# MIDDLETON-RALE

RALEIGH - LOT 00.0153 THE FARM AT NEILL'S CREEK

(MODEL# 2183)

ELEVATION 1- GL

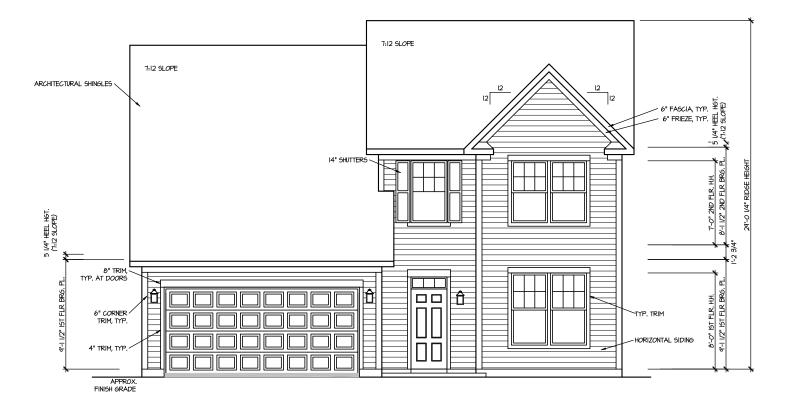


AREA CALCULATIONS  ELEVATION 1 FIRST FLOOR GARAGE FRONT PORCH - ELEVATION 1 SECOND FLOOR  OPTIONS BONUS ROOM	HEATED 1495 SF 692 SF	COVERED / UNHEATED  417 SF 34 SF	UNCOVERED
COVERED PORCH	0.00	124 SF	
4' REAR EXTENSION	160 SF	12+ 31	
BONUS ROOM ADD W/ 4' REAR EXT	61 SF		
BONGS ROOM ABB MY 1 NEAR EXT	01 01		
TOTAL	2718 SF	575 SF	

## 59 Little Branch Drive

LOT	SPECIFIC	
1		THE FARM AT NEILL'S CREEK
		MIDDLETON REV. RALE 2 ELEVATION 1
2	ADDRESS	59 LITTLE BRANCH DR LILLINGTON, NC 27546
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<u>INDEX</u>	



### FRONT ELEVATION I SCALE: 1/8" = 1'-0"



REAR ELEVATION I

SCALE: 1/8" = 1'-0"

DRAWN BY:

DATE: 05/05/2025 PLAN NO. 2183



ELEVATIONS 07 ₹ 07

HOUSE NAME:
MIDDLETON
DRAWING TITLE
FRONT & REAR

SHEET No.

A|.



## RIGHT ELEVATION I



LEFT ELEVATION I SCALE: 1/8" = 1'-0"

DRAWN BY: DATE: 05/05/2025 PLAN NO. 2183



HOUSE NAME:
MIDDLETON
DRAWING TITLE
RIGHT & LEFT E

SHEET No.

ROOF VENTILATION CALCULATIONS:
ROOF AREA = 20/2 So. FT.
ROOF ALEA = 20/2 So. FT.
OVERALL REGULED VENTILATION.
1 TO BOO = 1/0.1 So. FT.
1 TO FEEL AREA OF RIDDE VENT = 1/0.5 So. IN / LINEAR FT.
LOPEN VENTILABLE, BOUTOM 2.9 BOOS!
1 LINEAR TIET OF SOFTIT X 5.1 So. IN = 2.49 So. FT.
1 LINEAR TIET OF RIDDE X 10 So. IN = 4.5 So. FT.
1 AOO SO. LINEAR TIET OF RIDDE X 10 So. IN = 4.5 So. FT.
1 AOO SO. ALLONED)
1 TO SOO ALLONED)
1 TO TALK ROOF VENTILATION. 143 So. FT. > 1.0 I So. FT. (ROOD)

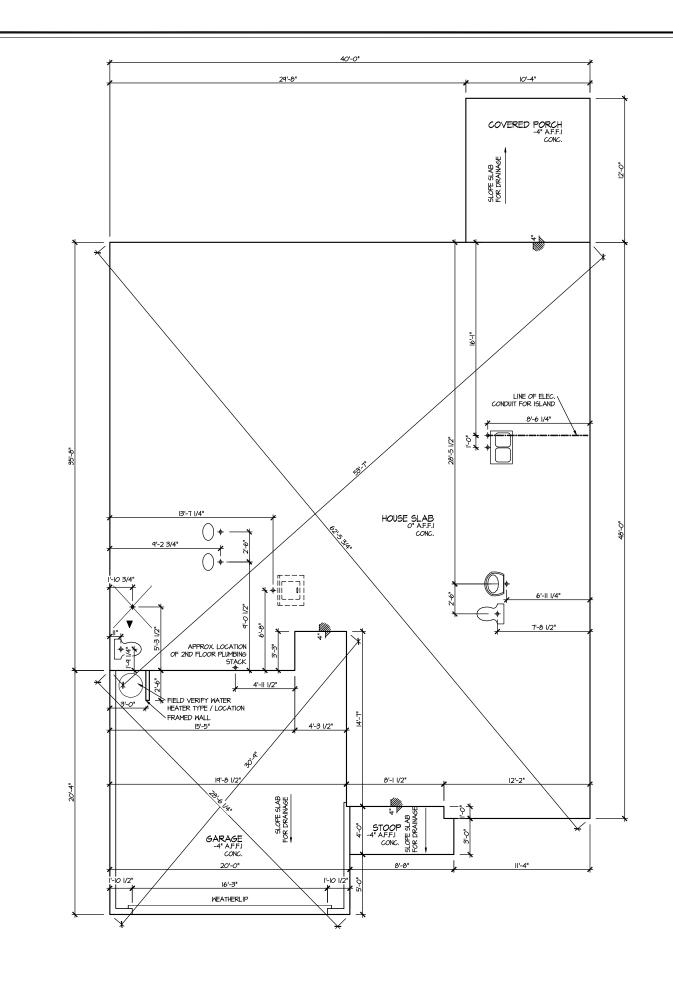
8:12 SLOPE 8:12 SLOPE 8:12 SLOPE 8:12 SLOPE LINE OF SECOND FLOOR 16 LF RIDGE VENT LINE OF SECOND FLOOR ATTIC ACCESS 12:12 SL*O*PE 12:12 SL*O*PE

ROOF PLAN ELEV. I SCALE: 1/8" = 1'-0" DRAWN BY: DATE: 05/05/2025 PLAN NO. 2183

HOUSE NAME:
MIDDLETON
DRAWING TITLE
ROOF PLAN

SHEET No.

AI.3



DEAMN BASTER PLAN INFORMATION

DEAMN BASTER PLAN INFORMATION

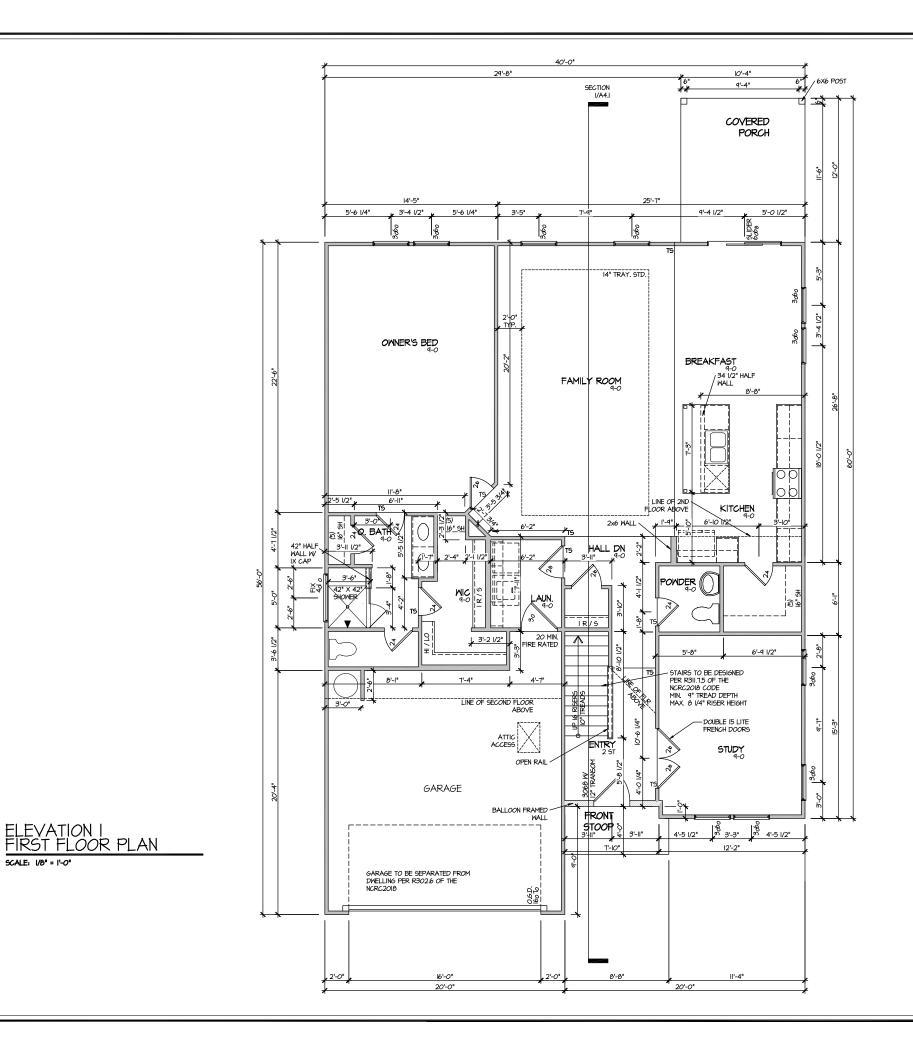
DEAMN BASTER PLAN INFORMATION

DEAMN BASTER PLAN INFORMATION

TO STATE TO ST

ELEVATION I SLAB PLAN SCALE: 1/8° = 1'-0" HOUSE NAME:
MIDDLETON
DRAWING TITLE
SLAB PLAN

SHEET No.



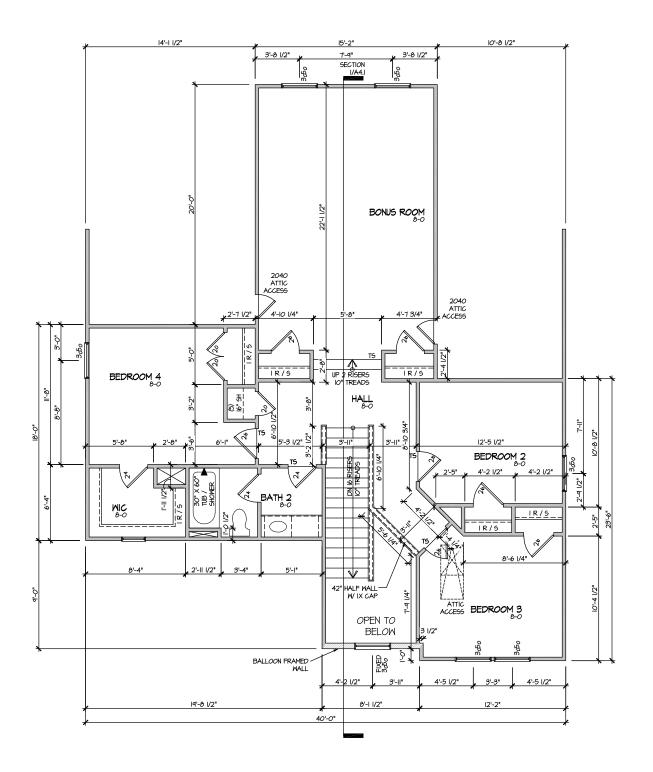
PLAN NO. 2183

DRAWN BY: DATE: 05/05/2025

HOUSE NAME:
MIDDLETON
DRAWING TITLE
FIRST FLOOR P

SHEET No. A3.

SCALE: 1/8" = 1'-0"

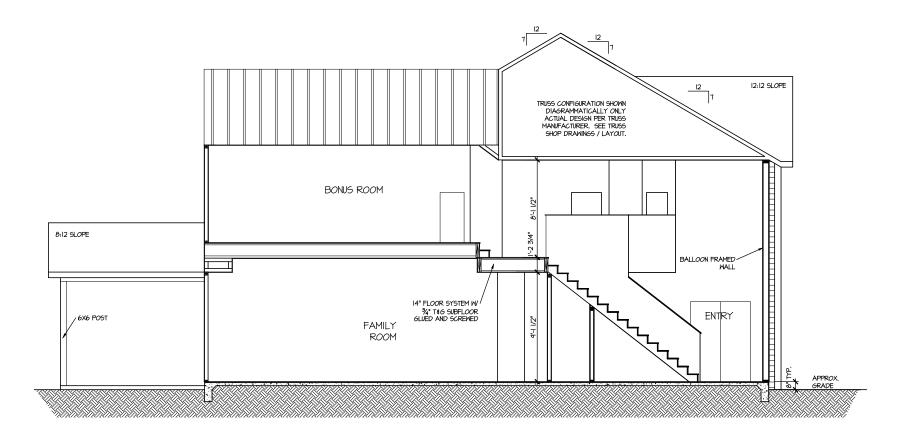


ELEVATION I SECOND FLOOR PLAN SCALE: 1/8" = 1'-0" 

HOUSE NAME:
MIDDLETON
DRAWING TITLE
SECOND FLOOR PLAN

SHEET No.

A3.2



SECTION 1 SCALE: 1/8" = 1'-0"

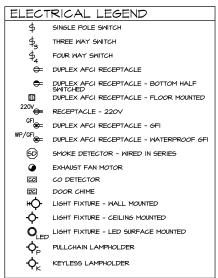
DRAWN BY:

DATE: 05/05/2025 PLAN NO. 2183

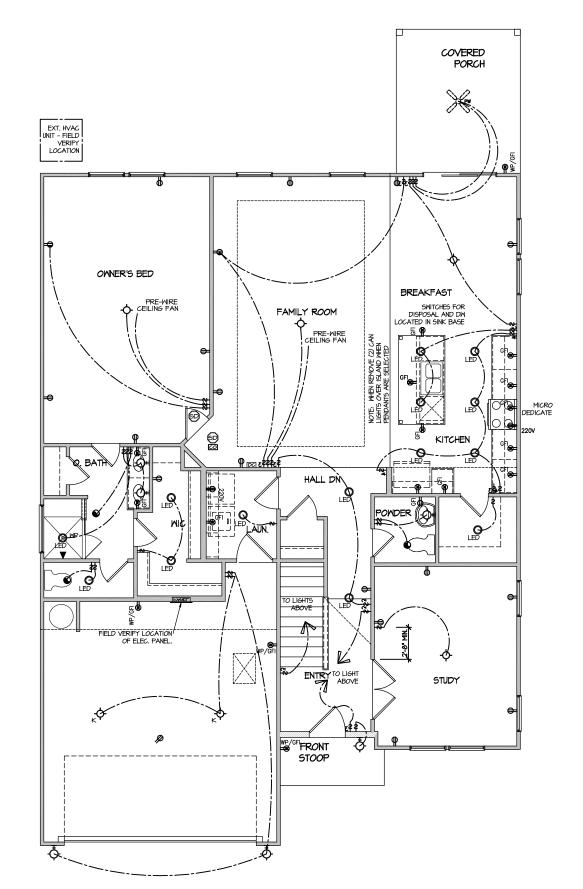


SECTION HOUSE NAME:
MIDDLETON
DRAWING TITLE
BUILDING SECTION

SHEET No. A4.1



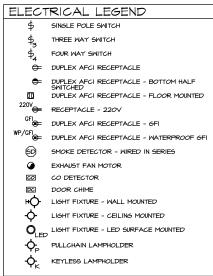
NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



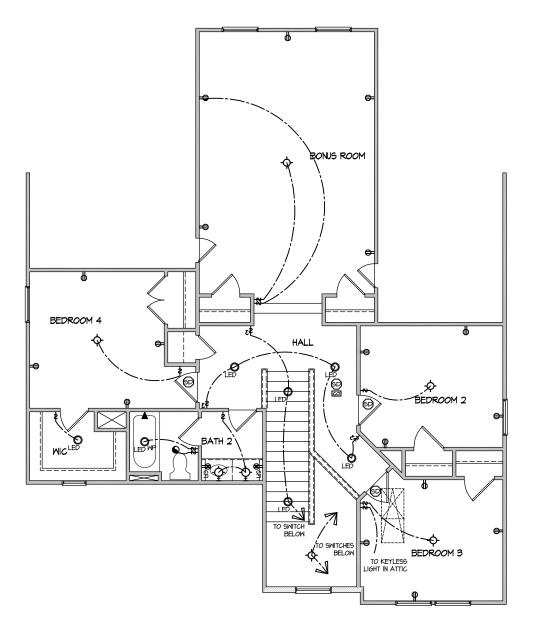
ELECTRICAL PLAN FIRST FLOOR - ELEV. I SCALE: 1/8" = 1'-0"

DRAWN BY: DATE: 05/05/2025 PLAN NO. 2183 ᇳ HOUSE NAME:
MIDDLETON
DRAWING TITLE

SHEET No.



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL PONER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



ELECTRICAL PLAN SECOND FLOOR - ELEV. I SCALE: 1/8" = 1'-0"

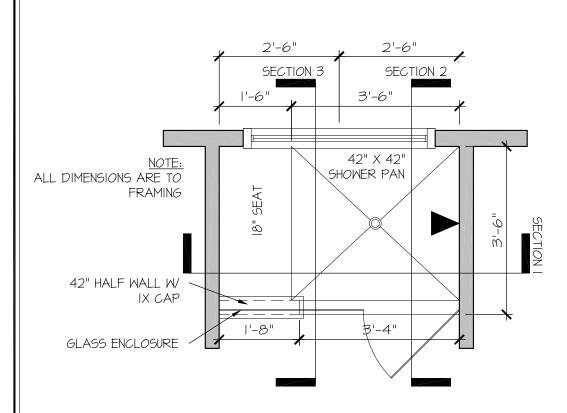
| TILE: LOT 00.0133.0Mg DATE: 3/3/2023 9:44 AM

HOUSE NAME:
MIDDLETON
DRAWING TITLE
SECOND FLOOR ELECT

SHEET No.

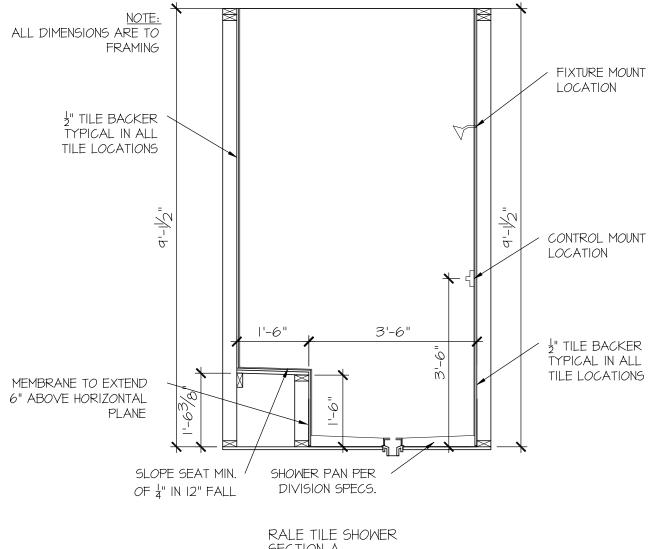
DRAWN BY: ITS DATE: 05/05/2025

PLAN NO. 2183



RALE TILE SHOWER 42" X 42" W 18" SEAT

SCALE: 1/2" = 1'-0"



SECTION A

SCALE: 1/2" = 1'-0"

CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE

24 X 36 SCALE



DETAIL SHOWER RALE

SHEET No.



SEAL

DRAWN BY:
L. BEAVERS
DATE: 9/1/22

PLAN NO.

24 X 36 SCALE

~ "

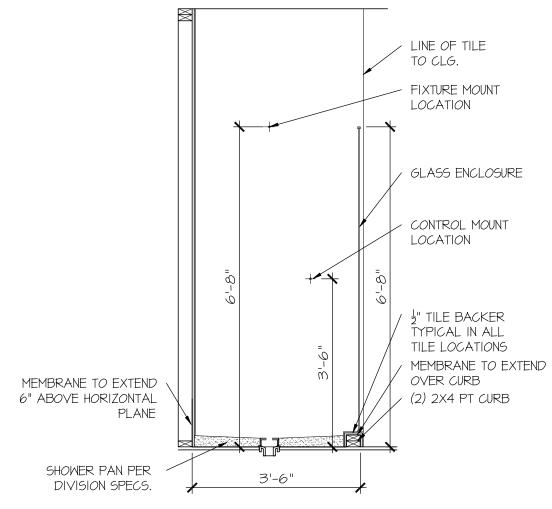


E ILE SHOWER DETAIL

OUSE NAME:

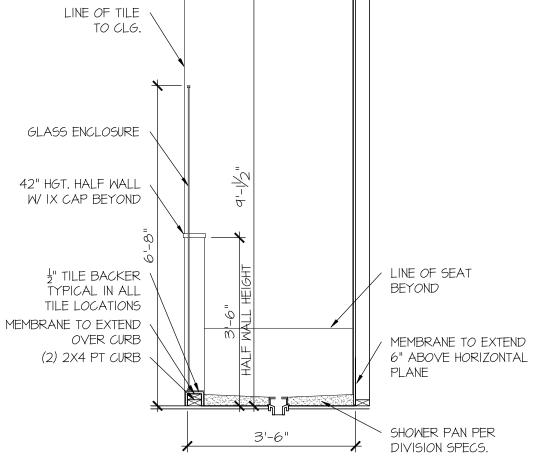
SHEET No.

P||.2



SCALE: 1/2" = 1'-0"





SCALE: 1/2" = 1'-0"

FOOTING DESIGN - 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED, BUILDER/CONTRACTOR MUST VERIFY.

FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING

• I/2" DIA. ANCHOR BOLTS • 6'-0" O.C, 7" MIN. EMBEDMENT

(CONC.) 15" MIN EMBEDMENT (CMU)

SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)

SIMPSON MAB23 ANCHOR STRAPS • 2'-8" O.C. (CMU)

(REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS) ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W/ CONCRETE

OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2. BUILDER TO VERIFY CORROSION-RESISTANCE COMPATIBILITY OF

HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.

BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 @ 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.

CONCRETE DESIGN BASED ON ACL 318 CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: ...... FOUNDATION WALLS

2,500 psi: ...... FOOTINGS € INTERIOR SLABS ON GRADE 3,000 psi: ...... GARAGE & EXTERIOR SLABS ON GRADE

BASEMENT FOUNDATION WALL DESIGN BASED ON:

. 9' OR IO' HEIGHT (AS NOTED ON PLANS) - TALLER WALLS MUST BE ENGINEERED.

NOMINAL WIDTH (9 1/2" FOR 10" THICK WALL).

BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC, ML-CL, OR CL).

BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.

PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS

• FOR OPENINGS UP TO 36", PROVIDE MINIMUM IO" CONCRETE DEPTH OVER OPENING OR (3)2x10 W (2)2x6 JACK STUDS, U.N.O

LARGER OPENINGS SHALL BE PER PLAN.

ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT

ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.

FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.

 PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP

. JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" O.C. (MAXIMUM) JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS

POSSIBLE (I.I RATIO), WITH A MAXIMUM OF 1.15 RATIO

• CONTROL JOINTS SHALL <u>NOT</u> BE INSTALLED IN STRUCTURAL

CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 & 530.I.

CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.

PROVIDE 2x8 x 16" LONG PT. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.

PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE.

DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

• BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

### ADDITIONAL NOTES FOR TRUSS \$ I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS NOTED OTHERWISE ON PLAN.

TRIGGES/ MIGTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUSH BEAMS DO NOT EXCEED THE FOLLOWING:

ROOF TRUSSES

FLOOR TRUSSES, ATTIC TRUSSES, \$ 1-JOISTS:

FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS:
LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

### GENERAL STRUCTURAL NOTES

DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE

WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS:

DEAD = 7 PSF T.C., IO PSF B.C. LOAD DURATION FACTOR = 1.25

LIVE = 40 PSE (30 PSE & SLEEPING AREAS) DEAD = 10 PSF (I-JOISTS & SOLID SAWN) IO PSF T.C.. 5 PSF B.C. (TRUSSES) (ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

### GENERAL FRAMING

• ALL TYP. NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS ALL NAILS SPECIFIED ARE MIN NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.

REFER TO FASTENING SCHEDULE TABLE R602.3(1) FOR ALL

FXT & INT BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS • 16" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. . WALLS OVER 12' TALL SHALL BE PER PLAN.

ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPE) OR SOUTHERN PINE #2 (SYP) LIMBER, OR BETTER (KILN-DRIED), ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY, CODE TABLES HAVE NOT BEEN USED.

ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED . 16" O.C. (MAX., U.N.O.) . HEADERS IN NON-LOAD BEARING WALLS SHALL BE

(1)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'. ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).

FIGURERED LUMBER BEAMS TO MEET OR EXCEED THE FOIL OWING

 'LSL' - Fb=2325 psi; Fv=3I0 psi; E=I.55xI0^6 psi LVL' - Fb=2600 psi; Fv=285 psi; E=2.0xl0^6 psi

'PSL' - FB=2900 PSI: FV=290 PSI: E=2.0XI0^6 PSI

M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF TH OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OF

FOR 2 \$ 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0,120" NAILS @ 8" O/C OR 2 ROWS 1/4"x3/6" SIMPSON 5D5 SCREWS (OR 3½" TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAIL S/SCREWS 2" FROM FDGE. SOLID 3 1/2" OR 5 1/2" BEAMS ARE ACCEPTABLE, USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8

FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 1/2 x6" SIMPSON SDS SCREWS (OR 6 3/4" TRUSSLOK OF 14" OR GREATER APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREW 2" FROM EDGE. A SOLID 1" BEAM IS ACCEPTABLE.

ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD \$ (1)2x KING STUD, MINIMUM.

- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,

ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER W/ 3"XO.I3I" IAILS @ 24" O.C. (MIN.), EACH PLY.

PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND BEARING. BLOCKING TO MATCH POST ABOVE.

FASTEN 2V WOOD PLATES TO TOP ELANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.

• ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4

### FLOOR FRAMING

-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED 1 /480 LIVE LOAD DEELECTION CRITERIA (EXCLUDES MARBLE FLOORS - CONTACT M&K FOR MARBLE FLOOR DESIGNS)

AT I-JOIST FLOORS, PROVIDE I I/8" MIN. OSB RIM BOARD.

• METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.

• FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND 2 1 x 0.131 NAILS @ 6"o.c. @ PANEL EDGES & @ 12"o.c. FIELD.

2 2" × 0120" NAILS @ 4" OC @ PANEL EDGES & @ 8" OC FIELD 2 3" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.

Õ× 2" MIN. SCREMS @ 6" O.C. @ PANEL EDGES \$ @ 12" O.C. FIELD

### ROOF FRAMING

\*BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.

FASTEN EACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL.) © ALL BEARING POINTS. PROVIDE (2) H25T CLIPS AT 2-PLY GIRDER TRUSSES, (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS

METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.

ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."

SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W. 2x4 LEDGER FASTENED TO: - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS)

- TRUSS VERTICALS w/ (3) 3"x0.131" NAILS @ 19.2" O.C. MAX. (FLOOR TRUSSES)

• ROOF SHEATHING SHALL BE 1/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS

- w/ 2 1 x 0.131 NAILS @ 6"0.c. @ PANEL EDGES & @ 12" O.C. FIELD.

- w/ 2 🐉 x 0,120" NAILS 👁 4"o.c. 👁 PANEL EDGES 🛊 👁 8" O.C. FIELD. - W/ 2 🖥 x 0.113" NAILS @ 3"o.c. @ PANEL EDGES \$ @ 6" O.C. FIELD.

### HOLD-DOWN SCHEDULE

SYMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA, ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM U.N.O.) -OR- MSTC66B3 ALTERNATE
<b>→</b> HD-3	SIMPSON STHDI4/STHDI4RJ

\* UTILIZE THE SSTB24 ANCHOR BOLT @ ALL MONOSLAB & INTERIOR RAISED FOOTING THICKNESS REQUIRED.

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN HREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR 15" (FOR 1/8" DIA.) MIN EMBEDMENT INTO CONCRETE

### MEANS & METHODS NOTES

DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY DETERMINE THE ERECTION PROCEDURES SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENTS IN CONTACT WITH FLOOR FRAMING ARE LEVEL, NCLUDING, BUT NOT LIMITED TO; FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING FLEMENTS IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TOLERANCES.

### LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: ) MPH WIND IN 2018 NCSBC:R(

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301.2.1.1) EXP. B. RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10 AS PERMITTED BY R301.13 OF THE 2018 NGSBG:RG, OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R60212 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602.3.5 R802.II.

### EXT. WALL SHEATHING SPECIFICATION

• 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W/ 2 ¾"x0.II3" NAILS ● 6" O.C. AT EDGES ♦ ● 12" O.C. IN THE PANEL FIELD. TYP, U.N.O

HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.

ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.

・ALT. STAPLE CONNECTION SPEC: 1½" 16 GA STAPLES (1/4" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD.

### BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 %" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 %" 16 GA STAPLES (1/6" CROWN) • 3" O.C. AT EDGES \$ • 6" O.C IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

### 3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF w/ 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING.

SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.

DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O. ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING.

PRE-MANUFACTURED PANELIZED WALLS: FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

> INDICATES EXTENT OF INT. OSB SHEARWALL OR 3" O.C. OSB SHEARWALL

INDICATES HOLDOWN BELOW

### LEGEND

IIIIIIII INTERIOR BEARING WALL

● □===□ BEARING WALL ABOVE

BEAM / HEADER

• = = INDICATES SHEAR WALL & EXTENT

 EXTENT OF OVERFRAMING JL METAL HANGER

INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP

### NON-BEARING HEADER SCHEDULE

SPAN	2x4 NON-BEARING PARTITION WALL	2x6 NON-BEARING PARTITION WALL	
UP TO 3'-0"	(I)2x4 FLAT	(I)2x6 FLAT	
UP TO 6'-0"	(2)2x4	(3)2×4	
UP TO 8'-0"	(2)2x6	(3)2×6	
UP TO 12'-0"	(2)2x8	(3)2×8	

### NOTES:

• ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 24" OC (MAX)

3'-0" 20 FT, MAX L3"x3"x/4" 3 FT MAX L3"x3"x/4" I2 FT, MAX L4"x3"x/4" 20 FT, MAX L5"x3K"x%" 3 FT. MAX L4"x4"x/4" 8'-0" I2 FT, MAX L5"x3K"x%." I6 FT. MAX L6"x3½"x%" I2 FT, MAX L6"x3½"x%" 9'-6" 2 FT MAX 17"24"26" \*\*

VENEER LINTEL SCHEDULE

### ALL LINTELS:

HALL SUPPORT 2 %" - 3 ½" VENEER w/ 40 psf MAXIMUM WEIGHT. 6' SHALL HAVE 4" MIN, BEARING

L8"x4"x/2" \*\*

3 FT, MAX

16' SHALL HAVE 8" MIN. BEARING 16' SHALL NOT BE FASTENED BACK TO HEADER. 16' SHALL BE FASTENED BACK TO WOOD HEADER IN WALL @48°0.0 w/½" DIA. x 3½" LONG LAG SCREMS IN 2" LONG VERTICALLY SLOTTED HOLES.

1AX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE

OPENING.
ALL LINTELS SHALL BE LONG LEG VERTICAL.
WHEN SUPPORTING VENEER ( 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 ½" WIDE OVE THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOIN

PRISHING.
SET STRICTURAL PLANS FOR ANY LINTEL CONDITION NOT SET STRICTURAL PLANS FOR ANY LINTEL CONDITION NOT DECOMPAGED BY THE ABOVE PARAMETERS. FOR ANY LINTEL FASTINED BACK TO BEAM, FASTINESS SHALL MAINTAIN A 35' (MINIMAN) CLEAR DISTRICK FROM BOTTOM OF BEAM.

FOR GALEN VENERS USE LA-36%.

FOR 35' VENERS ONLY, SEE PLAN FOR VENERS SUPPORT IF VENERS C. 35' THICK.

ENGINEERED BEAM MATERIAL SCHEDULE

BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION	
001	(2)194"×1176" - H	3½"×II%" - H	(2)154"×1176" - H	(2)2xl2 + (l) 片"xll片" STEEL FLITCH PLATE - H	NA	
AIOO	(2)1¾"×11%" - H	3½"xII%" - H	(2)194"x11%" - H	(2)2xi2 + (i) ½"xil¼" 5TEEL FLITCH PLATE - H	N/A	
002	(2)194"x18" - FT	5¼"×18" - FT	N/A	(3)2xi2 + (2)½"xii4" Steel Flitch Plates - FB	WI2xI4 - F	
600	(2)134"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2xl2 + (l) ¼"xll¼" STEEL FLITCH PLATE - FB	WI2xI4 - F	
004	(2)134"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2xi2 + (1)¼"xi¼" STEEL FLITCH PLATE - FB	WI2xI4 - F	
005	(2)134"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2xi2 + (1) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F	
006	(2)134"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2xi2 + (1) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F	
001	(2)1¾"×11%" - F	3½"xII%" - F	(2)1¾"×11¾" - F	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATE - F	MI0x12 - F	
008	(2)134"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2x12 + (1) ¼"xII¼" STEEL FLITCH PLATE - FB	WI2xI4 - F	
P00	(3)13/4"x18" - FT	5¼"×18" - FT	N/A	(4)2xl2 + (3)½"xll¼" STEEL FLITCH PLATES - FB	WI2x26 - F	
010	(3)13/4"x20" - FT	5¼"x20" - FT	N/A	(4)2xl2 + (3) %"xll4" STEEL FLITCH PLATES - FB	WI2x35 - F	
OII	(2)1¾"x11¾" - FB	3½"xii%" - FB	(2)13/4"×14" - FB	(2)2xi2 + (I) ¼"xil¼" STEEL FI ITCH PI ATF - FB	MI0xI2 - FB	

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM

- "FT" INDICATES FLUSH TOP BEAM - "FB" INDICATES FLUSH BOTTOM BEAM

- "D" INDICATES DROPPED BEAM

REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS

FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W/ (2) 3"XO.120" NAILS @ 8" O.C.

- "H" INDICATES DROPPED OPENING HEADER REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS

FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REGID. FASTEN PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C.

CONNECTION SPECIFICATIONS (TYP. U.N.O.)

	DESCRIPTION OF BLDG, ELEMENT	3"x0.131" NAILS	3"x0.120" NAIL5
	JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*
1	SOLE PLATE TO JOIST/BLK'G.	(3) NAILS @ 4" O.C.	(3) NAILS @ 4" O.C.
L	STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*
1	TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS
1	RIM TO TOP PLATE	TOENAILS • 8" O.C.	TOENAILS • 6" o.c.*
L	BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*
L	DOUBLE STUD	NAILS <b>●</b> 24" o.c.	NAILS @ 16" O.C.
L	DOUBLE TOP PLATE	NAILS <b>⊘</b> 24" o.c.	NAILS • 16" o.c.
L	DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA
	TOP PLATE LAP @ CORNERS \$	(2) NAILS	(2) NAILS

2½'x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3'x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE \* ARE SHOWN)

FLOOR LOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED I-JOISTS DESIGN AND CONSTRUCTION GUIDE, MINIMUM JOIST ALLOWARI E MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I-JOISTS. ALL ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST

### ALTERNATE F.J MANUFACTURERS

CONNECTIONS ARE PER THE JOIST MANUFACTURER.

 $\Box$  $\cap$  $\delta$ NEIL MIDDLET RM LO

5/14/2

ERN+KU

I&K project numbe

rawn by:

EVISIONS

126-22076

ssue date: 05-14-2

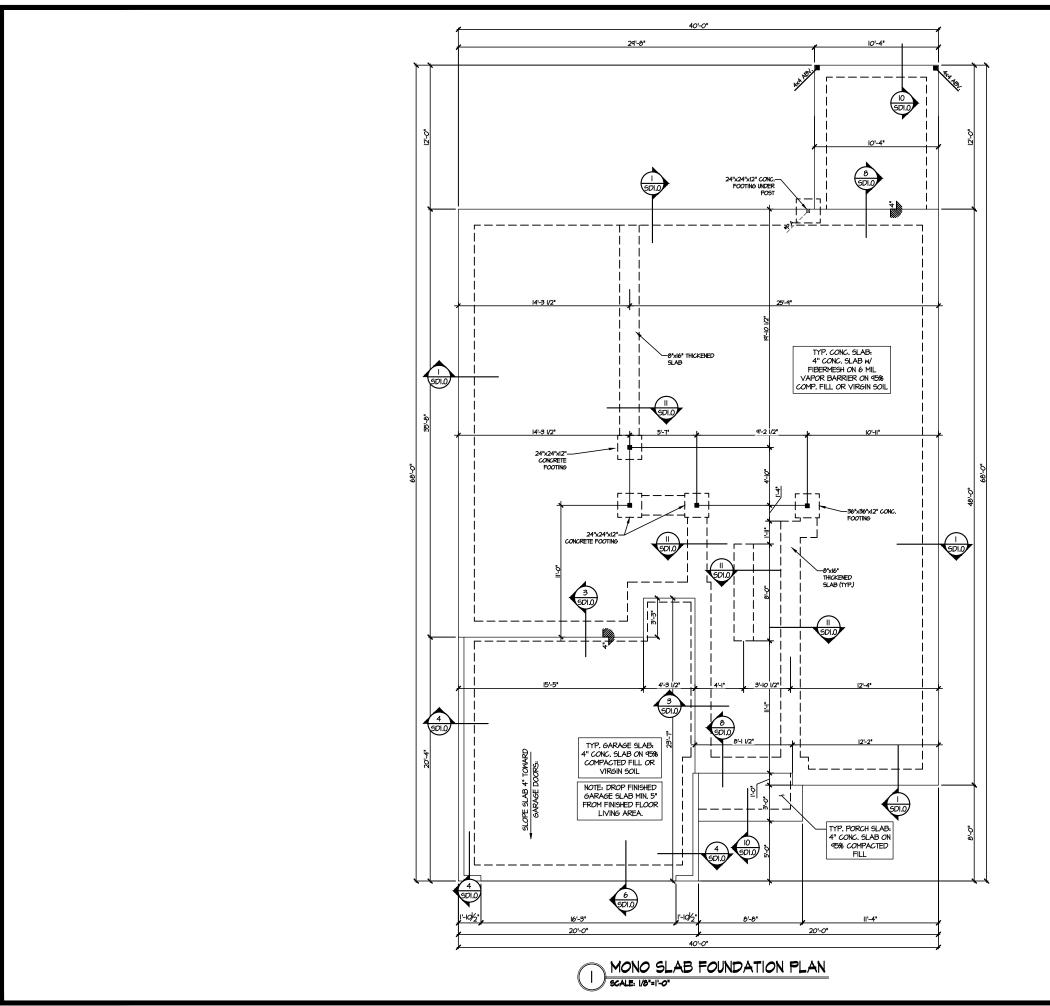
**JTR** 

initial:

H CAR

ROFESSIO

ENGINE



5/14/25

MULHERN+KULP

ESSIDENTIAL STRUCTURAL ENSINEERING

2005-Market Ave Balling 1- Ambre IA 19922

2785-955-9511 - Ambre IA 19922 Y

M&K project number:

126-22076

**JTR** drawn by: issue date: 05-14-25

CREEK

FARM AT NEIL'S LOT 153 - MIDDLETON 1 RALEIGH, NC

**S1.0** 

OUNDATION P

### LEGEND

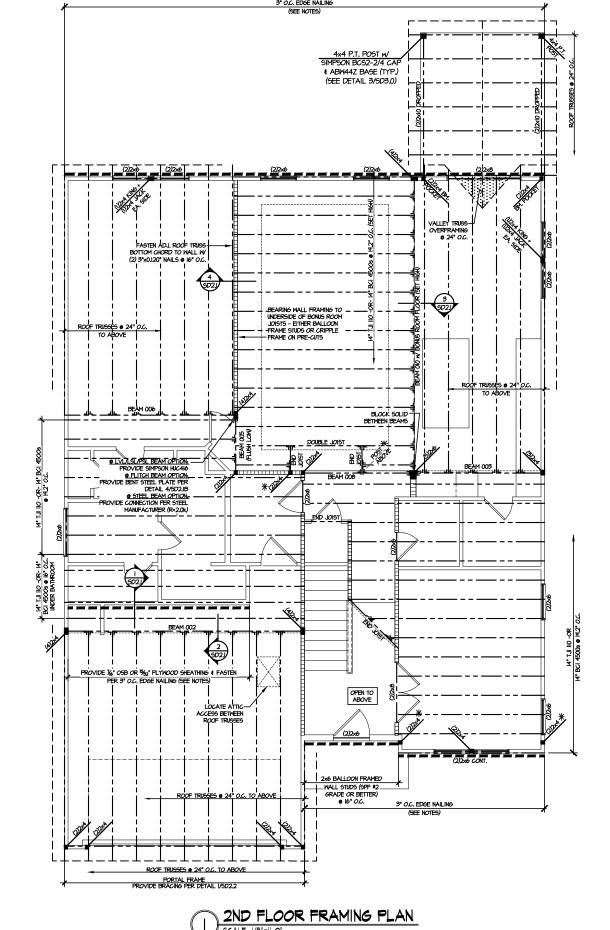
- IIIIII INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- — BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING

### JL METAL HANGER

- \* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP.
  REFER TO SCHEDULE.

TYPICAL STRUCTURAL NOTES # SCHEDULES

REFER TO SO.O FOR



5/14/2 H CAR SEPH T. R

MULHERN+KUL RESIDENTIAL STRUCTURAL ENGINEER SUBMARIA AN BAIRD 1- ANDAR PARSON \$755000001 per humanistan Y

M&K project number: 126-22076

**JTR** drawn by: issue date: 05-14-2

REVISIONS initial:

ALTERNATE F.J MANUFACTURERS

• FLOOR JOISTS BY MANUFACTURER'S OTHER THAN THOSE SHOWN ON PLAN SHALL CONFORM TO THE APA PERFORMANCE RELATED I-JOISTS DESIGN AND CONSTRUCTION GUIDE, MINIMUM JOIST PROPERTIES INCLUDING, BUT NOT LIMITED TO, ALLOWABLE SHEAR, ALLOWABLE MOMENT, STRENGTH, AND STIFFNESS, SHALL MEET OR EXCEED THOSE LISTED FOR THE PRI-60 SERIES I-JOISTS, ALL

# ALLOWABLE HOLES, BEARING STIFFENERS, AND JOIST TO JOIST CONNECTIONS ARE PER THE JOIST MANUFACTURER. ENGINEERED BEAM MATERIAL SCHEDULE

NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)134"x1136" - H	3½"×II%" - H	(2)1¾"x11%" - H	(2)2xl2 + (l) 片"xl片" STEEL FLITCH PLATE - H	N/A
OOIA	(2)194"x1176" - H	3½"×II%" - H	(2)1¾"x11%" - H	(2)2xi2 + (1)片"xi片" STEEL FLITCH PLATE - H	N/A
002	(2)194"x18" - FT	5¼"xl6" - FT	N/A	(3)2x12 + (2) ½"x1¼" STEEL FLITCH PLATES - FB	WI2xI9 - F
003	(2)13/4"×14" - F	3½"xi4" - F	(2)13/4"×14" - F	(2)2xi2 + (l)从"xi以" STEEL FLITCH PLATE - FB	WI2xI4 - F
004	(2)13/4"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2xi2 + (1) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
005	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xi2 + (1) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
006	(2)13/4"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xi2 + (I) 以"xi以" STEEL FLITCH PLATE - FB	WI2xI4 - F
001	(2)13/4"x113/6" - F	3½"xll%" - F	(2)1¾"x11%" - F	(2)2xl2 + (1) ¼"xll¼" STEEL FLITCH PLATE - F	MI0x12 - F
008	(2)13/4"×14" - F	3½"x14" - F	(2)13/4"×14" - F	(2)2xi2 + (1) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
004	(3)194"x18" - FT	5¼"xl6" - FT	N/A	(4)2xi2 + (3)火*xi以* Steel Flitch Plates - FB	WI2×26 - F
010	(3)1%*x20" - FT	5¼"×20" - FT	N/A	(4)2xl2 + (3) %"xll4" STEEL FLITCH PLATES - FB	WI2x35 - F
OII	(2)1¾"x11%" - FB	3½'x 1%' - FB	(2)19/4"×14" - FB	(2)2xl2 + (I)从"xll以" STEEL FLITCH PLATE - FB	MIOxI2 - FB

- BEAM NOTATION:
   "F" INDICATES FLUSH BEAM
   "FT" INDICATES FLUSH TOP BEAM
   "FB" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM

- "D" INDICATES DROPPED BEAM
   "H" INDICATES DROPPED OPENING HEADER
  REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
  REFER TO DETAIL E/SD2.0 FOR TYPICAL SITEL BEAM CONNECTIONS
  FOR FLUSH TOP DETAIN FROVINE 2 S TACKED PLATES BENEATH DEAM AS REQ\*D, FASTEN
  PLATES IN SUCCESSION W (2) 3"X0.120" NAILS € 8" O.C.
- FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W/ (2) 3"XO.120" NAILS @ 8" O.C.

CREEK

A AT NEIL'S (
MIDDLETON 1 FARM LOT 153 - M RALEIGH, N

LEGEND

• INTERIOR BEARING WALL

● □===□ BEARING WALL ABOVE • ---- BEAM / HEADER

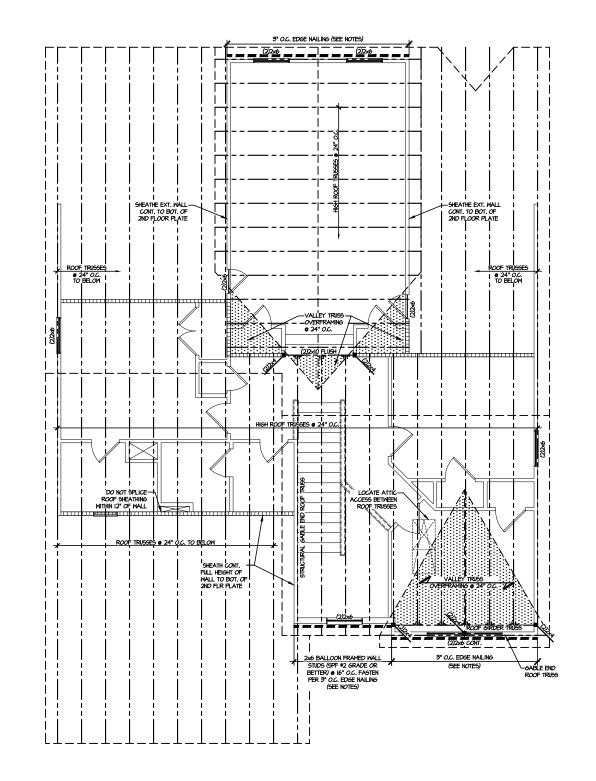
• = = INDICATES SHEAR WALL & EXTENT

EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES



ROOF FRAMING PLAN SCALE: 1/8"=1'-0"

5/14/25 TH CAR SEPH T. RI MULHERN+KULP

ESSIDENTIAL STRUCTURAL ENSINEERING

2008-Market Ava Builling 1- Ambril 70 1992

225-255-255-2511 - Ambrild-Ambril 70 1992



M&K project number: 126-22076

project mgr: JTR drawn by: XJG issue date: 05-14-25

REVISIONS:

initial:

CREEK

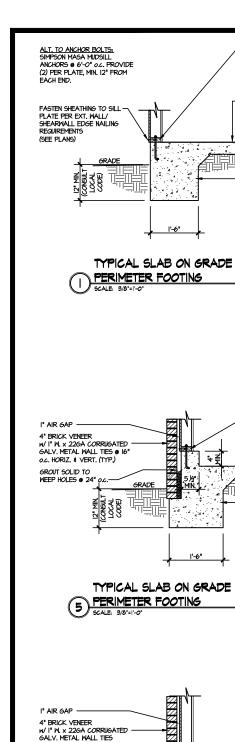
### LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- ---- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES # SCHEDULES

FARM AT NEIL'S (LOT 153 - MIDDLETON 1 ROOF **S3.0** 

FRAMING



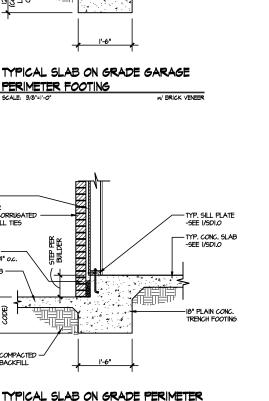
GROUT SOLID TO

TYP, PORCH SLAB

BACKFILL

FOOTING @ PORCH/PATIO

-SEE 8/SDI.0



-<u>TYP. SILL PLATE:</u> 2x4 P.T. PLATE w/ ½" DIA. x II" LONG A.B. **@** 6'-0" o.c. PROVIDE

-TYP, CONC, SLAB: 4" PLAIN CONC, SLAB W 6X6 WL4XI.4 WLMF, OR FIBERMESH ON 6 MIL VAPOR BARRIER ON 95% COMPACTED FILL OR VIRGIN SOIL

- TYP. SILL PLATE -SEE I/SDI.O

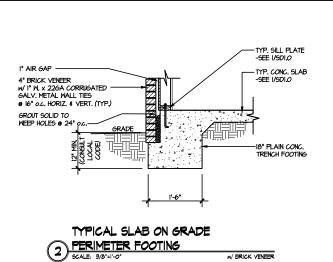
-TYP. CONC. SLAB

-SEE I/SDI.O

18" PLAIN CONC.

-18" PLAIN CONC. TRENCH FOOTIN

(2) PER PLATE, MIN. 12" FROM



TYPICAL SLAB ON GRADE GARAGE

6 ENTRY @ PERIMETER FOOTING
SCALE: 3/8"=1"-0"

TYPICAL FOOTING @ PORCH SLAB

-TYP. SILL PLATE BEYOND -SEE I/SDI.O

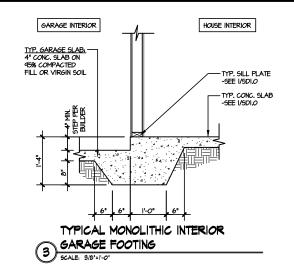
-TYP. PORCH SLAB -SEE 4/SDI.O

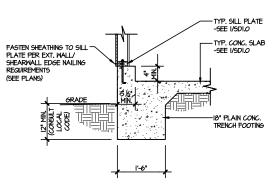
COMPACTED

-12" MIN, CONC. TRENCH FOOTING

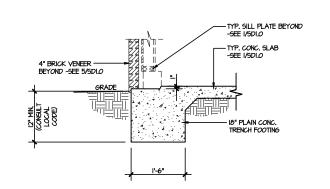
BACKFILL

— TYP. CONC. SLAB -SEE I/SDI.O

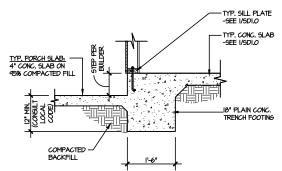




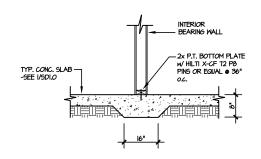




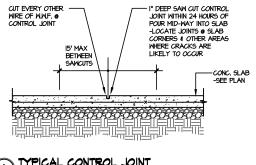




TYPICAL SLAB ON GRADE PERIMETER 8 FOOTING @ PORCH/PATIO









THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

 $\mathbb{Z}$  $\Box$  $\delta$ NEIL ON AT ARM Ō LOT

5/14/2

ERN+KU

M&K project number 126-22076

frawn by: issue date: 05-14-2

REVISIONS

**JTR** 

initial:

TH CAR

SEPH T. R

OFESSIO

SD1

LETTERED DETAILS ARE TYPICAL FOR

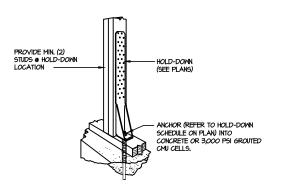
### TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

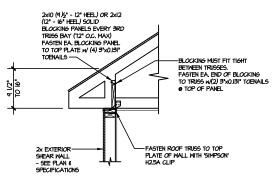
SCALE: 9/8"=1"-0" HEEL HEIGHT LESS THAT HEEL HEIGHT LESS THAN 9½" NO BLOCKING REQ'D

2x EXTERIOR SHEAR - Fasten Sole Plate to Rim Board W/ 3"x0.131" Nails @ 6" O.C. (SEE PLANS) - 2x EXTERIOR SHEAR

TYPICAL SHEAR TRANSFER DETAIL



TYPICAL HOLD DOWN INSTALLATION
SCALE: N.T.S.



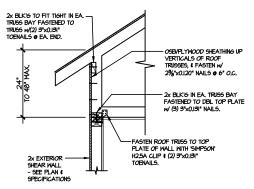
TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

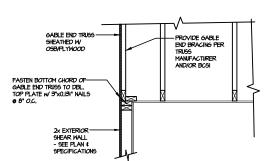
SCALE: 3/0"=1"-0" HEEL HEIGHT BETWEEN 1/2

BELLOWING BENDEN

HEEL HEIGHT BETWEEN 9 ½" - 16" BLOCKING REQ'D



TYPICAL SHEAR TRANSFER DETAIL @ RAISED HEEL TRUSS
SCALE: 3/8'=1'-0' HEEL HEIGHT UP TO 46" MAX.



TYPICAL GABLE END DETAIL

SCALE: 3/8°=1-0°

MULHERN+KULP

ESSIDENTIAL STRUCTURAL ENSINEERING

2008-Market Ava Builling 1- Ambril 70 1992

225-255-255-2511 - Ambrild-Ambril 70 1992 **Y** 

5/14/25

H CAR

SEPH T. RV

M&K project number: 126-22076

**JTR** drawn by: XJG issue date: 05-14-2

REVISIONS:

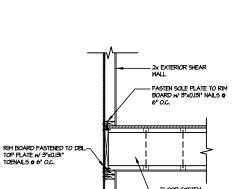
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CREEK DETAILS FARM AT NEIL'S (LOT 153 - MIDDLETON 1 RAMING

NUMBERED DETAILS ARE PLAN

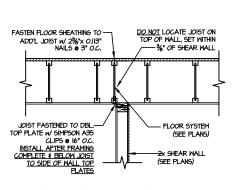
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.



BETWEEN FLOORS @ EXTERIOR WALL
SCALE 9/8/11-0\*

**SD2.0** 



FASTEN SHEATHING TO BLOCKING W/ 2%"x 0.120" NAILS @ 6" O.C. - 2x6 CONT. BLOCKING. FASTEN TO EA. STUD w/ (3) 3°x0.131" NAILS OR INSTALL SHEATHING -PRIOR TO INSTALLING ROOF TRUSSES -FASTEN SOLE PLATE OF SHEARWALL TO FLUSH BEAM W 3"X0.131" NAILS @ 6" O.C. OR 3"X0.120" NAILS @ 4" O.C. EXTEND SHEATHING TO BOT. OF BEAM & FASTEN PER SPECIFICATION ON PLAN | I-JOISTS || (SEE PLAN) TOP BEAM (SEE PLAN)

FASTEN SHEATHING TO - FASTEN 2x6 LEDGER OR TOP CHORD OF TRUGS TO TO EA. STUD w/ (3) 3"x0.131" NAILS OR (3) 3"x0.120" NAILS SHEARWALL ABOVE -ROOF TRUSS -- FASTEN SOLE PLATE OF SHEARWALL TO FLUSH BEAM W 3"x0.131" NAILS @ 6" O.C. OR 3"x0.120" NAILS @ 4" O.C. EXTEND SHEATHING — TO BOT, OF BEAM \$ FASTEN PER SPECIFICATION ON PLAN || 1-JOISTS || ||(SEE PLAN)|| LOW ROOF TRUSSES -(SEE PLAN) TOP BEAM (SEE PLAN) SHEAR TRANSFER DETAIL @

EXTERIOR SHEARMALL ABOVE

FASTEN SHEATHING TO -LEDGER W/ 23/8"x 0.113" NAILS @ 6" O.C. EPH T. R - FASTEN 2x6 LEDGER OR TOP CHORD OF TRUSS TO EA, STUD w/ (3) 3"x0,131" NAILS -2x EXTERIOR SHEAR WALL ABOVE NSTALL SHEATHING PRIOR TO INSTALLING LOW ROOF TRUSSES - Fasten sole plate *o*f Shearwall to Rim Board W 3"x0.131" nails **@** 6" o.c. LOW ROOF TRUSSES RIM BOARD FASTENED TO DBL.~ TOP PLATE W/ 3"x0.131" TOENAILS @ 6" O.C. -2x interior shear

TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ INTERIOR WALL

SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL BELOW

SHEAR TRANSFER DETAIL @

2 EXTERIOR SHEARMALL ABOVE

REVISIONS initial:

"H CAR

5/14/25

MULHERN+KUL RESIDENTIAL STRUCTURAL ENSINEER SUBMARIE AM BUILDE 1- AMARI, PA 1902 278-58-5001 - EMBARIE AMARI, PA 1902

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M&K project number:

frawn by:

126-22076

**JTR** 

XJG issue date: 05-14-25

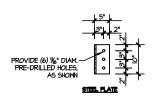
CREEK DETAILS FARM AT NEIL'S (LOT 153 - MIDDLETON 1 RALEIGH, NC RAMING

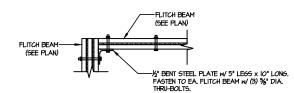
NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

SD2.1A

SHEAR TRANSFER DETAIL @ INTERIOR SHEARWALL BELOW
SCALE 8/47-11-07
PARAL PARALLEL TO FRAMING ONLY REQD WHERE NOTED ON PLAN

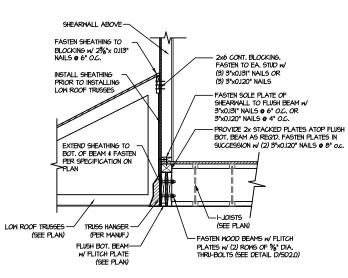




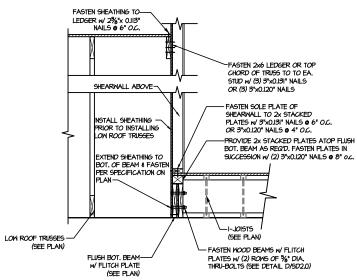
TYPICAL FLITCH BEAM TO FLITCH BEAM

CONNECTION DETAIL

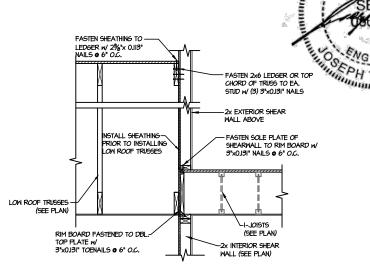
SCALE: 9/4"=1"-0"



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE SCALE. S/8"-11-0"



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



TYPICAL SHEAR TRANSFER DETAIL 3 BETWEEN FLOORS @ INTERIOR MALL M&K project number: 126-22076

> frawn by: issue date: 05-14-2! REVISIONS initial:

**JTR** 

7

5/14/2

H CAR



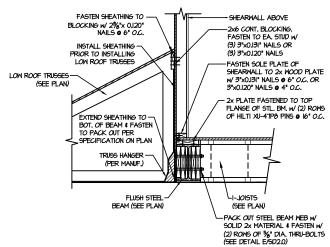
 $\Box$ A AT NEIL'S (
- MIDDLETON 1
I. NC Ŋ FARM LOT 153 - M RALEIGH, N

SD2.1B

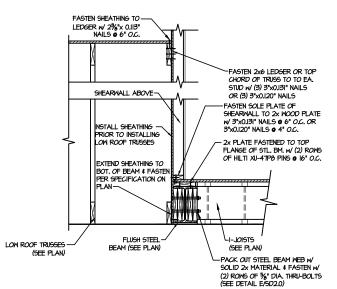
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

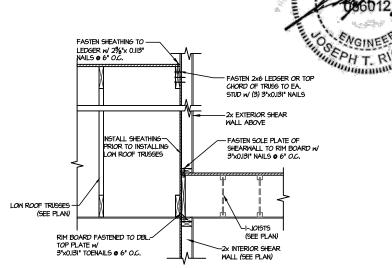
SHEAR TRANSFER DETAIL @ INTERIOR SHEARMALL BELOW SCALE S/4'=1'-0' PARA PARALLEL TO FRAMING ONLY REGID WHERE NOTED ON PLAN



SHEAR TRANSFER DETAIL @ EXTERIOR SHEARWALL ABOVE



SHEAR TRANSFER DETAIL @ 2 EXTERIOR SHEARMALL ABOVE SCALE SOF-1-0"



TYPICAL SHEAR TRANSFER DETAIL 3 BETWEEN FLOORS @ INTERIOR WALL

5/14/25

MULHERN+KUL RESIDENTIAL STRUCTURAL ENSINEER SUBMARIE AM BUILDE 1- AMARI, PA 1902 278-58-5001 - EMBARIE AMARI, PA 1902

Y

M&K project number:

frawn by:

REVISIONS

126-22076

issue date: 05-14-25

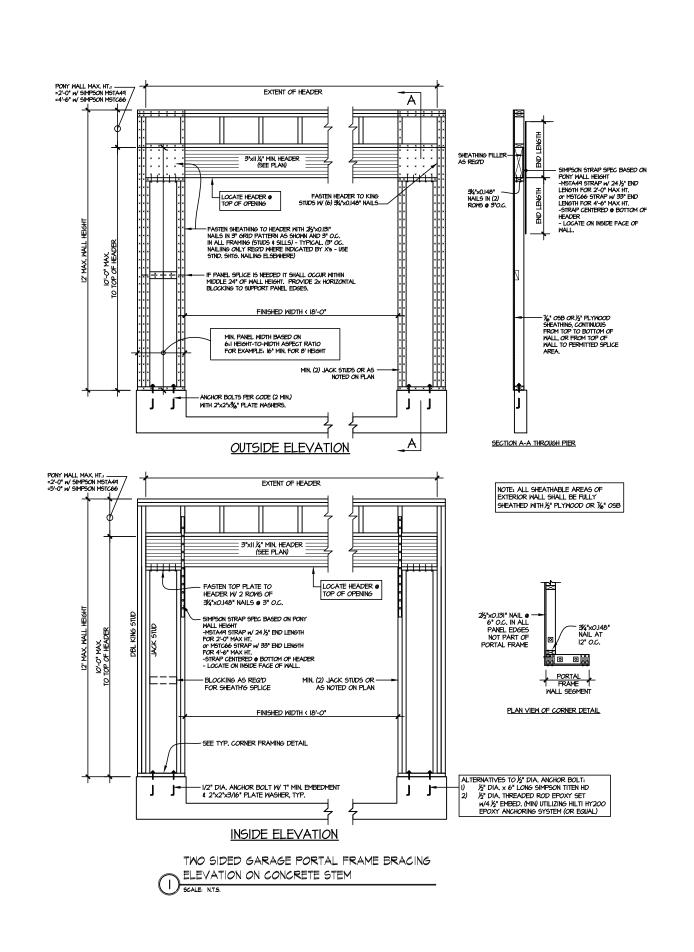
**JTR** 

initial:

H CAR

CRE DETAILS FARM AT NEIL'S (LOT 153 - MIDDLETON 1 RALEIGH, NC RAMING

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED



5/14/25 TH CAR SEPH T. RI

MULHERN+KULP

ESSIDENTIAL STRUCTURAL ENSINEERING

2008-Market Ava Builling 1- Ambril 70 1992

225-255-255-2511 - Ambrild-Ambril 70 1992 Y

M&K project number: 126-22076

roject mgr: **JTR** drawn by: XJG issue date: 05-14-25

REVISIONS:

initial:

CREE DETAILS FARM AT NEIL'S (LOT 153 - MIDDLETON 1 FRAMING

**SD2.2** 

MULHERN+KULP

ESSIDENTIAL STRUCTURAL ENSINEERING

2008-Market Ava Builling 1- Ambril 70 1992

225-255-255-2511 - Ambrild-Ambril 70 1992 Y

M&K project number: 126-22076

JTR drawn by: XJG issue date: 05-14-25

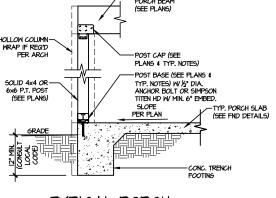
FARM AT NEIL'S CREEK
LOT 153 - MIDDLETON 1
RALEIGH, NC

**SD3.0** 

NUMBERED DETAILS ARE PLAN

WHERE SPECIFICALLY INDICATED

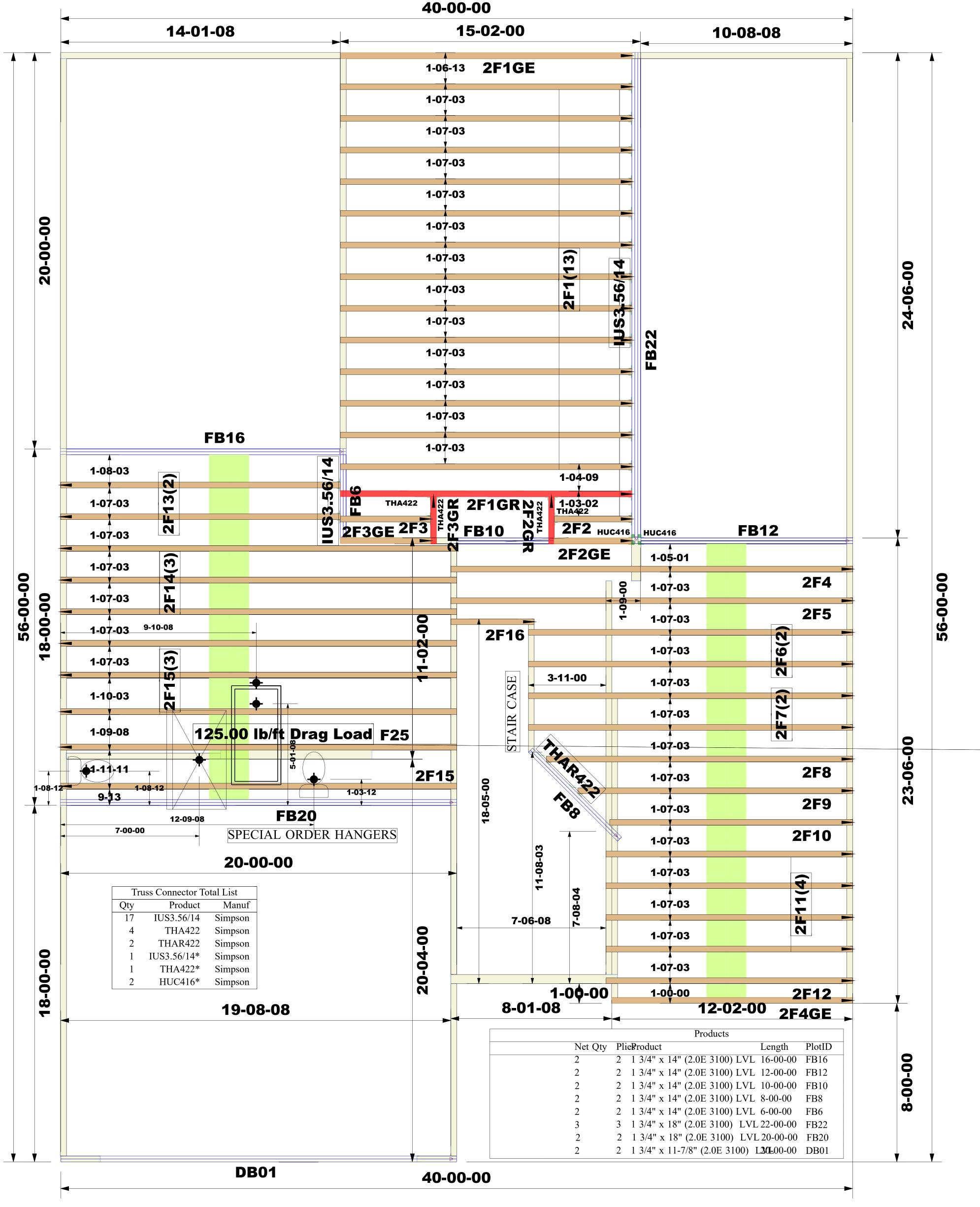
("CUT") ON THE PLANS.



TYPICAL PORCH
POST CONNECTION DETAIL
SCALE NONE

ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

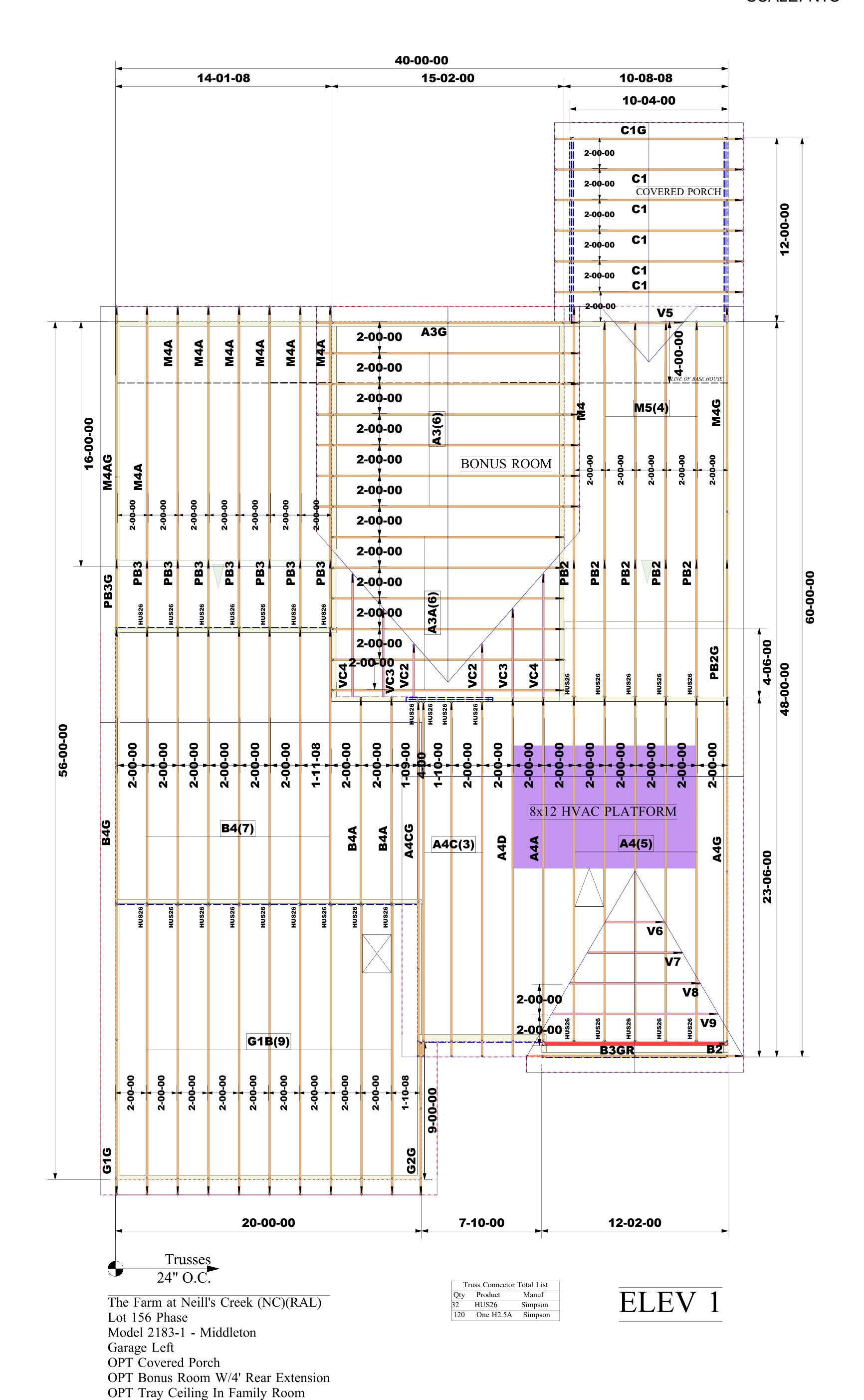
LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN SPECIFIC AND ARE ONLY REQUIRED

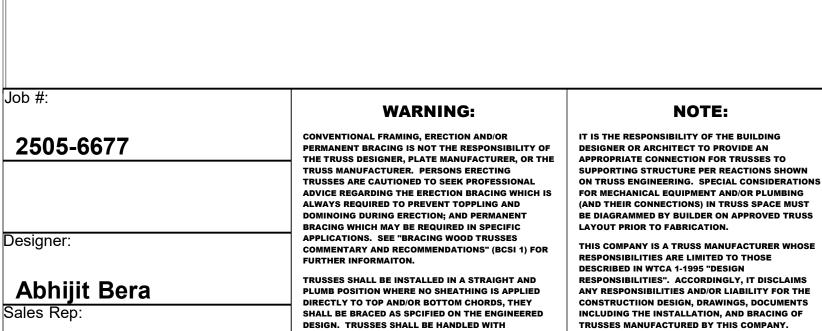


The Farm at Neill's Creek (NC)(RAL)
Lot 153 Phase
Model 2183-1 - Middleton
Garage Left
OPT Bonus Room W/4' Rear Extension

ELE 1

Job #: <b>2505-6673</b>	CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE	DESIGNER OR ARCHITECT TO PROVIDE AN	Customer:	DRB Raleigh		
Job Path:	THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	Job Name:	Farm at Neills Creek	Third-Party Quality Assurance Licensee TPI Plant W974	
Designer:  Abhijit Bera	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON.  TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS. THEY	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTIION DESIGN. DRAWINGS. DOCUMENTS	<u>Lot #:</u> 153		Structural, LLC 201 Poplar Avenue Thurmont, MD 21788	
Sales Rep:  Robbie Zarobinski	SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SWHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.	INCLUDING THE INSTALLATION, AND BRACING OF	Model Nan	<u>ne:</u> Middleton	Phone: 301-271-7591 Fax: 301-271-5441	





REASONABLE CARE DURING ERECTION TO PREVENT

DAMAGE OR PERSONAL INJURY.

Robbie Zarobinski

Customer: DRB Raleigh Job Name: The Farm at Neills Creek Lot 00.0153 Roof <u>ot #:</u> Lot 00.0153

Model Name: Middleton

TPI Plant W974 Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

