

Trenco

818 Soundside Rd
Edenton, NC 27932

Re: 25060061-A
572 Creekside Oaks North-Roof-Prelude B CP GLH

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: 175924994 thru 175925024

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



August 27, 2025

Gilbert, Eric

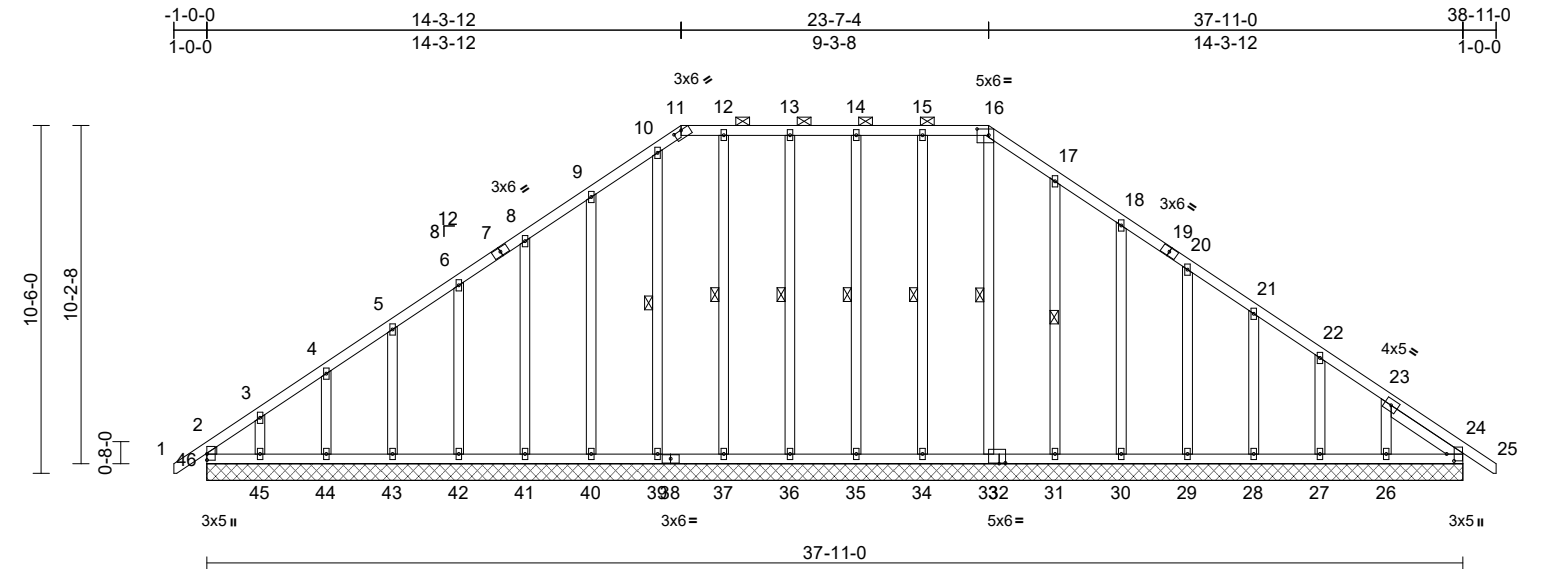
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175924994
25060061-A	A1	Piggyback Base Supported Gable	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

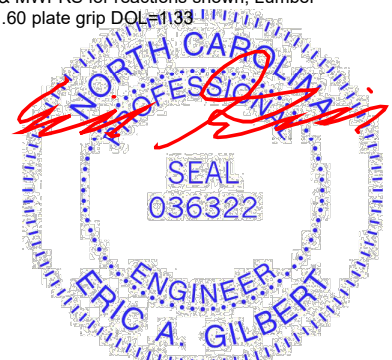
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Page: 1



Scale = 1:69.6									
Plate Offsets (X, Y): [11:0-3-0,0-0-2], [16:0-4-4,0-2-4], [24:0-2-8,0-2-11], [33:0-2-4,0-0-4]									
Loading		(psf)	Spacing		1-11-4	CSI		DEFL	
TCLL (roof)		20.0	Plate Grip DOL		1.15	TC		in	(loc)
Snow (Pf/Pg)		18.9/20.0	Lumber DOL		1.15	BC		l/defl	L/d
TCDL		10.0	Rep Stress Incr		YES	WB		999	999
BCLL		0.0*	Code		IRC2021/TPI2014	Matrix-MSH		Horz(CT)	n/a
BCDL		10.0						0.01	24
								n/a	n/a
								PLATES	
								GRIP	
								MT20	
								244/190	
								Weight: 300 lb	
								FT = 20%	

LUMBER		Max Grav		WEBS	
TOP CHORD		24=154 (LC 29), 26=200 (LC 30),		16-33=-102/9, 15-34=-137/57,	
BOT CHORD		27=162 (LC 30), 28=169 (LC 30),		14-35=-135/67, 13-36=-143/83,	
WEBS		29=167 (LC 30), 30=167 (LC 30),		12-37=-114/29, 10-39=-117/20,	
OTHERS		31=175 (LC 30), 33=141 (LC 32),		9-40=-164/131, 8-41=-151/102,	
SLIDER		34=162 (LC 35), 35=154 (LC 35),		6-42=-153/106, 5-43=-152/106,	
		36=157 (LC 36), 37=153 (LC 35),		4-44=-157/106, 3-45=-143/124,	
		39=156 (LC 29), 40=170 (LC 29),		17-31=-158/105, 18-30=-153/109,	
		41=167 (LC 29), 42=168 (LC 29),		20-29=-152/105, 21-28=-153/106,	
		43=168 (LC 29), 44=167 (LC 29),		22-27=-150/104, 23-26=-178/133	
		45=176 (LC 29), 46=198 (LC 30)			
BRACING		FORCES		NOTES	
TOP CHORD		(lb) - Maximum Compression/Maximum Tension		1) Unbalanced roof live loads have been considered for this design.	
BOT CHORD		TOP CHORD		2) Wind: ASCE 7-16; Vult=150mph (3-second gust)	
WEBS		2-46=-166/88, 1-2=0/45, 2-3=-219/195,		Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.	
		3-4=-165/160, 4-5=-136/138, 5-6=-120/124,		II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner	
		6-8=-105/136, 8-9=-131/205, 9-10=-181/289,		(3E) -0-11-8 to 2-10-0, Exterior(2N) 2-10-0 to 14-3-12,	
		10-11=-173/270, 11-12=-165/278,		Corner(3R) 14-3-12 to 18-1-4, Exterior(2N) 18-1-4 to	
		12-13=-165/278, 13-14=-165/278,		23-7-4, Corner(3R) 23-7-4 to 27-7-4, Exterior(2N) 27-7-4	
		14-15=-165/278, 15-16=-165/278,		to 38-10-8 zone; cantilever left and right exposed ; end	
		16-17=-190/305, 17-18=-147/234,		vertical left and right exposed; C-C for members and	
		18-20=-106/160, 20-21=-75/89,		forces & MWFRS for reactions shown; Lumber	
		21-22=-78/70, 22-23=-100/89, 23-24=-71/22,		DOL=1.60 plate grip DOL=1.33	
		24-25=0/39			
		45-46=-104/192, 44-45=-104/192,			
		43-44=-104/192, 42-43=-104/192,			
		41-42=-104/192, 40-41=-104/192,			
		39-40=-104/192, 37-39=-104/192,			
		36-37=-104/192, 35-36=-104/192,			
		34-35=-104/192, 33-34=-104/192,			
		31-33=-104/192, 30-31=-104/192,			
		29-30=-104/192, 28-29=-104/192,			
		27-28=-104/192, 26-27=-104/192,			
		24-26=-104/192			
REACTIONS (size)		24=37-11-0, 26=37-11-0,			
		27=37-11-0, 28=37-11-0,			
		29=37-11-0, 30=37-11-0,			
		31=37-11-0, 33=37-11-0,			
		34=37-11-0, 35=37-11-0,			
		36=37-11-0, 37=37-11-0,			
		39=37-11-0, 40=37-11-0,			
		41=37-11-0, 42=37-11-0,			
		43=37-11-0, 44=37-11-0,			
		45=37-11-0, 46=37-11-0			
Max Horiz		46=-259 (LC 11)			
Max Uplift		24=-26 (LC 10), 26=-80 (LC 14),			
		27=-44 (LC 14), 28=-50 (LC 14),			
		29=-48 (LC 14), 30=-51 (LC 14),			
		31=-49 (LC 14), 34=-21 (LC 10),			
		35=-19 (LC 10), 36=-26 (LC 9),			
		37=-6 (LC 10), 40=-63 (LC 13),			
		41=-47 (LC 13), 42=-48 (LC 13),			
		43=-51 (LC 13), 44=-38 (LC 13),			
		45=-109 (LC 13), 46=-99 (LC 9)			



August 27, 2025

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75924994
25060061-A	A1	Piggyback Base Supported Gable	1	1	Job Reference (optional)

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) All bearings are assumed to be SP No.2 .
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 46, 26 lb uplift at joint 24, 21 lb uplift at joint 34, 19 lb uplift at joint 35, 26 lb uplift at joint 36, 6 lb uplift at joint 37, 63 lb uplift at joint 40, 47 lb uplift at joint 41, 48 lb uplift at joint 42, 51 lb uplift at joint 43, 38 lb uplift at joint 44, 109 lb uplift at joint 45, 49 lb uplift at joint 31, 51 lb uplift at joint 30, 48 lb uplift at joint 29, 50 lb uplift at joint 28, 44 lb uplift at joint 27, 80 lb uplift at joint 26 and 26 lb uplift at joint 24.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

 **WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)



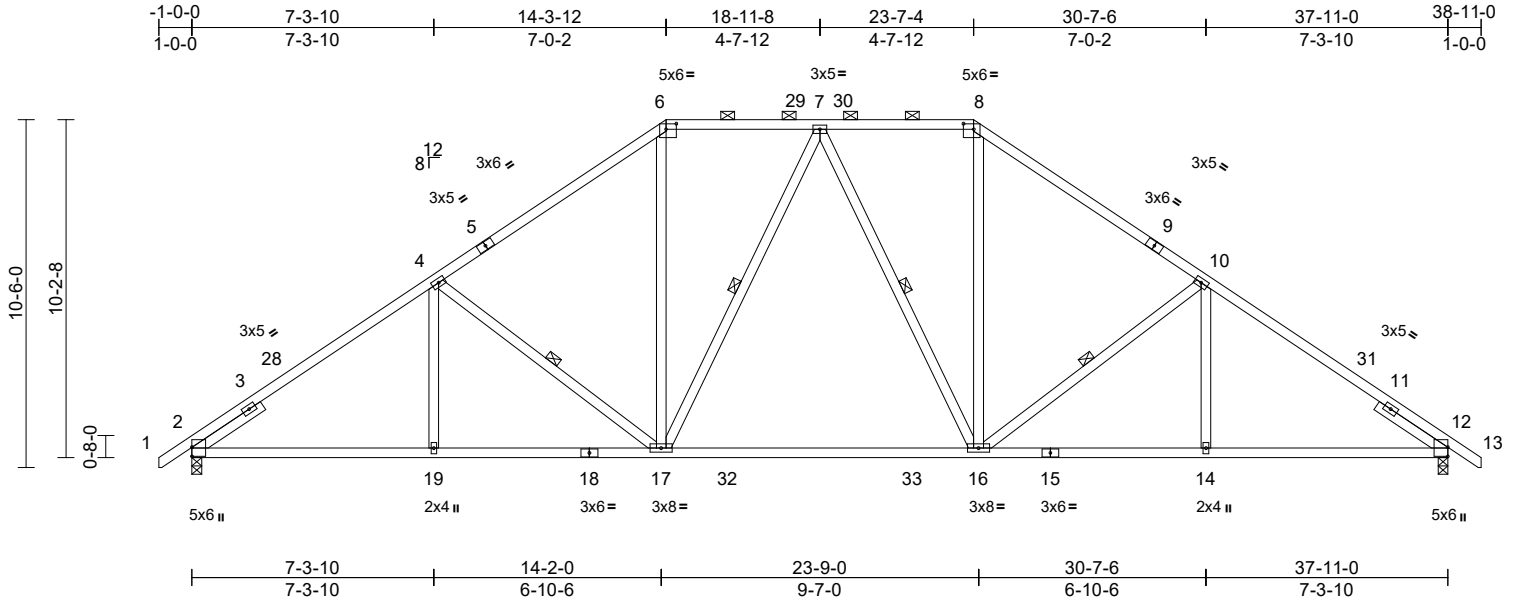
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175924995
25060061-A	A2	Piggyback Base	5	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:42
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Page: 1



Scale = 1:69.6

Plate Offsets (X, Y): [6:0-3-12,0-2-0], [8:0-3-12,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.36	16-17	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.86	Vert(CT)	-0.59	16-17	>775	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.11	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 232 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except* 18-15:2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 17-7, 16-7:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (4-2-11 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-17, 7-17, 7-16, 10-16

REACTIONS

(size) 2=0-3-8, 12=0-3-8
Max Horiz 2=-255 (LC 11)
Max Uplift 2=-96 (LC 13), 12=-96 (LC 14)
Max Grav 2=1726 (LC 29), 12=1726 (LC 30)

FORCES

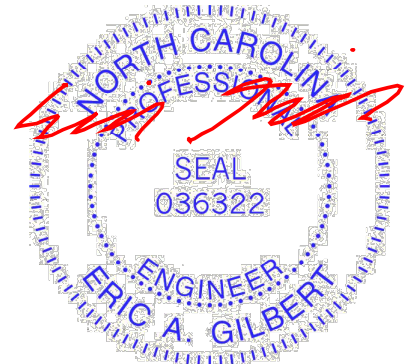
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-4=-2769/339, 4-6=-2316/378, 6-7=-1854/384, 7-8=-1854/384, 8-10=-2316/378, 10-12=-2769/339, 12-13=0/40
BOT CHORD 2-19=-238/2217, 17-19=-202/2217, 16-17=-65/1792, 14-16=-182/2219, 12-14=-182/2219
WEBS 4-19=0/217, 4-17=-604/203, 6-17=-63/866, 7-17=-285/182, 7-16=-285/182, 8-16=-63/866, 10-16=-604/204, 10-14=0/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 14-3-12, Exterior(2R) 14-3-12 to 19-8-2, Interior (1) 19-8-2 to 23-7-4, Exterior(2R) 23-7-4 to 28-11-10, Interior (1) 28-11-10 to 38-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 2 and 96 lb uplift at joint 12.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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ENGINEERING BY
TRENCO
A MITEK COMPANY

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Edenton, NC 27932

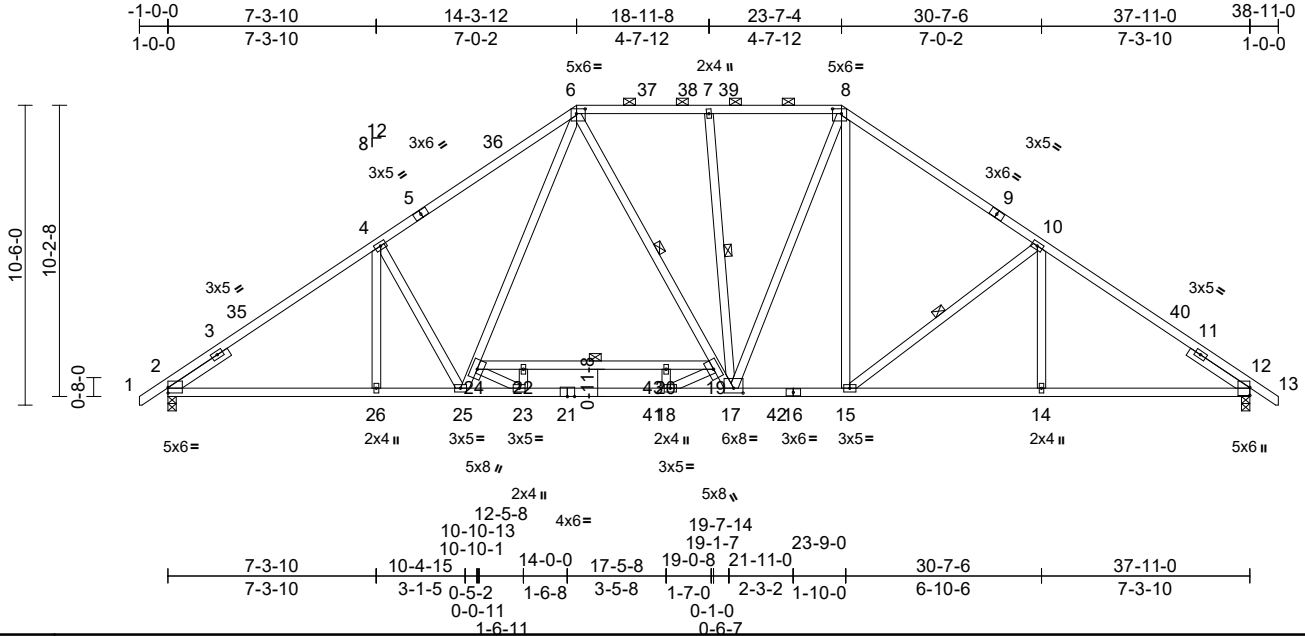
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75924996
25060061-A	A3	Piggyback Base	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:42

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Scale = 1:80.8

Plate Offsets (X, Y): [2:Edge,0-2-5], [6:0-3-12,0-2-0], [8:0-3-12,0-2-0], [17:0-4-0,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.79	Vert(LL)	-0.18	18-23	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.34	18-23	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(CT)	0.14	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 263 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP 2400F 2.0E *Except* 6-8:2x4 SP No.2, 1-5,9-13:2x4 SP No.1
BOT CHORD	2x4 SP No.2 *Except* 2-21:2x4 SP No.1
WEBS	2x4 SP No.3 *Except* 6-25,6-17,8-17:2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-6-11 max.): 6-8.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	1 Row at midpt 10-15, 6-17, 7-17

REACTIONS

(size)	2=0-3-8, 12=0-3-8
Max Horiz	2=255 (LC 11)
Max Grav	2=2086 (LC 29), 12=1952 (LC 30)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/40, 2-4=-3370/26, 4-6=-3181/159, 6-7=-2374/178, 7-8=-2347/173, 8-10=-2682/182, 10-12=-3156/141, 12-13=0/40
BOT CHORD	2-26=-165/2712, 25-26=0/2712, 23-25=0/2185, 18-23=0/2829, 17-18=0/2225, 15-17=0/2035, 14-15=-19/2533, 12-14=-95/2533, 22-24=-917/0, 20-22=-917/0, 19-20=-917/0
WEBS	4-26=0/154, 8-15=-48/605, 10-15=-614/205, 10-14=0/255, 24-25=-134/615, 6-24=-35/982, 6-19=-153/415, 17-19=-252/211, 4-25=-536/197, 8-17=-13/568, 7-17=-344/63, 22-23=-230/0, 23-24=0/894, 18-20=-217/0, 18-19=0/837

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 14-3-12, Exterior(2R) 14-3-12 to 19-8-2, Interior (1) 19-8-2 to 23-7-4, Exterior(2R) 23-7-4 to 28-11-10, Interior (1) 28-11-10 to 38-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the top chord, 14-3-12 from left end, supported at two points, 5-0-0 apart.
- Provide adequate drainage to prevent water ponding.
- All plates are 3x5 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP No.1 , Joint 12 SP No.2 .
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

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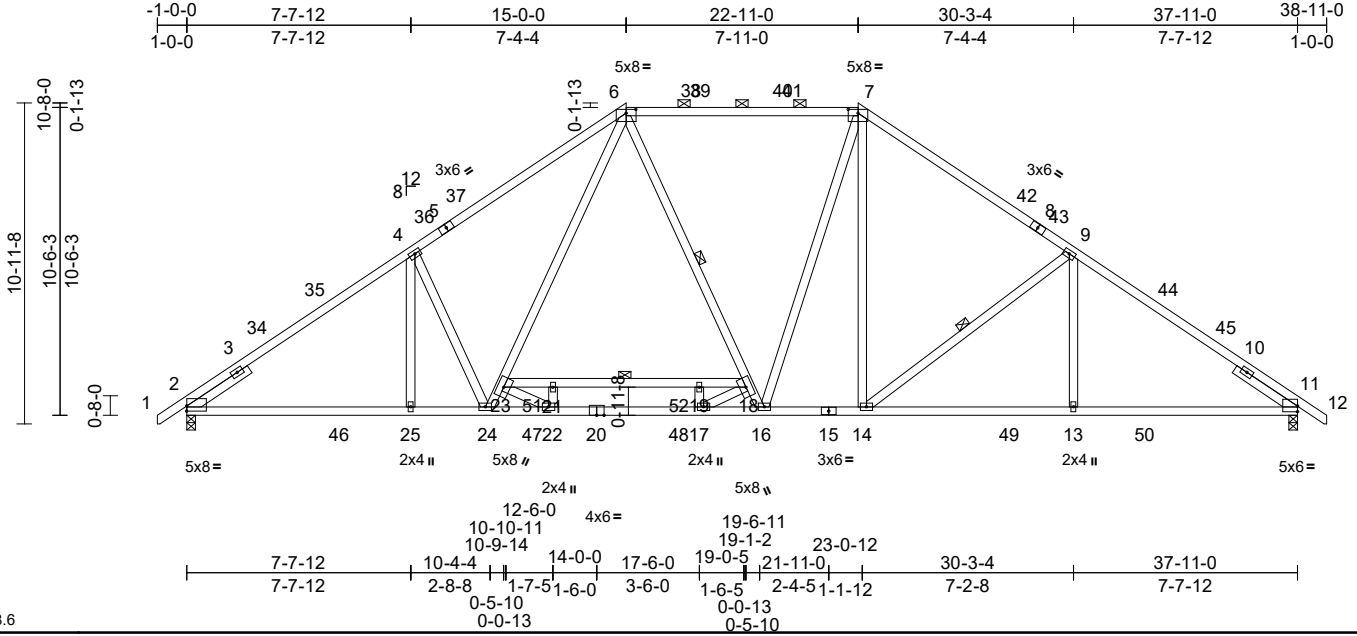
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75924997
25060061-A	A4	Hip	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:43

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Scale = 1:78.6

Plate Offsets (X, Y): [2:Edge,0-2-1], [6:0-4-0,0-1-9], [7:0-4-0,0-1-9], [11:Edge,0-2-5]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	1.00	Vert(LL)	-0.19	17-22	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.36	17-22	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.14	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 253 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* 1-5,8-12:2x4 SP No.1
 BOT CHORD 2x4 SP No.1 *Except* 20-15,23-18:2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* 14-7,6-24,6-16,7-16:2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied, except
 2-0-0 oc purlins (3-6-8 max.): 6-7.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

WEBS 1 Row at midpt 9-14, 6-16

REACTIONS (size) 2=0-3-8, 11=0-3-8
 Max Horiz 2=264 (LC 14)
 Max Grav 2=2217 (LC 56), 11=2092 (LC 58)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-4=-3601/30, 4-6=-3419/126, 6-7=-2336/172, 7-9=-2807/184, 9-11=-3410/141, 11-12=0/40
 BOT CHORD 2-25=-165/2901, 24-25=0/2901, 22-24=0/2276, 17-22=0/3079, 16-17=0/2278, 14-16=0/2105, 13-14=-13/2745, 11-13=-106/2745, 21-23=-1148/0, 19-21=-1148/0, 18-19=-1148/0
 WEBS 4-25=0/217, 7-14=-49/690, 9-14=-791/214, 9-13=0/359, 23-24=-157/771, 6-23=0/1236, 6-18=-52/326, 16-18=-367/0, 4-24=-638/246, 7-16=0/415, 21-22=-222/0, 22-23=0/1078, 17-19=-229/0, 17-18=0/1078

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 15-0-0, Exterior(2R) 15-0-0 to 20-4-6, Interior (1) 20-4-6 to 22-11-0, Exterior(2R) 22-11-0 to 28-3-6, Interior (1) 28-3-6 to 38-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) 200.0lb AC unit load placed on the bottom chord, 14-3-12 from left end, supported at two points, 5-0-0 apart.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 3x5 MT20 unless otherwise indicated.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) All bearings are assumed to be SP No.1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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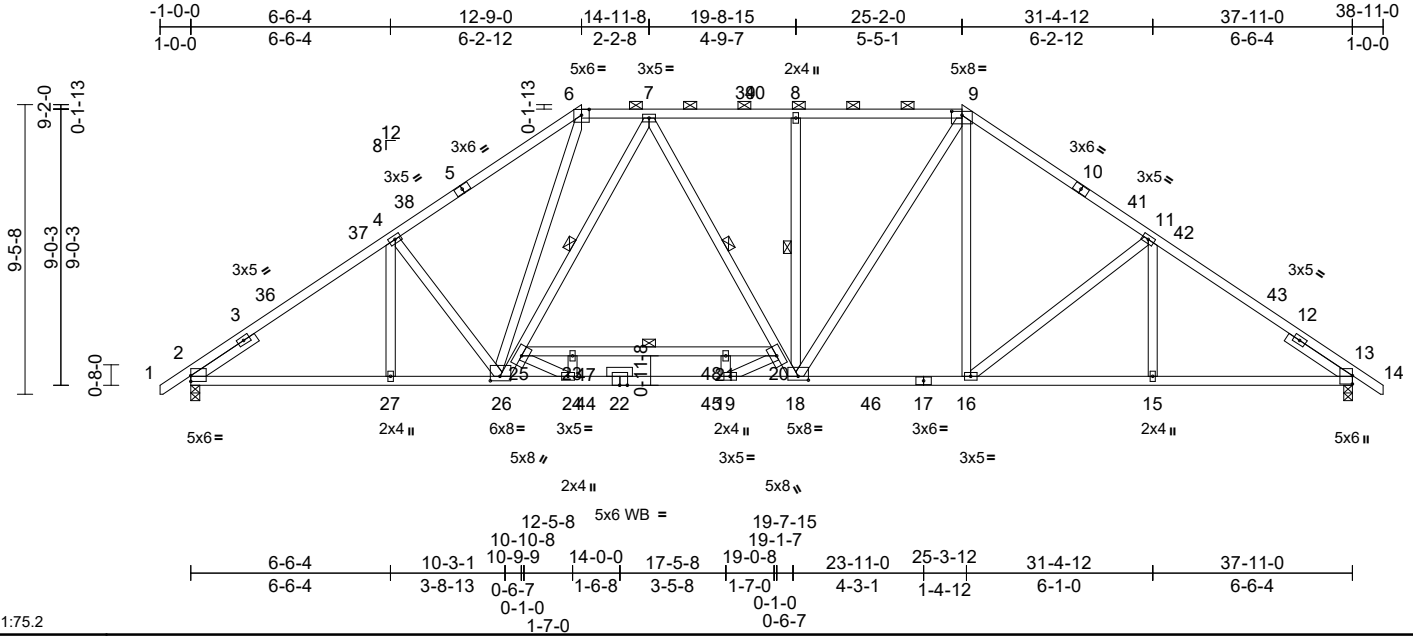
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175924998
25060061-A	A5	Hip	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:43

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.95	Vert(LL)	-0.19	19-24	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.80	Vert(CT)	-0.38	19-24	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.13	13	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 267 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* 5-6,9-10:2x4 SP No.1
BOT CHORD	2x4 SP No.1 *Except* 25-20:2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-2-3 max.): 6-9.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 5-5-10 oc bracing: 23-25 5-1-1 oc bracing: 21-23 5-5-3 oc bracing: 20-21.
WEBS	1 Row at midpt 7-26, 7-18, 8-18

REACTIONS

(size)	2=0-3-8, 13=0-3-8
Max Horiz	2=227 (LC 14)
Max Grav	2=2093 (LC 50), 13=1974 (LC 50)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/40, 2-4=-3464/33, 4-6=-3206/86, 6-7=-2201/172, 7-8=-2715/139, 8-9=-2717/141, 9-11=-2832/165, 11-13=-3280/134, 13-14=0/40
BOT CHORD	2-27=-129/2783, 26-27=0/2783, 24-26=0/2590, 19-24=0/3337, 18-19=0/2578, 16-18=0/2182, 15-16=-23/2634, 13-15=-73/2634, 23-25=-1113/0, 21-23=-1113/0, 20-21=-1113/0
WEBS	4-27=0/132, 9-16=-38/615, 11-16=-591/187, 11-15=0/202, 25-26=-1254/0, 7-25=-797/146, 7-20=-44/454, 18-20=-249/105, 4-26=-492/204, 6-26=0/1507, 9-18=-5/831, 8-18=-539/153, 23-24=-211/0, 24-25=0/999, 19-21=-222/0, 19-20=0/1008

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 12-9-0, Exterior(2R) 12-9-0 to 18-1-6, Interior (1) 18-1-6 to 25-2-0, Exterior(2R) 25-2-0 to 30-6-6, Interior (1) 30-6-6 to 38-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 14-11-8 from left end, supported at two points, 5-0-0 apart.
- Provide adequate drainage to prevent water ponding.
- All plates are 3x5 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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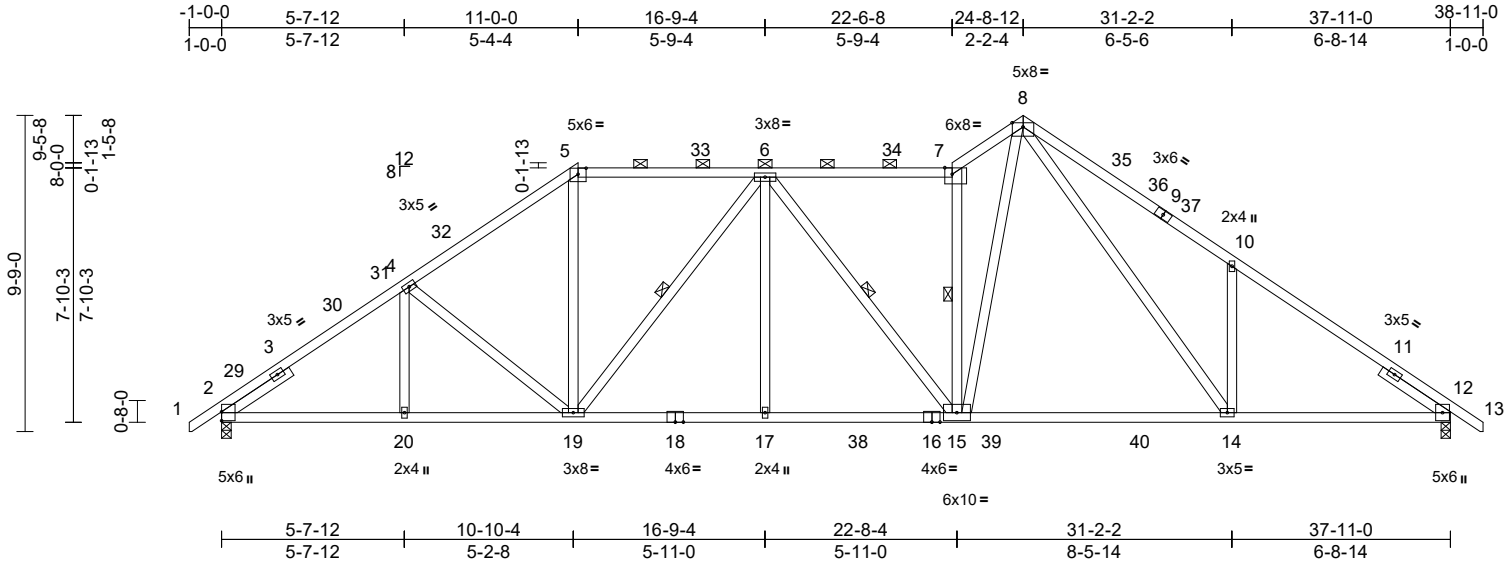
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75924999
25060061-A	A6	Roof Special	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:43

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Scale = 1:71.1									
Plate Offsets (X, Y): [5:0-3-0,0-2-3], [7:0-2-11,Edge], [12:0-3-5,0-0-7]									
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.32 14-15	>999	240
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	1.00	Vert(CT)	-0.55 14-15	>833	180
TCDL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.12 12	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH					
BCDL	10.0								
									PLATES MT20
									GRIP 244/190
									Weight: 244 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 8-9,9-13:2x4 SP No.1
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 14-8:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (3-0-5 max.): 5-7.
Rigid ceiling directly applied or 2-2-0 oc bracing.
BOT CHORD
WEBS 1 Row at midpt 6-19, 6-15, 7-15

REACTIONS (size) 2=0-3-8, 12=0-3-8
Max Horiz 2=-236 (LC 13)
Max Uplift 2=-161 (LC 15), 12=-89 (LC 16)
Max Grav 2=1728 (LC 3), 12=1804 (LC 34)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-4=-3014/484, 4-5=-2722/506, 5-6=-2233/469, 6-7=-2633/521, 7-8=-3179/651, 8-10=-3175/693, 10-12=-3054/475, 12-13=0/40
BOT CHORD 2-20=-295/2416, 19-20=-295/2416, 17-19=-228/2603, 15-17=-228/2603, 14-15=-112/1915, 12-14=-271/2441
WEBS 4-20=0/161, 4-19=-525/169, 5-19=-120/1100, 6-19=-874/125, 6-17=0/290, 6-15=-284/108, 7-15=-1988/438, 8-15=-394/2602, 8-14=-276/912, 10-14=-461/314

NOTES
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 11-0-0, Exterior(2R) 11-0-0 to 14-9-8, Interior (1) 14-9-8 to 24-8-12, Exterior(2R) 24-8-12 to 28-6-4, Interior (1) 28-6-4 to 38-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 2 and 89 lb uplift at joint 12.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

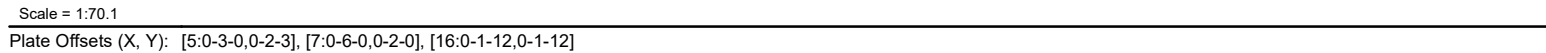
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:44 Page: 1
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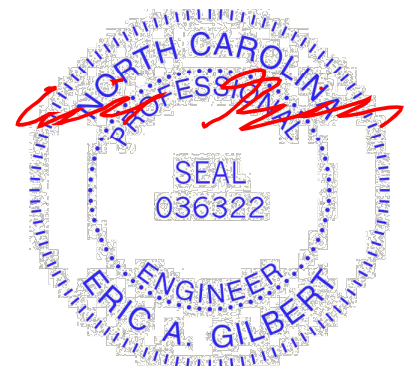


WIND	2x4 SP No.2 *Except* 8-9:2x4 SP No.1	Wind: ASCE 7-16; Vult=150mph (3-second gust)
TOP CHORD	2x4 SP No.2 *Except* 17-15:2x4 SP No.1	Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
BOT CHORD	2x4 SP No.3 *Except* 16-8:2x4 SP No.2	II; Exp B; Enclosed; MWFRS (envelope) and C-C
WEBS	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3	Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to
SLIDER	-- 2-6-0	9-0-0, Exterior(2R) 9-0-0 to 12-9-8, Interior (1) 12-9-8 to
		24-8-12, Exterior(2R) 24-8-12 to 28-6-4, Interior (1)
		28-6-4 to 38-10-8 zone; cantilever left and right
BRACING	Structural wood sheathing directly applied,	exposed ; end vertical left and right exposed;C-C for
TOP CHORD	except	members and forces & MWFRS for reactions shown;
	2-0-0 oc purlins (2-7-4 max.): 5-7.	Lumber DOL=1.60 plate grip DOL=1.33
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc	3) TCELL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
	bracing.	Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum
WEBS	1 Row at midpt 6-19, 6-16, 7-16	DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully
REACTIONS	(size) 2=0-3-8, 12=0-3-8	Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
	Max Horiz 2=-236 (LC 13)	4) Unbalanced snow loads have been considered for this
	Max Uplift 2=-161 (LC 15), 12=-89 (LC 16)	design.
	Max Grav 2=1741 (LC 54), 12=1790 (LC 34)	5) This truss has been designed for greater of min roof live
FORCES	(lb) - Maximum Compression/Maximum	load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on
	Tension	overhangs non-concurrent with other live loads.
TOP CHORD	1-2=0/48, 2-4=-2982/509, 4-5=-2852/496,	6) Provide adequate drainage to prevent water ponding.
	5-6=-2349/457, 6-7=-3237/584,	7) All plates are MT20 plates unless otherwise indicated.
	7-8=-3931/767, 8-10=-2921/578,	8) * This truss has been designed for a live load of 20.0psf
	10-12=-2987/495, 12-13=0/40	on the bottom chord in all areas where a rectangle
BOT CHORD	2-19=-337/2399, 18-19=-323/3045,	3-06-00 tall by 2-00-00 wide will fit between the bottom
	16-18=-323/3045, 14-16=-112/1890,	chord and any other members, with BCDL = 10.0psf.
	12-14=-282/2393	9) All bearings are assumed to be SP No.2 .
WEBS	4-19=-287/155, 5-19=-120/1197,	10) Provide mechanical connection (by others) of truss to
	6-19=-1205/186, 6-18=0/263, 6-16=-131/218,	bearing plate capable of withstanding 161 lb uplift at joint
	7-16=-2459/545, 8-16=-497/2989,	2 and 89 lb uplift at joint 12.
	8-14=-148/783, 10-14=-407/258	11) Graphical purlin representation does not depict the size
NOTES		or the orientation of the purlin along the top and/or
		bottom chord.

NOTES

1) Unbalanced roof live loads have been considered for this design.

LOAD CASE(S) Standard



August 27, 2025

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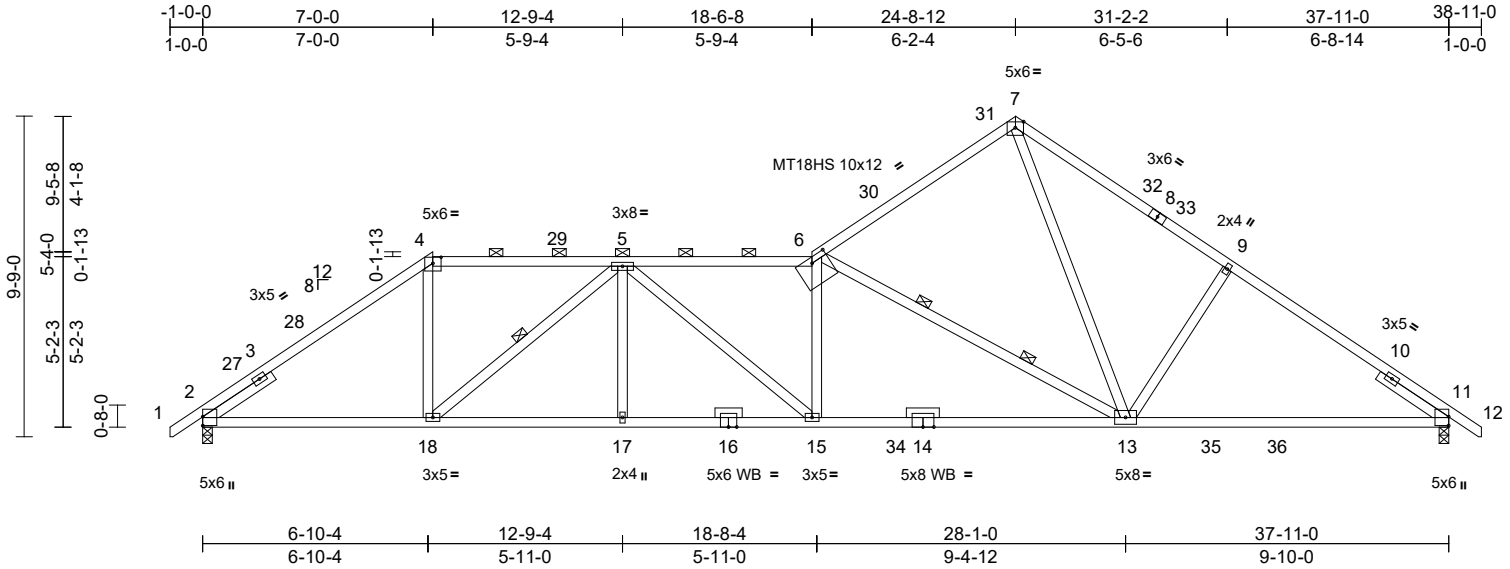
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925001
25060061-A	A8	Roof Special	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:44
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Page: 1



Scale = 1:70.1

Plate Offsets (X, Y): [4:0-3-0,0-2-3], [6:0-6-0,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.99	Vert(LL)	-0.31	13-15	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.61	13-15	>750	180	MT18HS	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(CT)	0.12	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 215 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 6-7,7-8:2x4 SP No.1
BOT CHORD 2x4 SP No.1
WEBS 2x4 SP No.3 *Except* 13-6:2x4 SP No.1
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied, except
2-0-0 oc purlins (2-2-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-18
WEBS 2 Rows at 1/3 pts 6-13

REACTIONS (size) 2=0-3-8, 11=0-3-8
Max Horiz 2=-236 (LC 13)
Max Uplift 2=-161 (LC 15), 11=-89 (LC 16)
Max Grav 2=1681 (LC 3), 11=1761 (LC 34)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-4=-2924/487, 4-5=-2384/464, 5-6=-4013/686, 6-7=-1859/409, 7-9=-2777/543, 9-11=-2790/497, 11-12=0/40
BOT CHORD 2-18=-313/2334, 17-18=-426/3533, 15-17=-426/3533, 13-15=-470/3929, 11-13=-282/2332
WEBS 4-18=-92/1235, 5-18=-1638/234, 5-17=0/207, 5-15=-62/594, 6-15=-125/221, 6-13=-2918/502, 7-13=-339/2176, 9-13=-409/248

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 7-0-0, Exterior(2R) 7-0-0 to 10-9-8, Interior (1) 10-9-8 to 24-8-12, Exterior(2R) 24-8-12 to 28-6-4, Interior (1) 28-6-4 to 38-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.1.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 2 and 89 lb uplift at joint 11.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

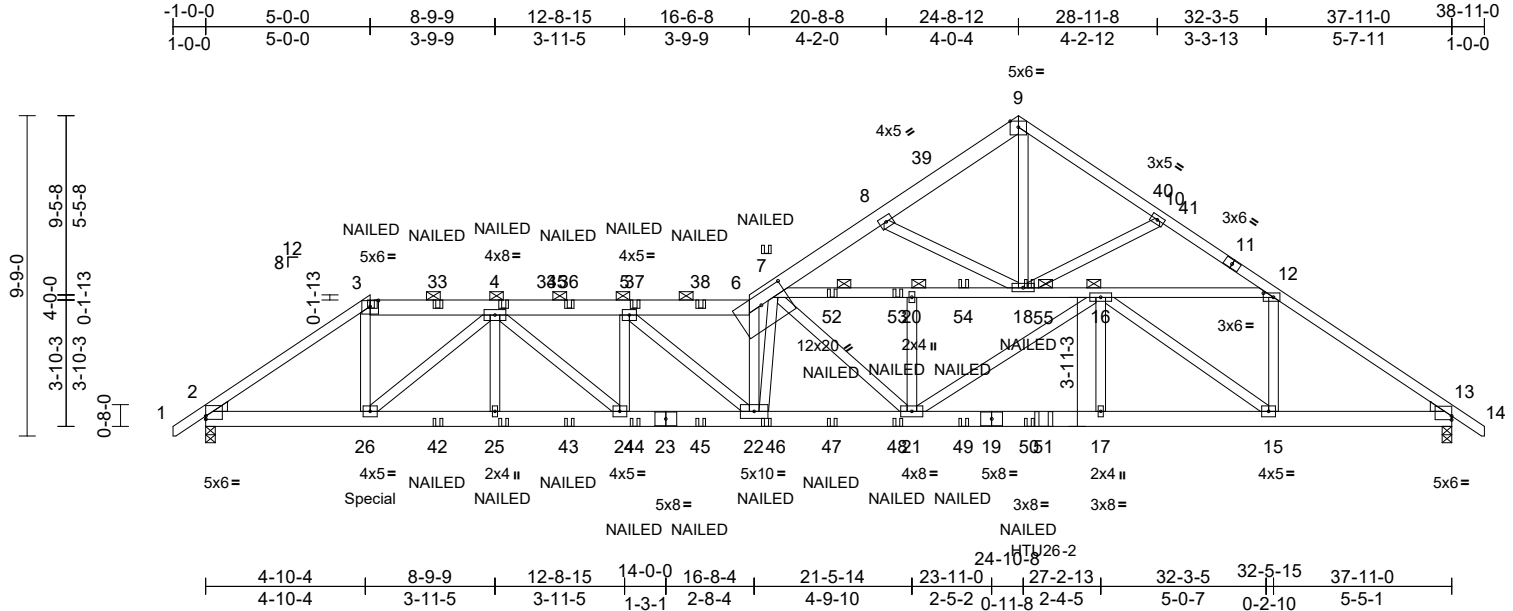
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175925002
25060061-A	A9	Roof Special Girder	1	2	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:45

Page: 1

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Scale = 1:70.1

Plate Offsets (X, Y): [2:Edge,0-1-2], [3:0-3-0,0-2-3], [6:0-10-0,0-4-0], [12:0-3-11,0-1-8], [13:Edge,0-1-6]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	-0.25	22	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.87	Vert(CT)	-0.53	21-22	>855	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(CT)	0.11	13	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 613 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 3-6,6-9;2x6 SP No.2
BOT CHORD 2x6 SP No.2 *Except* 7-12;2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or
4-7-4 oc purlins, except
2-0-0 oc purlins (4-8-9 max.): 3-6.
BOT CHORD Rigid ceiling directly applied or 4-1-7 oc
bracing.
JOINTS 1 Brace at Jt(s): 20,
16, 18

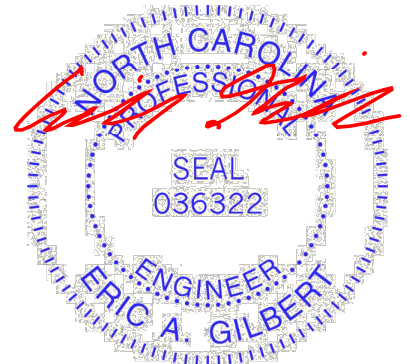
REACTIONS (size) 2=0-3-8, 13=0-3-8
Max Horiz 2=-239 (LC 9)
Max Uplift 2=-539 (LC 11), 13=-335 (LC 12)
Max Grav 2=3024 (LC 25), 13=2958 (LC 26)

FORCES (lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=0/48, 2-3=-4755/860, 3-4=-3804/740,
4-5=-8132/1291, 5-6=-8809/1273,
6-7=-9879/1423, 7-8=-1538/296,
8-9=-1226/277, 9-10=-1220/275,
10-12=-1474/300, 12-13=-4532/519,
13-14=0/40
BOT CHORD 2-26=-744/4009, 25-26=-1088/6351,
24-25=-1088/6351, 22-24=-1285/8066,
21-22=-1129/7803, 17-21=-592/5833,
15-17=-592/5833, 13-15=-334/3667,
7-20=-6390/788, 18-20=-6380/785,
16-18=-6291/800, 12-16=-2519/271

WEBS
3-26=-320/2382, 4-26=-3366/484,
4-25=0/236, 4-24=-281/2350,
5-24=-1268/219, 5-22=-44/888,
6-22=-4808/711, 12-15=-92/1404,
20-21=-292/127, 16-17=-75/821,
16-21=-386/1985, 15-16=-2731/327,
7-22=-658/4365, 7-21=-615/302,
9-18=-223/1131, 8-18=-486/152,
10-18=-383/143

NOTES
1) 2-ply truss to be connected together with 10d
(0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0
oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows
staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies,
except if noted as front (F) or back (B) face in the LOAD
CASE(S) section. Ply to ply connections have been
provided to distribute only loads noted as (F) or (B),
unless otherwise indicated.
3) Unbalanced roof live loads have been considered for
this design.
4) Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
II; Exp B; Enclosed; MWFRS (envelope); cantilever left
and right exposed ; end vertical left and right exposed;
Lumber DOL=1.60 plate grip DOL=1.33
5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum
DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully
Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
6) Unbalanced snow loads have been considered for this
design.
7) This truss has been designed for greater of min roof live
load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on
overhangs non-concurrent with other live loads.

- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 539 lb uplift at joint
2 and 335 lb uplift at joint 13.
- Graphical purlin representation does not depict the size
or the orientation of the purlin along the top and/or
bottom chord.
- Use Simpson Strong-Tie HTU26-2 (20-10d Girder,
14-10d Truss) or equivalent at 25-6-0 from the left end to
connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d
(0.148"x3.25") toe-nails per NDS guidelines.



August 27, 2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925002
25060061-A	A9	Roof Special Girder	1	2	Job Reference (optional)

16) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 393 lb down and 107 lb up at 5-0-0, and 49 lb down at 17-4-5 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-48, 3-6=-58, 6-9=-48, 9-14=-48, 27-30=-20, 7-12=-20
Concentrated Loads (lb)
Vert: 3=-49 (B), 26=-345 (B), 4=-45 (B), 25=-34 (B), 7=-60 (B), 18=-110 (B), 33=-45 (B), 36=-48 (B), 37=-95 (B), 38=-11 (B), 42=-34 (B), 43=-37 (B), 44=-52 (B), 45=-169 (B), 46=-38 (B), 47=-38 (B), 48=-38 (B), 49=-38 (B), 50=-38 (B), 51=-546 (B), 52=-110 (B), 53=-110 (B), 54=-110 (B)

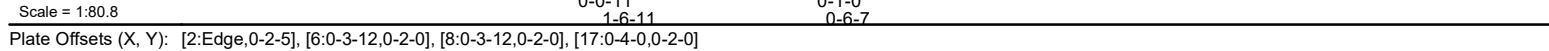
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818 Soundside Road
Edenton, NC 27932

Carter Components (Sanford, NC), Sanford, NC - 27332, Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:45 Page: 1
ID:hEhCDYARA6mKPxtvzGovrazeSOd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC7f



LUMBER		1) Unbalanced roof live loads have been considered for this design.
TOP CHORD	2x4 SP 2400F 2.0E *Except* 6-8:2x4 SP No.2, 1-5,9-13:2x4 SP No.1	2) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDDL=6.0psf; BCDDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-10-0, Interior (1) 2-10-0 to 14-3-12, Exterior(2R) 14-3-12 to 19-8-2, Interior (1) 19-8-2 to 23-7-4, Exterior(2R) 23-7-4 to 28-11-10, Interior (1) 28-11-10 to 38-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
BOT CHORD	2x4 SP No.2 *Except* 2-21:2x4 SP No.1	3) TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
WEBS	2x4 SP No.3 *Except* 6-25,6-17,8-17:2x4 SP No.2	4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
SLIDER	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0	5) 200.0lb AC unit load placed on the top chord, 14-3-12 from left end, supported at two points, 5-0-0 apart.
BRACING		6) Provide adequate drainage to prevent water ponding.
TOP CHORD	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-6-11 max.): 6-8.	7) All plates are 3x5 MT20 unless otherwise indicated.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.	8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDDL = 10.0psf
WEBS	1 Row at midpt 10-15, 6-17, 7-17	9) Bearings are assumed to be: Joint 2 SP No.1 , Joint 12 SP No.2 .
REACTIONS		10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
	(size) 2=0-3-8, 12=0-3-8	
	Max Horiz 2=-255 (LC 11)	
	Max Grav 2=2086 (LC 29), 12=1952 (LC 30)	
FORCES		
	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/40, 2-4=-3370/26, 4-6=-3181/159, 6-7=-2374/178, 7-8=-2347/173, 8-10=-2682/182, 10-12=-3156/141, 12-13=0/40	
BOT CHORD	2-26=-165/2712, 25-26=0/2712, 23-25=0/2185, 18-23=0/2829, 17-18=0/2225, 15-17=0/2035, 14-15=-19/2533, 12-14=-95/2533, 22-24=-917/0, 20-22=-917/0, 19-20=-917/0	
WEBS	4-26=0/154, 8-15=-48/605, 10-15=-614/205, 10-14=0/255, 24-25=-134/615, 6-24=-35/982, 6-19=-153/415, 17-19=-252/211, 4-25=-536/197, 8-17=-13/568, 7-17=-344/63, 22-23=-230/0, 23-24=0/894, 18-20=-217/0, 18-19=0/837	
LOAD CASE(S)		Standard

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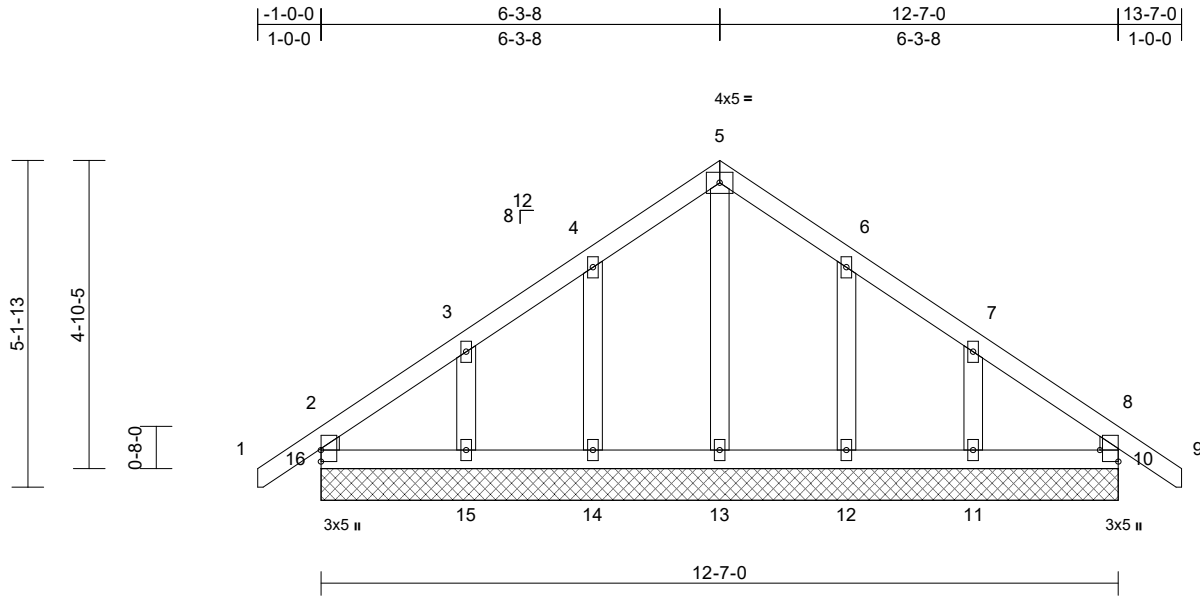
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925004
25060061-A	B1	Common Supported Gable	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:45
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Page: 1



Scale = 1:36.4

Plate Offsets (X, Y): [10:Edge,0-3-8]

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
										Weight: 66 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size)	10=12-7-0, 11=12-7-0, 12=12-7-0, 13=12-7-0, 14=12-7-0, 15=12-7-0, 16=12-7-0
Max Horiz	16=-133 (LC 11)
Max Uplift	10=-12 (LC 13), 11=-64 (LC 14), 12=-46 (LC 14), 14=-46 (LC 13), 15=-67 (LC 13), 16=-29 (LC 9)
Max Grav	10=154 (LC 36), 11=182 (LC 30), 12=172 (LC 30), 13=142 (LC 32), 14=171 (LC 29), 15=187 (LC 29), 16=154 (LC 35)

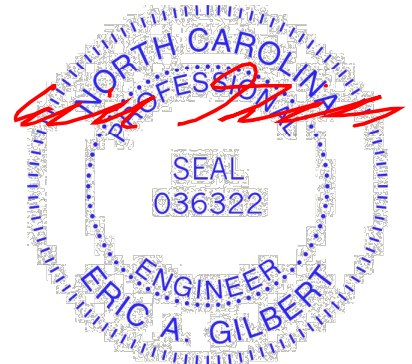
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-16=-160/165, 1-2=0/45, 2-3=-88/72, 3-4=-78/125, 4-5=-126/222, 5-6=-126/222, 6-7=-79/125, 7-8=-67/49, 8-9=0/45, 8-10=-159/166
BOT CHORD	15-16=-52/107, 14-15=-52/107, 13-14=-52/107, 12-13=-52/107, 11-12=-52/107, 10-11=-52/107
WEBS	5-13=-142/33, 4-14=-168/152, 3-15=-177/179, 6-12=-168/152, 7-11=-177/179

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-11-8 to 2-3-8, Exterior(2N) 2-3-8 to 6-3-8, Corner (3R) 6-3-8 to 9-3-8, Exterior(2N) 9-3-8 to 13-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 16, 12 lb uplift at joint 10, 46 lb uplift at joint 14, 67 lb uplift at joint 15, 46 lb uplift at joint 12 and 64 lb uplift at joint 11.

LOAD CASE(S) Standard



August 27, 2025

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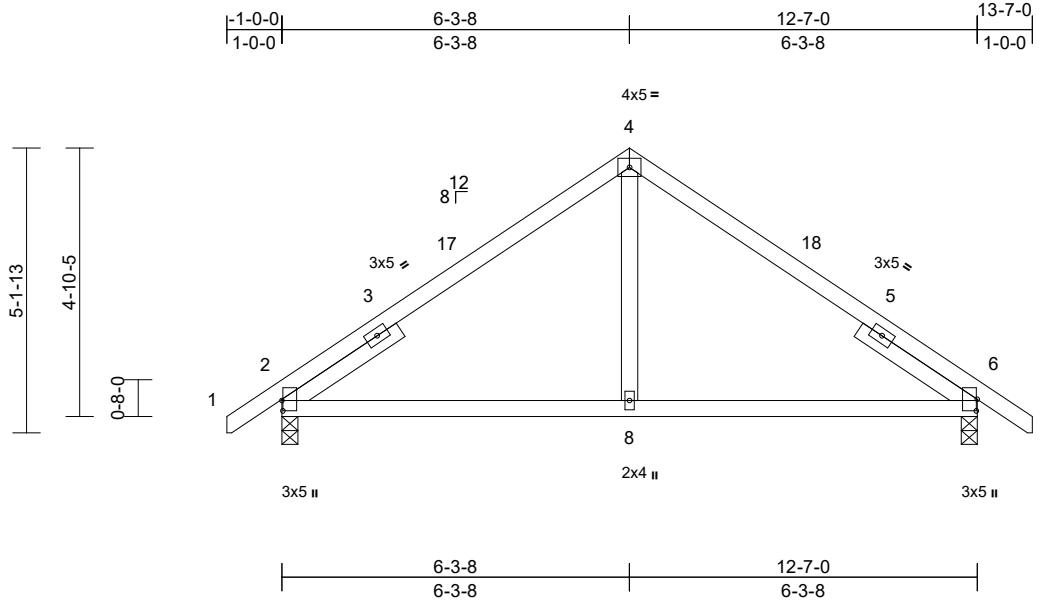
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925005
25060061-A	B2	Common	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:45
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Page: 1



Scale = 1:41.7

Plate Offsets (X, Y): [2:0-2-4,0-0-3], [6:0-2-9,0-0-3]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	0.05	8-11	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.38	Vert(CT)	-0.08	8-15	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 59 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-8, 6=0-3-8

Max Horiz	2=121 (LC 12)
Max Uplift	2=-48 (LC 13), 6=-48 (LC 14)
Max Grav	2=561 (LC 2), 6=561 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/40, 2-4=-584/218, 4-6=-584/218, 6-7=0/40
BOT CHORD	2-8=-200/441, 6-8=-211/441
WEBS	4-8=0/224

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 6-3-8, Exterior(2R) 6-3-8 to 9-3-8, Interior (1) 9-3-8 to 13-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 2 and 48 lb uplift at joint 6.

LOAD CASE(S) Standard



August 27, 2025

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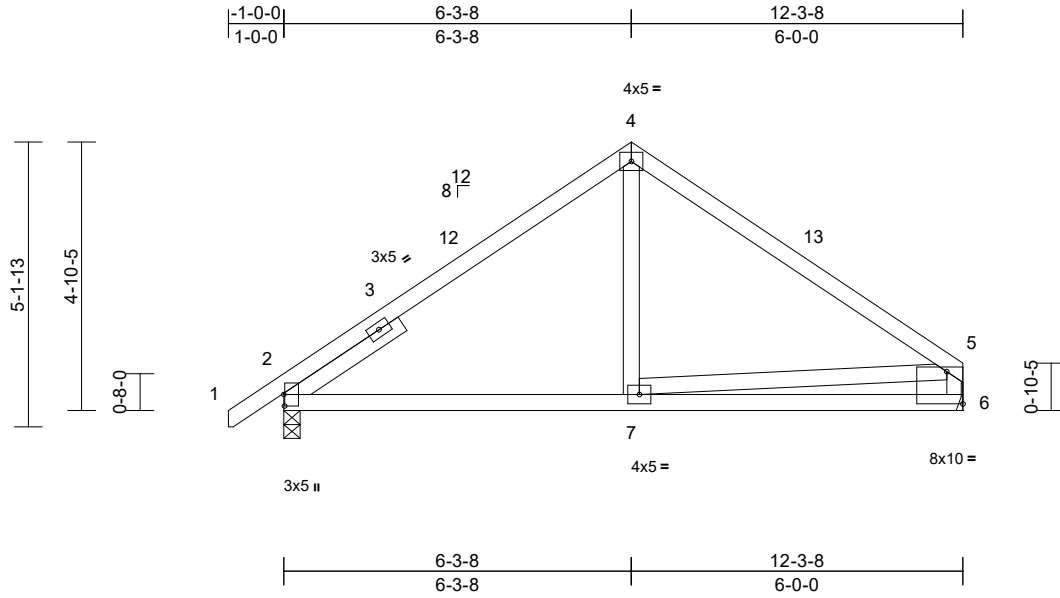
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925006
25060061-A	B3	Common	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:45
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Page: 1



Scale = 1:41.7

Plate Offsets (X, Y): [2:0-2-8,0-0-3], [6:Edge,0-7-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.47	Vert(LL)	0.05	7-10	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.38	Vert(CT)	-0.08	7-10	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 61 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=0-3-8, 6= Mechanical
Max Horiz 2=130 (LC 12)
Max Uplift 2=-47 (LC 13), 6=-30 (LC 14)
Max Grav 2=545 (LC 2), 6=484 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-4=-539/202, 4-5=-616/200, 5-6=-526/203

BOT CHORD 2-7=-250/419, 6-7=-123/256

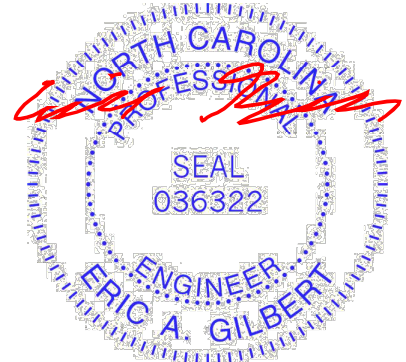
WEBS 4-7=0/162, 5-7=-64/248

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 6-3-8, Exterior(2R) 6-3-8 to 9-3-8, Interior (1) 9-3-8 to 12-1-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 6.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



August 27, 2025

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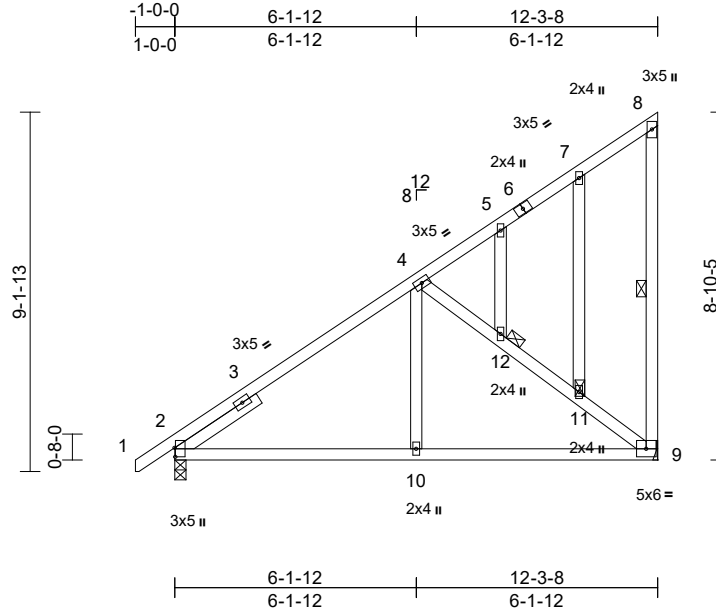
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925007
25060061-A	B4	Monopitch Structural Gable	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:58.6

Plate Offsets (X, Y): [2'-0"-2'-13", 0'-0"-3"]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.65	Vert(LL)	0.05	10-15	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.32	Vert(CT)	-0.07	9-10	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.48	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 88 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* 8-9:2x4 SP No.2
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 8'-11" oc bracing.
WEBS	1 Row at midpt 8-9
JOINTS	1 Brace at Jt(s): 11, 12

REACTIONS

(size)	2=0-3-8, 9= Mechanical
Max Horiz	2=334 (LC 12)
Max Uplift	2=-30 (LC 13), 9=-112 (LC 10)
Max Grav	2=545 (LC 2), 9=542 (LC 29)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/40, 2-4=-614/196, 4-5=-271/213, 5-7=-268/245, 7-8=-194/199, 8-9=-177/144
BOT CHORD	2-10=-554/635, 9-10=-390/635
WEBS	4-10=0/189, 4-12=-635/319, 11-12=-626/305, 9-11=-704/371, 7-11=-130/110, 5-12=-23/23

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 12-1-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Gable studs spaced at 2'-0" oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0"-0" tall by 2'-0"-0" wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 9.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



August 27, 2025

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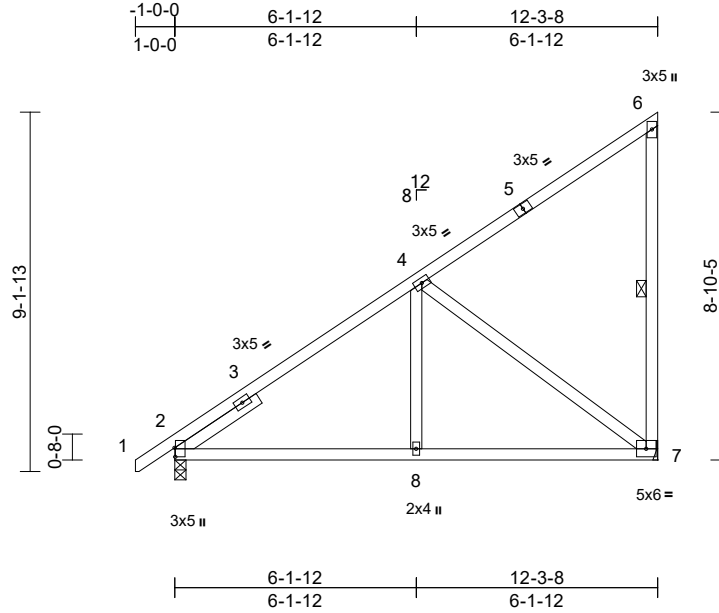
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925008
25060061-A	B5	Monopitch	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:58.6

Plate Offsets (X, Y): [2:0-2-13,0-0-3]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	0.05	8-11	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.30	Vert(CT)	-0.07	7-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 75 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 6-7:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-10-6 oc bracing.

WEBS 1 Row at midpt 6-7

REACTIONS (size) 2=0-3-8, 7= Mechanical
Max Horiz 2=334 (LC 12)
Max Uplift 2=-30 (LC 13), 7=-112 (LC 10)
Max Grav 2=545 (LC 2), 7=542 (LC 29)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-4=-549/199, 4-6=-256/231, 6-7=-257/210

BOT CHORD 2-8=-543/643, 7-8=-395/643

WEBS 4-8=0/187, 4-7=-656/331

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 12-1-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 7.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard



August 27, 2025

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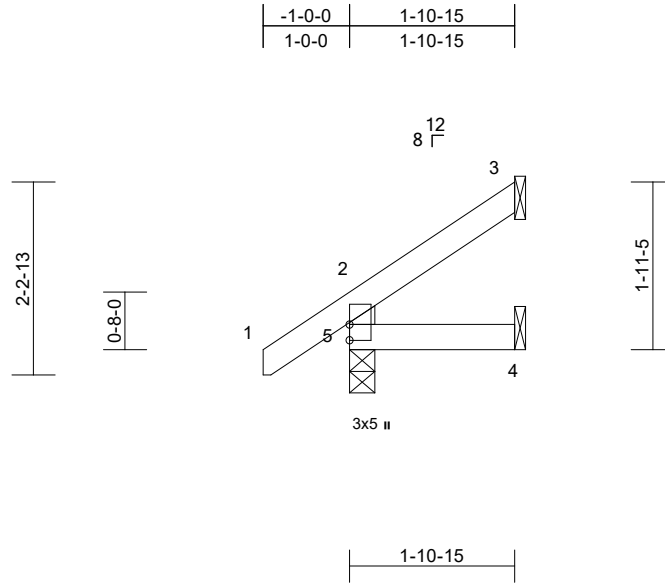
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175925009
25060061-A	CJ1	Jack-Open	2	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:46

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 9 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 3= Mechanical, 4= Mechanical,
5=0-3-8
Max Horiz 5=56 (LC 13)
Max Uplift 3=-30 (LC 13), 5=-7 (LC 13)
Max Grav 3=44 (LC 29), 4=19 (LC 11), 5=157 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-5=-157/149, 1-2=0/46, 2-3=-54/37
BOT CHORD 4-5=0/0

NOTES

- 1) Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- 5) Bearings are assumed to be: , Joint 5 SP No.2 .
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 5 and 30 lb uplift at joint 3.

LOAD CASE(S) Standard



August 27, 2025

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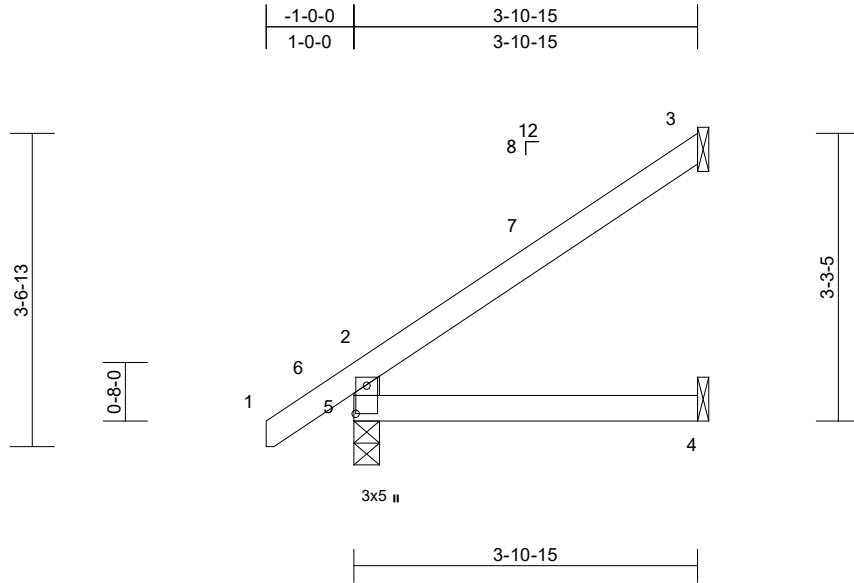
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925010
25060061-A	CJ2	Jack-Open	2	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:26.2

Plate Offsets (X, Y): [5:0-3-13,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	0.02	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	-0.02	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 15 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 3= Mechanical, 4= Mechanical, 5=0-3-8
Max Horiz 5=100 (LC 13)
Max Uplift 3=-62 (LC 13)
Max Grav 3=109 (LC 29), 4=46 (LC 29), 5=224 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-5=-214/163, 1-2=0/46, 2-3=-110/71
BOT CHORD 4-5=0/0

NOTES

- 1) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 3-10-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 5) Bearings are assumed to be: , Joint 5 SP No.2 .
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 3.

LOAD CASE(S) Standard



August 27, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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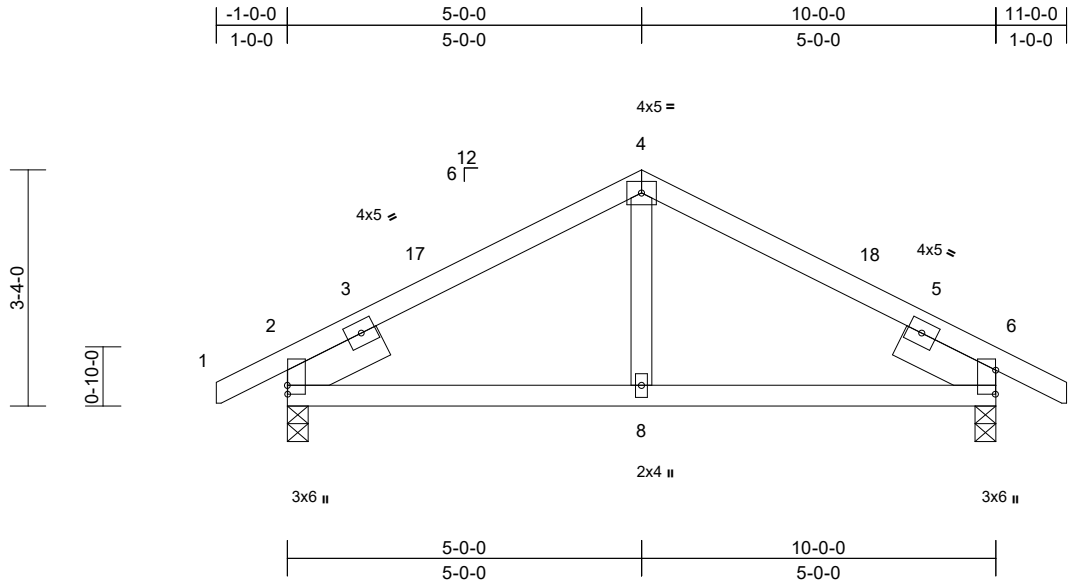
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925011
25060061-A	D1	Common	3	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:46
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Page: 1



Scale = 1:32.5

Plate Offsets (X, Y): [2:0-1-8,0-0-1], [6:0-4-1,0-0-1]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	-0.02	8-15	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.03	8-15	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 46 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP No.2 -- 1-6-0, Right 2x6 SP No.2 -- 1-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 6=0-3-8
Max Horiz 2=-41 (LC 13)
Max Uplift 2=-44 (LC 15), 6=-44 (LC 16)
Max Grav 2=458 (LC 2), 6=458 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-4=-487/300, 4-6=-488/299, 6-7=0/33
BOT CHORD 2-8=-196/396, 6-8=-192/396
WEBS 4-8=0/131

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-9 to 2-0-7, Interior (1) 2-0-7 to 5-0-0, Exterior(2R) 5-0-0 to 8-0-0, Interior (1) 8-0-0 to 10-11-9 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 2 and 44 lb uplift at joint 6.

LOAD CASE(S) Standard



August 27, 2025

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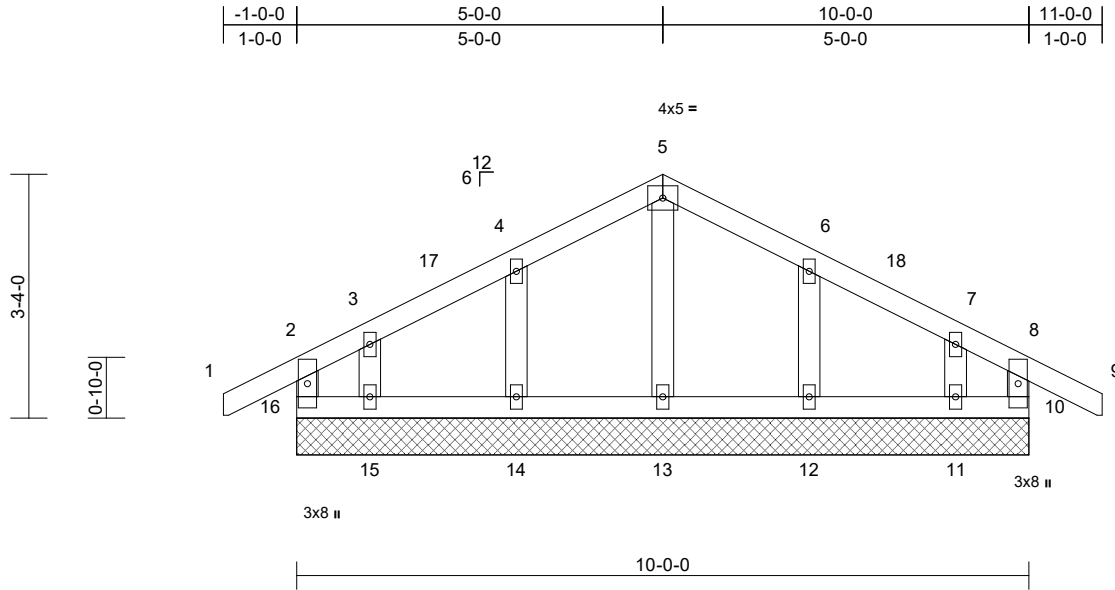
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175925012
25060061-A	D2	Common Supported Gable	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:31.5

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.03	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	10	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0									Weight: 48 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6'-0" oc bracing.

REACTIONS (size)	10=10'-0"-0, 11=10'-0"-0, 12=10'-0"-0, 13=10'-0"-0, 14=10'-0"-0, 15=10'-0"-0, 16=10'-0"-0
Max Horiz	16=62 (LC 13)
Max Uplift	10=-39 (LC 12), 11=-36 (LC 16), 12=-41 (LC 16), 14=-41 (LC 15), 15=-39 (LC 15), 16=-49 (LC 11)
Max Grav	10=121 (LC 21), 11=122 (LC 23), 12=215 (LC 23), 13=159 (LC 2), 14=215 (LC 22), 15=122 (LC 22), 16=121 (LC 21)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	2-16=-115/159, 1-2=0/37, 2-3=-29/35, 3-4=-23/73, 4-5=-58/173, 5-6=-57/173, 6-7=-24/75, 7-8=-17/26, 8-9=0/37, 8-10=-115/154
BOT CHORD	15-16=-29/90, 14-15=-29/90, 13-14=-29/90, 12-13=-29/90, 11-12=-29/90, 10-11=-29/90
WEBS	5-13=-119/0, 4-14=-173/200, 3-15=-108/126, 6-12=-173/200, 7-11=-106/124

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-11-9 to 2-0-7, Exterior(2N) 2-0-7 to 5-0-0, Corner (3R) 5-0-0 to 8-0-0, Exterior(2N) 8-0-0 to 10-11-9 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2'-0" oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-0"-0" tall by 2'-0"-0" wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 16, 39 lb uplift at joint 10, 41 lb uplift at joint 14, 39 lb uplift at joint 15, 41 lb uplift at joint 12 and 36 lb uplift at joint 11.

LOAD CASE(S) Standard



August 27, 2025

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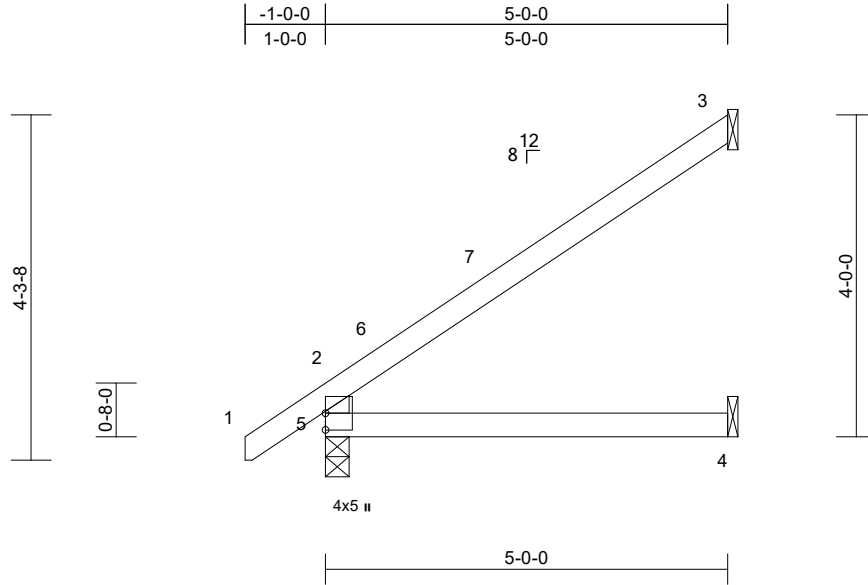
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925013
25060061-A	EJ1	Jack-Open	3	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:46

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Scale = 1:28.6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.51	Vert(LL)	0.04	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.35	Vert(CT)	-0.05	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.02	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 19 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 3= Mechanical, 4= Mechanical,
5=0-3-8
Max Horiz 5=125 (LC 13)
Max Uplift 3=-79 (LC 13)
Max Grav 3=143 (LC 29), 4=61 (LC 29),
5=265 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

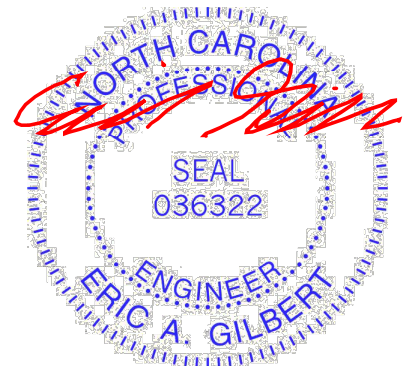
TOP CHORD 2-5=-250/172, 1-2=0/46, 2-3=-137/90
BOT CHORD 4-5=0/0

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 4-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- The Fabrication Tolerance at joint 5 = 12%, joint 5 = 12%

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 5 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 3.

LOAD CASE(S) Standard



August 27, 2025

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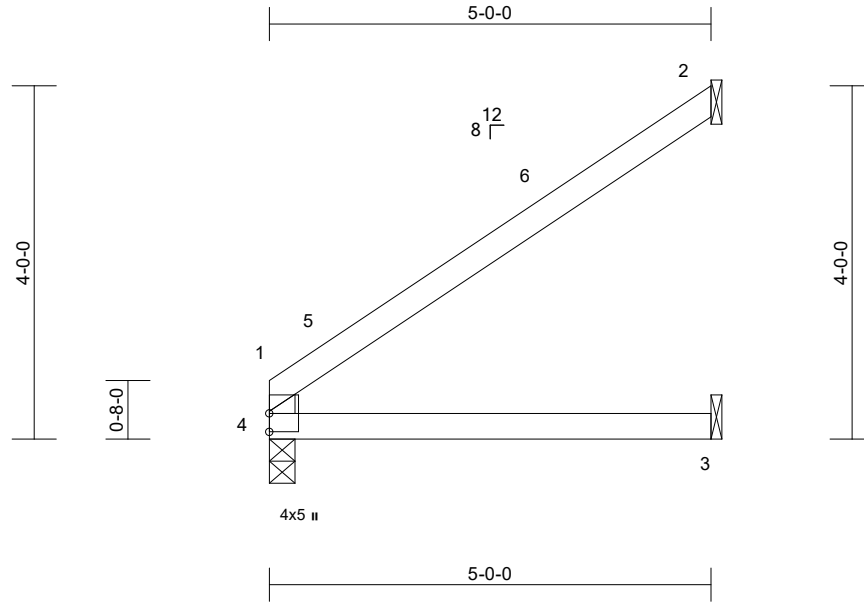
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925014
25060061-A	EJ2	Jack-Open	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:26.1

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.52	Vert(LL)	0.05	3-4	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.05	3-4	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.03	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 17 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
bracing.

REACTIONS (size) 2= Mechanical, 3= Mechanical,
4=0-3-8
Max Horiz 4=105 (LC 13)
Max Uplift 2=-80 (LC 13)
Max Grav 2=147 (LC 28), 3=64 (LC 28),
4=192 (LC 2)

FORCES (lb) - Maximum Compression/Maximum
Tension

TOP CHORD 1-4=-169/83, 1-2=-138/91
BOT CHORD 3-4=0/0

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
II; Exp B; Enclosed; MWFRS (envelope) and C-C
Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12 to
4-11-4 zone; cantilever left and right exposed ; end
vertical left and right exposed;C-C for members and
forces & MWFRS for reactions shown; Lumber
DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum
DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully
Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- The Fabrication Tolerance at joint 4 = 8%, joint 4 = 8%
- * This truss has been designed for a live load of 20.0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members.
- Bearings are assumed to be: , Joint 4 SP No.2 .

- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 80 lb uplift at joint
2.

LOAD CASE(S) Standard



August 27, 2025

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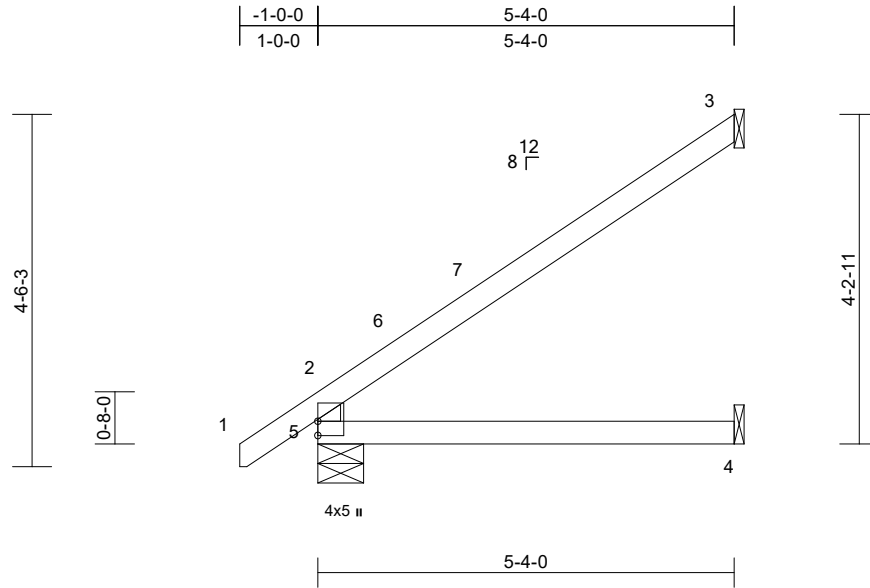
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925015
25060061-A	EJ3	Jack-Open	5	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:29.5

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.56	Vert(LL)	0.05	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.39	Vert(CT)	-0.07	4-5	>875	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.03	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 20 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 3= Mechanical, 4= Mechanical, 5=0-7-0
Max Horiz 5=132 (LC 13)
Max Uplift 3=-84 (LC 13)
Max Grav 3=153 (LC 29), 4=66 (LC 29), 5=278 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

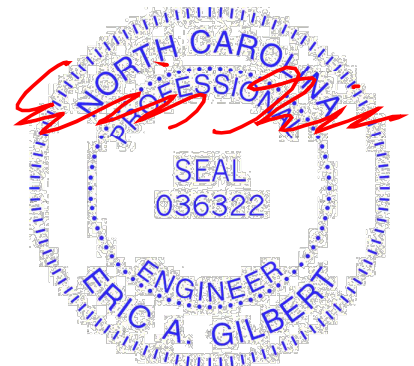
TOP CHORD 2-5=-261/175, 1-2=0/46, 2-3=-145/96
BOT CHORD 4-5=0/0

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 5-3-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- The Fabrication Tolerance at joint 5 = 4%, joint 5 = 4%

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 5 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3.

LOAD CASE(S) Standard



August 27, 2025

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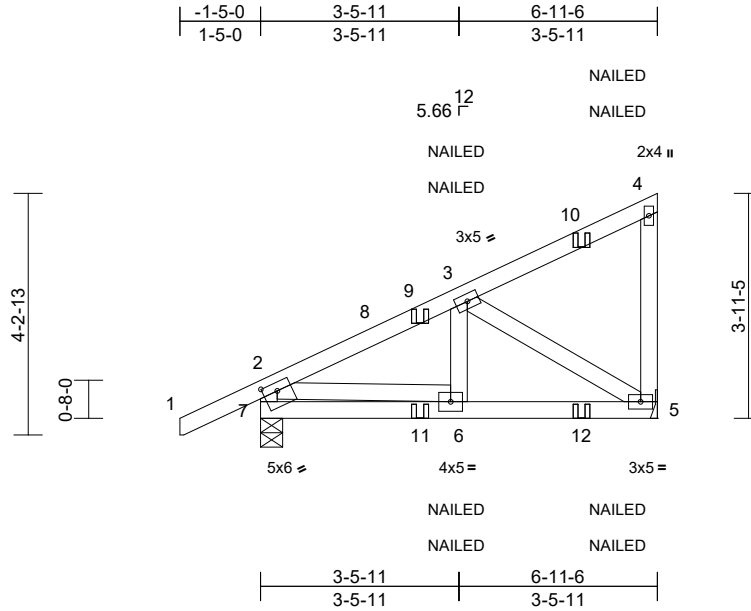
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925016
25060061-A	HJ1	Diagonal Hip Girder	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:46
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Page: 1



Scale = 1:40.4

Plate Offsets (X, Y): [7:0-3-0,0-1-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	0.00	5-6	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.01	5-6	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.13	Horz(CT)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 42 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS

(size) 5= Mechanical, 7=0-4-9
Max Horiz 7=153 (LC 8)
Max Uplift 5=-106 (LC 8), 7=-54 (LC 11)
Max Grav 5=357 (LC 25), 7=378 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-7=-354/67, 1-2=0/49, 2-3=-425/52,
3-4=-113/72, 4-5=-122/87
BOT CHORD 6-7=-151/22, 5-6=-95/317
WEBS 2-6=-22/373, 3-6=0/82, 3-5=-358/97

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06-00 tall by 2'-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 7 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 7 and 106 lb uplift at joint 5.
- "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidelines.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-48, 2-4=-48, 5-7=-20
Concentrated Loads (lb)
Vert: 10=-31 (F=-16, B=-16), 11=3 (F=1, B=1), 12=-28 (F=-14, B=-14)



August 27, 2025

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818 Soundside Road
Edenton, NC 27932

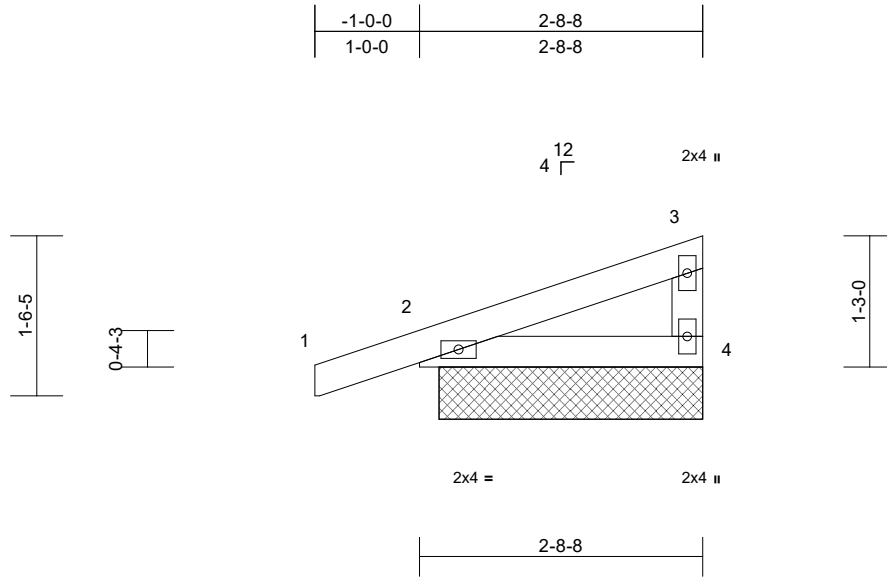
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175925017
25060061-A	M1	Monopitch Supported Gable	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:46

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Scale = 1:22

Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 11 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=2-6-4, 4=2-6-4
Max Horiz 2=40 (LC 14)
Max Uplift 2=-54 (LC 11), 4=-10 (LC 15)
Max Grav 2=185 (LC 22), 4=99 (LC 22)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/23, 2-3=-55/40, 3-4=-83/113
BOT CHORD 2-4=-27/67

NOTES

- 1) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-11-11 to 2-0-5, Exterior(2N) 2-0-5 to 2-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 .
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 4, 54 lb uplift at joint 2 and 54 lb uplift at joint 2.
- 10) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



August 27, 2025

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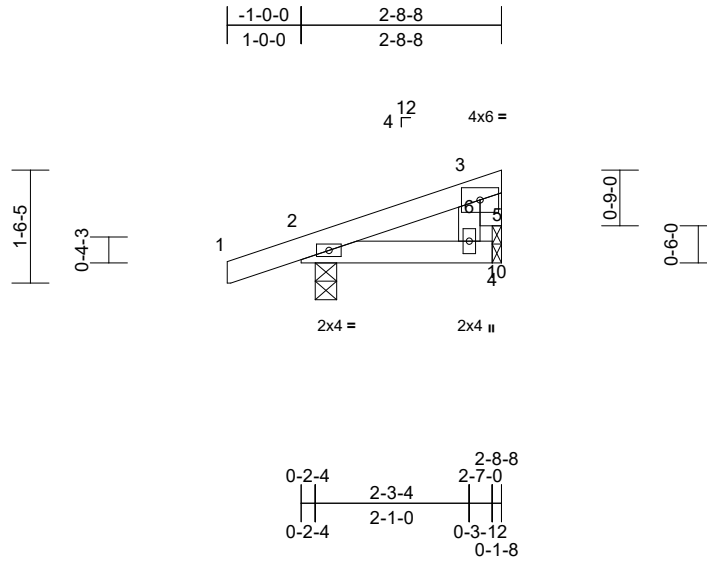
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925018
25060061-A	M2	Monopitch	3	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	0.00	5-9	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	0.00	5-9	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 11 lb	FT = 20%

LUMBER	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 2-8-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS	(size) 2=0-3-8, 10=0-1-8 Max Horiz 2=34 (LC 11) Max Uplift 2=-84 (LC 11), 10=-25 (LC 11) Max Grav 2=190 (LC 22), 10=78 (LC 22)
FORCES	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/23, 2-3=-86/73, 5-6=-41/39, 3-6=-31/26
BOT CHORD	2-5=-105/84, 4-5=0/0
WEBS	3-10=-49/58

- NOTES**
- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearings are assumed to be: Joint 2 SP No.2, Joint 10 SP No.3.
 - Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate at joint(s) 10.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 2 and 25 lb uplift at joint 10.
- LOAD CASE(S)** Standard



August 27, 2025

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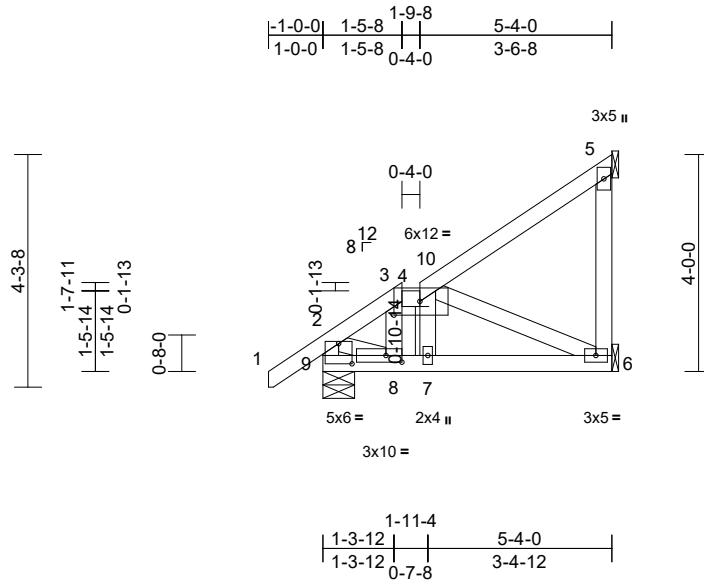
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175925019
25060061-A	M3	Roof Special	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:47
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Page: 1



Scale = 1:42.5

Plate Offsets (X, Y): [3:0-5-12,0-3-0], [8:0-3-8,0-1-8], [9:0-3-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	-0.01	6-7	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	-0.01	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(CT)	-0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 34 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 9-6-11 oc bracing.

REACTIONS (size) 5= Mechanical, 6= Mechanical,
8=0-7-0, 9=0-7-0
Max Horiz 9=150 (LC 12)
Max Uplift 5=-58 (LC 12), 8=-43 (LC 12),
9=-24 (LC 11)
Max Grav 5=159 (LC 56), 6=72 (LC 44),
8=165 (LC 56), 9=253 (LC 44)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/55, 2-3=-224/102, 3-4=-161/55,
4-5=-174/149, 5-6=0/0, 2-9=-344/150
BOT CHORD 8-9=-380/221, 7-8=-108/164, 6-7=-110/155
WEBS 3-8=-99/134, 4-7=-153/78, 4-6=-129/68,
2-8=-90/307

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
II; Exp B; Enclosed; MWFRS (envelope) and C-C
Exterior(2E) -0-11-8 to 1-9-8, Interior (1) 1-9-8 to 5-2-4
zone; cantilever left and right exposed; end vertical left
and right exposed; C-C for members and forces &
MWFRS for reactions shown; Lumber DOL=1.60 plate
grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum
DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully
Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0

- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 8 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 9, 43 lb uplift at joint 8 and 58 lb uplift at joint 5.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard



August 27, 2025

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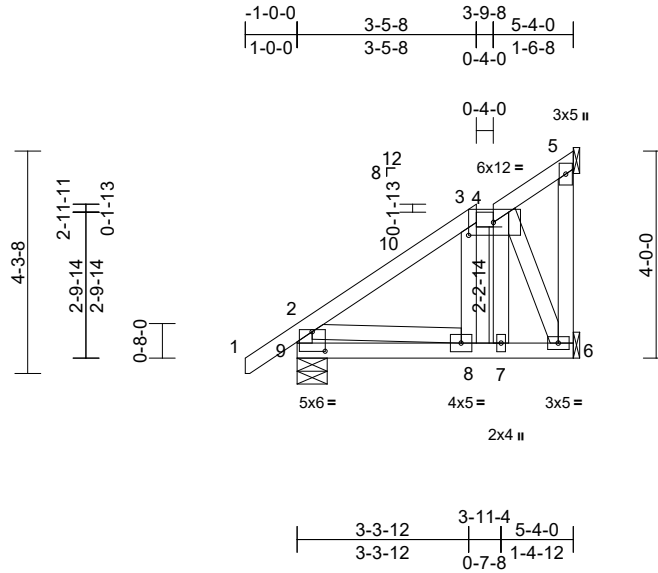
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925020
25060061-A	M4	Roof Special	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:47
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Page: 1



Scale = 1:44.5

Plate Offsets (X, Y): [3:0-5-12,0-3-0], [9:0-3-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.27	Vert(LL)	0.01	8-9	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	-0.01	8-9	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(CT)	-0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 41 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 9-5-1 oc bracing.

REACTIONS (size) 5= Mechanical, 6= Mechanical, 9=0-7-0
Max Horiz 9=150 (LC 12)
Max Uplift 5=-36 (LC 12), 6=-39 (LC 15), 9=-24 (LC 15)
Max Grav 5=70 (LC 56), 6=189 (LC 44), 9=381 (LC 44)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/55, 2-3=-235/53, 3-4=-164/93, 4-5=-116/106, 5-6=0/0, 2-9=-380/199
BOT CHORD 8-9=-384/221, 7-8=-143/186, 6-7=-154/200
WEBS 3-8=-140/166, 4-7=-198/247, 4-6=-318/203, 2-8=-58/253

NOTES

- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-8 to 2-0-8, Interior (1) 2-0-8 to 3-5-8, Exterior(2E) 3-5-8 to 3-9-8, Interior (1) 3-9-8 to 5-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 9 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 6, 24 lb uplift at joint 9 and 36 lb uplift at joint 5.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard



August 27, 2025

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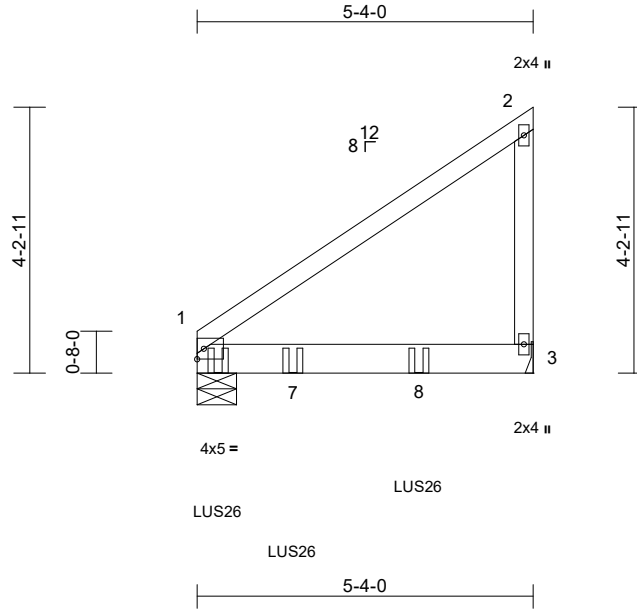
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH 175925021
25060061-A	M5	Jack-Closed Girder	1	2	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:47
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Scale = 1:36.6

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	0.04	3-6	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.07	3-6	>832	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.01	1	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 55 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5'-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS (size) 1=0-7-8, 3= Mechanical
Max Horiz 1=137 (LC 8)
Max Uplift 1=-169 (LC 9), 3=-171 (LC 6)
Max Grav 1=1149 (LC 21), 3=753 (LC 20)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-464/144, 2-3=-174/69
BOT CHORD 1-3=-237/119

NOTES

- 2-ply truss to be connected together as follows:
Top chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected with 10d (0.131"x3") nails as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-16; Vult=150mph (3-second gust)
Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed;
Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06-00 tall by 2'-00-00 wide will fit between the bottom chord and any other members.
 - Bearings are assumed to be: Joint 1 SP No.2 .
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 1 and 171 lb uplift at joint 3.
 - Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2'-0-0 oc max. starting at 0'-4-0 from the left end to 3'-6-4 to connect truss(es) to back face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-46, 3-4=-19
Concentrated Loads (lb)
Vert: 6=-397 (B), 7=-390 (B), 8=-390 (B)



August 27, 2025

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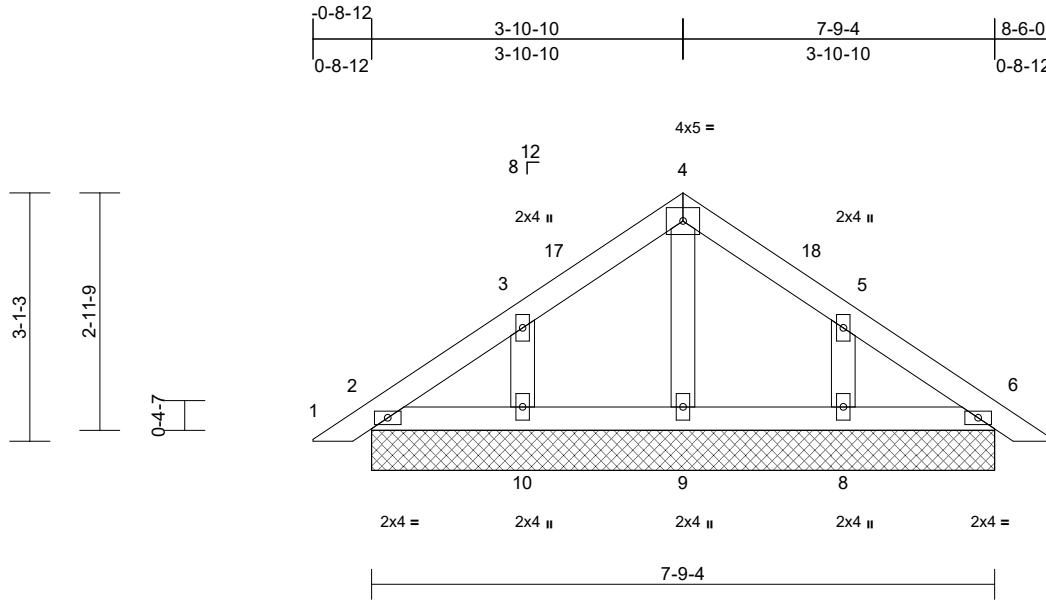
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925022
25060061-A	PB1	Piggyback	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Aug 13 2025 Print: 8.730 S Aug 13 2025 MiTek Industries, Inc. Tue Aug 26 13:57:47
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Loading	(psf)	Spacing	1-11-4	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.04	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	6	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0									Weight: 35 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=7-9-4, 6=7-9-4, 8=7-9-4, 9=7-9-4, 10=7-9-4
Max Horiz 2=-71 (LC 11)
Max Uplift 2=-5 (LC 9), 6=-1 (LC 14), 8=-61 (LC 14), 10=-61 (LC 13)
Max Grav 2=98 (LC 2), 6=98 (LC 2), 8=194 (LC 30), 9=107 (LC 2), 10=195 (LC 29)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-3=-53/49, 3-4=-91/131, 4-5=-90/131, 5-6=-35/27, 6-7=0/20
BOT CHORD 2-10=-32/108, 9-10=-32/108, 8-9=-32/108, 6-8=-32/108
WEBS 4-9=-71/0, 3-10=-199/240, 5-8=-199/240

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) 0-3-5 to 3-3-5, Exterior(2N) 3-3-5 to 4-7-12, Corner (3R) 4-7-12 to 7-7-12, Exterior(2N) 7-7-12 to 9-0-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 2, 1 lb uplift at joint 6, 61 lb uplift at joint 10, 61 lb uplift at joint 8, 5 lb uplift at joint 2 and 1 lb uplift at joint 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



August 27, 2025

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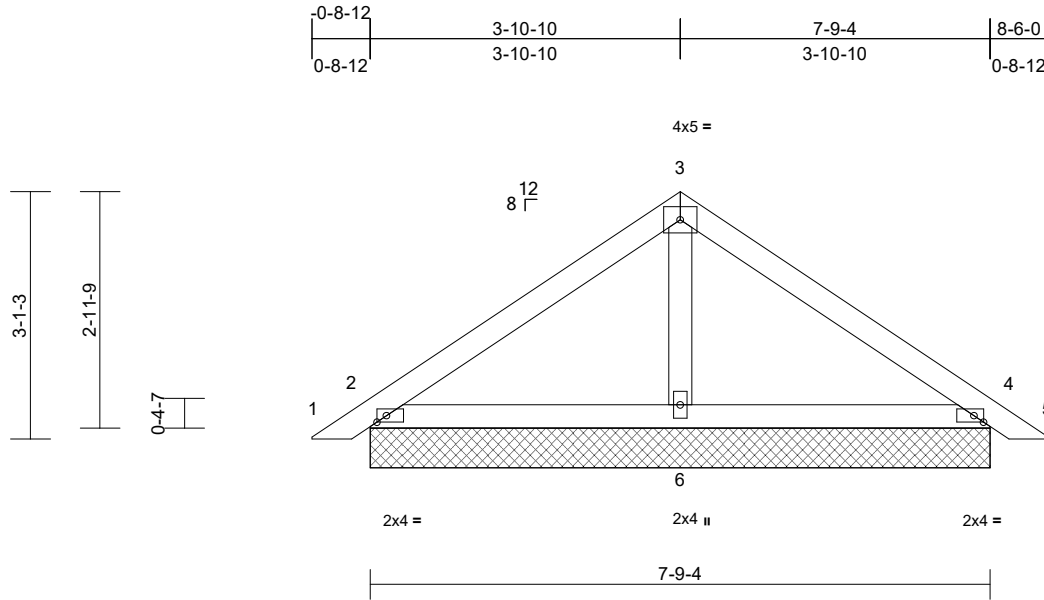
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925023
25060061-A	PB2	Piggyback	9	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:28.9

Plate Offsets (X, Y): [2:0-1-7,Edge], [4:0-1-7,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	2	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										
										Weight: 32 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=7-9-4, 4=7-9-4, 6=7-9-4
Max Horiz 2=-73 (LC 11)
Max Uplift 2=-35 (LC 13), 4=-42 (LC 14)
Max Grav 2=212 (LC 2), 4=212 (LC 2), 6=256 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

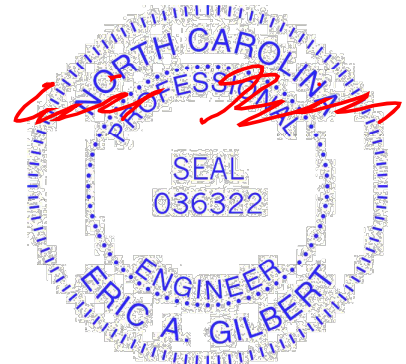
TOP CHORD 1-2=0/20, 2-3=-181/133, 3-4=-181/134, 4-5=0/20
BOT CHORD 2-6=-28/78, 4-6=-32/84
WEBS 3-6=-109/26

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-3-5 to 3-3-5, Interior (1) 3-3-5 to 4-7-12, Exterior(2R) 4-7-12 to 7-7-12, Interior (1) 7-7-12 to 9-0-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 2, 42 lb uplift at joint 4, 35 lb uplift at joint 2 and 42 lb uplift at joint 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



August 27, 2025

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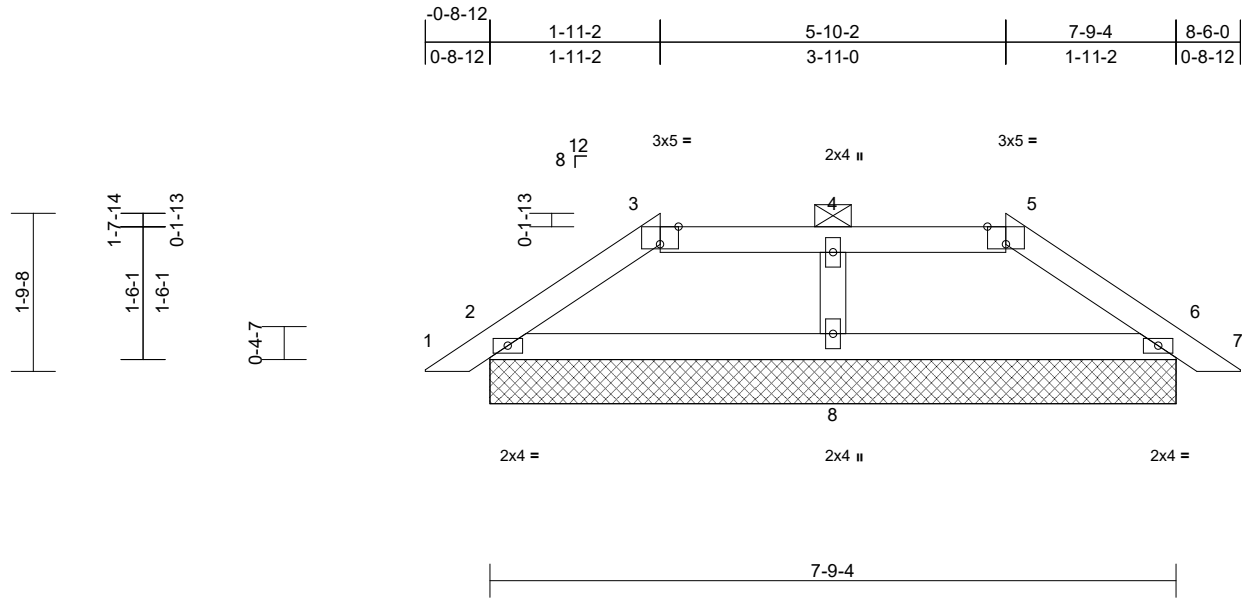
Job	Truss	Truss Type	Qty	Ply	572 Creekside Oaks North-Roof-Prelude B CP GLH I75925024
25060061-A	PB3	Piggyback	1	1	Job Reference (optional)

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:26.1

Plate Offsets (X, Y): [3:0-2-8,Edge], [5:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	n/a	-	n/a	999	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	13	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										
										Weight: 28 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 3-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=7-9-4, 6=7-9-4, 8=7-9-4
Max Horiz 2=-38 (LC 13)
Max Uplift 2=-26 (LC 15), 6=-27 (LC 16), 8=-8 (LC 12)
Max Grav 2=259 (LC 44), 6=259 (LC 44), 8=321 (LC 43)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/24, 2-3=-220/124, 3-4=-182/127, 4-5=-182/127, 5-6=-220/124, 6-7=0/24
BOT CHORD 2-8=-33/160, 6-8=-33/160
WEBS 4-8=-247/133

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 2, 27 lb uplift at joint 6, 8 lb uplift at joint 8, 26 lb uplift at joint 2 and 27 lb uplift at joint 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



August 27, 2025

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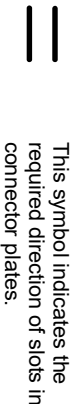
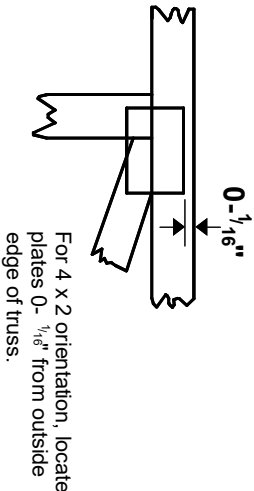
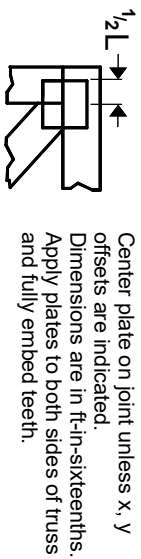
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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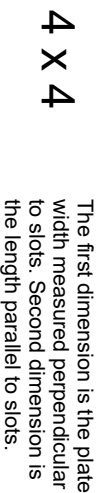
Symbols

PLATE LOCATION AND ORIENTATION

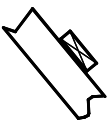


* Plate location details available in MITek software or upon request.

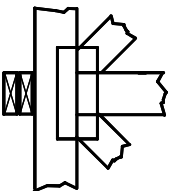
PLATE SIZE



LATERAL BRACING LOCATION

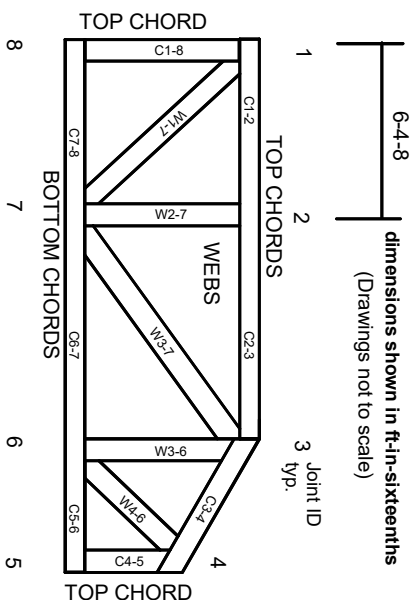


BEARING



Industry Standards:
ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-EES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.