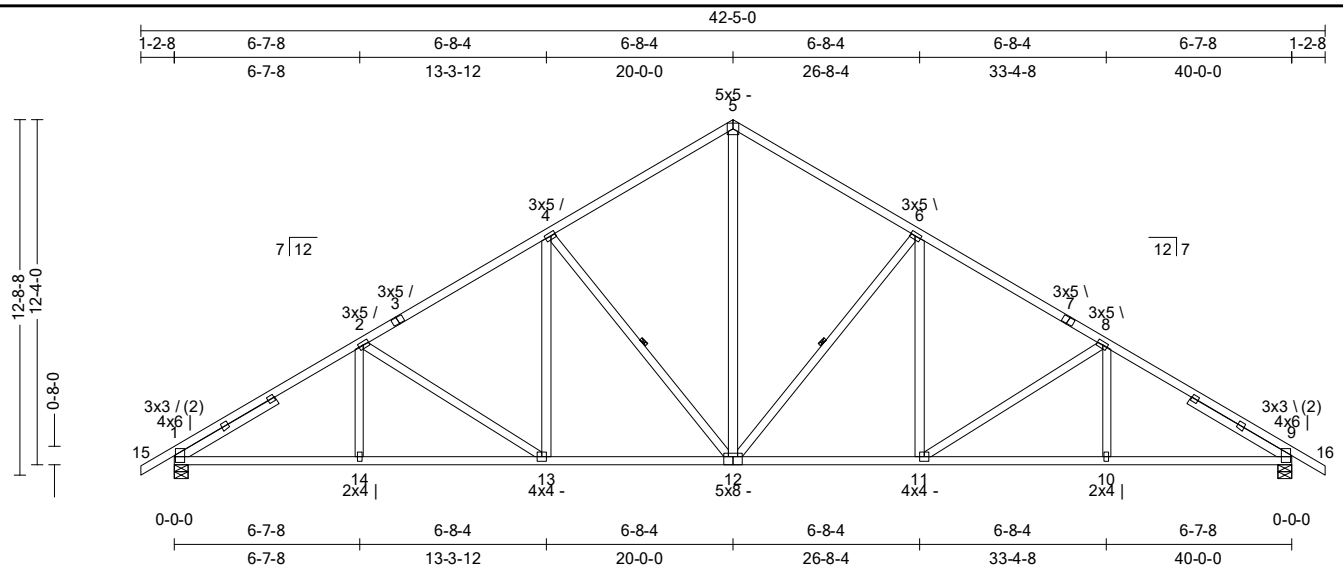
TOZA TRUSS LLC
6633 CARL COX RD
BENNETT NC 27208
Off: (336) 879-1212

Truss:T1
Job: Flowers Residence 25
Date: 05/22/25 15:55:17
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
40-0-0	7/12	18	1-2-8	1-2-8	0-0-0	0-0-0	1	24 in	236 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IRC 2018/	TC :	0.56 (5-6)	Vert TL:	0.25 in	L / 999	(11-12)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.47 (12-13)	Vert LL:	0.11 in	L / 999	12	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.50 (5-12)	Horz TL:	0.11 in		9	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.97 in	1,672 lbs	·	-152 lbs	-267 lbs	-267 lbs	110 lbs
9	1	5.5 in	1.97 in	1,672 lbs	·	-152 lbs	-267 lbs	-267 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-12, 6-12

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 20 psf Roof (GSL = 29 psf), Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2018 assuming slope = 7 /12 and area supported = 84.83 ft^2, DOL = 115 %.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.303	363 lbs	(-2,502 lbs)	4-5	0.561	390 lbs	(-1,668 lbs)	6-8	0.529	377 lbs	(-2,158 lbs)
	2-4	0.529	377 lbs	(-2,158 lbs)	5-6	0.561	390 lbs	(-1,668 lbs)	8-9	0.303	363 lbs	(-2,502 lbs)
BC	9-10	0.466	2,128 lbs		11-12	0.473	1,784 lbs		13-14	0.468	2,128 lbs	
	10-11	0.468	2,128 lbs		12-13	0.473	1,784 lbs		14-1	0.466	2,128 lbs	
Web	2-13	0.384		(-406 lbs)	5-12	0.498	1,181 lbs		8-11	0.384		(-406 lbs)
	4-13	0.059	355 lbs		6-12	0.337		(-761 lbs)				
	4-12	0.337		(-761 lbs)	6-11	0.059	355 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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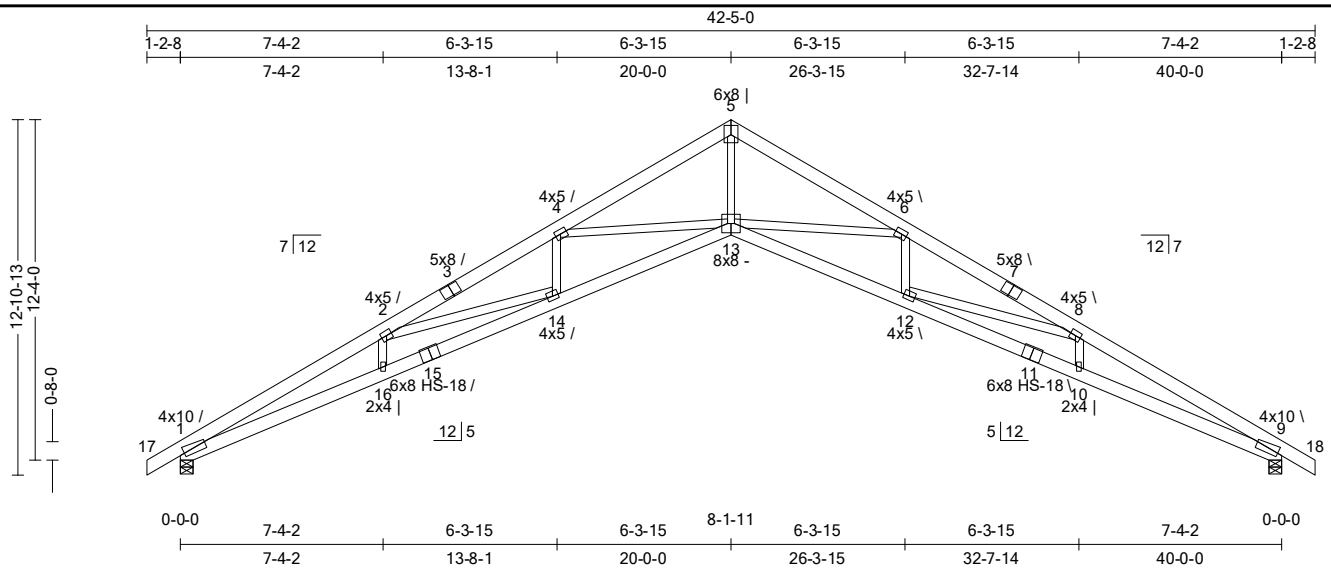
Eagle Metal Products

TOZA TRUSS LLC
6633 CARL COX RD
BENNETT NC 27208
Off: (336) 879-1212

Truss:T1A
Job: Flowers Residence 25
Date: 05/22/25 15:55:17
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
40-0-0	7/12	10	1-2-8	1-2-8	0-0-0	0-0-0	1	24 in	259 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 20	Bldg Code :	IRC 2018/	TC :	0.55 (8-9)	Vert TL:	1.24 in	L / 377	13	L / 240
TCDL : 10		TPI 1-2014	BC :	0.62 (9-10)	Vert LL:	0.62 in	L / 756	13	L / 360
BCLL : 0	Rep Mbr :	Yes	Web :	0.83 (4-13)	Horz TL:	1.25 in		9	
BCDL : 10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.53 in	1,672 lbs	·	-152 lbs	-267 lbs	-267 lbs	-110 lbs
9	1	5.5 in	1.53 in	1,672 lbs	·	-152 lbs	-267 lbs	-267 lbs	·

Material

TC: SYP DSS 2 x 6
BC: SYP DSS 2 x 6
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 20 psf Roof (GSL = 29 psf), Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2018 assuming slope = 7 /12 and area supported = 84.83 ft^2, DOL = 115 %.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.550	665 lbs	(-6,650 lbs)	4-5	0.278		(-5,153 lbs)	6-8	0.368	497 lbs	(-6,492 lbs)
	2-4	0.368	497 lbs	(-6,492 lbs)	5-6	0.278		(-5,153 lbs)	8-9	0.550	665 lbs	(-6,650 lbs)
BC	9-10	0.619	6,008 lbs	(-464 lbs)	12-13	0.399	6,037 lbs		14-16	0.597	6,111 lbs	(-470 lbs)
	10-12	0.597	6,111 lbs	(-470 lbs)	13-14	0.399	6,037 lbs		16-1	0.619	6,008 lbs	(-464 lbs)
Web	4-14	0.056	338 lbs		6-13	0.834	376 lbs	(-1,351 lbs)				
	4-13	0.834	376 lbs	(-1,351 lbs)	6-12	0.056	338 lbs					
	5-13	0.780	4,710 lbs									

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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Truss:T1AG
Job: Flowers Residence 25
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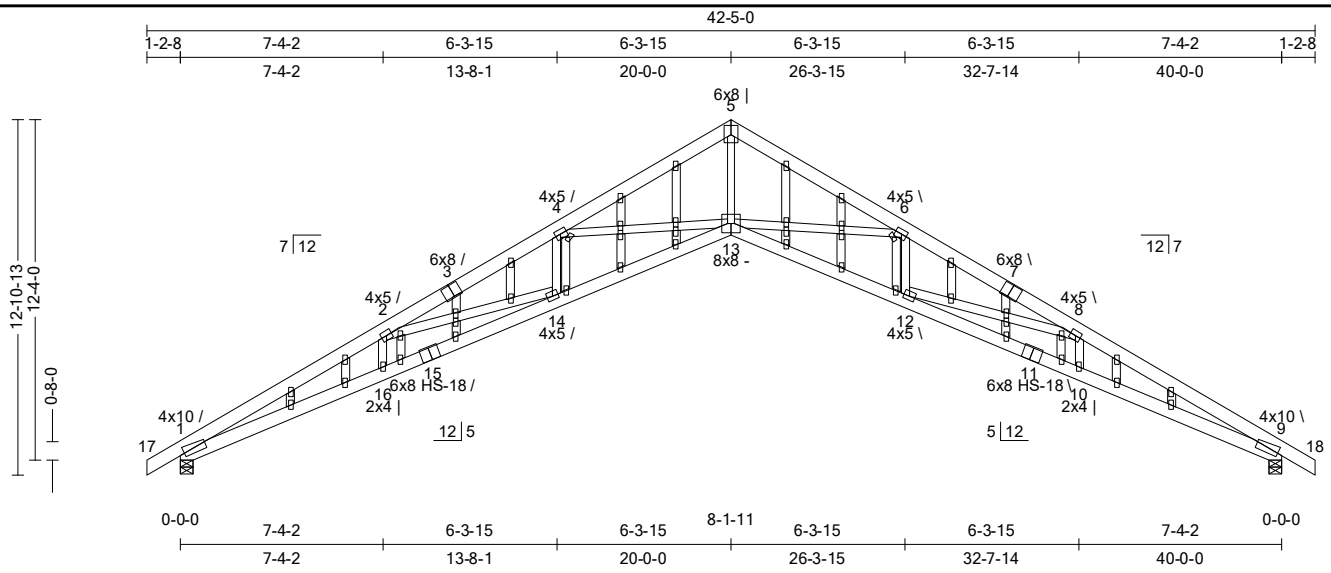
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
40-0-0	7/12	1	1-2-8	1-2-8	0-0-0	0-0-0	1	24 in	302 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IRC 2018/	TC :	0.64 (8-9)	Vert TL:	1.24 in	L / 377	13	L / 240
TCDL :	10		TPI 1-2014	BC :	0.70 (9-10)	Vert LL:	0.62 in	L / 756	13	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.83 (4-13)	Horz TL:	1.25 in		9	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.53 in	1,672 lbs	·	-152 lbs	-267 lbs	-267 lbs	-110 lbs
9	1	5.5 in	1.53 in	1,672 lbs	·	-152 lbs	-267 lbs	-267 lbs	·

Material

TC: SYP DSS 2 x 6
BC: SYP DSS 2 x 6
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 20 psf Roof (GSL = 29 psf), Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2018 assuming slope = 7 /12 and area supported = 84.83 ft^2, DOL = 115 %.
5) Non-concurrent minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.640	665 lbs	(-6,650 lbs)	4-5	0.305		(-5,153 lbs)	6-8	0.433	497 lbs	(-6,492 lbs)
	2-4	0.433	497 lbs	(-6,492 lbs)	5-6	0.305		(-5,153 lbs)	8-9	0.640	665 lbs	(-6,650 lbs)
BC	9-10	0.695	6,008 lbs	(-464 lbs)	12-13	0.442	6,037 lbs		14-16	0.669	6,111 lbs	(-470 lbs)
	10-12	0.669	6,111 lbs	(-470 lbs)	13-14	0.442	6,037 lbs		16-1	0.695	6,008 lbs	(-464 lbs)
Web	4-14	0.056	338 lbs		6-13	0.834	376 lbs	(-1,351 lbs)				
	4-13	0.834	376 lbs	(-1,351 lbs)	6-12	0.056	338 lbs					
	5-13	0.780	4,710 lbs									

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable webs placed at 24 " OC, U.N.O.
3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) A creep factor of 1.00 has been applied for this truss analysis.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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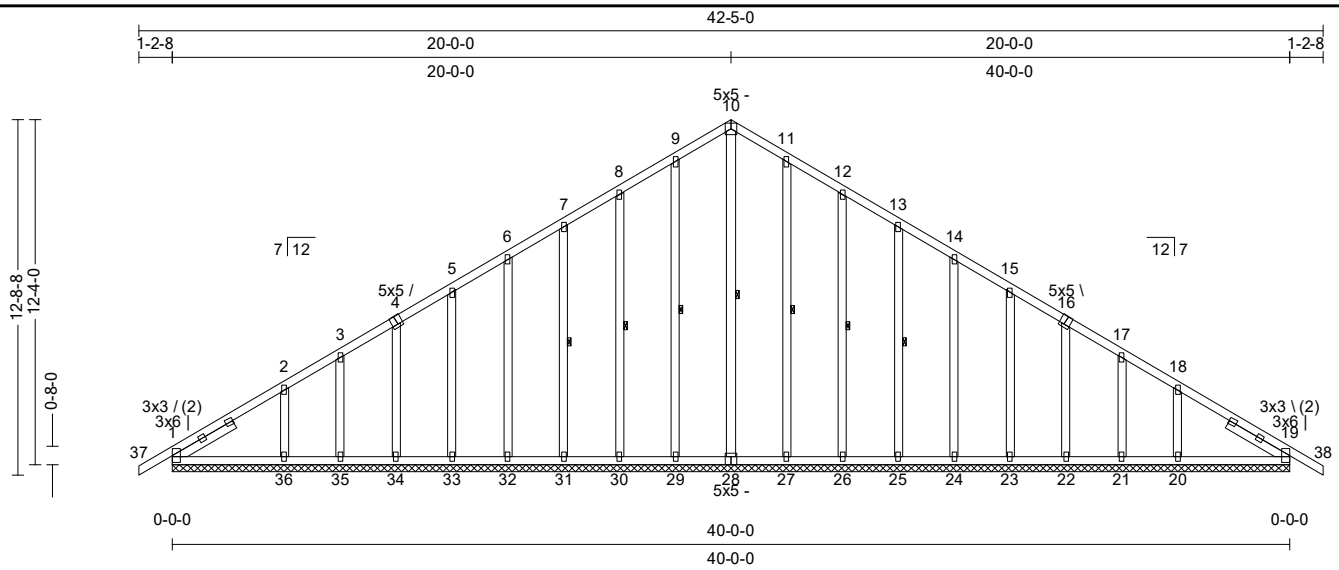
BENNETT NC 27208
Off: (336) 879-1212

Truss:T1G
Job: Flowers Residence 25
Date: 05/22/25 15:55:18

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
40-0-0	7/12	1	1-2-8	1-2-8	0-0-0	0-0-0	1	24 in	297 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 20	Bldg Code :	IRC 2018/	TC :	0.15 (19-38)	Vert TL:	0 in	L / 999	19	L / 240
TCDL : 10		TPI 1-2014	BC :	0.07 (19-20)	Vert LL:	0 in	L / 999	19	L / 360
BCLL : 0	Rep Mbr :	No	Web :	0.10 (14-24)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. :	115 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	429 lbs	113 plf	-130 lbs	-227 lbs	-221 lbs	-227 lbs	293 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 7-31, 8-30, 9-29, 10-28, 11-27, 12-26, 13-25

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 20 psf Roof (GSL = 29 psf), Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2018 assuming slope = 7 /12 and area supported = 84.83 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.143	(-351 lbs)
	18-19	0.143	(-351 lbs)
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
7) Gable must be sheathed on one side or lateral bracing applied appropriately.
8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
9) A creep factor of 1.00 has been applied for this truss analysis.
10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
11) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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6633 CARL COX RD
BENNETT NC 27208
Off: (336) 879-1212

Truss:T1G
Job: Flowers Residence 25
Date: 05/22/25 15:55:18

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
40-0-0	7/12	1	1-2-8	1-2-8	0-0-0	0-0-0	1	24 in	297 lbs

12) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 19, 1 may need to be considered.
13) Listed wind uplift reactions based on MWFRS & C&C loading.

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