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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 58877 JOB: 25-3336-R01 JOB NAME: LOT 0.0048 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 67 Truss Design(s)

Trusses:

GR01, GR02, GR03, GR04, GR05, GR06, GR07, J01, J02, J03, J04, J05, J06, J07, J07A, J08, J09, J11, J12, J13, J14, J15, J16, J17, J17A, J19, J20, J21, J22, J23, J24, J25, J26, J27, J28, P01, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, R19, R20, R21, R22, R23, R24, R24A, R24B, R25, R26, R27, VT01, VT02



My license renewal date for the state of North Carolina is 12/31/2025

Warning !-- Verify design parameters and read notes before use.



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELI	BY MEADOW LANE ANGIER, NC
25-3336-R01	GR02	Hip Girder	1	2	Job Reference (optional)	# 58877
			100 51	0.0004.0.		0 / A 00 / 7 / 7 57 0005 D

Run: 8.430 s_Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc._Sat Apr 26 17:17:57 2025_Page 2 ID:qqlfH?RqemZ1wWmxuKuRIUzBcTx-Or1w2VTX4axq_kd4gFtBpZstWjZjSRJ1BuEVkNzMoBu

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-5=-60, 5-8=-60, 2-7=-20

Concentrated Loads (lb)

Vert: 4=-83(F) 5=-83(F) 10=-97(F) 3=-48(F) 13=-58(F) 12=-29(F) 11=-29(F) 6=-48(F) 9=-58(F) 14=-63(F) 15=-15(F) 16=-83(F) 17=-83(F) 18=-15(F) 19=-63(F) 20=-43(F) 21=-97(F) 22=-29(F) 23=-29(F) 24=-43(F)





2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 4 and 3 lb uplift at joint 2.

LOAD CASE(S) Standard





NOTES-(12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 7 and 40 lb uplift at joint 4.
- 10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20





Max Uplift4=-27(LC 9), 5=-7(LC 12) Max Grav 7=259(LC 1), 4=108(LC 1), 5=86(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 4 and 7 lb uplift at joint 5.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 7=259/0-3-8 (min. 0-1-8), 4=68/Mechanical, 5=117/Mechanical Max Horz 7=125(LC 12) Max Uplift4=-8(LC 9), 5=-58(LC 12) Max Grav 7=259(LC 1), 4=68(LC 1), 5=120(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 4 and 58 lb uplift at joint 5.

LOAD CASE(S) Standard





- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 3.

LOAD CASE(S) Standard





NOTES- (12-16)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 7, 33 lb uplift at joint 4 and 19 lb uplift at joint 5.
- 10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NC
25-3336-R01	J01	Jack-Open Girder	1	1	Job Reference (optional)	# 58877
		Ru ID:G	n: 8.630 s Jul HOhT5MOV	12 2024 Pri 4FkLKIPfX	int: 8.630 s Jul 12 2024 MiTek Industries, Inc. S <2c9QzXMNI-KE9qTBVncCBXD1nSngvfv	at Apr 26 17:17:59 2025 Page 2 vIcXG8wM6KeCjboFzMoBs

12) Trusses designed with 2018 IRC also comply with 2015 IRC.

13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

6) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

 Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)

Vert: 3=-2(B) 6=-8(B) 8=-74(B)







Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HI	LLS 56 SHELBY MEADOW LANE ANGIER, NC
25-3336-R01	J04	Jack-Open	5		1	# 58877
			Run: 8.630 s Jul	12 2024 P	Job Reference (optional) Print: 8.630 s Jul 12 2024 MiTek I	ndustries, Inc. Sat Apr 26 17:18:00 2025 Page 1
		0 10 8 2 3 8	ID:GHOhT5MO	4FkLKIP	fX2c9QzXMNI-pQi2gXVPN	/JOrBLfLNQuRBUUNxcbfpYTtsS9KhzMoB
		0-10-8 2-3-8	1-8-8			
						Scale = 1:27.6
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			1			
		3x8	2x4			
		2-3-8	4-0-0			
	4 40 0 4 0]	2-3-8	1-8-8			
Plate Offsets (X,Y) [8:0	-4-12,0-1-8]		1			
LOADING (pst) TCLL (roof) 20.0	SPACING-	2-0-0 CSI .	DEFL.	in ((loc) I/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.16	Vert(LL)	0.02	6 >999 240 6 >999 180	MT20 244/190
TCDL 10.0	Rep Stress Incr	YES WB 0.00	Horz(CT)	-0.02	5 n/a n/a	
BCDL 10.0	Code IRC2021/TP	I2014 Matrix-AS				Weight: 21 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x4 SP No	.2		TOP CHORD	Structu	ral wood sheathing direct	ly applied, except end verticals.
BOT CHORD 2x4 SP No B2: 2x4 SP	.2 *Except*		BOT CHORD	Rigid c	eiling directly applied.	in a second s
WEBS 2x4 SP No	.3			be ins	k recommends that Stabili stabilities that Stabilities the stability of th	Izers and required cross bracing
				Instal	lation guide.	
REACTIONS. (lb/size) Max Horz	8=221/0-3-8 (min. 0-1-8), 4 8=136(LC 12)	=81/Mechanical, 5=62/Mechanical				
Max Uplift	4=-63(LC 12), 5=-32(LC 12)				
Max Grav	8=221(LC 1), 4=91(LC 20),	5=71(LC 20)				
FORCES. (Ib) - Max. Cor	mp./Max. Ten All forces 2	250 (lb) or less except when shown.				
1) Wind: ASCE 7-16: Vult	=120mph (3-second aust)	vasd=95mph: TCDL=5.0psf: BCDL:	=5.0psf: h=23ft: C	at. II: Exi	p B: Enclosed: MWFRS	
(envelope) gable end z	one and C-C Exterior(2E) z	one; end vertical left exposed;C-C	for members and f	orces &	MWFRS for reactions	
shown; Lumber DOL=1	.60 plate grip DOL=1.60	=1 15 Plate DOI =1 15): Pf=20.0 ps	sf (Lum DOI =1 15	Plate D	OI =1 15): Is=1 0: Rough	
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10				0E-1.10), 13-1.0, 100gh	
3) This truss has been de	signed for greater of min ro	of live load of 12.0 psf or 2.00 times	s flat roof load of 2	20.0 psf o	on overhangs	
4) This truss has been de	signed for a 10.0 psf bottor	n chord live load nonconcurrent with	n any other live loa	ads.		
5) * This truss has been c	lesigned for a live load of 3	0.0psf on the bottom chord in all are	eas where a rectar	ngle 3-6-	0 tall by 1-0-0 wide will fit	
6) Refer to girder(s) for tr	ord and any other members	S.				
7) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of withsta	anding 63 lb uplift	at joint 4	and 32 lb uplift at joint 5.	
8) This truss design requi	res that a minimum of 7/16	structural wood sheathing be appli	ied directly to the t	op choro	d and 1/2" gypsum	
9) Trusses designed with	2018 IRC also comply with	2015 IRC.				and the second s
10) Graphical bracing rep	resentation does not depic	the size, type or the orientation of t	the brace on the m	nember.	Symbol only indicates	WING TH CAROLINI
11) Bearing symbols are	t be braced. only graphical representation	ons of a possible bearing condition	Bearing symbols a	are not c	onsidered in the	AOFESSION STILL
structural design of th	e truss to support the loads	indicated.			in the second seco	1 A China and a
12) Web bracing shown is	s for lateral support of indiv	dual web members only. Refer to B	CSI - Guide to Go	od Pract	tice for Handling,	SEAL
13) SEE BCSI-B3 SUMM	ARY SHEET- PERMANEN	T RESTRAING/BRACING OF CHO	RDS & WEB MEN	IBERS F	FOR RECOMMENDED	28147
MINIMUM BRACING	REQUIREMENTS OF TOP	CHORD, BOTTOM CHORD, AND	WEB PLANES. II	N ADDIT		No. al I
MINIMUM GUIDELIN	ES, ALWAYS CONSULT T	HE PROJECT ARCHITECT OR EN	IGINEER FOR AD	UIION		VOINEE
						Mark K. MORTHUM
LOAD CASE(S) Standard	l					1050005
						A/JS/JUD5

Job	Truss	Truss Type	Qty	Ply LO	T 0.0048 HONEYCUTT HIL	LS 56 SHELBY MEADOW LANE ANGIER, NC
25-3336-R01	J05	Jack-Open	1	1		# 58877
			Pup: 8 630 c Jul	Jo	b Reference (optional)	π JOO//
			ID:GHOhT5MOv4	FkLKIPfX2c9C	ZXMNI-HcGRusW18pF	RFSLwrv5x7_P1eUKzcOGbc6WCit8zMoBq
		-0-10-8 <u>2-3-8</u> 0-10-8 2-3-8	0-4-8	4-0-0 1-4-0		
						Seels = 1:20.7
			4x4 = 4	5 🕅		Scale - 1.20.7
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		10	2x4			
		3x8				
		2-3-8	2-8-0	4-0-0		
Plate Offsets (X,Y) [4:0-	-2-8,0-2-0], [10:0-4-12,0-1-8	3]	0-4-8	1-4-0		
LOADING (psf)	SPACING	2.0.0	DEEL	in (loo)	l/dofl l/d	
TCLL (roof) 20.0	Plate Grip DOL	1.15 TC 0.14	Vert(LL)	0.02 8	>999 240	MT20 244/190
TCDL 10.0	Lumber DOL	1.15 BC 0.17	Vert(CT)	-0.02 8	>999 180	
BCLL 0.0 *	Code IRC2021/TP	I2014 Matrix-AS	Horz(CT)	-0.02 5	n/a n/a	Weight: 23 lb FT = 20%
			DRACING			
TOP CHORD 2x4 SP No	.2		TOP CHORD	Structural w	ood sheathing directl	y applied, except end verticals.
BOT CHORD 2x4 SP No	.2 *Except*		BOT CHORD	Rigid ceiling	g directly applied.	
WEBS 2x4 SP No	.3			MiTek rec	ommends that Stabili	zers and required cross bracing
				Installation	n guide.	
REACTIONS. (lb/size)	10=221/0-3-8 (min. 0-1-8),	5=66/Mechanical, 6=78/Mechanica	l			
Max Uplift	5=-20(LC 9), 6=-26(LC 12)					
Max Grav	10=221(LC2), 5=66(LC1),	6=78(LC 2)				
FORCES. (Ib) - Max. Cor	np./Max. Ten All forces 2	250 (Ib) or less except when shown.				
NOTES (11 15)						
1) Unbalanced roof live lo	ads have been considered	for this design.				
2) Wind: ASCE 7-16; Vult	=120mph (3-second gust)	Vasd=95mph; TCDL=5.0psf; BCDL:	=5.0psf; h=23ft; C	at. II; Exp B; I	Enclosed; MWFRS	
shown; Lumber DOL=1	.60 plate grip DOL=1.60	one, end vertical leit exposed, C-C			I I TO IOI TEACIIOTIS	
3) TCLL: ASCE 7-16; Pr=	20.0 psf (roof LL: Lum DOL	=1.15 Plate DOL=1.15); Pf=20.0 ps	sf (Lum DOL=1.15	Plate DOL=	1.15); ls=1.0; Rough	
4) This truss has been de	signed for greater of min ro	of live load of 12.0 psf or 2.00 times	s flat roof load of 2	20.0 psf on ov	verhangs	
non-concurrent with oth	ner live loads.	in a				
6) This truss has been de	signed for a 10.0 psf bottor	ng. n chord live load nonconcurrent with	n any other live loa	ads.		
7) * This truss has been d	esigned for a live load of 3	0.0psf on the bottom chord in all are	eas where a rectar	ngle 3-6-0 tall	l by 1-0-0 wide will fit	
8) Refer to girder(s) for tr	ord and any other members uss to truss connections.	5.				
9) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of withsta	anding 20 lb uplift	at joint 5 and	26 lb uplift at joint 6.	ANNIHIII COMPANY
sheetrock be applied	dires that a minimum of 7/10	o" structural wood sneatning be app	blied directly to the	e top chord ar	nd 1/2" gypsum	WINATH CARO
11) Trusses designed with	n 2018 IRC also comply wit	h 2015 IRC.			in the second se	OFESSID
12) Graphical bracing repl that the member must	resentation does not depict	the size, type or the orientation of t	the brace on the m	nember. Sym	bol only indicates	and the
13) Bearing symbols are o	only graphical representation	ons of a possible bearing condition.	Bearing symbols a	are not consid	dered in the	SEAL
structural design of the 14) Web bracing shown is	e truss to support the loads for lateral support of indivi	indicated. Idual web members only Refer to B	CSI - Guide to Go	od Practice f	or Handling	28147
Installing, Restraining	& Bracing of Metal Plate C	connected Wood Trusses for additio	nal bracing guidel	lines, includin	ng diagonal bracing.	
15) SEE BCSI-B3 SUMM	ARY SHEET- PERMANEN	T RESTRAING/BRACING OF CHO	RDS & WEB MEN	ABERS FOR	RECOMMENDED	A NOINEER S
MINIMUM GUIDELINE	ES, ALWAYS CONSULT T	HE PROJECT ARCHITECT OR EN	IGINEER FOR AD	DITIONAL B	RACING	WINK K. MORMUN
CONSIDERATIONS.						and the second s
						4/25/2025
Warning !	parameters and read notes be	efore use. This design is based only upon	parameters shown, a	and is for an ind	ividual building compone	ent to be installed and loaded
of individual web members	action parameters and proper in a Additional temporary brack	corporation of component is responsibility	y of building designer	r – not truss des of the erector	Additional permanent br	Dracing snown is for lateral support

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE ANGIER, NC
25-3336-R01	J05	Jack-Open	1	1	Job Reference (optional)	# 58877

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LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHEL	BY MEADOW LANE ANGIER, NC
25-3336-R01	J06	Jack-Open Girder	1	1	Job Reference (optional)	# 58877
		Run: ID:GHC	8.630 s Jul hT5MOv4F	12 2024 Pri kLKIPfX2	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. c9QzXMNI-HcGRusW18pRFSLwrv5x7	Sat Apr 26 17:18:01 2025 Page 2 P1dLKzMOGbc6WCit8zMoBg

12) Trusses designed with 2018 IRC also comply with 2015 IRC.

13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

 loads indicated.
 15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

(6) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

 Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-5=-60, 8-10=-20, 6-7=-20 Concentrated Loads (lb)

Vert: 3=-2(F) 4=-2(F) 9=-8(F) 8=-8(F)





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (12-16)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 7, 41 lb uplift at joint 4 and 12 lb uplift at joint 5.
- 10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE ANGIER, NC
25-3336-R01	J07	Jack-Open Girder	1	1	Job Reference (optional)	# 58877
		ID:	Run: 8.630 s Jul 2 GHOhT5MOv4F	12 2024 Pri FkLKIPfX2	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sa 2c9QzXMNI-Ipqp5CXgv7Z64VV1ToSMWc	at Apr 26 17:18:02 2025 Page 2 canTkFm7jhmLAxFPazMoBp

12) Trusses designed with 2018 IRC also comply with 2015 IRC.

13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)

Vert: 3=-21(B) 6=-19(B) 8=-21(B) 9=-19(B)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHEL	BY MEADOW LANE ANGIER, NC
25-3336-R01	J07A	Jack-Open	1	1	Job Reference (optional)	# 58877

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LOAD CASE(S) Standard



Jop	Truss	Truss Type	Qty	Ply	LOT	0.0048 HONEYCUTT HI	LLS 56 SHELBY MEADO	W LANE ANGIER, NC
25-3336-R01	J08	Jack-Open	2		1	Deference (entional)	#	58877
			Run: 8.630 s Jul	12 2024	Print: 8.6	30 s Jul 12 2024 MiTek li	ndustries, Inc. Sat Apr 26	17:18:02 2025 Page 1
		-0-10-8 3-2-0	ID:GHOn15MOV	4FKLKIP 4-6-0	-1X2C90	ZXMNI-Ipqp5CXgv7Zi	64VV110SMWcaozkH	I/JQmLAxFPazMoBp
		0-10-8 3-2-0	, i	1-4-0	,			
			4x4 = 3		4 🗖			Scale = 1:23.3
]		Æ	T2		[
		12.00 12						
	0-0-	T 1				0-		
	4		W2			3-8-4		
		2						
	q							
	0-10	/ /·						
]]				Δ	ll		
			6					
		7	2x4		5			
		3x8						
		3-2-0		4-6-0				
Plate Offsets (X,Y) [3:0-	-2-8,0-2-0], [7:0-4-12,0-1-8]		1				T	
LOADING (psf)	SPACING-	2-0-0 CSI .	DEFL.	in	(loc)	l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0 Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.22	Vert(LL)	0.04	6-7	>999 240	MT20	244/190
TCDL 10.0	Rep Stress Incr	1.15 BC 0.24 YES WB 0.04	Horz(CT)	-0.04 -0.05	6-7 4	>999 180 n/a n/a		
BCLL 0.0 ^ BCDL 10.0	Code IRC2021/TP	I2014 Matrix-AS					Weight: 23 lb	FT = 20%
LUMBER-			BRACING-					
TOP CHORD 2x4 SP No	.2		TOP CHORD	Struct	tural wo	ood sheathing direct	ly applied, except er	nd verticals.
WEBS 2x4 SP No	.2 .3		BOT CHORD	Rigid	ceiling	directly applied.	zoro and required or	and bracing
				be in	nstalled	during truss erection	on, in accordance wit	h Stabilizer
	7-240/0.3.8 (min 0.1.8)	-70/Mechanical 5-94/Mechanical		Insta	allation	guide.		
Max Horz	7=114(LC 12)							
Max Uplift	4=-14(LC 9), 5=-46(LC 12)	5-06/1 C 24)						
Wax Grav	7-240(LC T), 4-70(LC T), C	J-90(LC 24)						
FORCES. (Ib) - Max. Cor	mp./Max. Ten All forces 2	250 (lb) or less except when shown.						
NOTES- (11-15)								
1) Unbalanced roof live lo	ads have been considered	for this design.	-5 Opef: h-23ft: C	at II.E.	vn B· E	Inclosed: MW/EPS		
(envelope) gable end z	one and C-C Exterior(2E) z	zone; end vertical left exposed;C-C	for members and f	orces &	& MWF	RS for reactions		
shown; Lumber DOL=1	.60 plate grip DOL=1.60	-1 15 Plate DOI -1 15): Pf-20.0 p	ef (Lum DOI −1 15	Diato D	1– וחר	15): ls=1 0: Pouch		
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10				JOL-1	. 10), 13–1.0, 100ugii		
 This truss has been de non-concurrent with oth 	signed for greater of min ro	oof live load of 12.0 psf or 2.00 time	s flat roof load of 2	0.0 psf	on ove	erhangs		
5) Provide adequate drain	age to prevent water pond	ing.						
 6) This truss has been de 7) * This truss has been de 	signed for a 10.0 psf bottor lesigned for a live load of 3	n chord live load nonconcurrent wit 0 0psf on the bottom chord in all ar	h any other live loa eas where a rectar	ads. Indle 3-6	S-0 tall	by 1-0-0 wide will fit		
between the bottom ch	ord and any other members	S.		igic 0-0		by 100 wide will lit		
 8) Refer to girder(s) for tr 9) Provide mechanical control 	uss to truss connections.	s to bearing plate capable of withst	anding 14 lb unlift :	at ioint 4	4 and 4	46 lb unlift at joint 5		
10) This truss design requ	uires that a minimum of 7/1	6" structural wood sheathing be app	plied directly to the	top cho	ord and	d 1/2" gypsum	MUMBELLE CAD	11.
sheetrock be applied	directly to the bottom chord a 2018 IRC also comply wit	l. h 2015 IRC					IN BILL CAROL	Alle.
12) Graphical bracing rep	resentation does not depict	t the size, type or the orientation of	the brace on the m	nember.	. Symb	ol only indicates 🌋	POPOFES PNG	PIL
that the member must	t be braced. only graphical representation	ons of a possible bearing condition	Bearing symbols	are not a	consid	ered in the	SEAL	
structural design of th	e truss to support the loads	s indicated.	Doaring oynibolo c		oonola		28147	
14) Web bracing shown is Installing Restraining	s for lateral support of indivi & Bracing of Metal Plate C	idual web members only. Refer to E connected Wood Trusses for addition	BCSI - Guide to Go	od Prac	ctice fo	r Handling,	1	
15) SEE BCSI-B3 SUMM	ARY SHEET- PERMANEN	T RESTRAING/BRACING OF CHC	RDS & WEB MEN	IBERS	FOR F		A ANGINEER	- Mill
	REQUIREMENTS OF TOP		WEB PLANES. IN	N ADDI			ARK & MORP	ATTING STATES
CONSIDERATIONS.	LO, ALWATO CONSULT I		NGINEER FUR AD				Man A. Mounth	
Continued on page 2							4/25/202	5
Warning !—Verify design	parameters and read notes be	efore use. This design is based only upon	n parameters shown, a	nd is for	an indi	vidual building compone	ent to be installed and lo	aded
vertically. Applicability of o	design parameters and proper in only Additional temporary broad	corporation of component is responsibilit	y of building designer	- not tru	uss desi	gner or truss engineer.	Bracing shown is for lat	eral support

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS	S 56 SHELBY MEADOW LA	ANE ANGIER, NC
25-3336-R01	J08	Jack-Open	2	1	Job Reference (optional)	# 5	8877
		Run: i ID:0	3.630 s Jul GHOhT5M	12 2024 Pri Dv4FkLKI	nt: 8.630 s Jul 12 2024 MiTek Indu PfX2c9QzXMNI-D?OBJYYIgC	ustries, Inc. Sat Apr 26 17:18 Qizif4D0Wzb3q6zj8d5sAg	3:03 2025 Page 2 gvZqhpx0zMoBo

LOAD CASE(S) Standard





) SEE BOSIED SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMME MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Max Uplift3=-44(LC 12), 2=-8(LC 12) Max Grav 3=85(LC 20), 2=187(LC 1), 4=59(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 3 and 8 lb uplift at joint 2.
- 8) Trusses designed with 2018 IRC also comply with 2015 IRC.

LOAD CASE(S) Standard





4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 10 lb uplift at joint 2.

- 8) Trusses designed with 2018 IRC also comply with 2015 IRC.
- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the
- veb bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDER MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES IN ADDITION CONSIDERATIONS 12) SEE BČŠI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR ŘECOMMENDED
- CONSIDERATIONS.

LOAD CASE(S) Standard

25/202.5 and NOINEE K. MORR 4/25/2025



4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 10 lb uplift at joint 2.

- 8) Trusses designed with 2018 IRC also comply with 2015 IRC.
- 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the
- web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDER MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES IN THE PROJECTION CONSIDERATIONS 12) SEE BČŠI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR ŘECOMMENDED
- CONSIDERATIONS.

LOAD CASE(S) Standard

25/202.5 and NOINEE K. MORR 4/25/2025



responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NC
25-3336-R01	J14	Jack-Open Structural Gable	1	1	Job Reference (optional)	# 58877

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vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NC
25-3336-R01	J17A	Monopitch Supported Gable	1	1	Job Reference (optional)	# 58877
		Run ID:GH0	8.630 s Jul hT5MOv4I	12 2024 Pri FkLKIPfX2	int: 8.630 s Jul 12 2024 MiTek Industries, Inc. S c9QzXMNI-9OWxkEZYB2yhxzEc8w038F	Sat Apr 26 17:18:05 2025 Page 2 FCH9yIgK4oC18Aw0vzMoBm

LOAD CASE(S) Standard





8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Trusses designed with 2018 IRC also comply with 2015 IRC.

LOAD CASE(S) Standard






Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	BY MEADOW LANE ANGIER, NC
25-3336-R01	J21	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GH	.630 s Jul IOhT5MC	12 2024 Pri v4FkLKIP	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. S fX2c9QzXMNI-da4KxaaAyL4YZ6poieXII	Sat Apr 26 17:18:06 2025 Page 2 hSkQ3Lc83VkMFovTYLzMoB

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

5) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY N	ALADOW LANE ANGLE	R, NC
25-3336-R01	J22	Roof Special	1	1	Job Reference (optional)	# 58877	
		Run: 8 ID:GH	.630 s Jul OhT5MOv	12 2024 Pri 4FkLKIPf	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat X2c9QzXMNI-5mei8wbojfCPAGO?FL2XDg	Apr 26 17:18:07 2025 Pa Hdwl_vozZVUSf04nzl	age 2 MoBk

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

(5) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHEL	BY MEADOW LANE ANGIER, NC
25-3336-R01	J23	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GH	3.630 s Jul OhT5MOv	12 2024 Prii 4FkLKIPfX	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. <2c9QzXMNI-azB4MGcQUzKGoQzBp3	Sat Apr 26 17:18:08 2025 Page 2 Zmmtgrj9KjXPdej6PacEzMoB

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

(5) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard









Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NC
25-3336-R01	J26	Half Hip Girder	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GH0	.630 s Jul DhT5MOv	12 2024 Prii 4FkLKIPfX	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. S (2c9QzXMNI-29ISZcc3FGS7QaYNNm4?	at Apr 26 17:18:09 2025 Page 2 J5MyhZgrGrkoym879gzMoB

13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-5=-60, 6-8=-20 Concentrated Loads (lb)

Vert: 3=-2(F) 7=-7(F) 11=-2(F) 12=-7(F)











A Constraint of the set of t	Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HIL	LS 56 SHELBY MEADOW LANE ANGIE	R, NC
All ED MALED MALED <t< td=""><td>25-3336-R01</td><td>R01</td><td>Hip Girder</td><td>1</td><td>1</td><td></td><td># 58877</td><td></td></t<>	25-3336-R01	R01	Hip Girder	1	1		# 58877	
Line State The sta				Run: 8.630 s Jul	12 2024 Prin	Job Reference (optional) t: 8.630 s Jul 12 2024 MiTek Ir	Idustries, Inc. Sat Apr 26 17:18:12 2025 P	age 1
<pre>scale = 17:00</pre>	0.10	0000 660	10.0.0 12.6.0 15.7.12 20	ID:GHOhT5MO	v4FkLKIPfX	2c9QzXMNI-SkRbCdfxYB	qhH1Gy2ueiwj_NMmbET4_EekNnl?z	zMoBf
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NULLD NULLD <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td>Scale = 1</td><td>75.6</td></th<>							Scale = 1	75.6
$\frac{1}{1000000} = \frac{1}{10000000000000000000000000000000000$			NAILED	NA	AILED			
Image: State of the state			NAILED NAIL	ED NAILED				
$ \begin{array}{c} \text{NALED} & \text{AUE} = \frac{34 + 104}{94} - $		5x6 =	NAILED NAILED	NAILED	NAILE	ED NAILED		
Image: State of the state	ΝΔΙΙ Ε	D NAILED	$4x4 = 3x4 \parallel 4x4 =$	4x4 = 3x8	= 4x4 =	NAILED 5x6 =		
Image: Second		00 12 5 3	3 634 35 36 ⁷ 72 ⁸	37 9 39	10 40	_141 ⁴² 43 12 [3 NAIL	ED3.00 12	
Image: Provide the standard standar	4x4 🗧	nailed to the second s				1	NAILED	
Image: Provide and the second seco	9	7x8 = W4	W5 W6 B4 W7 V	V10			13 4×4 - 9	
Image: Section 1 Image: Section 2 Image: Section 2 <td< td=""><td>1</td><td>3 W1 W2</td><td></td><td>WY1</td><td>VVVI</td><td>WAZ WIS</td><td></td><td></td></td<>	1	3 W1 W2		WY1	VVVI	WAZ WIS		
[14] 3 24 str. 48 48 <td>1-1-3</td> <td></td> <td></td> <td> \/</td> <td></td> <td>W14</td> <td>W15 W16 W17 19 19</td> <td></td>	1-1-3			\/		W14	W15 W16 W17 19 19	
Solution		B B 1 C C C C C C C C C C	23 a B5 5 a f		ոս 🖬ո			
Avid = 0:5 = NULED		²⁶ NAILED 4x8 =	22 21	0 51 52 20	0 53 19	^{54 55} 18 ⁵⁶	17 ⁵⁷ 16	
NALED NALED NALED NALED NALED NALED NALED NALED NALED NALED Plate Offsets (X/Y)- 16.0-26,0-26,10,21,20-24,0-2-4,0,12-4,012	4	$x_4 = 5x_5 =$ NAILED	NAILED 3x6 6x8 =	NAILED 4x6	= 4x8 =	= NAILED 4x8 =	4x6 = 3x6 ∥	
NALED NALED NALED +238 +289 +289 +21-12 +22-14 +28-14 +28-14 +27-12 <		NAI	ED NAILED NAIL	ED NAILED		ED NAILED M	VAILED	
$\frac{23.8}{23.8} + \frac{44.0}{23.8} + \frac{13.40}{23.4} + \frac{15.7.0}{70.4} + \frac{22.6.14}{27.1.2} + \frac{29.6.0}{6.1.2} + \frac{33.7.0}{4.1.3} + \frac{37.4.0}{3.4.1.3} + \frac{37.4.0}{3.4.1.3}$ Plate Offsets (X/Y) - (16.2-8.0.6-8.0) [5.9-4.0.2-4.0] (12.9.2-2.0.3-0.0) [22.9.2-2.0.3-0.0] [23.9.2-4.0.3-0]			NAILED	NA	AILED	NAILED		
H 2-36 15-00 15-712 22-914 29-00 35-7.3 57-00 1 Plate Offsets (X,Y) = (40-24, 02-8, 01, 50-44, 02-41, 120-44, 02-41, 1210-4-00-240, 1210-2-00-20, 123-02-4, 0-30, 0-2								
Lock 23.8 64.0 136.0 157.12 22.44 28.12 33.73 37.00 Plate Offsets (XV)- 16.02.8.0-6.91, 60.44.0.2-41 [12.0.4-0.2-41, 12.10.20,0.3-01, [23.0-24.0.3-0] Image: Control of the control of t								
Link Link <thlink< th=""> Link Link <thl< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></thl<></thlink<>								
Plate Offsets (XY): L (40-24.0.6-81, [50-44.0.2-41, [12:0.4-0.0-3:0] PLATES GRIP COUNING (root) 20.0 Plate Chip Dott 115 Colo Verticul 0.63 24.25 599 240 MT20 244/190 Show (r) 0.0 Rep Stress in cr NO WB 0.67 Horz(CT) 0.06 16 No No Weight: 249 lb FT = 20% BCOL 10.0 Rep Stress in cr NO WB 0.67 Horz(CT) 0.06 16 No No Weight: 249 lb FT = 20% BCOL 10.0 Rep Stress in cr NO Matrix:MSH BRACING- TOP CHORD Xeg Stress in cr NO Weight: 249 lb FT = 20% BCOL 10.0 No 2 BC - 2500 Stress in cr NO Weight: 249 lb TT = 20% BCOL 10.0 No 2 BC - 2500 Stress in cr NO No Stress in cr NO No<		2-3-8 6-6-0	<u>13-6-0</u> 7-0-0 <u>15-7-12</u> 2-1-12	<u>22-6-14</u> 6-11-2	<u>29</u> 6-1	<u>-6-0 33-7-3</u> 11-2 4-1-3	<u></u>	
LOADING (pist) TCLL (root) SPACING- 200 2-00 ISB OFFL In DFL In DFL MT20 244/190 Snow (P) 200 Filted Gip DOL 1.15 BC 0.68 Vert(CT) 0.02425 >999 180 BOLL 0.0 Code IRC2021/TPI2014 WB 0.67 0.06 1.6 n'a Weight: 249 lb FT 2.04 DOP CHORD 24.9 SP No.2 Force TOP CHORD Statuting directly applied or 4-4-15 oc purlins, except and verticals Weight: 249 lb FT = 20% UMBER 22.24 SP No.3 BT TOP CHORD Statuting vertical and except and verticals Not and verticals Statuting vertical and verticals PL/ST = 20% WEBS 24.4 SP No.3 WEBS BT TOP CHORD Statuting vertical and	Plate Offsets (X,Y) [4:0	-2-8,0-6-8], [5:0-4-4,0-2-4],	[12:0-4-4,0-2-4], [21:0-2-0,0-3-0], [2	23:0-2-4,0-3-0]				
TCLL (root) 20.0 Piase Grip DOL 1.15 TC 0.80 Vert(L) -0.02 ±2.25 > 599 24.0 MT20 244/190 TCD 10.0 Rep Stress hor. NO WB 0.67 Horz(CT) 0.10 24-25 > 599 24.0 Weight: 249 lb FT = 20% UMBER Code IRC2021/TPI2014 Matrix-MSH BRACING- TOP CHORD 24.45 P No.2 Weight: 249 lb FT = 20% UMBER 22.45 P No.2 TOP CHORD Structural wood sheathing directly applied or 4.4-15 oc purlins, except BCT CHORD 24.55 P No.2 BT TOP CHORD Structural wood sheathing directly applied or 4.4-15 oc purlins, except BCT CHORD 24.55 P No.3 BCT CHORD 24.55 P No.3 BCT CHORD 24.55 P No.4 Rigid celling directly applied or 4.4-15 oc purlins, except BCT CHORD 24.55 P No.3 Rigid celling directly applied or 4.4-15 oc purlins, except BCT CHORD 24.55 P No.3 BCT CHORD 24.55 P No.3 BCT CHORD 24.55 P No.3 Rigid celling directly applied or 4.4-15 oc purlins, except BCT CHORD 24.55 P No.3 BCT CHORD 25.50(C-13) BCT CHORD 24.55 P No.3 Rigid cel	LOADING (psf)	SPACING-	2-0-0 CSI	DEFL	in (lo	c) l/defl l/d	PLATES GRIP	
TCOL 0.0 Lumber DOL 1.15 BC 0.68 Vert(CT) -0.10 24-25 -999 180 BCLL 0.0 Code IRC2021/TPI2014 WB 0.67 Horz(CT) 0.00 12 Iso No BCDL 0.0 Code IRC2021/TPI2014 WB 0.67 Horz(CT) 0.00 24 SP No 2 UMBER TOP CHORD 2x4 SP No 2 Description BRACMG. TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purtins, except end verticals BOT CHORD 2x4 SP No 3 BOT Description BOT CHOR of the origination origination of the origination origination origination origination originatin originatin origination origin origination origin origin origi	TCLL (roof) 20.0 Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.80	Vert(LL)	-0.06 24-2	25 >999 240	MT20 244/190	
BCLL 0.0 Code iRC2021TP1201 Matrix-MSH INDUC1/ 000 To Take The Weight: 249 to FT = 20% LUMBER. TOP CHORD 2x4 SP No.2 Top CHORD 2x4 SP No.2 BEXCING- TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 4.4.15 oc purlins, except end wettals. BOT CHORD 2x4 SP No.3 BB2 XAMS BOT CHORD 2x4 SP No.3 Structural wood sheathing directly applied or 6.4.0 cb charge. WEBS 2x4 SP No.3 BDT CHORD 2x4 SP No.3 WEBS Structural wood sheathing directly applied or 6.4.0 cb charge. REACTIONS. (Ibizze) 2=55(IC 62) Max Horz2=35(IC 62) Max Grav2=732(IC 39), 16=986(IC 46), 21=3332(IC 38) BDT CHORD 2x4 SP No.3 Mitter weight: 249 Ib FT = 20% FORCES. (Ib) -Max Comp. Max Ten All forces 250 (Ib) or less except when shown. TOP CHORD 2x4 SP No.3 Mitter weight: 343 SP 200, 33-343 SP 200, 6-358 - 342/1129, 5-368 - 342/1129, 5-368 - 342/1129, 5-368 - 342/1129, 7-	TCDL 10.0	Lumber DOL Rep Stress Incr	1.15 BC 0.68	Vert(CT)	-0.10 24-2	25 >999 180		
Budd 100 1 1 1 Dig CHORD 2x4 SP No.2 BRACING- TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins, except end verticals. BOT CHORD 2x4 SP No.2, B4: 2x4 SP No.3 BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc bracing. WEBS 2x4 SP No.3 BOT CHORD WEBS If and werticals. Reactions. Reactions. (Ibisize) 2=529/0-3-8 (min. 0-1-8), 16=952/0-3-8 (min. 0-1-8), 21=2743/0-3-8 (min. 0-2-4) Imit and werticals. Imit and werticals. Max biorz Z=s6(LG 62) Max biorz Z=s6(LG 62) Imit and werticals. Imit and werticals. Imit and werticals. POP CHORD 2-3=-857/195, 3-4=-1705/458, 4-31=-837/206, 3-12=-552/192, 5-32=-442/204, 5-33=-347/104, 3-33-43-87/206, 3-43=-47/204, 3-33=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-39=-47/1/443, 3-49=-489/1713, 4-48=-48	BCLL 0.0 *	Code IRC2021/TF	I2014 Matrix-MSH	1012(01)	0.00 1	0 11/a 11/a	Weight: 249 lb FT = 20%	J
TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins, except end year of the set of the se								
BOT CHORD 2x6 SP No.2*Except* BZ 2x4 SP No.3 BC 2x6 SP No.3 BC 2x6 SP No.3 BC CHORD Rigid celling directly applied or 6-0-0 oc bracing. WEBS 2x4 SP No.3 BC CHORD Rigid celling directly applied or 6-0-0 oc bracing. I Row at midpt 9-21 Reactions. (bisize) 2=529/0-3-8 (min. 0-1-8), 16=952/0-3-8 (min. 0-1-8), 21=2743/0-3-8 (min. 0-2-4) Max Horz 2=95(LC 62) Max Upit72=-143(LC 12), 16=-284(LC 13), 21=-1174(LC 9) Max Grav27=73(LC 29), 16=958/0C 43, 21=-3322(LC 38) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2-3=-857/195, 3-4=-17054/36, 4-31=-587/206, 31-32=-552/192, 5-32=-442/204, 5-33=-3877(200, 6-34=-3877(200, 6-35=-3877(2112), 3-36=-42/1129, 7-36=-342/1129, 74=-349/1141, 8-37=-471/1443, 3738=-471/1443, 3-39=-471/1443, 9-33=-471/144, 9-459/405, 12-43=-956/405, 12-44=-11034/61, 13-44=-11224/52, BOT CHORD 2-28=-1746/16, 24-28=4-956/405, 12-44=-11034/61, 13-44=-12124/52, BOT CHORD 2-28=-1746/16, 24-28=-135073(10-41-5-600/273, 11-41=-60/8713, 1-14=-956/405, 42-44=-960/405, 12-44=-11034/61, 13-44=-12124/52, BOT CHORD 2-28=-1746/16, 24-28=-135073(06, 13-58=-330/669, 14-58=-348/65, 3-25=-319/1109, 14-17=-4331/368 NOTES (13-17) 1) Unbalanced roo five loads have been considered for this design. 5) This truss has been designed for atil to ge for this de	TOP CHORD 2x4 SP No	0.2		TOP CHORD	Structura	I wood sheathing directl	y applied or 4-4-15 oc purlins, exc	cept
WEBS 22.24 SP No.3 BC1 CHORD High Celling update graphies of evolution that the stabilizers and required cross bracing bit for the stabilizers and required cross bracing bit installed during trues erection, in accordance with Stabilizer Installed during under the stabilizer Installed during trues erection, in accordance with Stabilizer Installed during true serection, in accordance with Stabilizer Instable during true serection accord server serection ac	BOT CHORD 2x6 SP No	0.2 *Except*			end vertio	cals.	0.0 as brasing	
MText recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS. (Ib/size) 2=529/0.3-8 (min. 0-1-8), 16=952/0.3-8 (min. 0-1-8), 21=2743/0-3-8 (min. 0-2-4) Max Horz 2=-95(LC 62) Max Upint2=-143(LC 12), 16=-284(LC 13), 21=-1174(LC 9) Max Grave-732(LC 39), 16=-284(LC 13), 21=-1174(LC 9) Max Grave-732(LC 39), 16=-284(LC 13), 21=-1174(LC 9) Max Grave-732(LC 39), 16=-284(LC 13), 21=-1174(LC 9) To 2.3=-857/195, 3-4=-1705/458, 4-31=-567/206, 3-32=-562/192, 5-32=-442/129, 7-36=-342/1	WEBS 2x4 SP No	2 NO.2, B4: 2X4 SP NO.3 0.3		WEBS	1 Row at	midpt 9-21	0-0 oc bracing.	
EACTIONS. (b/size) 2=529/0-3-8 (min. 0-1-8), 16=952/0-3-8 (min. 0-1-8), 21=2743/0-3-8 (min. 0-2-4) Max Horz 2=35(LC 62) Max Grav 2=732(LC 39), 16=952(U-C 13), 21=-1174(LC 9) Max Grav 2=732(LC 39), 16=926(LC 45), 21=3332(LC 38) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2=3=857(195, 3-4=-1705/458, 4-31=-587/200, 33-32=-552/192, 5-32=-442/204, 5-33=387(200, 33-43=-387(200, 6-34=-387(200, 6-34=-387(200, 6-35=-342(1129, 35-36=-348(95, 32-26=-34)(1109, 36-36=-36(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-57-456(1448, 36-36)-36(14), 36-36-36(149, 36-36					MiTek r	ecommends that Stabili	zers and required cross bracing	
REACTIONS. (Ibsize) 2=5280:3-8 (min. 0-1-8), 16=9520-3-8 (min. 0-1-8), 21=2743/0-3-8 (min. 0-2-4) Max Hortz 2=-95(LC 62) Max Uplit2=143(LC 12), 16=-284(LC 13), 21=-1174(LC 9) Max Core Z-413(LC 12), 16=-284(LC 13), 21=-1174(LC 9) Max Core Z-413(LC 12), 16=-284(LC 13), 21=-332(LC 38) FORCES. (ib) - Max. Comp./Max. Ten All forces 250 (ib) or less except when shown. TOP CHORD, 23=-857/195, 34=-1705/458, 4-31=-587/206, 31-32=-552/192, 5-32=-442/204, 5-33=-387/200, 3-34=-387/200, 6-34=-387/200, 6-35=-342/1129, 35-36=-342/1129, 5-33=-387/200, 33-4=-387/200, 6-34=-387/200, 6-35=-342/1129, 35-36=-342/1129, 5-33=-387/200, 33-4=-387/200, 6-34=-387/200, 6-35=-342/1129, 35-36=-342/1129, 5-36=-342/1129, 7-8=-349/1414, 37-48=-471/443, 33-39=-47/11443, 9-39=-47/11443, 9-40=-509/273, 10-41=-509/273, 11-41=-509/273, 11-42=-995/405, 2-42=-395/405, 12-44=-1163/461, 13-44=-1123/462, 13-14=-148/4486, 14-16=-917/285 BOT CHORD, 2-26=-174/6148, 5-26=-26-309669, 19-53=-330/669, 19-54=-330/669, 0-54=-330/669, 5-455=-330/669, 19-55=-330/669, 19-55=-330/669, 19-54=-330/669, 5-455=-330/669, 19-55=-330/669, 19-55=-330/669, 19-54=-330/669, 5-415=-310/8435, 9-21=-207/1032, 11-20=-791/311, 11-18=-43/423, 12-18=-71/306, 13-18=-619/183, 13-17=-285/132, 3-26=-348/95, 3-25=-319/1109, 14-17=-433/1368 NOTES. (13-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16, FH=20.0 psf (cord L1: Lum DOL=1.15 Piate DOL=1.60) psf (Lum DOL=1.15 Piate DOL=1.15); Is=1.0; Rough (arevice)eg able end 20-end enginet exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16, FH=20.0 psf (roof L1: Lum DOL=1.15 Piate DOL=1.15); H=20.0 psf nuble and f12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with ather live load5. 2) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bothom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fin between the bottom chord and any other members. Continued on page 2 War					be insta	illed during truss erectio	n, in accordance with Stabilizer	
Max Horz 2-95(LC 62) Max Grav 2-732(LC 39), 16-986(LC 45), 21=332(LC 39) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2-38-857/105, 3-48-1705/458, 4-31=-6572(5), 3-25-52/192, 5-32=-442/204, 5-33=-387/200, 6-34=-387/200, 6-34=-387/200, 6-34=-387/200, 6-35=-342/1129, 35-36=-342/1129, 	REACTIONS. (lb/size)	2=529/0-3-8 (min. 0-1-8),	16=952/0-3-8 (min. 0-1-8), 21=2743	3/0-3-8 (min. 0-2-4	.)	Jon galaol		
Max Grav2=732(LC 30), 16=966(LC 45), 21=3332(LC 38) FORCES. (ib) - Max. Comp./Max. Ten All forces 250 (ib) or less except when shown. TOP CHORD 2:3==857/195, 3:4=:1705/458, 4:31==587/206, 3:1:32=:552/192, 5:32=:442/204, 5:33=:387/200, 3:34=:387/200, 5:34=:387/200, 5:35=:342/1129, 3:5:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/1129, 7:36=:342/129, 7:31=4:1=:509/273, 1:14:1=:509/273, 1:14:1=:509/273, 1:14:1=:509/273, 1:14:1=:509/273, 1:14:1=:509/273, 1:14:2=:95/405, 1:2:43=:995/405, 1:2:44=:1163/461, 1:3:44=:1212/452, 1:3:14::148/486, 1:4:16:917/285 BOT CHORD 2:26=:174/616, 2:52:6=:63/274, 4:25:=104/481, 2:5:45=:4369/1713, 4:54=:-448/1713, 2:4=:2:460/1840, 6:2:4=::260/172, 0:53:=:300/869, 1:8:55=:330/869, 1:8:55=:330/869, 1:8:56=::330/869, 1:8:56=::330/869, 1:8:56=::330/869, 1:8:56=::330/869, 1:2:42=::143/357, 8:22=::2:44/1153, 3:41:2:2:-78111, 1:11:8:=:43/423, 1:2:2:-781111, 1:11:8:=:43/423, 1:2:2:-781111, 1:11:8:=:43/423, 1:2:2:-781111, 1:11:8:=:43/423, 1:2:2:-7813/3388 NOTES: (13-17) 1) Uhbalanced roof live loads have been considered for this design. 2) Wind: A:SC 7-16; VILI=120mph (3:second gust) Vasd=95mph; TCDL=5.0psf, B:CDL=5.0psf, h=231; Cat. II; Exp B; Enclosed; MWFRS, (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.16 plate grip DOL=1.60 3) TCLL: ASC 7-16; VILI=120mph (3:second gust) Vasd=95mph; TCDL=5.0psf, B:CDL=5.0psf, h=231; Cat. II; Exp B; Enclosed; MWFRS, (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.15, Pl=20.0 psf (cut II) DUL=1.15; Pl=20.0 psf (cut II) DUL=1.0; Pl=20.0 psf (cut III) Pl=20.0 psf (cut III) Pl=20.0 psf (cut III) Pl=20.0 psf (cut II	Max Horz Max Unlift	2=-95(LC 62) 2=-143(LC 12) 16=-284(L)	$(13) 21 = -1174(1 \times 9)$					
FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2:3-8:57(195, 3-4:-1705/458, 4:31=-587/206, 3:1-32=-552/192, 5:32=-442/204, 5:33=-587/200, 3:3-4=-387/200, 6:3-6=-367/21(129, 3:5-38=-342/1129, 3:3-38=-471/1443, 3:39=-471/1443, 3:45=-489/1713, 1:4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:1-4=-509/273, 1:2-2=-319/170, 1:1-8=-30/4823, 1:2-18=-701/032, 1:1-20=-791/311, 1:1-8=-43/423, 1:2-18=-710/612, 1:2-20-619/163, 1:1-20-20-791/032, 1:1-20=-791/311, 1:1-8=-43/423, 1:2-18=-710/612, 1:1-10, 1:1-18=-100-1=-1.50, 1:1-60, 1:1-	Max Grav	2=732(LC 39), 16=986(LC	45), 21=3332(LC 38)					
TOP CHORD 2-3=87/195, 3-4=-1705/488, 4-31=-867/206, 31-52=-552/192, 5-32=-442/204, 5-33=-387/200, 33-34=-387/200, 6-34=-387/200, 6-35=-342/1129, 5-33=-387/200, 33-34=-387/200, 6-35=-342/1129, 7-36=-342/1129, 7=8-349/1141, 8-31=-867/206, 31-52=-552/192, 5-32=-442/204, 5-33=-387/200, 33-34=-387/200, 6-34=-387/206, 31-52=-552/192, 3-32=-442/204, 5-33=-387/200, 33-34=-387/200, 6-34=-387/206, 31-52=-502/73, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-41=-509/273, 11-42=-60/274, 42-33=-95/405, 12-43=-95/405, 12-43=-95/405, 12-43=-95/405, 12-43=-95/405, 12-43=-95/405, 12-45=-480/1713, 45-46=-480/1713, 24-46=-489/1714, 7-23=-260/73, 20-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-54=-330/869, 19-54=-330/869, 19-53=-330/869, 19-54=-330/869, 19-54=-330/869, 19-53=-330/869, 19-54=-330/869, 19-53=-330/869, 19-54=-330/869, 13-55=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-54=-330/869, 19-54=-330/869, 19-53=-330/869, 19-53=-330/869, 19-53=-330/869, 19-54=-30/862, 19-54=-36=-36=-36=-36=-36=-36=-36=-36=-36=-36	FORCES (Ib) Max Co	mp /Max Ten All forces	250 (lb) or less except when shown					
 5-33387/200, 3-34387/200, 6-34387/200, 6-35342/1129, 55-63-342/1129, 57-36342/1129, 57-36342/1129, 57-36342/1143, 38-39471/1443, 9-39471/1443, 13-441212/452, 13-141484/486, 14-16917/255 BOT CHORD 2-26171/4616, 25-2663/274, 4-25104/1418, 25-45489/1713, 45-46489/1713, 24-47-439/30869, 9-45-330/869, 9-45330/869, 6-24266/880, 6-23130/482, 21-23483/537, 8-23247/152, 8-244715, 8-21-1718/43, 33-17285/132, 3-26348/95, 3-25319/1109, 14-17433/1368 NOTES- (13-17) 1 Unbalanced roof live loads have been considered for this design. 1) Whit :ASCE 7-16; Pr=20.0 psf (root LL: Lum DOL=-1.15; Pf=20.0 psf (Lum DOL=-1.15; Is=-1.0; Routh Carbon chord in a sequence of 100 ps for 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with the live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with the live load of 30.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by	TOP CHORD 2-3=-857	/195, 3-4=-1705/458, 4-31	=-587/206, 31-32=-552/192, 5-32=-4	442/204,				
 1-36=-342/1128, 1-6=-349/1141, 6=-37=47/1143, 37=36=-47/11443, 35=36=-47/11443, 93=36=-47/11443, 94=509/273, 11=41=-509/273, 11=-619/183, 13=-7=285/132, 3=26=-348/95, 3=25=-319/1109, 11=-7=-385/366, 6=23=-368/95, 3=25=-319/1109, 11=-7=-208/722, 9=20=-270/1032, 11=20=-791/311, 11=18=-34/23, 12=, 12=-71/306, 13=-619/183, 13=-7=-285/132, 3=26=-348/95, 3=25=-319/1109, 11=7=-385/1368 NOTES- (13-17)	5-33=-38	37/200, 33-34=-387/200, 6-	34=-387/200, 6-35=-342/1129, 35-3	6=-342/1129,				
 11-42=-995/405, 12-43=-995/405, 12-44=-1163/461, 13-44=-1212/452, 13-14=-1484/486, 14-16=-917/285 BOT CHORD 2:26=-174/616, 25-26=-63/274, 4-25=-104/481, 25-45=-489/1713, 45-46=-489/1713, 24-46=-489/1714, 7-23=-260/73, 20-53=-330/869, 19-53=-330/869, 19-54=-330/869, 18-55=-330/869, 19-23=-244/715, 8-21=-1018/435, 9-21=-2048/722, 9-20=-270/1032, 11-20=-791/311, 11-18=-43/423, 12-18=-71/306, 13-18==-619/183, 13-17=-285/132, 3-26=-348/95, 3-25=-319/1109, 14-17=-433/1368 NOTES- (13-17)	9-39=-47	/1/1443, 9-40=-509/273, 10	-40=-509/273, 10-41=-509/273, 11-	41=-509/273,				
BOT CHORD 2.26E-174/616, 25:26E-63/274, 42:5=-104/481, 25:45=-489/1713, 45:46=-489/1713, 24:46=-489/1714, 7:23=-260/73, 20:53=-330/869, 19:53=-330/869, 19:54=-330/869, 54:55=-330/869, 18:56=-456/1448, 56:57=-456/1448, 75:57=-456/1448, 42:48=-1359/366, 6:23=-1300/492, 21:23=-1483/537, 8:21=-1018/435, 9:21=-2048/722, 9:20=-270/1032, 11:20=-791/311, 11:18=-43/423, 12:18=-71/306, 13:18=-619/183, 13:17=-285/132, 3:26=-348/95, 3:25=-319/1109, 14:17=-433/1368 NOTES- (13:17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS, 14:17=-433/1368 NOTES- (13:17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS, Carelog gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Vr=2:0, psf (roof LL: Lum DOL=1.15); Pf=2:0.0 psf (Lum DOL=1.15); Is=1.0; Rough (3) TCLL: ASCE 7-16; Vr=2:0, psf (roof LL: Lum DOL=1.15); Pf=2:0.0 psf (Lum DOL=1.15); Is=1.0; Rough (4) Unbalanced sonw loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12:0 psf or 2:00 times flat roof load of 2:0.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 1:0.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Varning I=-Verify design parameters and read notes before use. This design is based only unon parameters shown, and is for an individual building component to be installed and loaded	11-42=-9	95/405, 42-43=-995/405, 1	2-43=-995/405, 12-44=-1163/461, 1	3-44=-1212/452,				
 2446=-489/1714, 7-23=-260/73, 20-53=-330/869, 19-53=-330/869, 19-54=-330/869, 54-55=-330/869, 54-55=-330/869, 18-56=-456/1448, 56-57=-456/1448, 17-57=-456/148, 17-57=-456/14	BOT CHORD 2-26=-17	484/486, 14-16=-917/285 4/616, 25-26=-63/274, 4-2	5=-104/481, 25-45=-489/1713, 45-4	6=-489/1713.				
 S4-55=-330/869, 18-55=-330/869, 18-56=-456/1448, 17-57=-456/1448 WEBS 4-24=-1359/366, 6-24=-266/880, 6-23=-1360/492, 21-23=-1483/537, 8-23=-244/715, 8-21=-1018/435, 9-21=-2048/722, 9-20=-270/1032, 11-20=-791/311, 11-18=-43/423, 12-18=-71/306, 13-18=-619/183, 13-17=-285/132, 3-26=-348/95, 3-25=-319/1109, 14-17=-433/1368 NOTES- (13-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Vrut=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.50 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Vrut=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.150; Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rotgh (at B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any ot	24-46=-4	89/1714, 7-23=-260/73, 20	-53=-330/869, 19-53=-330/869, 19-	54=-330/869,				
 NCLOS 8-21 20040722, 9-202070/1032, 11-20701/31, 11-18=-43/423, 12-18=-71/306, 13-18=-619/183, 13-17=-285/132, 3-26=-348/95, 3-25=-319/1109, 14-17=-433/1368 NOTES- (13-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 3) TCLL: ASCE 7-16; Vrlt=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough (3-second gust) Vasd=95mph; TCDL=5.0psf; b=23f; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough (3-second gust) Vasd=95mph; TCDL=5.0psf; b=20.0 psf (Lum DOL=1.15); Is=1.0; Rough (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; BC	54-55=-3	330/869, 18-55=-330/869, 1 59/366 6-24=-266/880 6-	8-56=-456/1448, 56-57=-456/1448, 23=-1360/492, 21-23=-1483/537, 8-	17-57=-456/1448 23=-244/715				
 12-18=-71/306, 13-18=-619/183, 13-17=-285/132, 3-26=-348/95, 3-25=-319/1109, 14-17=-433/1368 NOTES- (13-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vull=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; PT=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough (Lat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning !Verify design parameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded 	8-21=-10)18/435, 9-21=-2048/722, 9	-20=-270/1032, 11-20=-791/311, 11	-18=-43/423,				
 NOTES- (13-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning I—Verify design parameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded 	12-18=-7	/1/306, 13-18=-619/183, 13	-17=-285/132, 3-26=-348/95, 3-25=	-319/1109,				
 NOTES- (13-17) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS, (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15); Is=1.0; Roth Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning I—Verify design parameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded 	14-17=-4	33/1308						
 Unbalanced root live loads have been considered for this design. Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 Unbalanced snow loads have been considered for this design. This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. Provide adequate drainage to prevent water ponding. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning I—Verify design parameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded 	NOTES- (13-17)		·				WHITH CARCING	
 (envelope) gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.61, plate grip DOL=1.61, plate DOL=1.15); Is=1.0; Rough (at B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 (b) Unbalanced snow loads have been considered for this design. (c) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs (c) Provide adequate drainage to prevent water ponding. (c) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. (c) Provide adequate drainage to prevent water ponding. (c) This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. (c) Continued on page 2 	 Unbalanced roof live ic Wind: ASCE 7-16: Vult 	bads have been considered t=120mph (3-second quist)	for this design. Vasd=95mph [:] TCDI =5 0psf: BCDI	=5 0nsf: h=23ft: Ca	at II: Exn F	3. Enclosed: MWERS	Second N'II	
 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough (astronomy constraints); Ca=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning !Verify design narameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded 	(envelope) gable end z	zone; end vertical right exp	osed; Lumber DOL=1.60 plate grip [DOL=1.60	и. п, шкр с		A OF LOS PAGE A IT	
 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a live load of 30.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning I—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded 	3) TCLL: ASCE 7-16; Pr=	20.0 psf (roof LL: Lum DO	_=1.15 Plate DOL=1.15); Pf=20.0 p	sf (Lum DOL=1.15	Plate DOL	_=1.15); ls=1.0; Rough	EAL T	
 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a live load of 30.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 Warning I-Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded. 	4) Unbalanced snow load	ls have been considered fo	r this design.				28147	
non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit work with the work will fit work with the bottom chord and any other members. Continued on page 2 Warning IVerify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded	5) This truss has been de	signed for greater of min r	oof live load of 12.0 psf or 2.00 time	s flat roof load of 20	0.0 psf on	overhangs	1 1 5	
 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members. Continued on page 2 4/25/2025 Warning - Verify design parameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded. 	non-concurrent with ot	her live loads. hage to prevent water non-	ina			Ame	Now of I	
 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 6) * Continued on page 2 6) * Continued on page 2 6) * Continued on page 2 7) * Continued on page 2 8) * Continued on page 2 8)	7) This truss has been de	signed for a 10.0 psf botto	m chord live load nonconcurrent wit	h any other live loa	ıds.	and the second se	ARE	
Continued on page 2 <u>4/25/2025</u> Warning !	8) * This truss has been of	designed for a live load of 3	0.0psf on the bottom chord in all are	eas where a rectan	igle 3-6-0 t	all by 1-0-0 wide will fit	Minner K. MORNIN	
4/23/2023 Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown and is for an individual building component to be installed and loaded	Continued on page 2	iora ana any outer member	J.				1/25/2025	
	Warning !	parameters and read notes b	efore use. This design is based only upon	parameters shown an	nd is for an i	ndividual building compone	$\tau/2J/202J$	

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY MEADOW LANE ANGIER, I	٩C
25-3336-R01	R01	Hip Girder	1	1	Job Reference (optional) # 58877	
		Run: 8 ID:GH	.630 s Jul OhT5MOv	2 2024 Pri	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Apr 26 17:18:12 2025 Page X2c9QzXMNI-SkRbCdfxYBghH1Gy2ueiwj NMmbET4 EekNnl?zM	32 2∂ 3B1

NOTES-(13-17)

- 9) Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=143, 16=284, 21=1174.
- 11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 Trusses designed with 2018 IRC also comply with 2015 IRC.
- 14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

- Vert: 1-5=-60, 5-12=-60, 12-13=-60, 13-14=-60, 14-15=-60, 26-27=-20, 23-25=-20, 16-22=-20
- Concentrated Loads (lb) Vert: 5=-21(B) 12=-40(B) 13=-41(B) 23=-42(B) 7=-21(B) 24=-42(B) 8=-40(B) 18=-23(B) 21=-23(B) 31=-41(B) 32=-8(B) 33=-21(B) 34=-21(B) 35=-21(B) 37=-40(B) 37=-39=-40(B) 40=-40(B) 41=-40(B) 42=-40(B) 43=-40(B) 44=-10(B) 45=-34(B) 46=-58(B) 47=-42(B) 48=-42(B) 49=-42(B) 50=-23(B) 51=-23(B) 52=-23(B) 53=-23(B) 53=-23 54=-23(B) 55=-23(B) 56=-59(B) 57=-86(B)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHEL	BY MEADOW LANE ANGIER, NC
25-3336-R01	R02	Нір	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GH	3.630 s Jul OhT5MOv	12 2024 Pri 4FkLKIPf>	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. K2c9QzXMNI-ww?zPzfZJVyYvBr8cc9x1	Sat Apr 26 17:18:13 2025 Page 2 TXXWkAuVCTwNtO6LIRzMoBe

NOTES- (12-16)

- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Trusses designed with 2018 IRC also comply with 2015 IRC.
- 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SH	ELBY MEADOW LANE ANGIER, NC
25-3336-R01	R03	Hip	1	1	Job Reference (optional)	# 58877
		F ID:	un: 8.630 s Jul GHOhT5MOv	12 2024 Pri 4FkLKIPfX	nt: 8.630 s Jul 12 2024 MiTek Industries, In 2c9QzXMNI-O6ZLdJqB4p4PWLQLA	c. Sat Apr 26 17:18:14 2025 Page 2 JqA083jfaDVxwNX52sugtzMoBd

NOTES- (11-15)

- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

11) Trusses designed with 2018 IRC also comply with 2015 IRC.

- 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY MEADOW LA	ANE ANGIER, NC
25-3336-R01	R04	Нір	1	1	Job Reference (optional) # 5	8877
		Run:	3.630 s Jul D:ydjkN6vi	12 2024 Pri IydZf_Qy5	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Apr 26 17:18 5ICaT4RzWPfe-sJ6jqfhpr6CG8V?Xj1BPYMcvH_YfgPf	8:15 2025 Page 2 gKibRMKzMoBc

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

(4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NC
25-3336-R01	R05	Piggyback Base	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:	3.630 s Jul ydjkN6vuy	12 2024 Pri dZf_Qy5l0	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. S CaT4RzWPfe-LVg51?iScQK7mfajHkie5Z	at Apr 26 17:18:16 2025 Page 2 940NuuPsvqZML?umzMoBb

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE ANGIER	₹, NC
25-3336-R01	R06	Piggyback Base	1	1	Job Reference (optional)	# 58877	
	·	Run: 8 ID:y	.630 s Jul ydjkN6vuy	i 2 2024 Pri dZf_Qy5l0	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat CaT4RzWPfe-phEUFLj4NkS_Np9wrSEtdnł	: Apr 26 17:18:17 2025 Pa hCrnDi8Hyzn04YRCzM	ige 2 ⁄loBa

NOTES- (10-14)

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Trusses designed with 2018 IRC also comply with 2015 IRC.

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY M	EADOW LANE	ANGIER, NC
25-3336-R01	R07	Piggyback Base	1	1	Job Reference (optional)	# 5882	77
		Run: 8	.630 s Jul ID:ydjkN	12 2024 Pri 6vuydZf_(nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat A Qy5ICaT4RzWPfe-HuosSgji81ar?yk6P9I6A_	Apr 26 17:18:18 2 EMJBZitjd70g	2025 Page 2 Jq6zfzMoBZ

NOTES- (10-14)

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Trusses designed with 2018 IRC also comply with 2015 IRC.

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY MEADOW LANE AND	GIER, NC
25-3336-R01	R08	Piggyback Base	1	1	Job Reference (optional) # 58877	7
		Run: 8	.630 s Jul ID:ydjkN6	12 2024 Pri vuydZf_C	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Apr 26 17:18:19 2029 0y5ICaT4RzWPfe-I4MEg0kKuLjid6IIytGLiCnXtbvCcA3GFKZfV	5 Page 2 /5zMoBY

NOTES- (10-14)

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Trusses designed with 2018 IRC also comply with 2015 IRC.

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY M	IEADOW LANE	ANGIER, NC
25-3336-R01	R09	Piggyback Base	1	1	Job Reference (optional)	# 5882	77
		Run: 8 ID:	.630 s Jul ydjkN6vu	12 2024 Pri /dZf_Qy5l	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat CaT4RzWPfe-DGwctMlyffrZEGtVWanaFPJ	Apr 26 17:18:20 2 j1?FLLe1PU_J	2025 Page 2 C2XzMoBX

NOTES- (10-14)

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Trusses designed with 2018 IRC also comply with 2015 IRC.

- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCU	JTT HILLS 56 SHE	ELBY MEADO	W LANE ANGIER, NC
25-3336-R01	R10	Roof Special	1	1			#	58877
			Rup: 8.630 s. Jul	12 2024 Prir	Job Reference (opti	onal) MiTek Industries Ind	n Sat Apr 26	17:18:21 2025 Page 1
_			ID:GHOhT5MOv4	FkLKIPfX2	c9QzXMNI-hTU_5in	naQyzQsQSh4HI	podsymOaN	I45tZid2ma_zMoBW
-0- 0-'	10-8 8-1-15 10-8 8-1-15	8-1-1 1-3-0	23-9-0	+	30-10-4 7-1-4	<u>37-0-0</u> 6-1-12	37-10-8 0-10-8	
-						• • •		
		5x8 =						Scale = 1:79.7
				5v9 —				
Ŧ		8.00 12 4 5	x6 =	5.00 -				
ΙŢ			264	6 ⊒n∖				т
			21 V/6					
	578	25 72			\₹5			
	0.00	4x8		11	27 5x	8 📎		
9		3	W4	11	7			
-11-4	//			11	<u>A</u>	<		-9-(
12		W2	1	MA		\square		2
	I			vie		46 42	x6 📎	
	. //	vv1 //		$> \parallel$	^{W9} W10		8	
Q1			x B3 x			V		φ
ģ	B1		<u></u>			-B4_W11		φ Ο
	28	20 29 19 18 17 3 4	14 ^{31 35}	13 3	²² 12 33 11		10	
	4x6 =	2x4 4x8 = 4x4 =	2x4	4x8 =	3x8 = 5x5 =	= 3	x4 =	
		2x4 =	2x4	2x4 =				
	0.4.45	15.0.0						
	8-1-15	<u> </u>	$\frac{0}{0}$ + $\frac{25-0-0}{5-0-0}$		30-10-4 5-10-4	<u> </u>		
Plate Offsets (X,Y) [2:0	-0-0,0-0-10], [3:0-4-0,0-3-0	, [6:0-5-12,0-2-0], [7:0-4-0,0-3-0], [8	3:0-3-0,0-1-12], [10	:Edge,0-1	-8]			
LOADING (psf)	SPACING-	2-0-0 CSI	DEEL	in (lo	c) l/defl l/d	PL	ATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL	1.15 TC 0.63	Vert(LL)	-0.67 16-	17 >660 240	MT	20	244/190
TCDL 10.0	Lumber DOL	1.15 BC 1.00	Vert(CT)	-1.02 16-	17 >434 180			
BCLL 0.0 *	Code IRC2021/TP	I2014 Matrix-AS		0.09	10 11/a 11/a	We	eight: 250 lb	FT = 20%
BCDL 10.0							0	
TOP CHORD 2x4 SP No	2		TOP CHORD	Structura	al wood sheathing	directly applied	excent en	d verticals
BOT CHORD 2x4 SP No	.2 *Except*		BOT CHORD	Rigid cei	ling directly applie	d. Except:	oxcopt on	
B3: 2x4 SF	P No.1, B2: 2x4 SP SS		WERE	3-1-0 oc	bracing: 15-17	10 12 01		
WEDGE 2X4 SP NO			JOINTS	1 Brace	at Jt(s): 21	-10, 13-21		
Left: 2x4 SP No.3				MiTek	recommends that \$	Stabilizers and r	required cro	oss bracing
				be insta	alled during truss e	erection, in acco	rdance with	n Stabilizer
REACTIONS. (lb/size)	2=1613/0-3-8 (min. 0-2-4),	10=1636/0-3-8 (min. 0-2-2)		Installa	lion guide.			
Max Horz	2=240(LC 11)							
Max Uplift Max Grav	2=-67(LC 12), 10=-92(LC 1 2=1913(LC 20)_10=1810(L	3) C 3)						
	2-1010(2020), 10-1010(2	0.07						
FORCES. (Ib) - Max. Con	mp./Max. Ten All forces 2	250 (lb) or less except when shown.	- 2525/205					
6-26=-25	35/305, 6-27=-2046/159, 7	-27=-2164/162, 4-55196/404, 5-26	2535/305, 0=-1733/122					
BOT CHORD 2-28=-10	3/2431, 20-28=-103/2431,	20-29=-103/2428, 19-29=-103/2428	3, 18-19=-103/2428	3,				
18-30=0/ 12-33=-2	1831, 14-30=0/1831, 14-31 4/1734 11-33=-24/1734	=0/1831, 31-32=0/1831, 13-32=0/1	831, 12-13=-24/17	34,				
WEBS 3-20=0/3	13, 3-18=-694/221, 17-18=	-35/913, 4-17=0/1121, 4-21=-385/1	973, 13-15=-352/1	05,				
6-13=0/6	655, 7-11=-289/46, 8-11=0/	1642, 5-21=-1755/263, 6-21=-126/9	926					
NOTES- (10-14)								
1) Unbalanced roof live lo	ads have been considered	for this design.						
2) Wind: ASCE 7-16; Vult (envelope) gable end z	=120mph (3-second gust) rone and C-C Exterior(2E) -	Vasd=95mph; TCDL=5.0pst; BCDL 0-10-8 to 3-11-2_Interior(1) 3-11-2 t	=5.0pst; h=23tt; Ca to 11-5-6_Exterior(at. II; Exp 2R) 11-5-	B; Enclosed; MVVF 6 to 16-3-0	RS		
Exterior(2E) 16-3-0 to	17-6-0, Interior(1) 17-6-0 to	18-11-6, Exterior(2R) 18-11-6 to 28	8-6-10, Interior(1) 2	8-6-10 to	33-0-14, Exterior(2	?E)	unin.	
33-0-14 to 37-10-8 zon	e; end vertical right expose	d;C-C for members and forces & M	WFRS for reaction	is shown;	Lumber DOL=1.60	KINGTH	CARO	11.
3) TCLL: ASCE 7-16; Pr=	20.0 psf (roof LL: Lum DOI	.=1.15 Plate DOL=1.15); Pf=20.0 ps	sf (Lum DOL=1.15	Plate DO	L=1.15); ls=1.0; Ro	and a douc	FSBIA	Ville
Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10								
4) I his truss has been de non-concurrent with off	signed for greater of min ro	ior rive road of 12.0 pst or 2.00 times	s hat roof load of 2	u.u pst on	overnangs	El .	FAL	
5) Provide adequate drair	hage to prevent water pond	ing.				11 21	8147	
6) This truss has been de	signed for a 10.0 psf botto	n chord live load nonconcurrent with	h any other live loa	ids.	tall by 1 0 0 wide ·	AVIE fit		1 5
between the bottom ch	ord and any other member	s, with BCDL = $10.0psf$.	cas where a rectan	iyie 3-0-0	tan by 1-0-0 wide \	A SNI	WEER	- Martin
8) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of withst	anding 100 lb uplift	at joint(s)	2, 10.	ARE	APA	CHINE .
9) This truss design requi sheetrock be applied d	9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum							
Continued on page 2						11	25/2024	5
Warning !—Verify design	parameters and read notes h	efore use. This design is based only upon	parameters shown a	nd is for an	individual building co	4/	$\Delta J/\Delta U \Delta z$	v aded

vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY M	EADOW LANE ANGIEF	R, NC
25-3336-R01	R10	Roof Special	1	1	Job Reference (optional)	# 58877	
		Rur ID:GH	8.630 s Jul 0hT5MOv4	12 2024 Pri FkLKIPfX	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat A 2c9QzXMNI-hTU 5imaQyzQsQSh4HIpodsyr	Apr 26 17:18:21 2025 Pa mOaN45tZid2ma zM	age 2 IoBW

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT H	ILLS 56 SHELBY MEA	DOW LANE ANGIER, NC		
25-3336-R01	R11	Roof Special	1	1	lah Deference (entional)		# 58877		
			Run: 8.630 s Jul	12 2024 Pri	nt: 8.630 s Jul 12 2024 MiTek	Industries, Inc. Sat Apr	26 17:18:21 2025 Page 1		
-Q-	10 _r 8 8-1-15	16-3-0 20	1D:GHOn 15MOV4)-6-0 1 26	IFKLKIPTX2 -9-0	2c9QZXMNI-n1U_5imaQy2	ZQSQSn4HipodsxZO 37-0-0 37/-10-8	cL44DZId2ma_zIMoBW		
0-	10-8 8-1-15	8-1-1 4	-3-0 6-	3-0	4-11-12	5-3-4 0-10-8			
		5x8 =					Scale = 1:79.7		
1		8.00 12 4							
			√3 5x6 =		5x8 =				
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	5x8 ·	24	25	4			[
			W4		2x4	Ш			
1-4-(W3	A ANG		7				
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	,	W1		<u> //</u>	ew /	8	_		
ୁମ ମୁୀ	EHW1 B1		B3		15184	W10	0 20 		
1 1 91	26	27 19 18 17 28	29 34	15 13 ³	¹⁰ 31 12 32				
		4x8 = 4x4 =	14 2×4 II	6x12 =	$3x8 = \frac{11}{6x8} = \frac{11}{11}$	10 3×1 II			
	4x6 =	3x6 =	2x4 2x4	2x4 =	0x0 —	574			
	<u>8-1-15</u> 8-1-15	<u> </u>	0 <u>25-0-0</u> 5-0-0		31-8-12 6-8-12	37-0-0 5-3-4			
Plate Offsets (X,Y) [2:0	-0-0,0-0-6], [3:0-4-0,0-3-0],	[6:0-5-12,0-2-0], [8:0-1-4,0-1-8]	1						
LOADING (psf)	SPACING-	2-0-0 CSI .	DEFL.	in (le	oc) l/defl L/d	PLATES	GRIP		
Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.64 1.15 BC 0.87	Vert(LL) Vert(CT)	-0.64 16-	17 >687 240 17 >430 180	MT20	244/190		
TCDL 10.0 BCII 0.0 *	Rep Stress Incr	YES WB 0.99	Horz(CT)	0.09	10 n/a n/a				
BCDL 10.0	Code IRC2021/TP	2014 Matrix-AS				Weight: 250) lb FT = 20%		
LUMBER-	. 0		BRACING-	Structur	al wood choothing direc	the applied avaant	andvarticals		
BOT CHORD 2x4 SP No	0.2 *Except*		BOT CHORD	Rigid ce	iling directly applied.	ily applied, except	enu verticais.		
B3,B2: 2x4 WEBS 2x4 SP No	I SP SS 3 *Excent*		WEBS	1 Row a 2 Rows	t midpt 3-18, 4 at 1/3 pts 5-13	4-15, 6-11			
W3,W4: 2	4 SP No.1			MiTek	recommends that Stabi	lizers and required	cross bracing		
WEDGE Left: 2x4 SP No.3				be inst	alled during truss erection	on, in accordance v	vith Stabilizer		
	2-1612/0.3.8 (min 0.2.4)	10-1633/0.3.8 (min 0.2.3)		motulie	alon guide.]		
Max Horz	2=1012/0-3-8 (mm. 0-2-4), 2=240(LC 11)	10-1033/0-3-8 (11111. 0-2-3)							
Max Uplift Max Grav	2=-68(LC 12), 10=-93(LC 1 2=1929(LC 20), 10=1848(L	3) C 3)							
	z=1929(E0 20), 10=10+0(E								
TOP CHORD 2-3=-288	mp./Max. Ten All forces 2 4/119. 3-24=-2306/129. 4-2	/50 (lb) or less except when shown. 24=-2190/156. 4-5=-3924/426. 5-25:	=-1971/149.						
6-25=-19	71/149, 6-7=-2143/234, 7-8	=-2158/123, 8-10=-1794/123	10 10- 105/045	,					
18-28=0/	1439, 14-28=0/1439, 14-29	=0/1439, 29-30=0/1439, 13-30=0/1	439, 13-31=0/180 [°]	, 1,					
12-31=0/ 16-34=-1	1801, 12-32=0/1801, 11-32 00/411 15-34=-100/411	=0/1801, 17-33=-100/411, 16-33=-1	100/411,						
WEBS 3-20=0/3	20, 3-18=-699/223, 17-18=	-40/933, 4-17=-1/1111, 4-15=-394/2	2491,						
13-15=-4	83/2656, 5-13=-2657/370,	5-13=0/990, 7-11=-311/180, 8-11=0	/1692						
NOTES- (10-14)	ada hava haan aanaidarad	for this design							
2) Wind: ASCE 7-16; Vul	=120mph (3-second gust)	/asd=95mph; TCDL=5.0psf; BCDL:	=5.0psf; h=23ft; Ca	at. II; Exp	B; Enclosed; MWFRS				
(envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 11-5-6, Exterior(2R) 11-5-6 to 16-3-0, Exterior(2E) 16 3 0 to 20 6 0, Interior(1) 20 6 0 to 21 11 6, Exterior(2R) 21 11 6 to 21 8 12, Interior(2R) 11-5-6 to 16-3-0,									
33-0-14 to 37-10-8 zone; end vertical right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60									
a) TCLL: ASCE 7-16: Pr=	20.0 psf (roof LL: Lum DOL	.=1.15 Plate DOL=1.15): Pf=20.0 ps	sf (Lum DOL=1.15	Plate DO	L=1.15): Is=1.0: Rough	OFESSION	NATT		
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10	of live load of 12.0 pof == 0.00 firm	flot roof load of 0	0.0	overbange	1º 1	E. I		
High russ has been de non-concurrent with ot	ner live loads.	or rive load of 12.0 pst of 2.00 times	s hat foot load of 2	u.u pst or	overnangs	SEAL			
5) Provide adequate drain	hage to prevent water pond	ng. n chord live load nonconcurrent with	h any other live los	ads	11HW	20147			
7) * This truss has been of	lesigned for a live load of 3	0.0psf on the bottom chord in all are	eas where a rectar	igle 3-6-0	tall by 1-0-0 wide will the	A NOINEER	10 1		
between the bottom ch 8) Provide mechanical co	between the bottom chord and any other members, with BCDL = 10.0psf. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10								
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum									
Continger ପ୍ରତିକାହାରୀ ସ	nectly to the pottom chord.					4/25/20	25		
Warning !—Verify design	narameters and read notes h	fore use. This design is based only upon	narameters shown a	nd is for an	individual building comport	ent to be installed and	loaded		

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE ANGIER, NC
25-3336-R01	R11	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: ID:GH0	8.630 s Jul DhT5MOv4	12 2024 Pri FkLKIPfX2	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sa 2c9QzXMNI-hTU 5imaQyzQsQSh4HIpods	it Apr 26 17:18:21 2025 Page 2 sxZOcL44DZid2ma_zMoBW

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard


Job	Truss	Truss Type	Qty	Ply LOT 0.0048 H	IONEYCUTT HILLS 56 S	HELBY MEADOW I	ANE ANGIER, NC
25-3336-R01	R12	Roof Special	1	1		# 4	58877
			Run: 8.630 s Jul	Job Referer 12 2024 Print: 8.630 s Jul	nce (optional) 12 2024 MiTek Industries,	Inc. Sat Apr 26 17:	18:22 2025 Page 1
-Q-	-10 _r -8 8-1-15	16-3-0	ID:GHOhT5MOv 23-6-0	4FkLKIPfX2c9QzXMNI 29-9-0	-9f2NI2mCBG5HUa1te 37-0-0	e?p2KqP3goyNpb 37 ₁ -10 ₋ 8	₁TixHoJ6QzMoB\
0-	10-8 8-1-15	8-1-1	7-3-0	6-3-0	7-3-0	0-10-8	
		5x8 =					Scale = 1:80.3
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	5.0	23 72	T3 24	F 0 7	v8 —		
	500			5x6 = '			
4-0		3 W3		25		Ī	
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0-9-	TT	₩2		W46	75	6x6 =	0-0-
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ୁ ମ ମୁ 1	EHW1 B1		B3 VB5				
	27	10 28 18 17 16 29	12 30 33	12 34 11 1	0		
	1/6 —	4x8 = 4x4 =	13 2x4	7x8 = 5x	0 5 =	9 3x6 =	
	4x0 —	2x4 =	2x4	2x4 = 3x8 =			
	9 1 15	15.0.0 20.0.0	25.0.0	20.0.0	27.0.0		
Plata Offacta (X V) [2:0	<u>8-1-15</u>	6-10-1 5-0-0	5-0-0	4-9-0	7-3-0		
LOADING (psf)	-0-0,0-0-10], [3:0-4-0,0-3-0]	, [0.0-0-4,0-2-4], [9.Edge,0-1-6]					
TCLL (roof) 20.0	Plate Grip DOL	2-0-0 CSI. 1.15 TC 0.88	DEFL. Vert(LL)	In (loc) I/defl -0.67 15-16 >658	L/d F 240 N	PLATES C MT20 2	3RIP 244/190
TCDL 10.0	Lumber DOL Rep Stress Incr	1.15 BC 0.89 YES WB 0.74	Vert(CT) Horz(CT)	-1.07 15-16 >415 0.09 9 n/a	180 n/a		
BCLL 0.0 * BCDL 10.0	Code IRC2021/TP	2014 Matrix-AS	11012(01)	0.00 0 11/4	V	Veight: 236 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP No T3: 2x4 SF	2 ^Except^ P SS		BOT CHORD	Rigid ceiling directly	athing directly applie	ed, except end v	/erticals.
BOT CHORD 2x4 SP No B3.B2: 2x4	.2 *Except*		WEBS	1 Row at midpt	3-17, 4-14, 5-1	2 d required eress	braaing
WEBS 2x4 SP No	.3 *Except*			be installed during	truss erection, in ac	cordance with S	stabilizer
WEDGE	F 110.2			Installation guide.			
Left: 2x4 SP No.3							
REACTIONS. (lb/size) 2 Max Horz	2=1613/0-3-8 (min. 0-2-4), 2=240(I C 11)	9=1636/0-3-8 (min. 0-2-2)					
Max Uplift	2=-67(LC 12), 9=-92(LC 13)					
Max Grav.	2-1914(LC 20), 9-1801(LC	, 3)					
FORCES. (lb) - Max. Cor TOP CHORD 2-3=-285	mp./Max. Ten All forces 2 6/134, 3-23=-2281/145, 4-2	/50 (lb) or less except when shown. /3=-2164/171, 4-24=-3759/342, 5-24	1=-3850/315,				
5-25=-25 BOT CHORD 2-27=-10	44/139, 6-25=-2544/139, 6 3/2434 19-27=-103/2434	-26=-2101/132, 7-26=-2207/113, 7-9	9=-1716/138 17-18=-104/243	1			
17-29=0/	1731, 13-29=0/1731, 13-30	=0/1731, 30-31=0/1731, 12-31=0/1	731, 11-12=0/174	1,			
WEBS 3-19=0/3	15, 3-17=-699/222, 16-17=	-37/919, 4-16=0/1127, 4-14=-284/23	336,				
12-14=-3	29/2260, 5-12=-2461/312,	6-12=0/1357, 7-10=0/1590					
NOTES- (10-14)	ads have been considered	for this design					
2) Wind: ASCE 7-16; Vult	=120mph (3-second gust)	/asd=95mph; TCDL=5.0psf; BCDL=	=5.0psf; h=23ft; C	at. II; Exp B; Enclose	d; MWFRS		
(envelope) gable end 2 21-0-10 to 24-11-6, Ext	terior(2R) 24-11-6 to 33-0-1	4, Exterior(2E) 33-0-14 to 37-10-8 z	one; end vertical	right exposed;C-C for	members	minini	
and forces & MWFRS 1 3) TCLL: ASCE 7-16: Pr=	for reactions shown; Lumbe 20.0 psf (roof LL: Lum DOL	er DOL=1.60 plate grip DOL=1.60 .=1.15 Plate DOL=1.15): Pf=20.0 ps	f (Lum DOL=1.15	Plate DOL=1.15): Is-	=1.0: Rough	H LAROLIN	11,
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10	of live lead of 12.0 pcf or 2.00 times	flat roof load of 2	0.0 pef on overhands	in the	DESCIONES	Inent
non-concurrent with oth	ner live loads.			0.0 psi on overhange	in the second	SEAL	
b) Provide adequate drain6) This truss has been de	age to prevent water pond signed for a 10.0 psf bottor	ng. n chord live load nonconcurrent with	n any other live loa	ads.		28147	1114
7) * This truss has been d	lesigned for a live load of 3 ord and any other members	0.0psf on the bottom chord in all are s with BCDL = 10 0psf	eas where a rectar	ngle 3-6-0 tall by 1-0-0	0 wide wiŒfit	1 0/	ung
8) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of withsta	nding 100 lb uplif	t at joint(s) 2, 9.	THE AP	GINEE	MARS .
sheetrock be applied d	irectly to the bottom chord.	suuctural wood sneathing be appli	ed directly to the t	op chord and 1/2" gy	psulli annak	K. Monun	
Continued on page 2					2	4/25/2025	

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	BY MEADOW LANE ANGIER, NC
25-3336-R01	R12	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GH0	.630 s Jul DhT5MOv	12 2024 Pri 4FkLKIPfX	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. 2c9QzXMNI-9f2NI2mCBG5HUa1te?p2k	Sat Apr 26 17:18:22 2025 Page 2 (qP3qoyNpbTixHoJ6QzMoBV

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HIL	LS 56 SHELBY MEADOW LANE ANGIER, NC
25-3336-R01	R13	Roof Special	1	1		# 58877
L			Run: 8.630 s_Jul	12 2024 Pri	1300 Reierence (optional) nt: 8.630 s Jul 12 2024 MiTek li	ndustries, Inc. Sat Apr 26 17:18:23 2025 Page 1
	-Q-10 , 8 8-1-15	16-3-0	ID:GHOhT5M 24-3-0	Ov4FkLKII 26-6-	PfX2c9QzXMNI-drblVOnrya	aD75kc3BiKHt2xHnCJTY0KsAxXteszMoBL 37-0-0 37-10-8
	0-10-8 8-1-15	8-1-1	8-0-0	2-3-0	0 6-3-0	4-3-0 0-10-8
		5x8 =	=			Scale = 1:78.9
		8.00 12 4				
		TR.				
	5	24 T2	25			
			N/5	2x4	11	
4-0		3 W3		5	_5x8 = 5×	x6 =
ŧ,	/				б Т4	27
		W2		//	P 26 F	
0-	It	W1		$\langle \ \rangle$	W9 W9	
4-6	2		W6	WT	, wit	
	P,1 ∰W1 B1_B1		2012			
	o [∕ 🕅 28	17 29 16 15	2x4 14 2x4	₌ 13	12 11	⊠ 10
	Auc —	3x8 = 4x4 =	2x4	6x8 =	= 4x8 = 4x8	= 3x4
	4x0 —					
	o / /=	15.0.0				
	<u>8-1-15</u> 8-1-15	<u> </u>	-0-0 <u>25-0-</u> -0-0 5-0-0		32-9-0 7-9-0	<u> </u>
Plate Offsets (X,Y) [2	2:0-0-0,0-0-10], [3:0-4-0,0-3-	0], [7:0-4-4,0-2-4], [8:0-2-14,0-2-0]				
LOADING (psf)	SPACING-	2-0-0 CSI .	DEFL.	in (lo	oc) l/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.66	Vert(LL)	-0.20	14 >999 240 14 >931 180	MT20 244/190
TCDL 10.0	Rep Stress Incr	YES WB 0.89	Horz(CT)	0.09	10 n/a n/a	
BCDL 10.0	Code IRC2021/T	PI2014 Matrix-AS				Weight: 234 lb FT = 20%
LUMBER-			BRACING-	-		
TOP CHORD 2x4 SP I T3 [.] 2x4	No.2 *Except* SP SS		TOP CHORD	Structura Rigid ce	al wood sheathing directl iling directly applied	y applied, except end verticals.
BOT CHORD 2x4 SP	No.2		WEBS	1 Row a	t midpt 3-15, 4	-13, 6-11
WEBS 2x4 SP I WEDGE	No.3			MiTek	recommends that Stabili	zers and required cross bracing
Left: 2x4 SP No.3				Installa	ition guide.	in, in accordance with Stabilizer
REACTIONS. (lb/size)	2=1526/0-3-8 (min. 0-1-1	3). 10=1536/0-3-8 (min. 0-1-13)				
Max Ho	rz 2=240(LC 11)					
Max Op Max Gra	av 2=1555(LC 20), 10=1536	(LC 1)				
	Comp May Ton All fores	250 (lb) or loss eveent when shown				
TOP CHORD 2-3=-2	248/198, 3-24=-1701/216, 4	-24=-1560/243, 4-25=-2869/454, 5-2	ı. 25=-2999/427,			
5-6=-2	785/271, 6-26=-1349/163, 7	-26=-1349/163, 7-27=-1526/165, 8-2	27=-1663/163,			
BOT CHORD 2-28=-	164/1930, 17-28=-164/1930	, 17-29=-165/1926, 16-29=-165/192	6, 15-16=-165/1926	б,		
14-15=	=0/1199, 13-14=0/1199, 12-1	3=-147/2620, 11-12=-147/2620	1401E			
13-19=	-385/1878, 5-13=-496/268,	6-13=-885/127, 6-11=-1543/121, 7-	11=0/565,			
8-11=-	59/1361					
NOTES- (10-14)						
1) Unbalanced roof live	e loads have been considere	d for this design. Vasd=95mpb; TCDI =5 0psf; BCDI	-5 Opef: h=23ft: C	at II: Evo	B. Enclosed: MW/EPS	
(envelope) gable end	d zone and C-C Exterior(2E)	-0-10-8 to 3-11-2, Interior(1) 3-11-2	to 11-5-6, Exterior(2R) 11-5-	6 to 21-0-10, Interior(1)	
21-0-10 to 27-11-6, I	Exterior(2R) 27-11-6 to 33-0	-14, Exterior(2E) 33-0-14 to 37-10-8	zone; end vertical i	ight expo	sed;C-C for members	ANNAL HILLIGHT
3) TCLL: ASCE 7-16; F	Pr=20.0 psf (roof LL: Lum DC	DL=1.15 Plate DOL=1.15); Pf=20.0 p	osf (Lum DOL=1.15	Plate DO	L=1.15); ls=1.0; Rough	WINGTH CARO
Cat B; Partially Exp.;	; Ce=1.0; Cs=1.00; Ct=1.10	roof live load of 12.0 pcf or 2.00 time	as flat roof load of 2	0 0 pef or	overbangs	OFESSID No"
non-concurrent with	other live loads.			0.0 p31 01	i overnangs	2the Age and
5) Provide adequate dr	ainage to prevent water pon	ding. om chord live load nonconcurrent wi	th any other live loa	de		SEAL
7) * This truss has been	n designed for a live load of	30.0psf on the bottom chord in all a	reas where a rectar	igle 3-6-0	tall by 1-0-0 wide wind fit	28147
8) Provide mechanical	chord and any other member	ers, with BCDL = 10.0psf.	tanding 100 lb unliff	at ioint/c) except (it=lb) 2-111	No. al I
10=142.		iss to bearing plate capable of WILINS	randing 100 lb uplill	a joint(S	/ 6706pt (Jt=ib) 2=111	A VOINEER OS INT
9) This truss design red	quires that a minimum of 7/1	6" structural wood sheathing be app	olied directly to the t	op chord	and 1/2" gypsum	MARK K. MORMUN
Continued on page 2		1.				
						4/25/2025

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NO
25-3336-R01	R13	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:G	.630 s Jul HOhT5M	12 2024 Pri Dv4FkLKI	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. S PfX2c9QzXMNI-drbIVOnryaD75kc3BiKHt	at Apr 26 17:18:23 2025 Page 2 2xHnCJTY0KsAxXteszMoBl

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 S	HELBY MEADOW LANE ANGIER, NC
25-3336-R01	R14	Roof Special Girder	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GI	.630 s Jul HOhT5MC	12 2024 Pri Dv4FkLKIF	nt: 8.630 s Jul 12 2024 MiTek Industries, PfX2c9QzXMNI-6297jkoTjtL_jtBGIQs	Inc. Sat Apr 26 17:18:24 2025 Page 2 sWPFUUXcqvHXI?ObHQBIzMoBT

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

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15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-60, 6-8=-60, 8-10=-60, 10-11=-60, 11-12=-60, 13-21=-20

Concentrated Loads (lb)

Vert: 14=0(F)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILL	S 56 SHELBY MEADOW LA	NE ANGIER, NC
25-3336-R01	R15	Roof Special	1	1	Job Reference (optional)	# 58	8877
		Run: 8 ID:Gł	.630 s Jul HOhT5MC	12 2024 Pri V4FkLKIP	nt: 8.630 s Jul 12 2024 MiTek Ind fX2c9QzXMNI-6297jkoTjtL j	dustries, Inc. Sat Apr 26 17:18 jtBGIQsWPFUXrcdXHV ?	3:24 2025 Page 2 ObHQBIzMoBT

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4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE	ANGIER, NC
25-3336-R01	R16	Common	2	1	Job Reference (optional)	# 588	77
		Run: IE	8.630 s Jul GHOhT5N	12 2024 Pri IOv4FkLK	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sa IPfX2c9QzXMNI-aEjVw4p5UBTrL1mSJ7N	at Apr 26 17:18:25 NyT1ia0zf00N80	2025 Page 2 dF0zjlzMoBS

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

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SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

(3) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE	ANGIER, NC
25-3336-R01	R17	Common	1	1	Job Reference (optional)	# 5882	77
		Run: ID:	8.630 s Jul GHOhT5M	12 2024 Pri Ov4FkLKI	int: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sa IPfX2c9QzXMNI-aEjVw4p5UBTrL1mSJ7NI	t Apr 26 17:18:25 2 lyT1iZ0zg00N8d	2025 Page 2 F0zjlzMoBS

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

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(3) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHEL	BY MEADOW LANE ANGIER, NO
25-3336-R01	R18	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:G	.630 s Jul HOhT5M0	12 2024 Pri Dv4FkLKIF	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. PfX2c9QzXMNI-2QHt8QqjFVbiyBLetru	Sat Apr 26 17:18:26 2025 Page 2 VgZuOPJ2IPcIsvmXFBzMoBF

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

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(5) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELE	BY MEADOW LANE ANGIER, NC
25-3336-R01	R19	Roof Special	1	1	Job Reference (optional)	# 58877
		Run: ID:GF	8.630 s Jul OhT5MOv	12 2024 Pri 4FkLKIPfX	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. <2c9QzXMNI-WdrGLlqL0ojZaLwrQYPD1	Sat Apr 26 17:18:27 2025 Page 2 I u62jpeFUvZR5ZV4ndzMoBQ

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LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELBY	MEADOW LANE	ANGIER, NC
25-3336-R01	R20	Roof Special	1	1	Job Reference (optional)	# 5882	77
		Run: 8 ID:GF	.630 s Jul OhT5MO	i 2 2024 Pri v4FkLKIP	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sa fX2c9QzXMNI-WdrGLlqL0ojZaLwrQYPD1	at Apr 26 17:18:27 2 u61lpitUswR5Z\	2025 Page 2 /4ndzMoBQ

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5) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELE	BY MEADOW LANE ANGIE	ER, NC
25-3336-R01	R21	Нір	1	1	Job Reference (optional)	# 58877	
		Run: ID:GH0	8.630 s Jul 0hT5MOv4	12 2024 Pri FkLKIPfX2	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. 2c9QzXMNIpPeZ5rzn6rQCVU1_GwSa	Sat Apr 26 17:18:28 2025 F a5f6UD0tDNYbJDFeK4z	Page 2 zMoBP

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LOAD CASE(S) Standard





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER, NC
25-3336-R01	R22	Hip Girder	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GH	3.630 s Jul OhT5MO\	12 2024 Pri 4FkLKIPf.	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. S X2c9QzXMNI-S?z0mRsbYQzHpf3DYzRh	at Apr 26 17:18:29 2025 Page 2 h6JBltdOdylJkYt BsWzMoBC

NOTES- (12-16)

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

12) Trusses designed with 2018 IRC also comply with 2015 IRC.

13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated

15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.

16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRĂCINĞ OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-9=-60, 9-10=-60, 11-20=-20

Concentrated Loads (lb)

Vert: 4--49(F) 9=-49(F) 18=-61(F) 15=-31(F) 6=-49(F) 12=-31(F) 23=-60(F) 24=-15(F) 25=-49(F) 26=-49(F) 27=-49(F) 28=-49(F) 29=-49(F) 30=-49(F) 31=-49(F) 32=-49(F) 32=-49(F) 33=-49(F) 33=-31(F) 40=-31(F) 42=-31(F) 42=-31(F) 43=-31(F) 44=-31(F) 45=-31(F) 46=-74(F) 47=-51(F)









Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELE	BY MEADOW LANE ANGIER, NC
25-3336-R01	R24A	Common	3	1	Job Reference (optional)	# 58877
		Run: ID:GH	8.630 s_Jul OhT5MOv₄	12 2024 Pri 4FkLKIPfX	int: 8.630 s Jul 12 2024 MiTek Industries, Inc. {2c9QzXMNI-wBWOzntEJj58RoeQ6gywl	Sat Apr 26 17:18:30 2025 Page 2 WkZB1kxhKrunXkkOyzMoBN

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SH	HELBY MEADOW LANE ANGIER, NC
25-3336-R01	R24B	COMMON	2	1	Job Reference (optional)	# 58877
		R ID:G	n: 8.630 s Jul IOhT5MOv4	12 2024 Pri FkLKIPfX2	nt: 8.630 s Jul 12 2024 MiTek Industries, Ir c9QzXMNI-OO4mB7ts41D?3yDcfOL	nc. Sat Apr 26 17:18:31 2025 Page 2 J9CkHl9Q5sQk91?BTIwOzMoBM

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHE	ELBY MEADOW LANE ANGIER, NC
25-3336-R01	R25	Hip Girder	1	2	Job Reference (optional)	# 58877
		ID:	Run: 8.630 s Jul GHOhT5MOv4F	2 2024 Pri kLKIPfX2	nt: 8.630 s Jul 12 2024 MiTek Industries, In c9QzXMNI-LmCXcpv6beTjIGN npWc	c. Sat Apr 26 17:18:33 2025 Page 2 H9M49EjTuYvKTVyO?HzMoBK

NOTES- (16)

 Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-1-4 from the left end to 7-1-4 to connect truss(es) R18 (1 ply 2x4 SP), R19 (1 ply 2x4 SP), R20 (1 ply 2x4 SP), R21 (1 ply 2x4 SP) to front face of bottom chord.
Use Simpson Strong-Tie HTU26 (20-10d Girder, 14-10dx1 1/2 Truss, Single Ply Girder) or equivalent at 9-1-4 from the left end to connect truss(es) R22 (1 ply 2x6 SP) to front

12) Use Simpson Strong-Tie HTU26 (20-10d Girder, 14-10dx1 1/2 Truss, Single Ply Girder) or equivalent at 9-1-4 from the left end to connect truss(es) R22 (1 ply 2x6 SP) to front face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 18 lb down and 3 lb up at 1-4-15, 18 lb down and 3 lb up at 3-4-15, and 18 lb down and 3 lb up at 5-4-15, and 18 lb down and 3 lb up at 7-4-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 16) Trusses designed with 2018 IRC also comply with 2015 IRC.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-6=-60, 6-8=-60, 13-16=-20

Concentrated Loads (lb)

Vert: 2=-2(B) 6-2(B) 12=-8(B) 9=-8(B) 11=-1262(F) 5=-2(B) 19=-2(B) 20=-2(B) 21=-2(B) 22=-2(B) 23=-1262(F) 24=-1262(F) 25=-8(B) 26=-8(B) 27=-1262(F) 28=-8(B) 29=-1981(F) 30=-8(B) 31=-8(B) 31=-8





Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHE	ELBY MEADOW LANE ANGIER, NC
25-3336-R01	R26	GABLE	1	1	Job Reference (optional)	# 58877
		Run: 8 ID:GHOhī	.630 s Jul I5MOv4Fl	12 2024 Pri LKIPfX2c	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc 9QzXMNI-LmCXcpv6beTjIGN_npWdF	c. Sat Apr 26 17:18:33 2025 Page 2 H9Mz4EmdueLKTVyO?HzMoBK

13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0048 HONEYCUTT HILLS 56 SHELB	Y MEADOW LANE ANGIER,	NC
25-3336-R01	R27	Monopitch	7	1	Job Reference (optional)	# 58877	
		Run: 8 ID:GH	.630 s Jul OhT5MOv	2 2024 Pri 4FkLKIPf	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. S X2c9QzXMNI-pzmvp9wkMybawQyBLW1	Sat Apr 26 17:18:34 2025 Pag 1spMu8qe5sd5bTi9iyXjzM	je 2 oBJ

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

(4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





