to 18-11-6, Interior (1) 18-11-6 to 16) 7 23-11-15, Exterior(2R) 23-11-15 to 27-5-14, Interior (1) 1 27-5-14 to 35-8-0 zone; cantilever left and right 1 exposed; end vertical left and right 1						16) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.				
FORCES TOP CHORD	(lb) - Maximum Com Tension 1-2=-542/38, 2-4=-19 5-6=-1476/144, 6-7= 7-8=-1486/116, 8-9= 9-10=-1363/83, 10-1 11-12=-1373/15, 12- 13-14=-21/357, 14-1 14-16=-196/156	pression/Maximum 932/114, 4-5=-1474/1 -1486/116, -1302/100, 1=-1377/58, 13=0/389, 5=0/47, 1-26=-408/40	52, ³⁾ 4)	members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL = 1.15 Plate DOL= 1.15); Is=1.0; Rough Cat B; Partially SEAL						AROUNING SILANA				
BOT CHORD	26-27=0//0, 25-26=-1 22-23=0/1424, 20-22 18-19=-292/52, 17-1	2/1631, 23-25=0/1379 2=0/1089, 19-20=-292 8=-292/52, 16-17=-29	^{9, 5)} 2/52, ⁵⁾ 9/91 6) 7) 8)	Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0 Unbalanced snow loads have been considered for this design. This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads. Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated.						LITAN.			JEER AND	

- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.

May 28,2025

Page: 1



Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE WARNING Design valid for use only with MTek connectors. This design is based only upon parameters and property incorporate this design is based only upon parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

	Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof		
		H3AG	Roof Special Structural Gable	1	1	Job Reference (optional)	173750788	
Structural, LLC, Thurmont, MD - 21788,			Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:29					

17) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

18) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



ID:Falt9NpfWfjANha8kplqKizeeWd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication for the trust structure Bucking Component Advancement and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss Truss Type		Qty	Ply	Elmhurst Rev 3-Elev 5-Roof			
	A2A	Common	1	1	Job Reference (optional)	173750789		

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:24 ID:bgSfFIWI2bhwR4rVXCcjwLzeesy-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:74.7

Plate Offsets	s (X, Y):	[7:0-4-15,0-2-8],	[9:Edge,0-1-8]
---------------	-----------	-------------------	----------------

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL LUMBER TOP CHORD BOT CHORD WEBS BRACING	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0 2x4 SP No.2 2x4 SP SS 2x4 SP No.3 *Excep	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021 3)	I/TPI2014 TCLL: ASCE Plate DOL=1 1.15 Plate D Exp.; Ce=1.0 Unbalanced	CSI TC BC WB Matrix-AS 7-16; Pr=20.0 psf; 1.5); Pg=20.0 psf; 0L = 1.15); Is=1.0; ; Cs=1.00; Ct=1.1 snow loads have b	1.00 0.55 0.48 (roof Ll Pf=15.4 Rough 0 peen cor	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL) :: Lum DOL=: psf (Lum DC Cat B; Partia usidered for th	in -0.25 -0.34 0.04 0.03 1.15 DL = Ily nis	(loc) 10-11 11-12 9 11-12	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20HS MT20 Weight: 183 lb	GRIP 187/143 244/190 FT = 20%	
TOP CHORD	Structural wood sheat except end verticals. Rigid ceiling directly	athing directly applie applied. 2.11 - 6.11	d, 5)	design. This truss ha load of 12.0 overhangs ne	s been designed for osf or 2.00 times floon-concurrent with	or great at roof le other li	er of min roof bad of 15.4 ps ve loads.	live sf on						
REACTIONS	(size) 9=0-3-8, 1 Max Horiz 13=-173 (I Max Grav 9=1448 (L	2-11, 6-11 3= Mechanical LC 12) .C 35), 13=1391 (LC	6) 7) 34) 8)	All plates are Plates check about its cen This truss ha	MT20 plates unle ed for a plus or mi ter. s been designed fo	ss other nus 5 de or a 10.	wise indicate egree rotation	d.						
FORCES TOP CHORD BOT CHORD WEBS	(lb) - Maximum Com Tension 1-2=-2065/67, 2-4=- 4-6=-1568/127, 6-7= 1-13=-1394/61, 7-9= 13-14=0/0, 12-13=-1 9-10=-31/568 2-12=-8/289, 2-11=-6 6-11=-677/85, 6-10= 7-10=0/1182	pression/Maximum 1569/127, -2139/67, 7-8=0/51, -1469/97 03/469, 10-12=0/173 614/86, 4-11=-1/102 0/329, 1-12=0/1323,	 chord live loa * This truss h on the bottor 3-06-00 tall b chord and ar) Refer to girdd) This truss ha load of 250.0 panels and a Bottom Chor 	The bottom chord in all areas where a rectangle 36-00 tall by 2-00-00 wide will fit between the bottom ord and any other members, with BCDL = 10.0psf. fier to girder(s) for truss to truss connections. is truss has been designed for a moving concentrated ad of 250.0lb live and 3.0lb dead located at all mid nels and at all panel points along the Top Chord and thom Chord nonconcurrent with any other live loads.										
NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E Exterior(2F 32-2-0 zon vertical left forces & M DOL=1.60	sign requires that od sheathing be a 2" gypsum sheetro hord. Standard	a minim pplied d ick be a	um of 7/16" rectly to the t oplied directly	op / to		Mannun	The second secon	SEA 0363		Name and Andrews				

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org)

and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



GI minin May 28,2025

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof		
	НЗА	Roof Special	1	1	Job Reference (optional)	173750790	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:29 ID:KX0AGsvctm1oB_SqIUTvXVzeWYV-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:76.9

Plate Offsets (X, Y): [6:0-2-0,Edge], [7:0-5-10,0-1-12]

Loading TCLL (roof) Snow (Pf/Pg) TCDL	(psf) 20.0 20.4/20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	/TDI2014	CSI TC BC WB Matrix-AS	0.86 0.67 0.27	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.23 -0.42 0.14	(loc) 14-16 16-17 12 17-19	l/defl >999 >999 n/a	L/d 360 240 n/a 240	PLATES MT20 MT20HS M18AHS	GRIP 244/190 187/143 186/179
BCDL	10.0	0000	1102021		Mainx 710		(LL)	0.00	11 10	2000	210	Weight: 217 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER	2x4 SP No.2 *Excep SS 2x4 SP SS 2x4 SP No.3 Left 2x6 SP No.2 1 2-0-0	t* 8-9,4-1,9-13:2x4 \$ I-6-0, Right 2x6 SP	2) SP No.2	Wind: ASCE Vasd=95mpt II; Exp B; En Exterior(2E) Exterior(2E) 20-9-13, Exter 24-3-12 to 35	7-16; Vult=120 n; TCDL=6.0pst closed; MWFR 0-0-0 to 3-5-14 15-8-8 to 17-0- erior(2R) 20-9-1 5-11-0 zone; ca	 13) This truss design requires that a minimum of 2DL=6.0psf; h=25ft; Cat. nvelope) and C-C erior (1) 3-5-14 to 15-8-8, Interior (1) 17-0-13 to 224-3-12, Interior (1) ever left and right 13) This truss design requires that a minimum of structural wood sheathing be applied directly chord and 1/2" gypsum sheetrock be applied to the bottom chord. 14) Graphical purlin representation does not dep or the orientation of the purlin along the top bottom chord. 						nimum of 7/16" ed directly to the top be applied directly to is not depict the size g the top and/or	
BRACING TOP CHORD BOT CHORD	Structural wood sheat except 2-0-0 oc purlins (3-1 Rigid ceiling directly	athing directly applie 0-10 max.): 5-8. applied.	ed, 3)	exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=				r n; :1.15 OL =	LOAD C	ASE(S)	Star	ndard	

	2-0-0 00 putilits (3-10-10 max.). 5-8.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 7-17, 10-16, 7-16, 3-17
REACTIONS	(size) 1=0-3-8, 12=0-3-8
	Max Horiz 1=-153 (LC 14)
	Max Grav 1=1686 (LC 34), 12=1770 (LC 63)
FORCES	(lb) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-3=-2716/191, 3-5=-2343/0, 5-6=-237/51,
	6-7=-235/107, 5-7=-2004/0, 7-8=-1972/0,
	8-10=-2344/0, 10-12=-2812/206, 12-13=0/41
BOT CHORD	1-19=-20/2217, 17-19=0/2217, 16-17=0/1903,
	14-16=0/2303, 12-14=0/2303
WEBS	8-16=0/731, 7-17=-196/175, 10-14=0/306,
	10-16=-541/129, 3-19=0/298, 5-17=0/720,
	7-16=-177/170, 3-17=-391/116
NOTES	

1) Unbalanced roof live loads have been considered for this design.

1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0 Unbalanced snow loads have been considered for this 4) desian. This truss has been designed for greater of min roof live 5)

load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads. 250.0lb AC unit load placed on the bottom chord, 17-5-8

from left end, supported at two points, 5-0-0 apart. 7)

Provide adequate drainage to prevent water ponding.

- All plates are MT20 plates unless otherwise indicated. 8) 9) Plates checked for a plus or minus 5 degree rotation about its center
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf 11) on the bottom chord in all areas where a rectangle
- 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 12) This truss has been designed for a moving concentrated
- load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- SEAL 036322 G mmm May 28,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancement description (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof			
	A2AT	Roof Special	1	1	Job Reference (optional)	173750791		

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:24 Page: 1 ID:bgSfFIWI2bhwR4rVXCcjwLzeesy-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:83.9

Plate Offsets (X, Y): [8:0-4-15,0-2-8], [10:Edge,0-1-8], [15:0-2-8,0-3-0]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-AS	0.72 0.75 0.80	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.19 -0.28 0.10 0.04	(loc) 10-11 10-11 10 16-17	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 200 lb	GRIP 244/190 187/143 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD WEBS	2x4 SP SS *Except* 2x4 SP No.2 *Except No.3, 17-15,12-10:2x 2x4 SP No.3 *Except Structural wood sheat except end verticals. Rigid ceiling directly (size) 10=0-3-8, Max Horiz 19=-173 (I Max Grav 10=1302 ((lb) - Maximum Com Tension 1:2=-2317/48, 2-3=-2 5-7=-1502/143, 7-8= 1:19=-1218/21, 8-10 19-20=0/0, 18-19=-7 2:17=0/309, 16-17=- 14-15=0/175, 13-14= 10-11=-27/561 2:16=-596/94, 3-16= 7-11=0/302, 8-11=0/ 5:13=-63/333, 13-15	4-1,6-9:2x4 SP No.2 * 18-2,15-14:2x4 SP x4 SP SS * 10-8:2x6 SP No.2 athing directly applied applied. 19= Mechanical LC 12) LC 2), 19=1231 (LC pression/Maximum 2033/64, 3-5=-1483/1 -1777/70, 8-9=0/51, =-1260/99 8/159, 17-18=-51/24 37/2209, 15-16=0/16 =-141/113, 11-13=0/1 0/433, 3-15=-682/77 940, 5-15=0/739, =0/1061, 7-13=-489/	3) 4) 5) 6) 7) 2) 8) 9) 116, 9) 110, 6, 10) 69, 11) 90, 12)	TCLL: ASCE Plate DOL=1 1.15 Plate DO Exp.; Ce=1.0 Unbalanced : design. This truss ha load of 12.0 tp overhangs no All plates are Plates check about its cen This truss ha chord live loa * This truss ha on the bottom 3-06-00 tall b chord and an Plates to girdd This truss ha load of 250.0 panels and a Bottom Chorr	7-16; Pr=20.0 psf 15); Pg=20.0 psf; DL = 1.15); Is=1.0; ; Cs=1.00; Ct=1.1; snow loads have b s been designed find s been designed find for a plus or miter. s been designed find n concurrent with MT20 plates unle ed for a plus or miter. s been designed find n chord in all areas y 2-00-00 wide willy y other members. ar(s) for truss to tru- s been designed find b live and 3.01b find t all panel points ad n cnocncurrent with b live and 3.01b find t all panel points ad n conconcurrent with b live and 3.01b find t all panel points ad n conconcurrent with sign requires that b and the panel points ad the panel	(roof LL Pf=15.4 Rough open cor or greate at roof le other liv so other liv so other liv so other liv so other liv so other liv liv so other liv so other liv liv liv liv liv liv liv liv liv liv	:: Lum DOL=' psf (Lum DC Cat B; Partia isidered for th er of min roof pad of 15.4 ps re loads. wise indicate agree rotation 0 psf bottom other live loa e load of 20.0 a rectangle reen the botto a rectangle reen the botto ections. ring concentri- ted at all mid other live loa um of 7/16"	1.15 DL = Ily iis iive sf on d. ds. Dpsf om ated ind ds. op			and the second	TH CA	ROLIN	
NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; E Exterior(2E S2-2-0 20m vertical left forces & M DOL=1.60	ed roof live loads have E 7-16; Vult=120mph iph; TCDL=6.0psf; BCI Enclosed; MWFRS (en E) 0-3-4 to 3-4-15, Inte R) 15-5-8 to 18-7-3, Int e; cantilever left and ri t and right exposed;C-1 IWFRS for reactions sf plate grip DOL=1.60	been considered for (3-second gust) DL=6.0psf; h=25ft; C velope) and C-C rior (1) 3-4-15 to 15-5 erior (1) 18-7-3 to ight exposed ; end C for members and hown; Lumber	LO at. 5-8,	the bottom ch AD CASE(S)	standard	ok de a	shuan nu acrit			A THINK .		SEAL O3632	ER. Kun	THUR DE LE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road Edenton, NC 27932

May 28,2025

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	B1GE	Common Supported Gable	1	1	Job Reference (optional)	173750792

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:25 ID:EMpxyo3shm5PEjzQ9dW7GczEGKq-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:47.6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	n/a	-	n/a	999			
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	10	n/a	n/a			
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS									
BCDL	10.0										Weight: 55 lb	FT = 20%	
			2) Wind ASCI	= 7-16: Vult=120	mnh (3-sec	ond aust)	_	14) This	s truss h	as hee	n designed for a	moving concen	trated

LUMBER		
TOP CHORD	2x4 SP N	0.2
BOT CHORD	2x4 SP N	0.2
WEBS	2x4 SP N	0.3
OTHERS	2x4 SP N	0.3
BRACING		
TOP CHORD	Structural except er	l wood sheathing directly applied, id verticals.
BOT CHORD	Rigid ceil	ing directly applied.
REACTIONS	(size)	10=10-7-0, 11=10-7-0, 12=10-7-0, 13=10-7-0, 14=10-7-0, 15=10-7-0, 16=10-7-0
	Max Horiz	16=-77 (LC 14)
	Max Uplift	10=-17 (LC 13), 11=-20 (LC 17), 12=-9 (LC 17), 14=-9 (LC 16), 15=-26 (LC 13), 16=-27 (LC 12)
	Max Grav	10=297 (LC 69), 11=312 (LC 68), 12=338 (LC 67), 13=334 (LC 66), 14=338 (LC 65), 15=312 (LC 64), 16=297 (LC 63)
FORCES	(lb) - Max Tension	imum Compression/Maximum
TOP CHORD	2-16=-28 3-4=-85/6 6-7=-85/6 8-10=-28	5/102, 1-2=0/47, 2-3=-44/95, /7, 4-5=-83/128, 5-6=-83/128, /8, 7-8=-44/95, 8-9=0/47, 5/100
BOT CHORD	15-16=-4	0/57, 14-15=-40/57, 13-14=-40/57, 0/57, 11-12=-40/57, 10-11=-40/57
WEBS	5-13=-27 6-12=-28	1/0, 4-14=-289/98, 3-15=-269/77, 9/98, 7-11=-269/76
NOTES		

 Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -1-0-0 to 2-0-0, Exterior(2N) 2-0-0 to 5-3-8, Corner (3R) 5-3-8 to 8-3-8, Exterior(2N) 8-3-8 to 11-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Plates checked for a plus or minus 5 degree rotation about its center.
- B) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads.
 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 16, 17 lb uplift at joint 10, 9 lb uplift at joint 14, 26 lb uplift at joint 15, 9 lb uplift at joint 12 and 20 lb uplift at joint 11.

14) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

Page: 1

- 15) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932

Job	Truss	Truss Type	Qty Ply		Elmhurst Rev 3-Elev 5-Roof	
	B2	Common	3	1	Job Reference (optional)	173750793

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:25 Page: 1 ID:T5sLrtAVZXE7p598B0AE8WzEGKh-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:38.4

Plate Offsets (X, Y): [2:0-7-15,Edge], [6:0-7-15,Edge]

		-				-								
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.53	Vert(LL)	-0.08	8-15	>999	360	MT20	244/190	
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15		BC	0.64	Vert(CT)	-0.09	8-15	>999	240			
TCDL	10.0	Rep Stress Incr	YES		WB	0.12	Horz(CT)	0.02	2	n/a	n/a			
BCLL	0.0*	Code	IRC20	21/TPI2014	Matrix-AS		Wind(LL)	0.01	8-11	>999	240			
BCDL	10.0											Weight: 50 lb	FT = 20%	
LUMBER			Ę) This truss ha	is been designed	for great	er of min root	live						
TOP CHORD	2x4 SP No.2			load of 12.0	psf or 2.00 times	flat roof lo	oad of 15.4 p	sf on						
BOT CHORD	2x4 SP No.2		on-concurrent wit	th other liv	r live loads.									
WEBS	2x4 SP No.3		6	 Plates check 	ed for a plus or n	ninus 5 de	egree rotatior	ו						
SLIDER	Left 2x6 SP No.2 1	1-6-0, Right 2x6 SP	No.2	about its cen	ter.	(AO)								
	1-6-0) This truss ha	is been designed	TOF a 10.0	J pst bottom	do						
BRACING				 CHOID IVE ID * This trues b 		d for a liv	o load of 20	lus. Doct						
TOP CHORD	Structural wood she	athing directly applie	ed. '	on the bottor	n chord in all area	as where	a rectande	opsi						
BOT CHORD	Rigid ceiling directly	applied.		3-06-00 tall b	ov 2-00-00 wide w	vill fit betv	veen the bott	om						
REACTIONS	(size) 2=0-3-8, 6	6=0-3-8		chord and ar	y other members	5.								
	Max Horiz 2=59 (LC	(15) 9) This truss h			s been designed	for a mov	ing concenti	ated						
	Max Grav 2=494 (LC	23), 6=494 (LC 24	·)	load of 250.0	lb live and 3.0lb	dead loca	ited at all mic	l						
FORCES	(lb) - Maximum Com	pression/Maximum		panels and a	t all panel points	along the	Top Chord a	and						
		166 4 6- 405/166		Bottom Chor	d, nonconcurrent	with any	other live loa	ids.						
TOP CHORD	1-2=0/41, 2-4=-425/ 6-7=0/41	100, 4-0=-425/100,	-	 I his truss de de 	sign requires that	t a minim	imum of 7/16"							
BOT CHORD	2-8=-54/342 6-8=-5	3/342		chord and 1/	2" avosum sheet	applied d	applied directly	uto						
WEBS	4-8=0/325	0/012		the bottom c	hord.	IOCK DE A		y 10						
NOTES			I	OAD CASE(S)	Standard									
1) Unbalance	ed roof live loads have	been considered for	r	.,										
this desigr	າ.											minin	UIL.	
2) Wind: ASC	CE 7-16; Vult=120mph	(3-second gust)										IN'LY CA	ROUL	
Vasd=95n	nph; TCDL=6.0psf; BC	DL=6.0psf; h=25ft; 0	Cat.								1.1	R		
II; Exp B; I	Enclosed; MWFRS (en	velope) and C-C								/	S.	OFESS	Di Vil	
Exterior(2)	E) -1-0-0 to 2-0-0, Inter	rior (1) 2-0-0 to 5-3-8	8,							2	17		non	
Exterior(2)	R) 5-3-8 to 8-3-8, Interi tilover left and right ovr	IOF (1) 8-3-8 to 11-7-	-U loft									:0	7: 3	
and right e	avposed C-C for memb	pers and forces &	ieit							-		CEA	r 1.3	
MWFRS f	or reactions shown. I u	mber DOI =1 60 pla	ite								:	SEA	L : :	
grip DOL=	:1.60									1	:	0363	22 : =	
3) TCLL: AS	CE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1	1.15							-	6		4 3	
Plate DOL	.=1.15); Pg=20.0 psf; F	Pf=15.4 psf (Lum DC	DL =								-	·	1 1 E	
1.15 Plate	DOL = 1.15); ls=1.0; F	Rough Cat B; Partial	lly								1.0	N. SNOW	EFR. AN	
Exp.; Ce=	1.0; Cs=1.00; Ct=1.10										1	PL	5. 64 1	
Unbalance	Unbalanced snow loads have been considered for this							IL BENN						

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



G mm May 28,2025

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof		
	B2A	Common	1	1	Job Reference (optional)	173750794	

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:25 ID:ipvlkyH8SIMrOULtDPrM?PzEGKY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:30.7

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-AS	0.53 0.64 0.12	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.08 -0.09 0.02 0.01	(loc) 6-13 6-13 1 6-9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 47 lb	GRIP 244/190 FT = 20%
LUMBER 6) This truss ha TOP CHORD 2x4 SP No.2 chord live loa BOT CHORD 2x4 SP No.2 7) * This truss ha WEBS 2x4 SP No.3 on the bottor SLIDER Left 2x6 SP No.2 1-6-0, Right 2x6 SP No.2 3-06-00 tall t - 1-6-0 This truss ha chord and ar BD CHORD This truss ha chord and ar BD CHORD This truss ha chord and ar BD CHORD This truss ha 8) This truss ha					s been designed ad nonconcurrent has been designed n chord in all area by 2-00-00 wide w hy other members o been designed	for a 10.0 with any d for a liv is where ill fit betw) psf bottom other live loa e load of 20.1 a rectangle yeen the bott	ads. Opsf om					
BRACING TOP CHORD BOT CHORD REACTIONS	Structural wood she Rigid ceiling directly (size) 1=0-3-8, 5 Max Horiz 1=-49 (LC Max Grav 1=465 (LC	athing directly applie applied. 5=0-3-8 : 12) C 43), 5=458 (LC 46)	8) ed. 9)	Inis truss ha load of 250.0 panels and a Bottom Chor This truss de structural wo chord and 1/	Ib live and 3.0lb c t all panel points a d, nonconcurrent sign requires that od sheathing be a 2" gypsum sheetr	lead loca along the with any a minim applied di ock be at	ted at all mid Top Chord a other live loa um of 7/16" rectly to the oplied directly	ated I and ads. top v to					
FORCES TOP CHORD BOT CHORD WEBS	(lb) - Maximum Com Tension 1-3=-434/153, 3-5=- 1-6=-66/344, 5-6=-6 3-6=-0/327	pression/Maximum 434/153 6/344	LC	the bottom c DAD CASE(S)	bord. Standard			,					
NOTES 1) Unbalance this design 2) Wind: ASC Vasd=95m II; Exp B; I Exterior(2I Exterior(2I Zone; cani and right e MWFRS fr grip DOL= 3) TCLL: AS Plate DOL 1.15 Plate	ed roof live loads have n. CE 7-16; Vult=120mph nph; TCDL=6.0psf; BC Enclosed; MWFRS (er E) 0-0-0 to 3-0-0, Interi R) 5-3-8 to 8-3-8, Inter tilever left and right exp exposed; C-C for memb or reactions shown; Lu =1.60 CE 7-16; Pr=20.0 psf; E] DDL = 1.15); Is=1.0; f DDL = 1.15); Is=1.0; f	been considered for (3-second gust) DL=6.0ps; h=25f; (vivelope) and C-C ior (1) 3-0-0 to 5-3-8 ior (1) 8-3-8 to 10-7- posed ; end vertical ioers and forces & mber DOL=1.60 pla roof LL: Lum DOL=1 2f=15.4 psf (Lum DC Rough Cat B; Partial	r , 0 left te I.15 L = Iv							Within		ORTH CA ORTHESE SEA 0363	ROLINITIE

Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this

design. 5) Plates checked for a plus or minus 5 degree rotation

about its center.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Elmhurst Rev 3-Elev 5-Roof	
	V7	Valley	1	1	Job Reference (optional)	173750795

Run: 25.20 S May 13 2025 Print: 25.2.0 S May 13 2025 MiTek Industries, Inc. Tue May 27 08:52:32 ID:3x2xP0bGO_78ynLA_SVHjCzEGYL-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.34	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.44	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-AS								
BCDL	10.0										Weight: 21 lb	FT = 20%

IOF CHORD	2X4 3P IN	0.2
BOT CHORD	2x4 SP No	0.3
OTHERS	2x4 SP No	0.3
BRACING		
TOP CHORD	Structural	wood sheathing directly applied
BOT CHORD	Rigid ceili	ng directly applied.
REACTIONS	(size)	1=6-7-2, 3=6-7-2, 4=6-7-2
	Max Horiz	1=-28 (LC 12)
	Max Uplift	1=-32 (LC 50), 3=-32 (LC 48)
	Max Grav	1=279 (LC 47), 3=279 (LC 51),
		4=474 (LC 44)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	-

TOP CHORD 1-2=-231/232, 2-3=-231/232 BOT CHORD 1-4=-164/186, 3-4=-164/186 WEBS 2-4=-352/107

NOTES

Unbalanced roof live loads have been considered for 1) this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 4) Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.

- 7) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc. 8)
- This truss has been designed for a 10.0 psf bottom 9) chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1 and 32 lb uplift at joint 3.
- 12) This truss has been designed for a moving concentrated load of 250.0lb live and 3.0lb dead located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.
- 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design and the second design much reacting of design and the second design much reacting and and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

