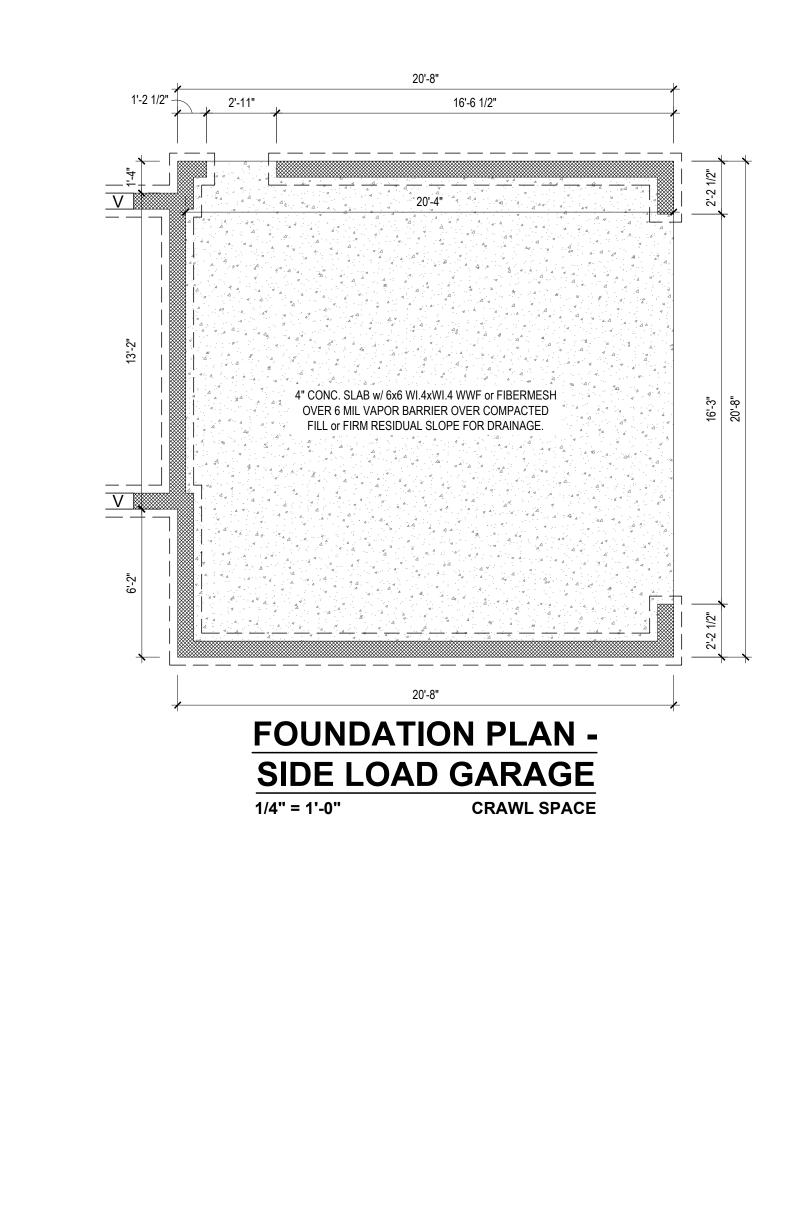


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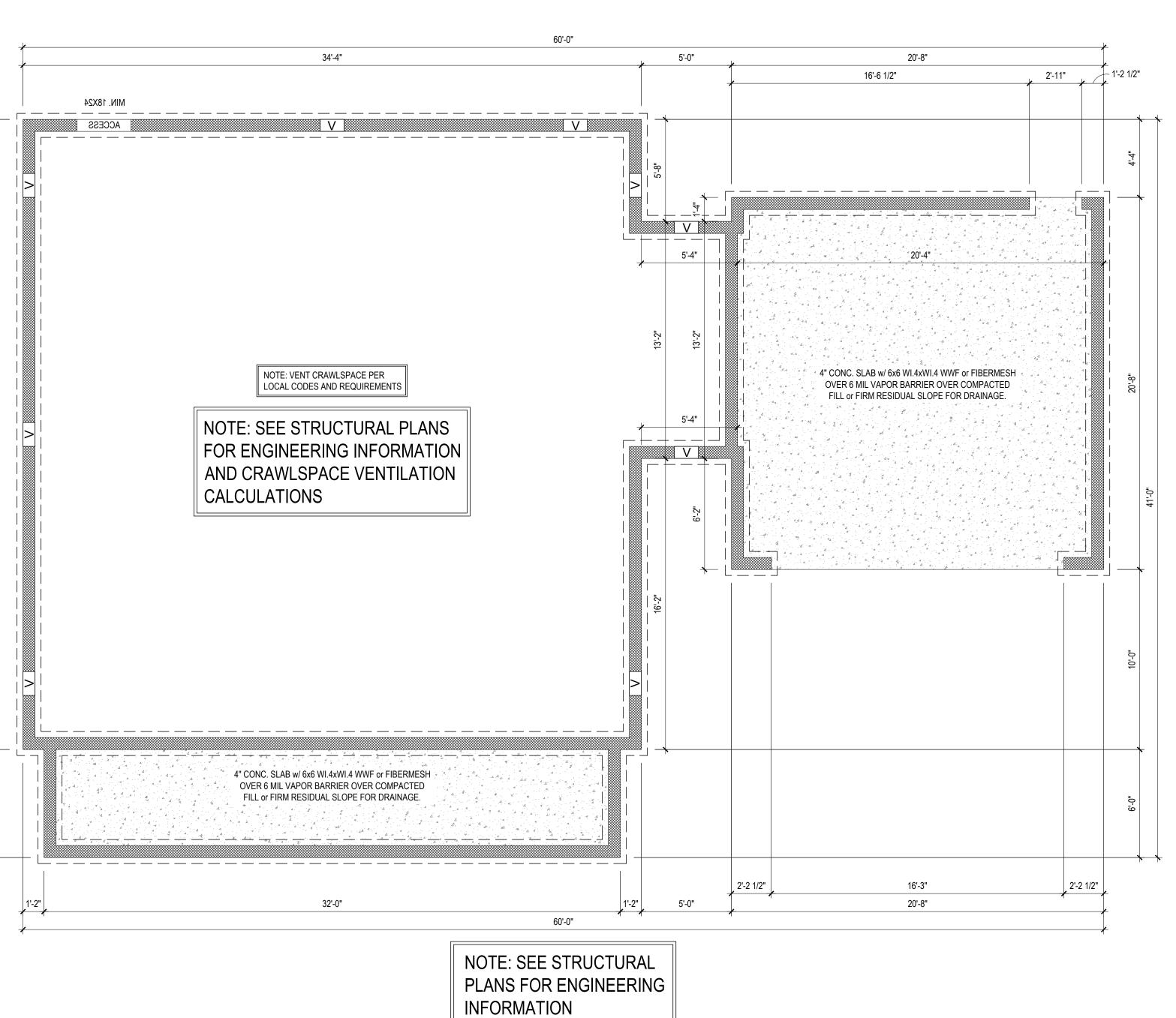
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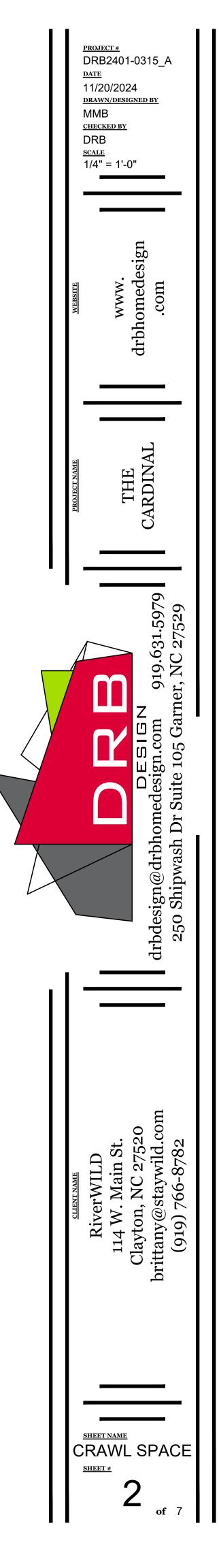
- DRB DESIGN assumes no liability for any home constructed from this plan.
   All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code",
- in addition to all local codes and regulations.3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the
- services of a structural engineer after notifying DRB DESIGN that such services are required.Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.
- 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- Communication is imperfect and every contingency cannot be anticipated.
   Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB
- DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.
  8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences.
- Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB DESIGN of responsibility for any and all consequences arriving out of such changes.
   Written dimensions on these plans always have precedence over scaled dimensions.
- 11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square footage errors once construction has begun.
- 12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

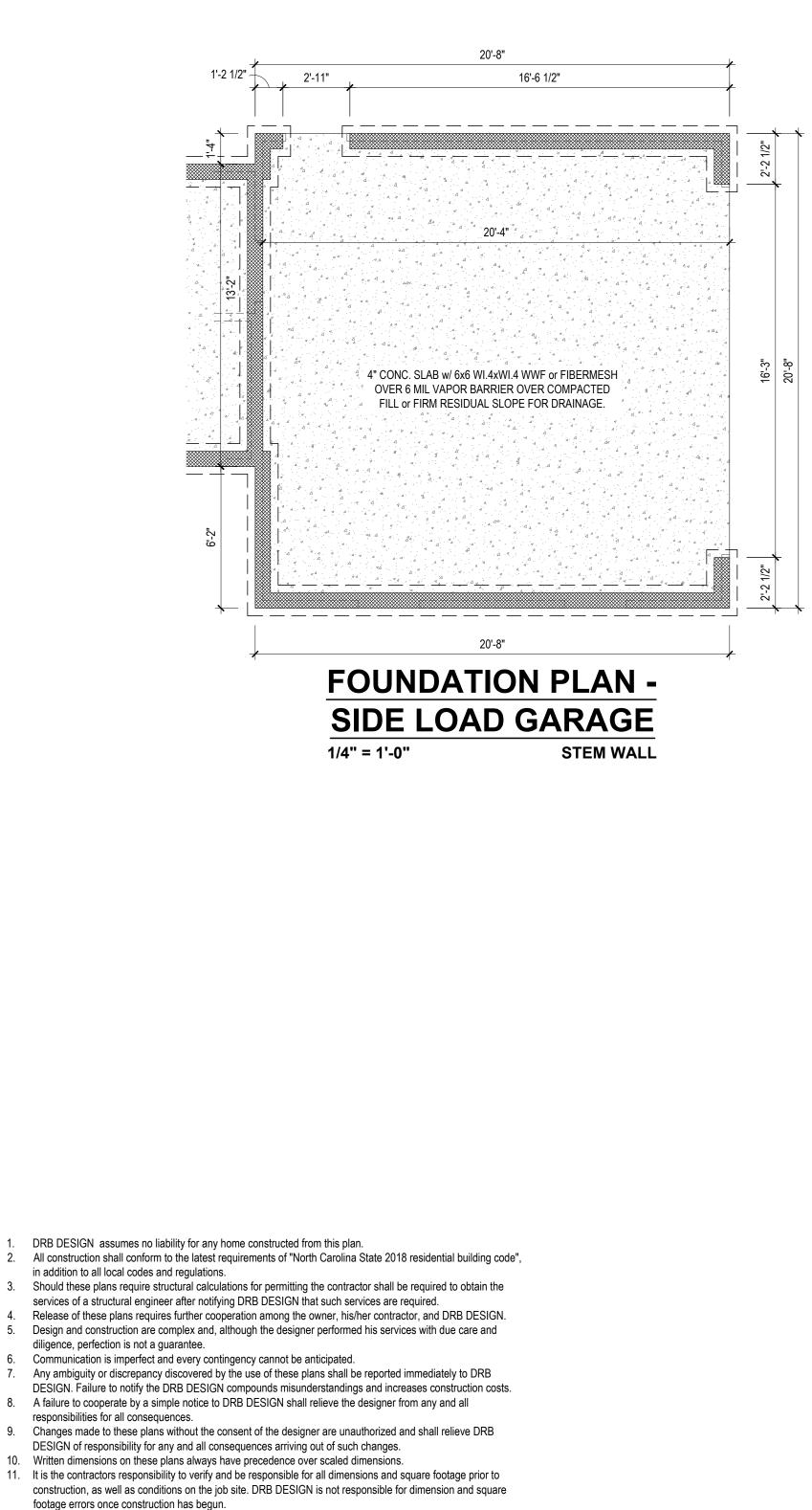


FOUNDATION PLAN

1/4" = 1'-0"

CRAWL SPACE

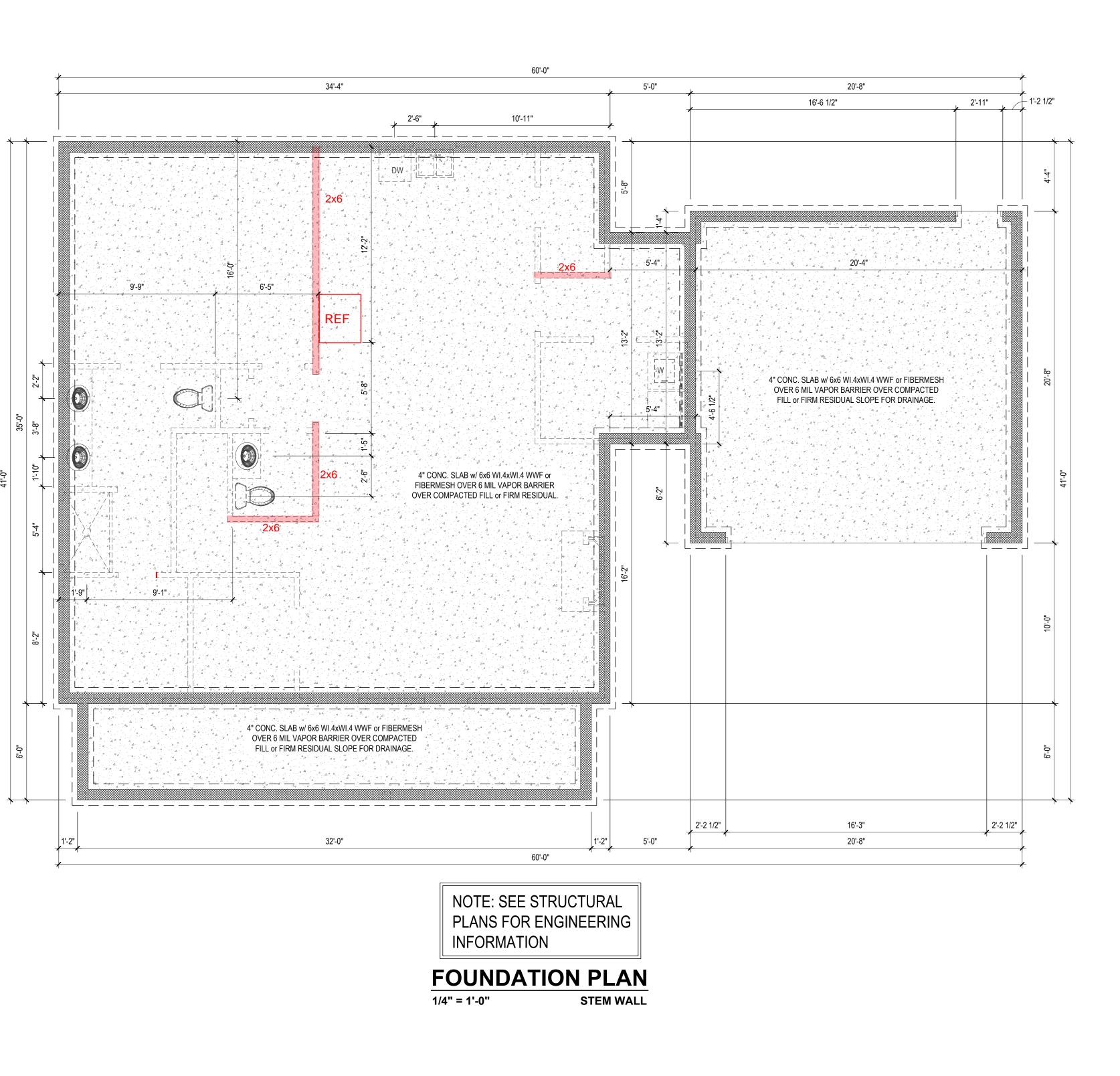


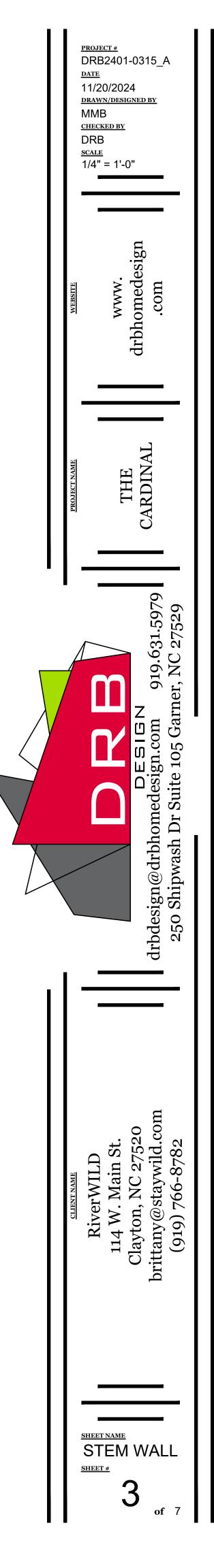


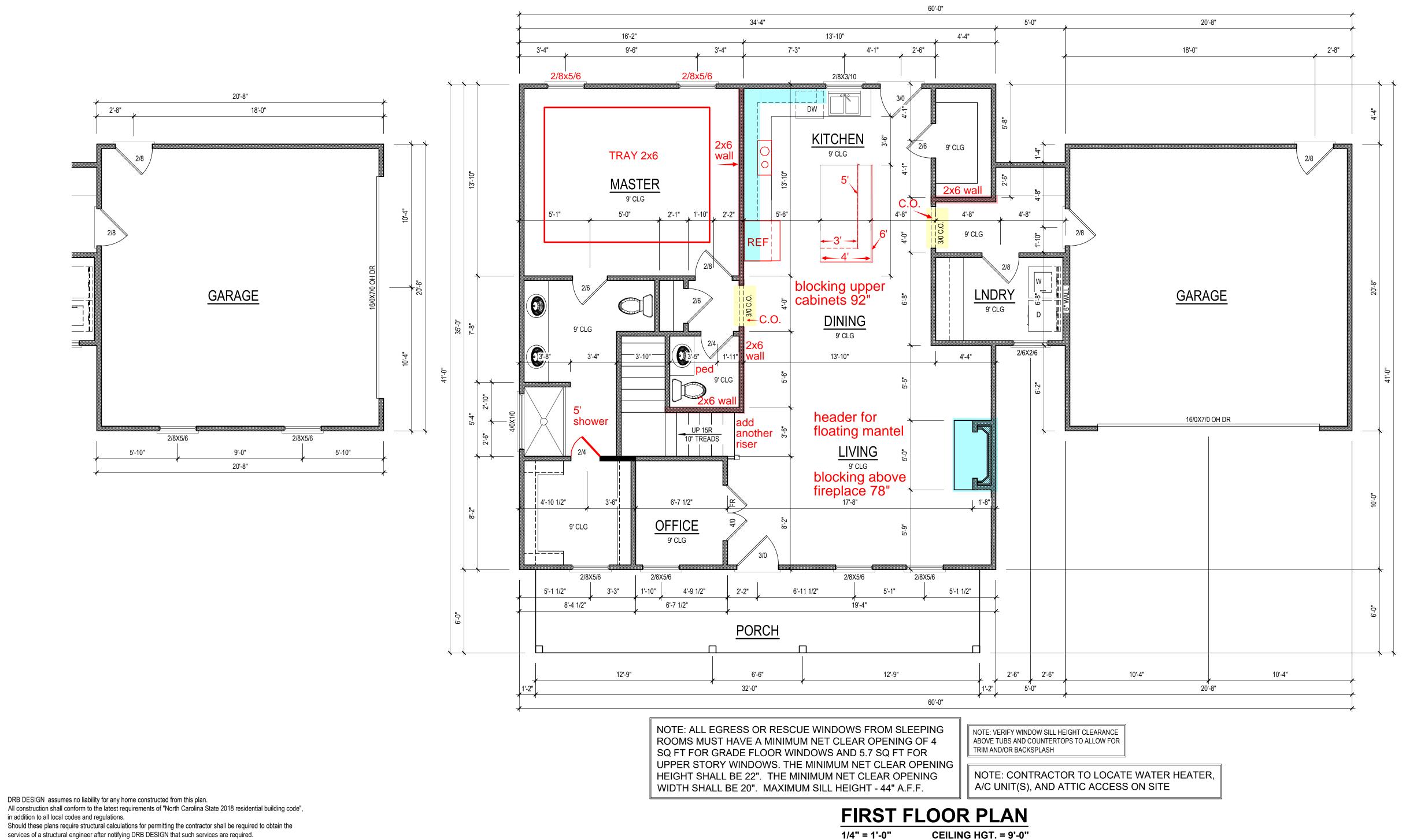
12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

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9.







DRB DESIGN assumes no liability for any home constructed from this plan.

- in addition to all local codes and regulations.
- services of a structural engineer after notifying DRB DESIGN that such services are required.
- Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN. Design and construction are complex and, although the designer performed his services with due care and
- diligence, perfection is not a guarantee. Communication is imperfect and every contingency cannot be anticipated.
- Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all
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- Z:\Raleigh Office\DR8\DR8\_2024\DR82401-0315\_RIVERWILD-CARDINAL\DR82401-0315\_A\_RIVERWILD-CARDINAL\CAD\_FILES\DR82401-0315\_A\_RIVERWILD-CARDINAL.dwg, 11/22/2024 2:17:12 PM

HEATED SQUARE FC First Floor Second Floor	DOTAGE 1272 897
TOTAL HEATED	2169
<u>UNHTD SQUARE FC</u> Garage Front Porch Unfinished Bonus	0OTAGE 423 192 385
TOTAL UNHEATED	1000
TOTAL SQ FT	<u>3169</u>

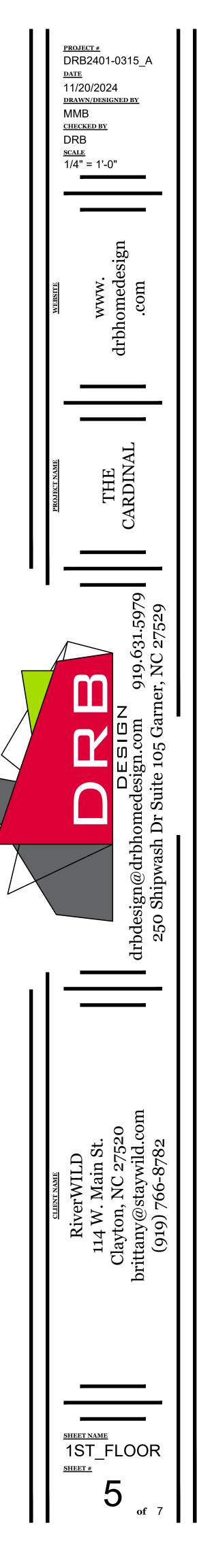
# NOTE: SEE ELEVATIONS FOR WINDOW HDR HGTS

<u>NOTE:</u> ALL DOORS ARE 6'-8" TALL UNO NOTE: ALL EXTERIOR WALLS ARE NOMINAL 4" UNO NOTE: ALL INTERIOR WALLS

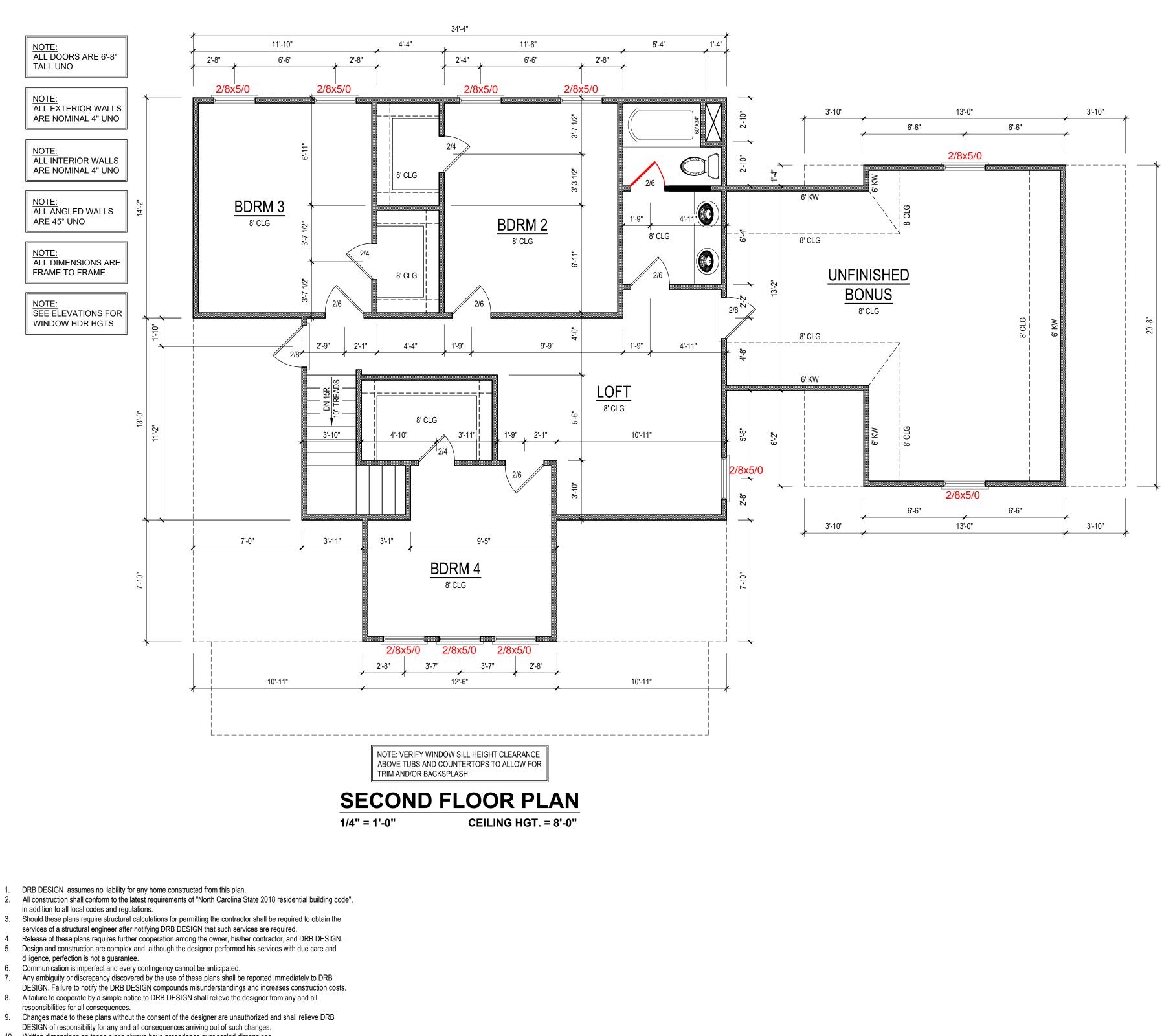
ARE NOMINAL 4" UNO NOTE: ALL ANGLED WALLS

ARE 45° UNO

NOTE: ALL DIMENSIONS ARE FRAME TO FRAME



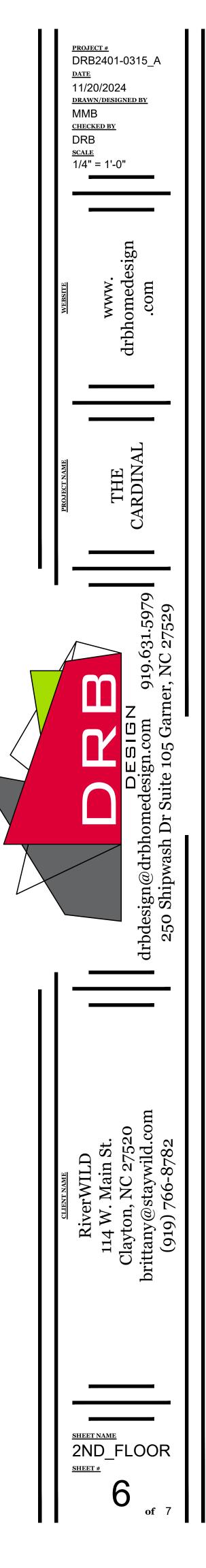


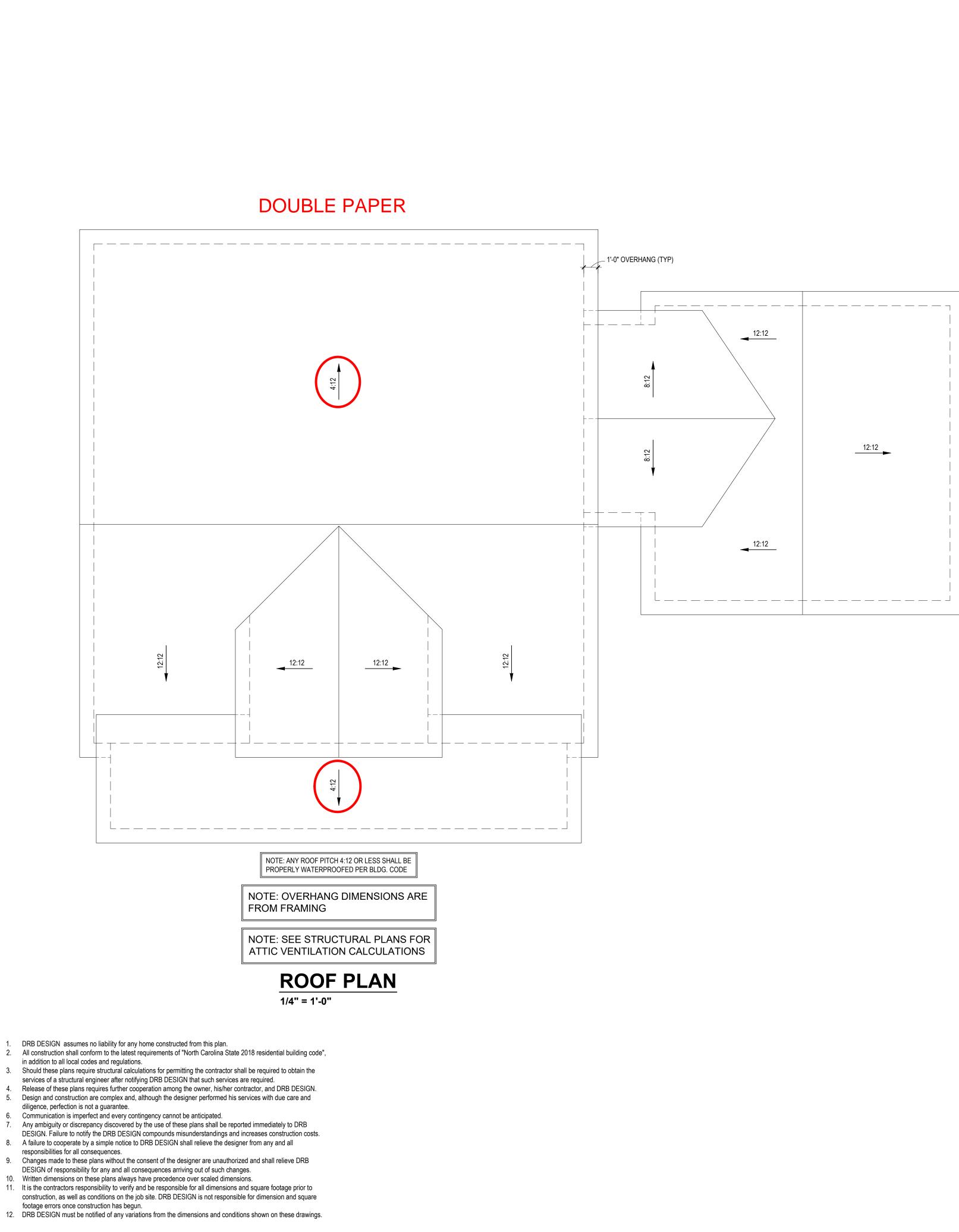


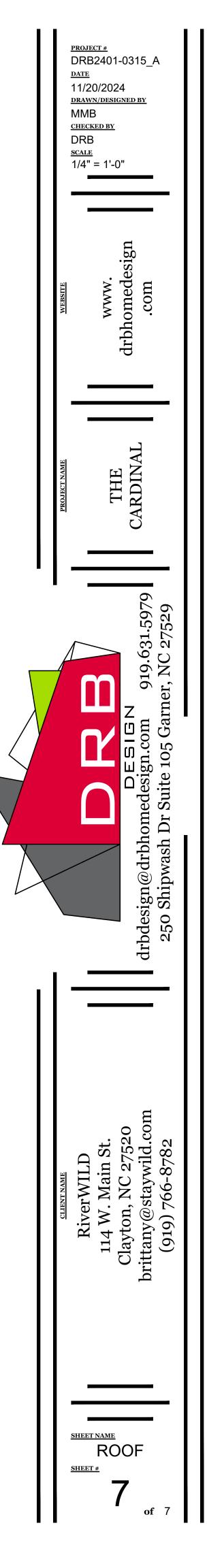
Written dimensions on these plans always have precedence over scaled dimensions.
 It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square footage errors once construction has begun.

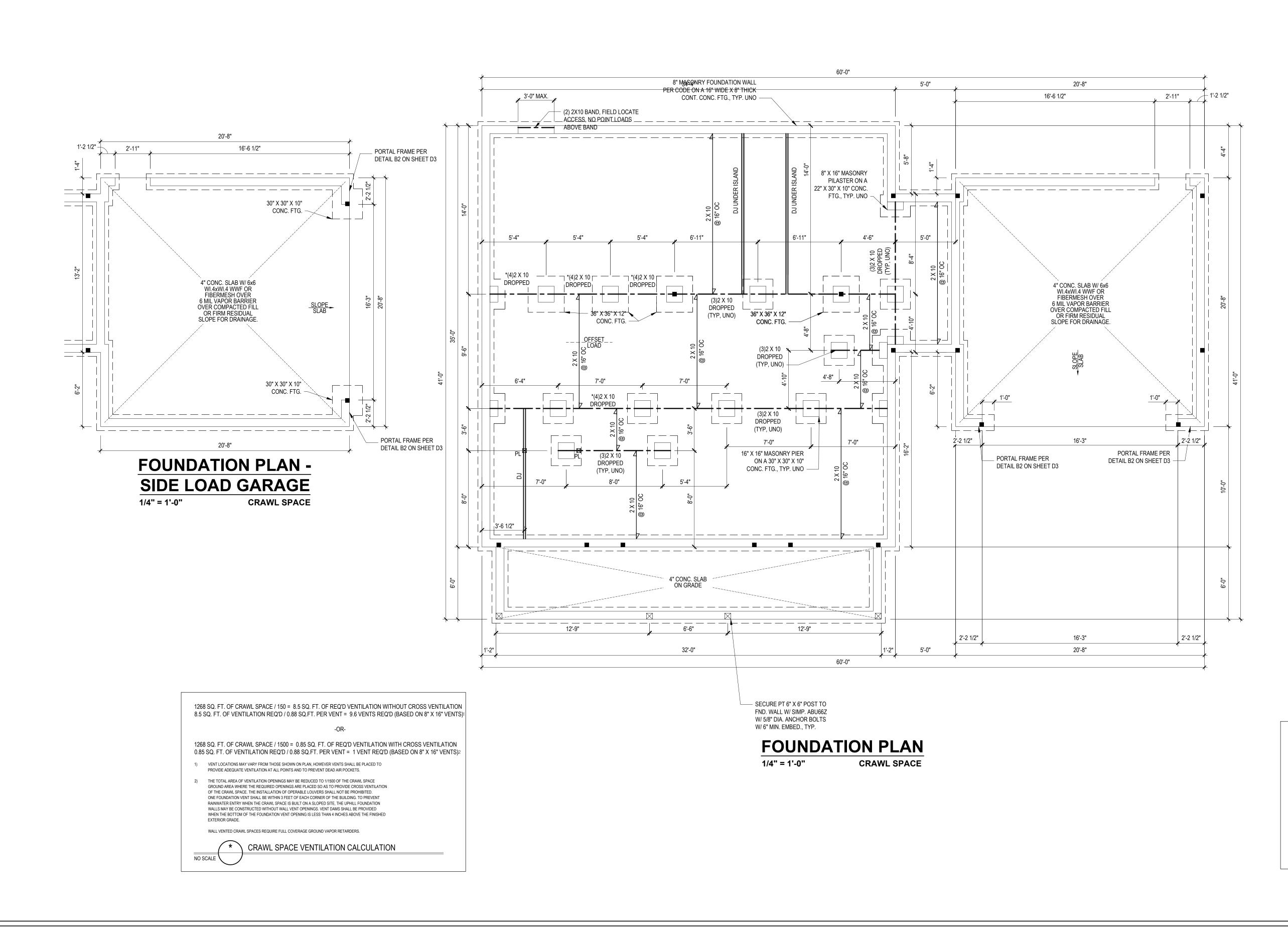
12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

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DESIGN LOADS

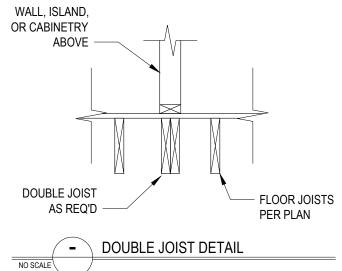
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION							
	()	(****)	LL	TL						
FLOOR (primary)	40	10	L/360	L/240						
FLOOR (secondary)	40	10	L/360	L/240						
ATTIC (w/ storage)	20	10	L/240	L/180						
ATTIC (no access)	10	5	L/240	L/180 L/240						
EXTERNAL BALCONY	40	10	L/360							
ROOF	20	10	L/240	L/180						
ROOF TRUSS	20	20	L/240	L/180						
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)									
SEISMIC	BASED ON SEISMIC ZONES A, B & C									

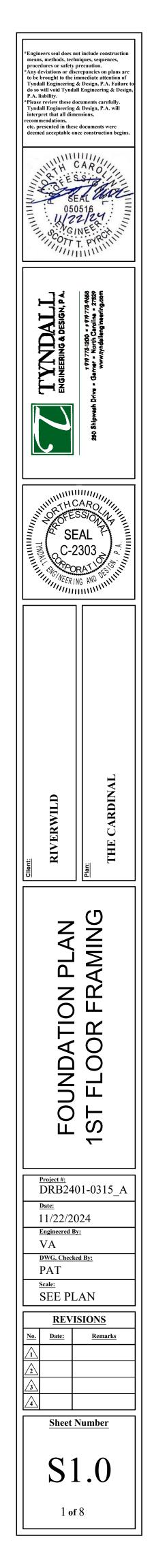
STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
   ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND
   Fb = 2600 PSI, E = 1.9M PSI (OR GREATER)
   (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
  ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)
  ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
- (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER
- TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
   REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- REFER TO 2018 NO BUILDING CODE SECTION R602 FOR CONSTRUCTION
   OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
  11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
  PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
   PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

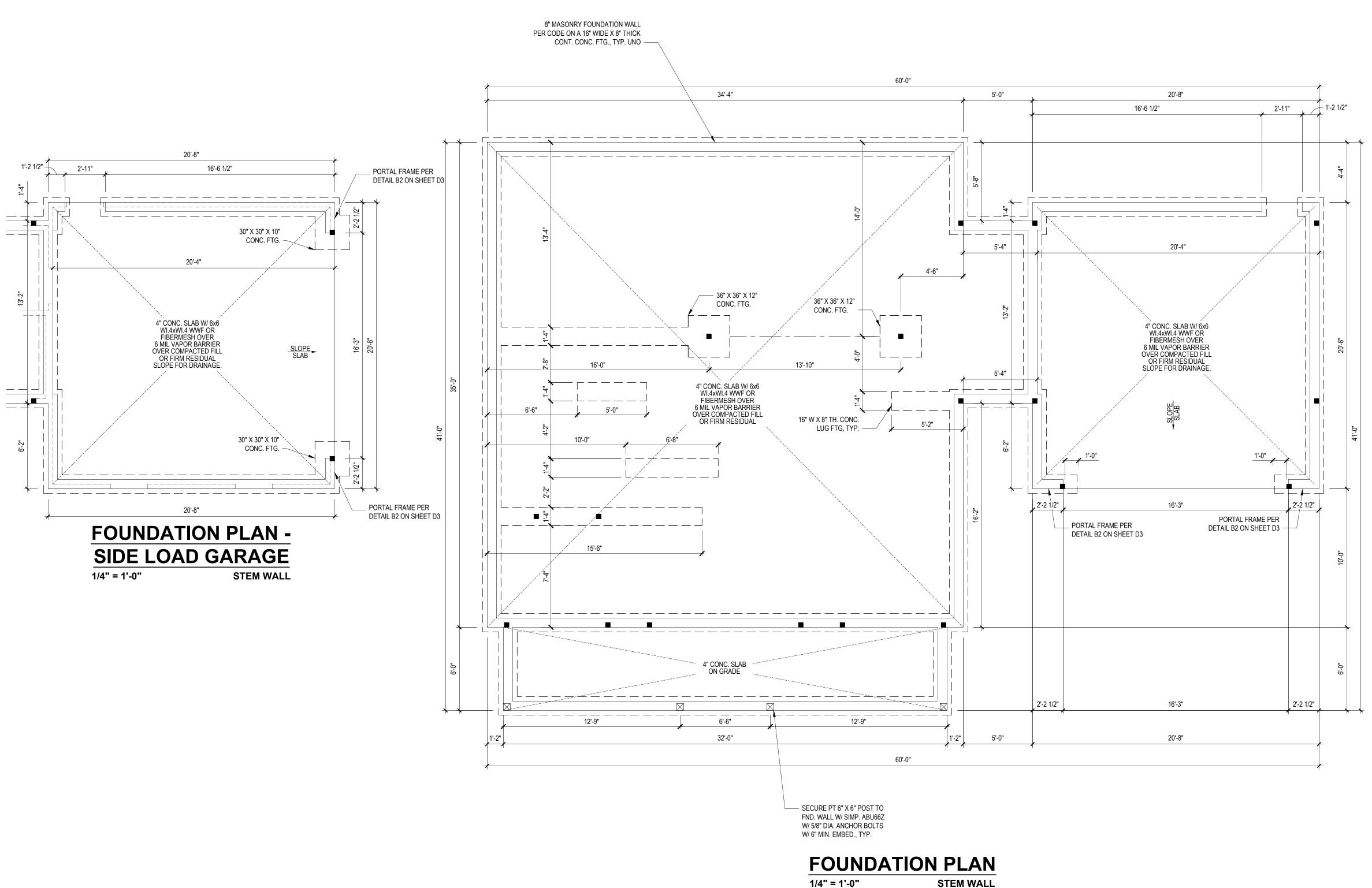


NOTE: ADDITIONAL JOISTS INSTALL A DOUBLE JOIST UNDER NON-LOAD BEARING WALLS, BUILT-INS, AND CABINETRY ABOVE THAT ARE PARALLEL TO THE FRAMING SYSTEM ON THIS PAGE, TYP UNO, BUILDER TO INSTALL AS REQUIRED, VIF DIMENSIONS









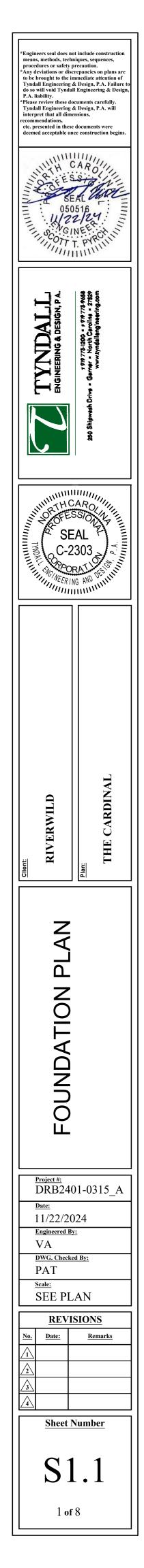
1/4" = 1'-0"

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION							
	· · · ·		LL	TL						
FLOOR (primary)	40	10	L/360	L/240						
FLOOR (secondary)	40	10	L/360	L/240						
ATTIC (w/ storage)	20	10	L/240	L/180						
ATTIC (no access)	10	5	L/240	L/180						
EXTERNAL BALCONY	40	10	L/360	L/240						
ROOF	20	10	L/240	L/180						
ROOF TRUSS	20	20	L/240	L/180						
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)									
SEISMIC	BASED ON SEISMIC ZONES A, B & C									

STRUCTURAL NOTES:

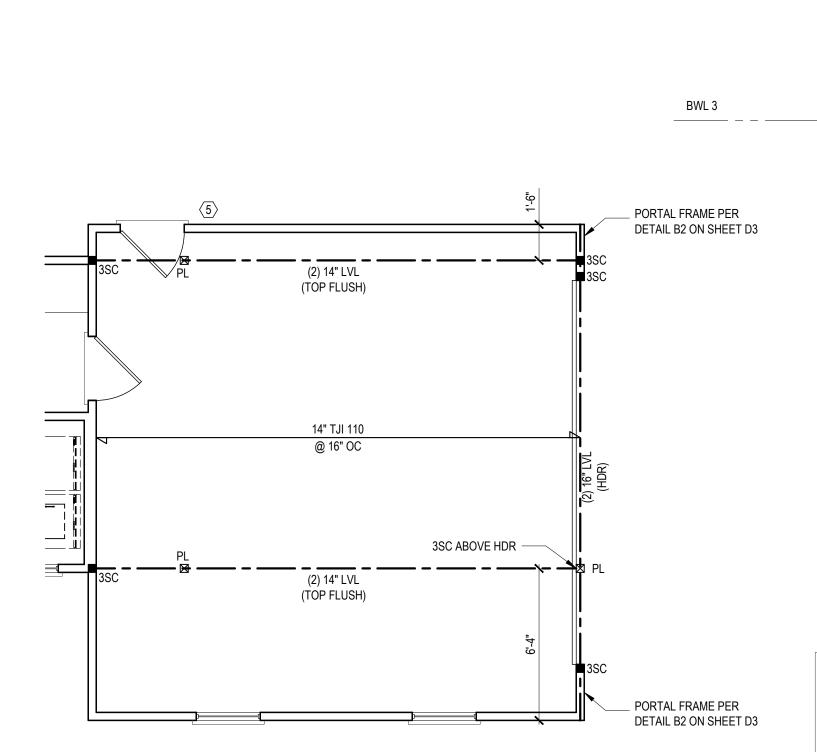
- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER) ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF 10) 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

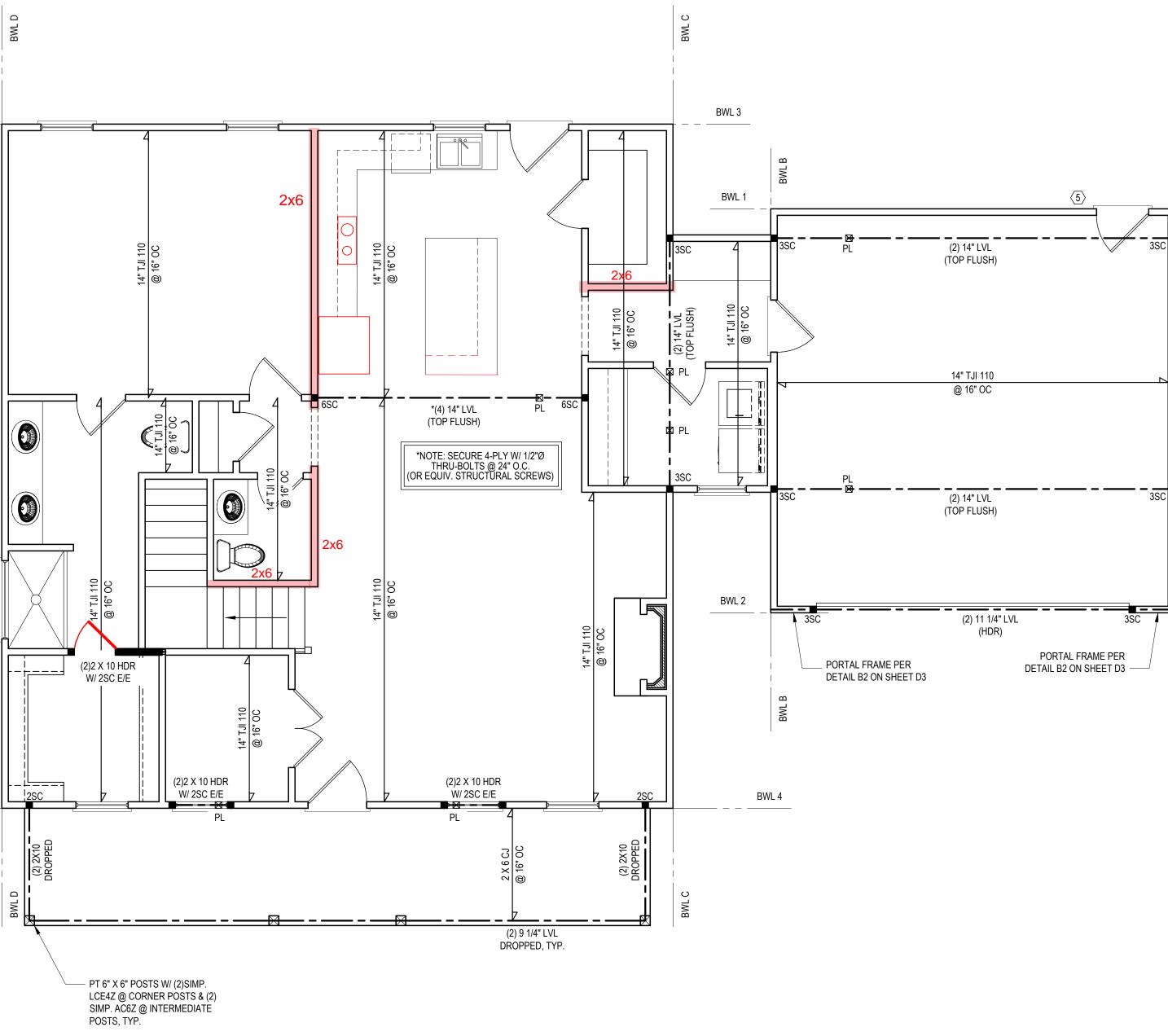


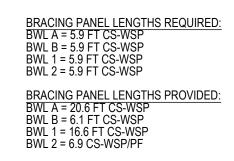
KING STUD SCHEDULE												
MIN. # OF FULL HEIGHT STUDS (KING) E.E. OF OPENING PER WALL DEPTH												
HEADER SPAN (FT)	2 X 4 STUD WALL	2 X 6 STUD WALL										
UP TO 3'-0"	1	1										
3'-1" TO 6'-0"	2	1										
6'-1" TO 9'-0"	3	2										
9'-1" TO 12'-0"	4	2										
12'-1" TO 15'-0"	5	3										
15'-1" TO 18'-0"	15'-1" TO 18'-0" 6 3											

a. TABLE DENOTES REQUIRED MINIMUM NUMBER OF STUDS EE OF HEADER, TYP UNO ON PLANS NUMBER OF KING STUDS LISTED ABOVE ARE BASED 10' NOMINAL WALL HEIGHT, STUD SPACING OF 16" O.C., AND ULTIMATE WIND SPEED OF 120 MPH (EXPOSURE B) HEADER SPANS IN TABLE ARE BASED ON ROUGH OPENINGS. INTERPOLATION BETWEEN SPAN VALUES IS PERMITTED, ROUND UP NUMBER OF KING STUDS, EXTRAPOLATION IS PROHIBITED. CONTACT

TYNDALL ENGINEERING AND DESIGN IF HEADER SPANS EXCEED TABLE VALUES







BWL 4



BRACING PANEL LENGTHS REQUIRED: BWL C = 8.8 FT CS-WSP BWL D = 8.8 FT CS-WSP BWL 3 = 9.0 FT CS-WSP BWL 4 = 5.7 FT CS-WSP BRACING PANEL LENGTHS PROVIDED: BWL C = 21.6 FT CS-WSP BWL D = 30.3 FT CS-WSP BWL 3 = 20.8 FT CS-WSP BWL 4 = 14.7 FT CS-WSP

CEILING HGT. = 9'-0" 1/4" = 1'-0"

# DESIGN LOADS

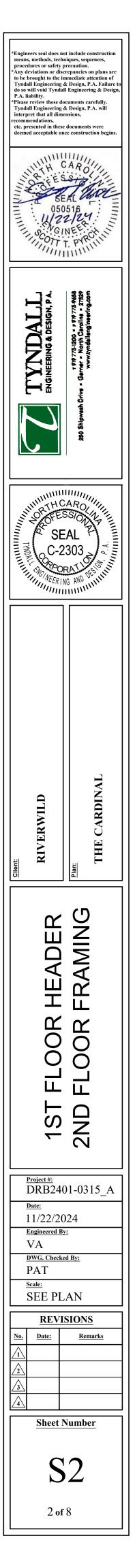
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION					
	( - /	( - )	LL	TL				
FLOOR (primary)	40	10	L/360	L/240				
FLOOR (secondary)	40	10	L/360	L/240				
ATTIC (w/ storage)	20	10	L/240	L/180				
ATTIC (no access)	10	5	L/240	L/180				
EXTERNAL BALCONY	40	10	L/360	L/240				
ROOF	20	10	L/240	L/180				
ROOF TRUSS	20	20	L/240	L/180				
WIND LOAD	BAS	SED ON 120 MPH (E	N 120 MPH (EXPOSURE B)					
SEISMIC	BAS	ED ON SEISMIC ZC	NES A, B & C					

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS. 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER) ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS
- BWL 1 FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
  - (UNO)REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION 6) OF ALL WALLS OVER 10'-0" IN HEIGHT.
  - 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
  - ALL EXTERIOR LUMBER TO BE #2 SYP PT
  - ALL CONCRETE, fc = 3000 PSI MIN.
  - PRESUMPTIVE BEARING CAPACITY = 2000 PSF 10) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE 11) THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
  - 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
  - AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
  - NCRC. 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
  - LEAST HORIZONTAL DIMENSION. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
  - ANCHORED TO THE FOUNDATION.
  - 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES BWL 2

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS. 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF
- THE 2018 NCRC. BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. 3)
- REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- $\langle 1 \rangle$  REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS. 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
- BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN
- 67% OF WALL HEIGHT
- 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
- 48" FOR OPENINGS GREATER THAN 85% OF
- WALL HEIGHT
- $\langle \overline{4} \rangle$  SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- (5) MINIMUM 800# HOLD-DOWN DEVICE



## KING STUD SCHEDULE

MIN. # OF FULL HEIGHT STUDS (KING) OF OPENING PER WALL DEPTH												
	OF OPEINING PER WALL DEPTH											
HEADER SPAN (FT)	2 X 4 STUD WALL	2 X 6 STUD WALL										
UP TO 3'-0"	1	1										
3'-1" TO 6'-0"	2	1										
6'-1" TO 9'-0"	3	2										
9'-1" TO 12'-0"	4	2										
12'-1" TO 15'-0"	5	3										
15'-1" TO 18'-0"	6	3										

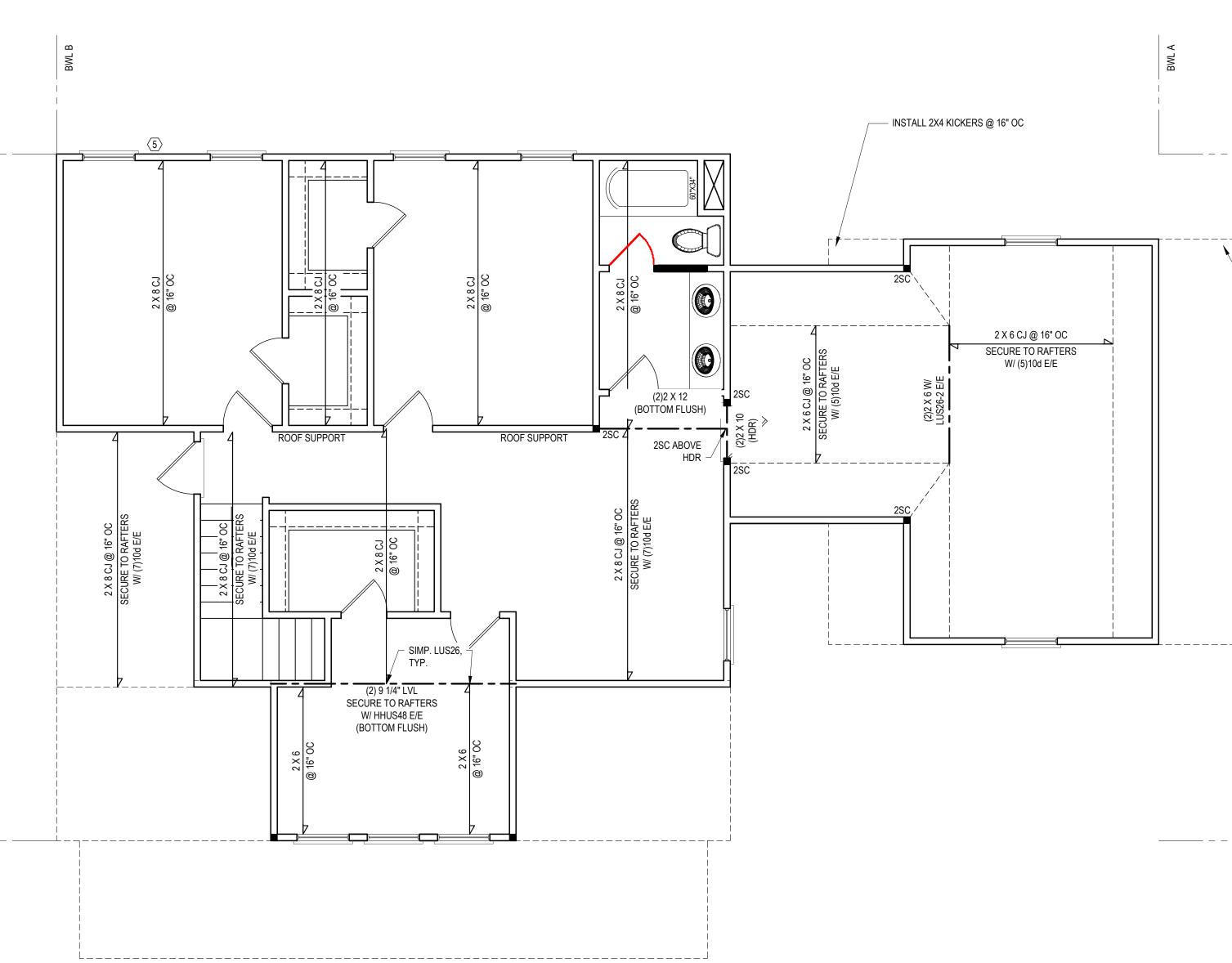
a. TABLE DENOTES REQUIRED MINIMUM NUMBER OF STUDS EE OF HEADER, TYP UNO ON PLANS
b. NUMBER OF KING STUDS LISTED ABOVE ARE BASED 10' NOMINAL WALL HEIGHT, STUD SPACING OF 16"

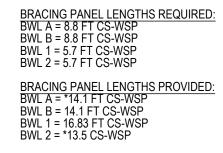
O.C., AND ULTIMATE WIND SPEED OF 120 MPH (EXPOSURE B)
 c. HEADER SPANS IN TABLE ARE BASED ON ROUGH OPENINGS. INTERPOLATION BETWEEN SPAN VALUES

IS PERMITTED, ROUND UP NUMBER OF KING STUDS, EXTRAPOLATION IS PROHIBITED. CONTACT TYNDALL ENGINEERING AND DESIGN IF HEADER SPANS EXCEED TABLE VALUES

BWL 1

BWL 2





# SECOND FLOOR PLAN

1/4" = 1'-0"

CEILING HGT. = 8'-0"

# DESIGN LOADS

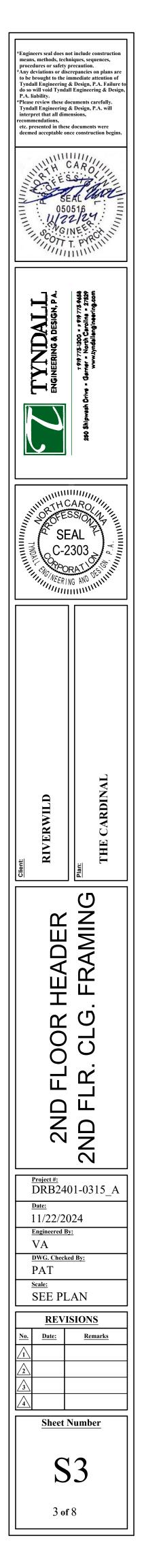
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION							
	( - /	( - )	LL	TL						
FLOOR (primary)	40	10	L/360	L/240						
FLOOR (secondary)	40	10	L/360	L/240						
ATTIC (w/ storage)	20	10	L/240	L/180						
ATTIC (no access)	10	5	L/240	L/180						
EXTERNAL BALCONY	40	10	L/360	L/240						
ROOF	20	10	L/240	L/180						
ROOF TRUSS	20	20	L/240	L/180						
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)									
SEISMIC	BASED ON SEISMIC ZONES A, B & C									

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER)
  ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER)
  ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
  (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE
- WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS
- FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
  REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT. 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
   1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
  13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
  PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

# STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS.
  WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
  - REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
  - 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
  - 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
  7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
- BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN
  - 67% OF WALL HEIGHT
  - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
  - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- $\langle \overline{4} \rangle$  SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE



BWL 1

V.

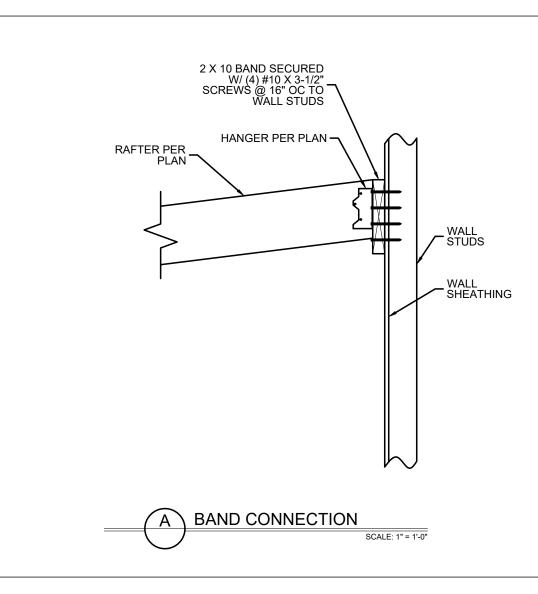
INSTALL 2X4 KICKERS @ 16" OC

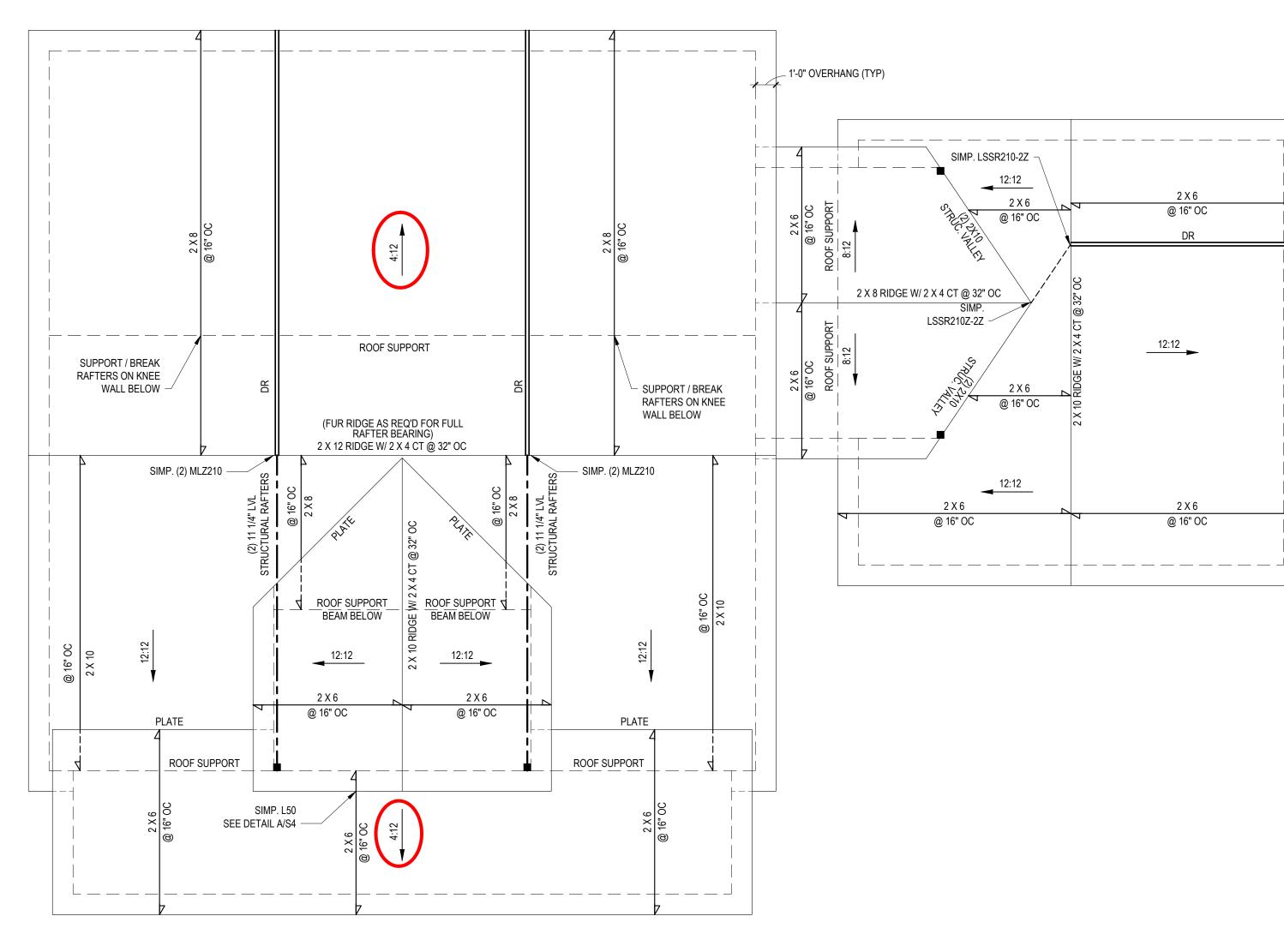
 $\langle 1 \rangle$ 

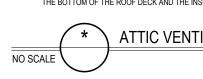
 $\langle 2 \rangle$ 

 $\langle 3 \rangle$ 

BWL 2





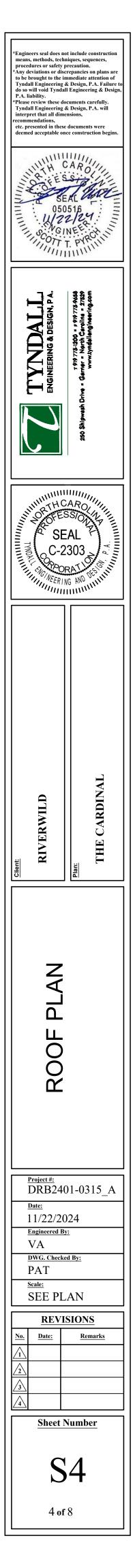


\* ATTIC VENTILATION CALCULATION

- CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.
- CALCULATION BASED ON VENTILATORS USED AT LEAST 3'-0" ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- 1695 SQ. FT. OF ATTIC / 300 = 5.65 SQ. FT. INLETS/OUTLETS REQUIRED



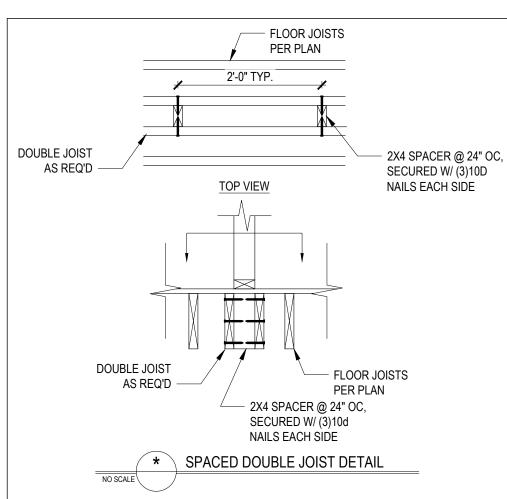
**ROOF PLAN** 1/4" = 1'-0"

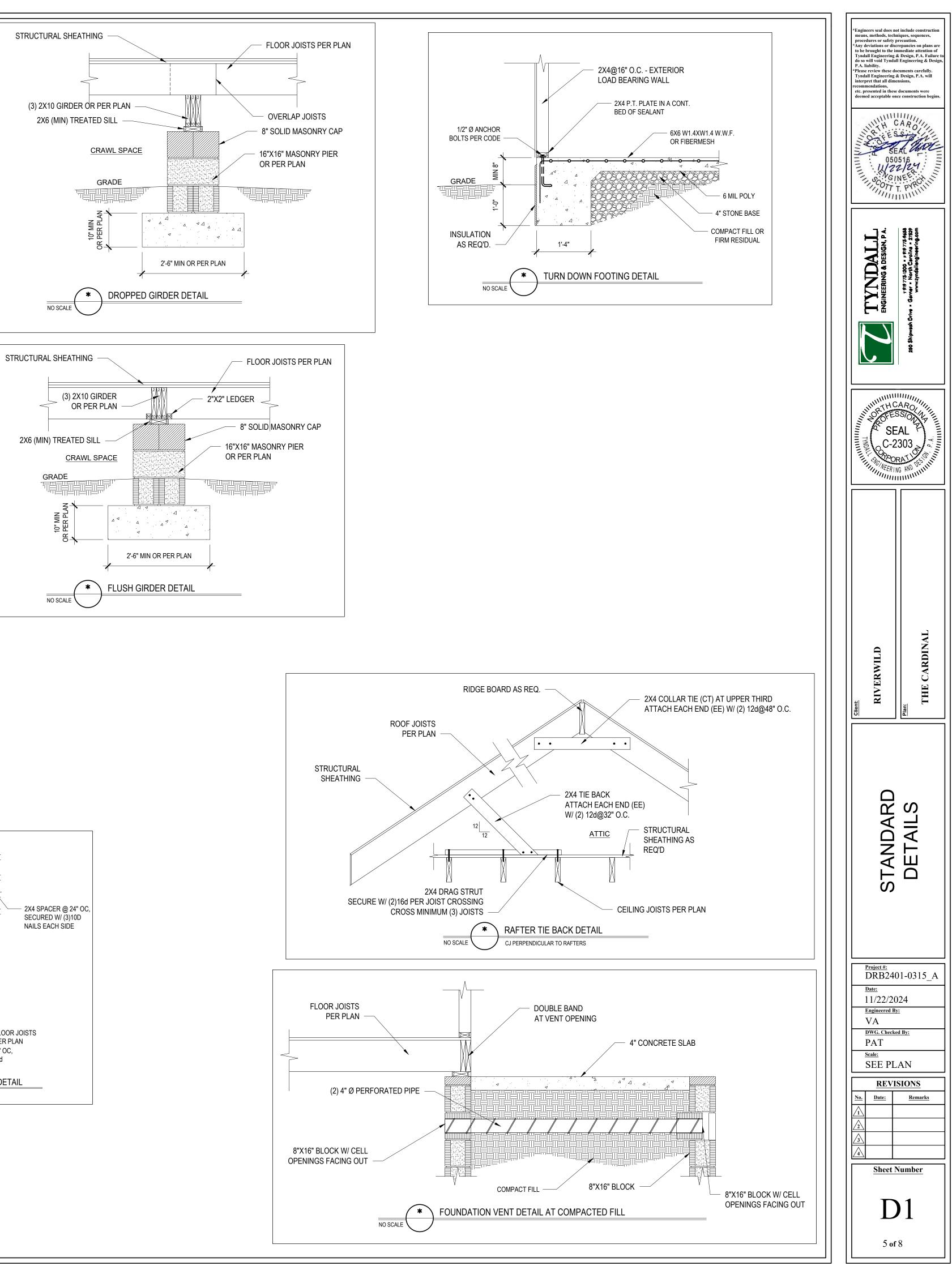


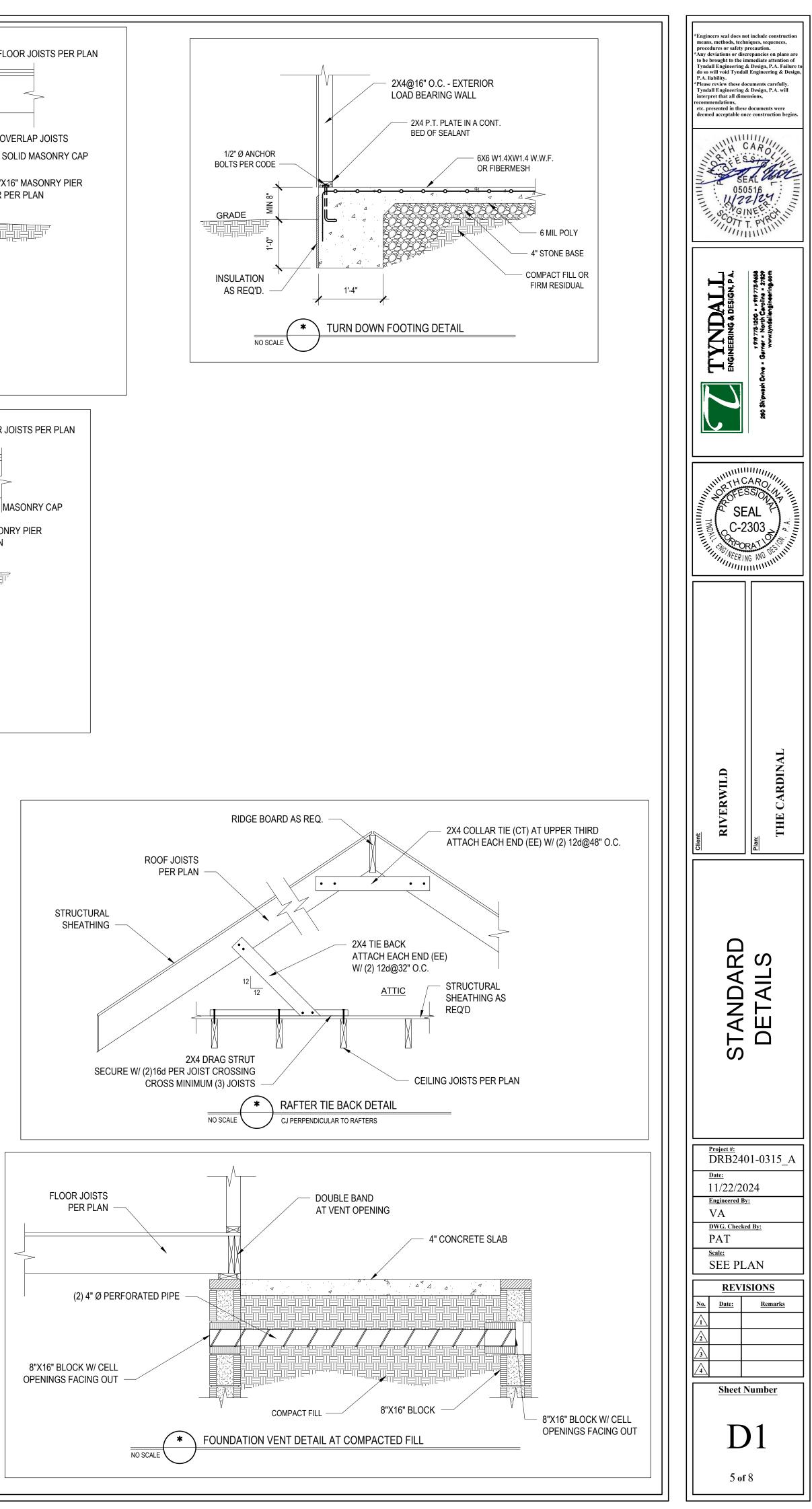
				STRUCTUR	AL NOTES							DEFINITIONS FOR C	OMMON ABBR	EVIATIONS		
1)		N SHALL CONFORM TO THE I		EMENTS OF "NORTH C	AROLINA STATE 20 <sup>°</sup>	18 RESIDENTIA	L BUILDING	i		ALT CAN		LTERNATE ANTILEVER	MANUF MAX	= Manufac = Maximum	TURER	
2)	CODE", IN ADDITION	N TO ALL LOCAL CODES AND	DREGULATIONS.							CJ CML	= (	ANTILEVER EILING JOIST ONCRETE MASONRY UNIT	MIN NOM	= MAXIMOM = MINIMUM = NOMINAL		
2)	DESIGN LOADS.			LIVE LOAD	DEAD LOAD		DEFLEC	CTION		COL	= (	OLUMN ONCRETE	O.C. PL	= ON CENTE = POINT LOA		
				(PSF)	(PSF)		L	TL	_	CON	T = (	ONTINUOUS OLLAR TIE	PT REINF	= PRESSUR = REINFORC	E TREATED	
		ALL FLOOF ATTIC (w/ walk u		40	10 10	L/3 L/3	360	L/240 L/240		DBL DIA	= [	OUBLE IAMETER	REQ'D RJ	= REQUIRED = ROOF JOIS	)	
		ATTIC (pull down	n access)	20	10	L/2	240	L/180		DJ	= [	OUBLE JOIST OUBLE RAFTER	RS SC	= ROOF SUF = STUD COL	PORT	
		ATTIC (no ac EXTERNAL BAI	,	10 40	5	L/2		L/180 L/240		DSP EA	= C	OUBLE STUD POCKET	SCH SPEC	= SCHEDULI = SPECIFIEI	E	
		ROOF ROOF TRU		20 20	10 20	L/2		L/180 L/180		EE FJ		ACH END LOOR JOIST	TH TJ	= THICK = TRIPLE JC		
		WIND LOA		20		0 MPH (EXPOS		2,100		FND FTG		OUNDATION OOTING	TRTD TSP	= TREATED = TRIPLE ST	JD POCKET	
		SEISMIC	с		SEISMIC	ZONES A, B &	С			GAL HOF	IZ = H	ALVANIZED ORIZONTAL	TYP UNO		OTED OTHERWISE	
3)		LE SOIL BEARING PRESSURI	RE = 2000 PSE							HT JSC KS	= J	EIGHT ACK STUD NG STUD	W WWF XJ	= WIDE FLAN = WELDED \ = EXTRA JO	VIRE FABRIC	
4)		HAVE A MINIMUM 28 DAY COI		ENGTH OF 3000 PSI AN	ND A MAXIMUM SLU	MP OF FIVE INC	CHES			i to				Extration		
- /	UNLESS NOTED OTH															
5)	BRACING. REFER TO	F UNBALANCED FILL AGAINS O SECTION R404 OF 2018 NC	C BUILDING CODE													
		YPE, AND UNBALANCED BAC														
6)	ALL FRAMING LUMB	BER SHALL BE SYP #2 (Fb = 8 BER EXPOSED TO THE ELEM	IENTS SHALL BE 1	TREATED MATERIAL.						1) N		OF DECK SUPPORT POSTS AS F				
	ALL LSL LUMBER TO	D BE 1.75" WIDE NOMINAL EA	CH SINGLE MEMB	BER AND Fb = 2325 PSI,	É = 1.6M PSI (Ù.N.C	).) <sup>´</sup>				1) I						
-)		O BE 3.5" WIDE NOMINAL EAC			, , , , , , , , , , , , , , , , , , ,	,					POST SIZE	MAX. POST HEIGHT	*			
()		EXTERIOR HEADERS SHALL OR HEADER SPANS FOR INTE									4 x 4	8'-0"				
8)		STEEL W-SHAPES (I-BEAMS) \$ 5, PLATES, AND C-CHANNELS									6 x 6	20'-0" OVER 20'-0"				
		ALL BE ASTM A53 GRADE B.		I A30.												
9)		LL BE SUPPORTED AT EACH ARING FROM BEAM SUPPOR								* 7		ED ON NO. 2 TREATED SOUTHE RIBUTARY AREA IS BASED ON 1		FEET		
	LAG SCREWS (1/2"Ø	3 x 4" LONG). LATERAL SUPP THE SOLE PLATES ARE NAIL	ORT IS CONSIDE	RED ADEQUATE PROV	IDED THE JOISTS A		( )				ROM TOP OF FOO	BE LOCATED AT DIFFERENT LE FING TO BOTTOM OF GIRDER				
10)		BOLT PLACEMENT PER SECT			C	ND PLACED 12	" FROM			*** [		HEIGHTS OVER 20'-0" SHALL BI A PROFESSIONAL ENGINEER O		CHITECT.		
10)	THE END OF EACH F	PLATE SECTION. ANCHOR BO INCRETE OR MASONRY. THE	OLTS SHALL BE S	SPACED AT 3'-0" O.C. F	OR BASEMENTS. AN	NCHOR BOLT S	HALL			/		RACED TO PROVIDE LATERAL	STABILITY BY ONE C	)F		
		MINIMUM TWO ANCHOR BOL									HESE METHODS:					
11)	FOUNDATION DRAIN	NAGE-DAMP PROOFING OR V	WATERPROOFING	G PER SECTION 405 AN	ND 406 OF NC BUILD	ING CODE.				A. 1	ATTACHED	IEIGHT IS LESS THAN 4'-0" AND O THE STRUCTURE IN ACCOR	DANCE WITH SECTION	ON (4)		
12)	WALL AND ROOF CL WALL CLADDING SH	LADDING VALUES: HALL BE DESIGNED FOR 28.0	) POUNDS PER SO	QUARE FOOT (LBS/SQF	T) OR GREATER PO	OSITIVE AND NE	EGATIVE PF	RESSURE.		B. 4	x 4 WOOD KNEE	ERAL BRACING IS NOT REQUIR RACES MAY BE PROVIDED ON	EACH COLUMN IN			
		H POSITIVE AND NEGATIVE S ROOF PITCHES 0/12 TO 1.5/1		LOWS:	,						AT A POINT	TIONS. THE KNEE BRACES SHA	ST LENGTH FROM T	ΉE		
		ROOF PITCHES 1.5/12 TO 6/1 ROOF PITCHES 6/12 TO 12/12									45° AND 60°	POST, AND THE BRACES SHAL FROM THE HORIZONTAL. KNEE T AND GIRDER WITH ONE 5/8"&	BRACES SHALL BE	BOLTED		
	**MEAN ROOF HEIGH									C F	BOLT AT EA	CH END OF THE BRACE. G DECKS WITHOUT KNEE BRAC		ANIZED		
13)		FROM 2/12 THROUGH 4/12, E			FELT PAPER.					О. г	BRACING, L	TERAL STABILITY MAY BE PRO	VIDED BY EMBEDD	ING THE		
14)		R602.3 FOR FRAMING OF AL											······			
15)											POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER	
16) 17)											4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"	
17)		1102.1 FOR PRESCRIPTIVE B			NLINI UKITEKIA.						6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"	
18)		M OF 500# UPLIFT & LATERAL	,	,		S (UNO)					<u>.</u>				·J	1
20)		Y PEIR HEIGHT SHALL NOT E				,				D. 2	(2) PERPENI	RTICAL CROSS BRACING MAY E	ESTANDING DECKS	OR PARALLEL		
21)		TORS RESPONSIBILITY TO V					ION.				THE 2 x 6s S	UCTURE AT THE EXTERIOR CO HALL BE ATTACHED TO THE PO	STS WITH ONE 5/8"	ØHOT		
,		RING & DESIGN, PA IS NOT R						ON BEGINS.		E. F		VANIZED BOLT AT EACH END C OF PILES IN COASTAL REGIONS		IEMBER.		
			01.4755		000					000000						
		on skylight <sup>b</sup> fen		CEILING m FRAM	ED WALL	MASS WALL -VALUE	FLOOR R-VALUE	BASEMENT <sup>C, C</sup> WALL R-VALUE	R-VALUE	CRAWL S WALI R-VAL	.					
ZONE 3	0.35	0.55	0.30	38 or 30 <u>1</u>	<u>5</u> or 5	5/13 or	R-VALUE 19	<u>5/13</u>	AND DEPTH 0	5/13			[			
3	0.35	0.00	0.30	<u>cont</u> 13	+ <u>2.5</u> <sup>h</sup> <u>5/</u>	/10 cont										FLOOR SPER PLA
4	0.35	0.55	<u>0.30</u>	cont <sup>j</sup> 13	+ <u>2.5</u> <sup>h</sup> <u>5/</u>	<u>5/13 or</u> (10 cont	19	<u>10/15</u>	10	<u>10/15</u>					 *	2'-0" TYP.
5	<u>0.35</u>	0.55	NR		L	3/17 <u>or</u> 12.5 cont	<b>30</b> <sup>g</sup>	<u>10/15</u>	10	<u>10/19</u>						
					-				L	1						

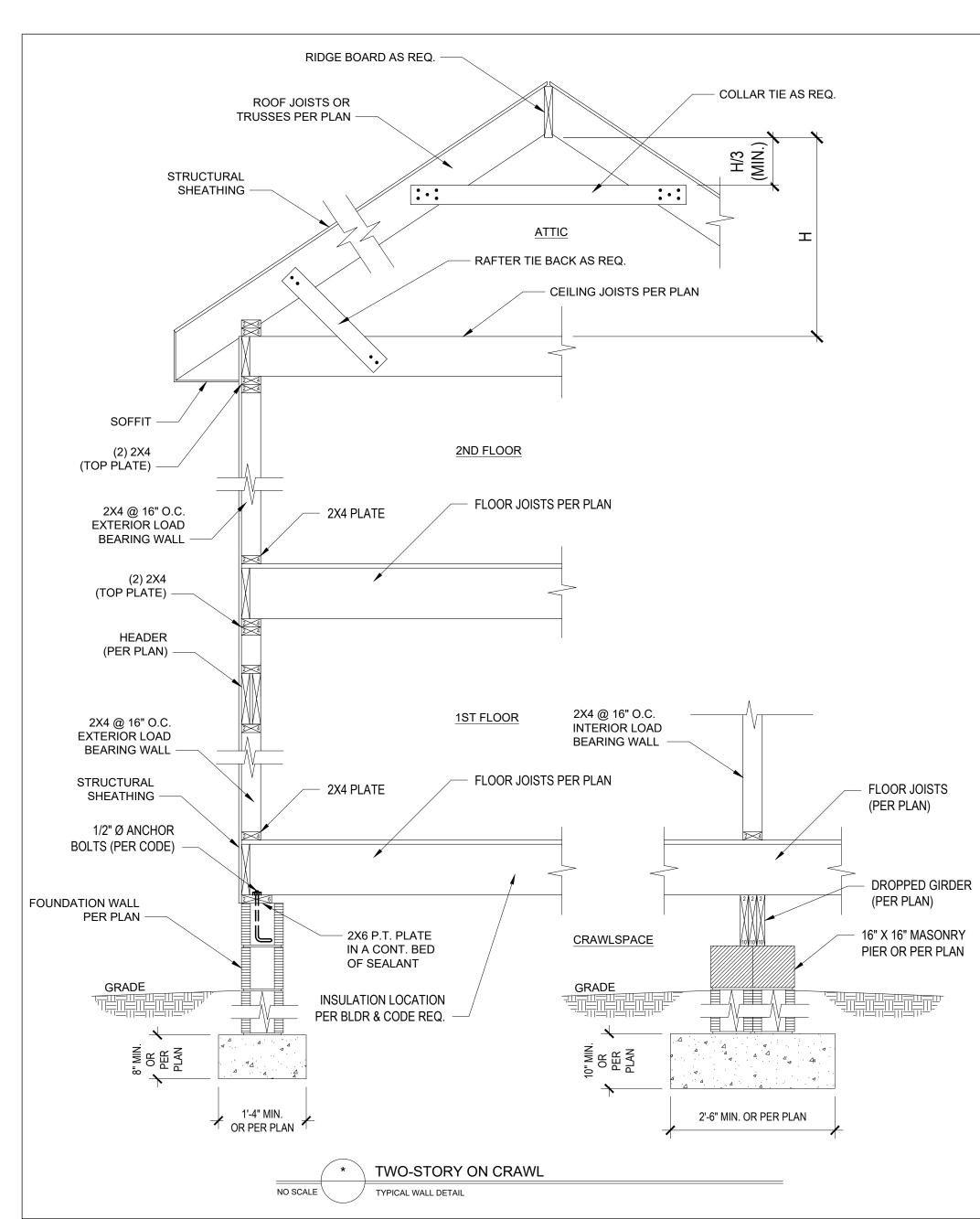
TABLE N1102.1 CLIMATE ZONES 3-5 NO SCALE a. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE. b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.

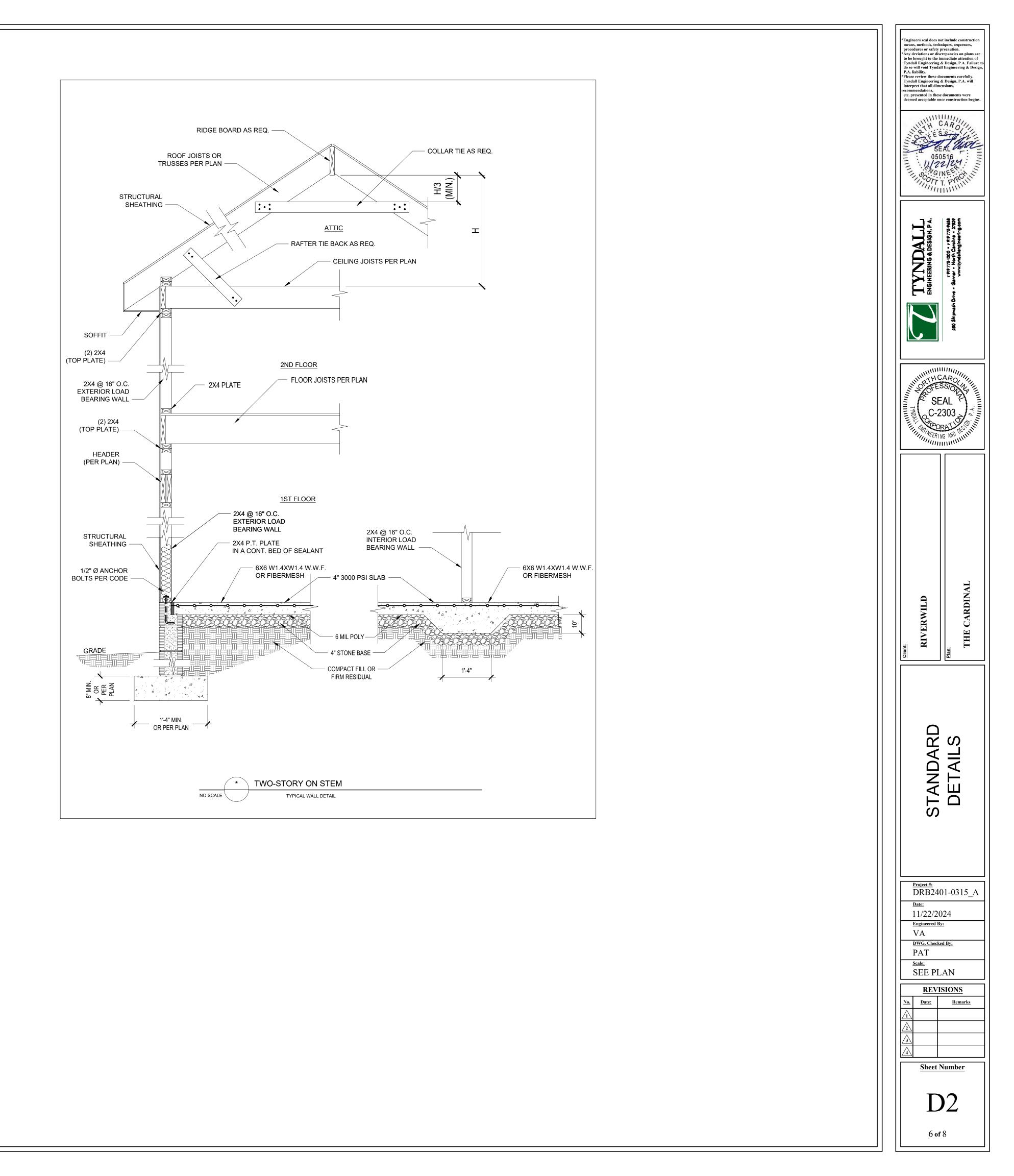
- c. "10/15" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME
- OR R-15 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL. d. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM
- OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. R-5 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS.
- e. <u>DELETED</u> f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE N1101.7 AND TABLE N1101.7.
- g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
- h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 INSULATED SHEATHING. "15+3" MEANS R-15 CAVITY INSULATION. PLUS R-3 INSULATED SHEATHING. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR,
- INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVITY
- INSULATION PLUS R-2.5 SHEATHING i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL.
- j. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- L R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH
- OF THE ATTIC ROOF DECK. m, TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE.
- n R-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2X4 WALL IS NOT DEEMED TO COMPLY. 9. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.



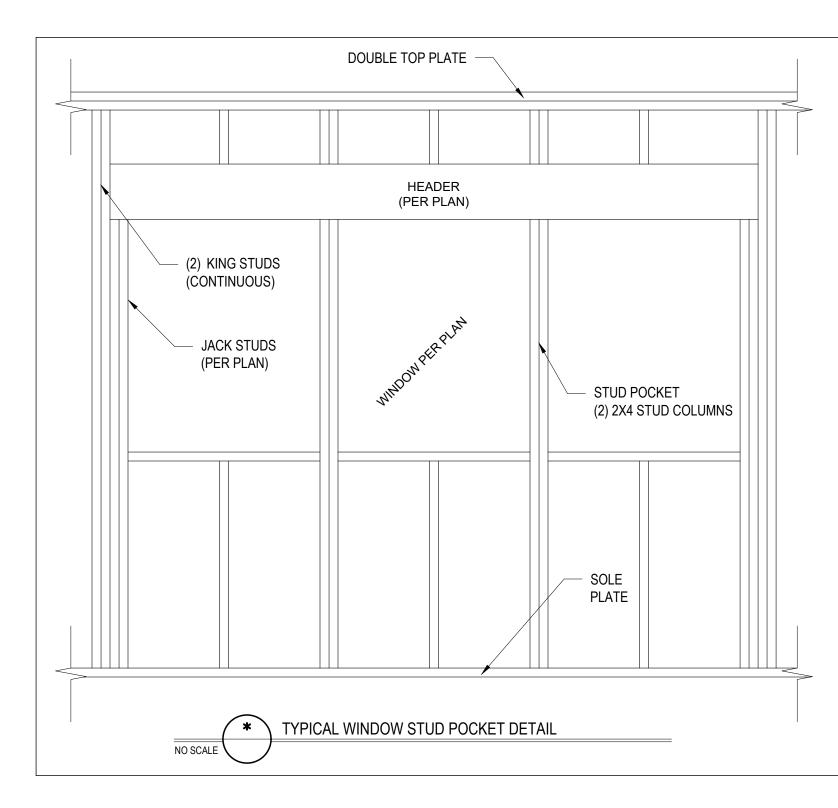




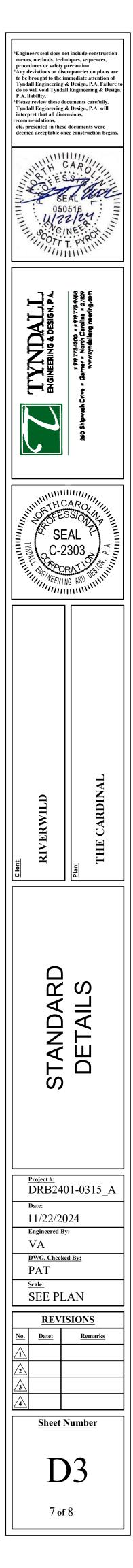


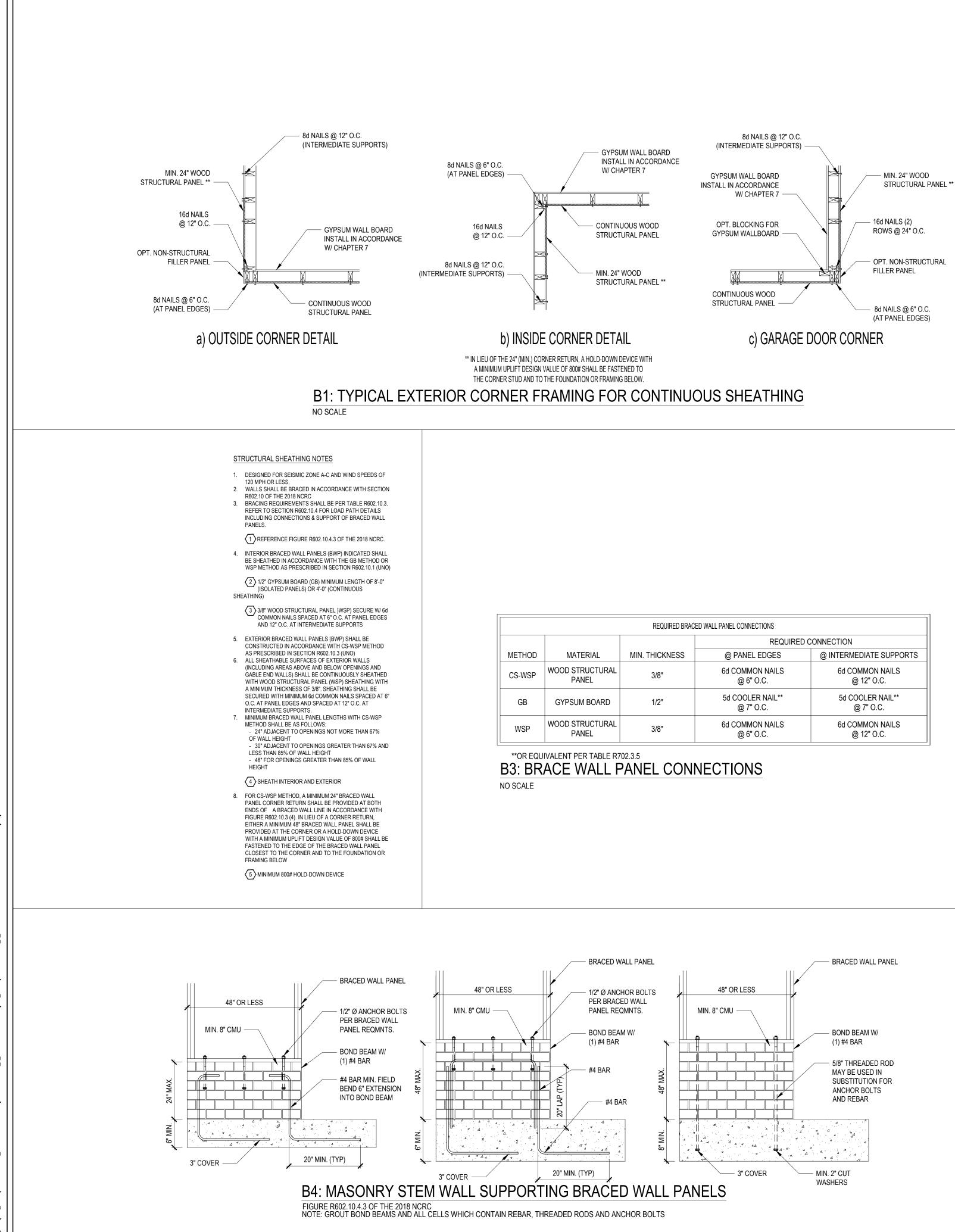


RIDGE BOARD AS REQ. -- COLLAR TIE AS REQ. ROOF JOISTS OR TRUSSES PER PLAN -H/3 (MIN.) STRUCTURAL SHEATHING -••• .... ATTIC Т RAFTER TIE BACK AS REQ. - CEILING JOISTS PER PLAN SOFFIT -(2) 2X4 (TOP PLATE) — 2ND FLOOR - FLOOR JOISTS PER PLAN 2X4 @ 16" O.C. EXTERIOR LOAD - 2X4 PLATE  $\mathsf{BEARING}$  WALL -(2) 2X4 (TOP PLATE) — HEADER (PER PLAN) — 1ST FLOOR 2X4 @ 16" O.C. EXTERIOR LOAD BEARING WALL -2X4 @ 16" O.C. STRUCTURAL INTERIOR LOAD SHEATHING -/----- 2X4 P.T. PLATE BEARING WALL IN A CONT. BED OF SEALANT 1/2" Ø ANCHOR BOLTS (PER CODE) -- 6X6 W1.4XW1.4 W.W.F. OR FIBERMESH — 4" 3000 PSI SLAB -\_\_**o**\_\_\_\_o GRADE 4 4 A - 6 MIL POLY -. A . - 4" STONE BASE -. . ∢ - Compact fill or -Firm Residual 1'-4" 1'-4" \* TWO-STORY ON SLAB NO SCALE TYPICAL WALL DETAIL

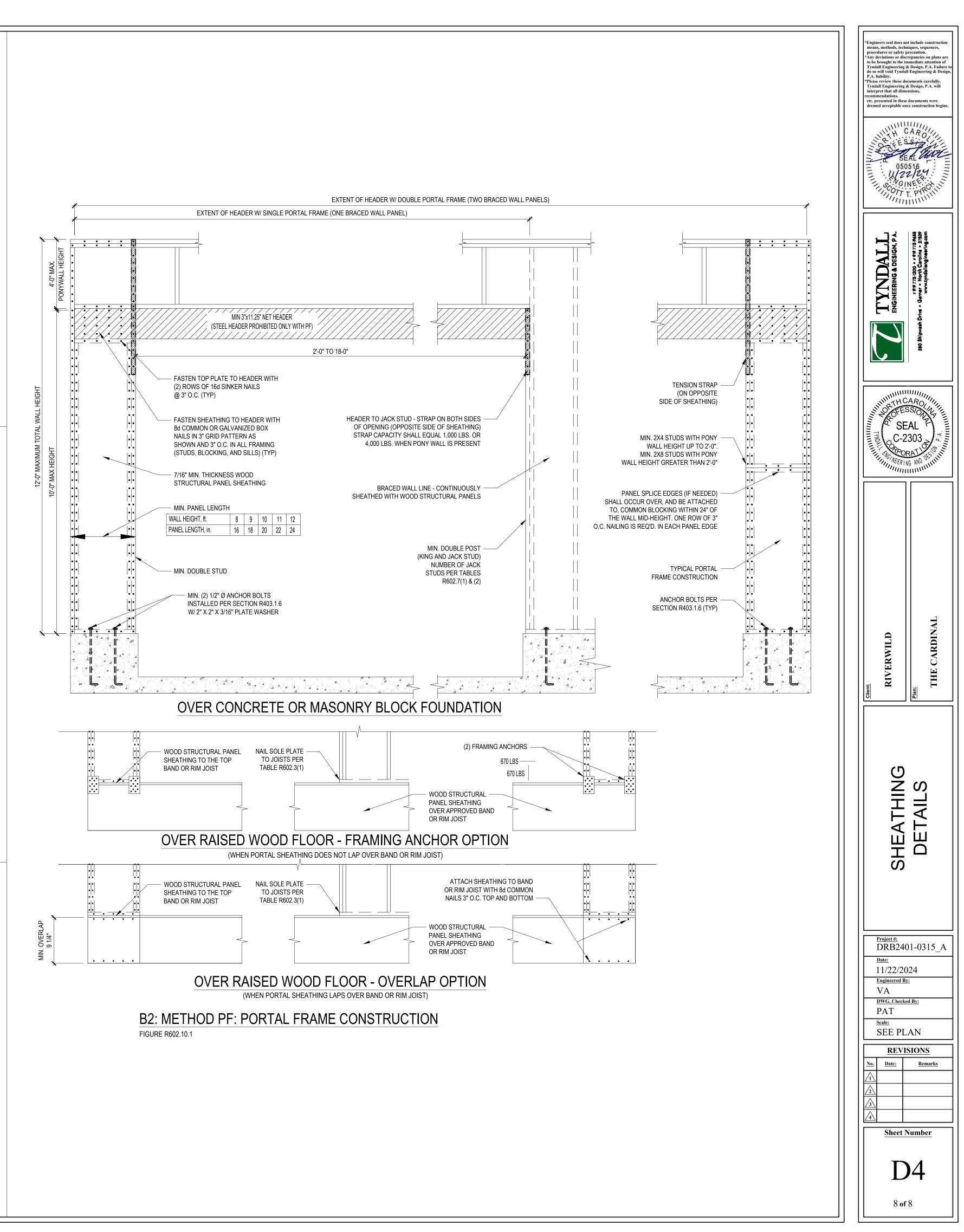


- 6X6 W1.4XW1.4 W.W.F. OR FIBERMESH

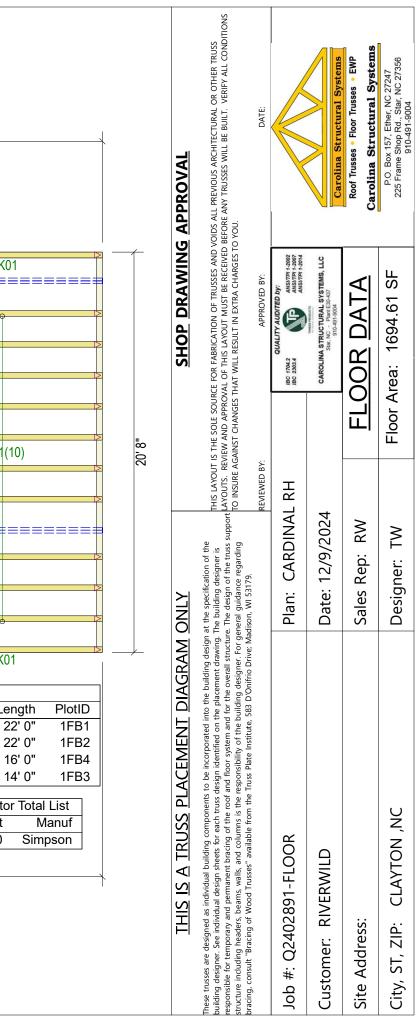




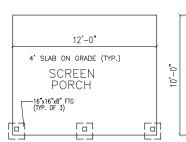
_											
CED WALL PANEL CONNECTIONS											
	REQUIRED C	CONNECTION									
	@ PANEL EDGES @ INTERMEDIATE SUPPOR										
	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.									
	5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.									
	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.									



							15' 10'						34' 4"									4'	8"	_		5' 0"		<u></u>		20' 8"
	, IKOR		<u> </u>		<u>N</u>		=05(10	<u> </u>					0			1	A IF04(9	)						1403	5.8"		<u> </u> , 4"	3	2 <mark>2</mark>	1K0
	13' 10"																											1'8	<b>≥</b> ==== 3/16"	======================================
	-	1/8/16"	3/16"	3/16"	3/16"					3/16"	3/16"	11/16	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"		3/16"	1 0/16" 1' 0 1/8"		2' 0"	10 7/8"				1'7	3/16"	0
		10,	1.7	1' 7	1' 7	1'7	1.5	1' 7	1' 7	1' 7	1' 7	13	1' 7	1' 7	1' 7	1' 7	1' 7	1' 7	1' 7	1' 7	+ +	-						1'7	3/16"	
																												1'7	3/16"	
26' 10"		-														===	S410-							1FB3				1'7	3/16"	
	.0							F10(2)		F11(4						-LU	S410-		1FB	4					<b>1</b>	02(3)	Ð	1'7	3/16"	1F01(1
	4							0 0 0 0	• •		<b>7</b>																	·	3/16" /	
	_/ <u>/</u>								ł													┓┺	┯┛				1	=========	3/16" ====	======================================
				7' 0'			3'	6"	<b>\</b>	5' 4																		-	7/16" 3/16"	
																											6'2"	·	3/16"	
									<u>v</u>	_																			3/16"	0
										1F12	!															-	<u> </u>		/	1K01
																														Products
																											Net	Qty Plies 2 2 2.2		Product Ler Lam SP LVL 1-3/4 x 14 22
	1600			1	F09(5)	)	•	•	F08(4	)	•	1F13	•			1F	07(9)				-0	1 <b>F</b> 06	( <mark>2)</mark>	1K02				2 2 2.2	1 Rigidl	Lam SP LVL 1-3/4 x 14 22
	~	-																												Lam SP LVL 1-3/4 x 14 16
		16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	7/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"	3/16"		2/10				2 2 2.2	Rigidi	Lam SP LVL 1-3/4 x 14 14
		7 5/16"	1.73	1.73	1.73	1.73	1.73	1.73	1.73	1.73	1.73	1.57	1'73	1.73	1.73	1.73	1.73	1.73	1.73	1.73	1.73	1.7 2	- <u>r</u>							Truss Connector
																														Qty Product 18 LUS410
		M			1		<u> </u>	M			M		M 34' 4"	M	M	M	M	M	M	M	M	M	M	M		5' 0"				20' 8"
																								Ť						



### FLOOR PLAN DETAIL



EXISTING HOUSE

